CRITERIA

1. ALL MATERIALS, WORKMANSHIP, DESIGN, AND CONSTRUCTION SHALL CONFORM TO THE DRAWINGS, SPECIFICATIONS, THE INTERNATIONAL BUILDING CODE (2015 EDITION), & SEATTLE BUILDING CODE MODIFICATIONS TO THE INTERNATIONAL BUILDING CODE.

2. DESIGN LOADING CRITERIA:

FLOOR LIVE LOAD (RESIDENTIAL)	PSI
FLOOR LIVE LOAD (RESIDENTIAL DECKS) 60	PSI
ROOF SNOW LOAD (Pf)	PSI
WIND:	
BASIC WIND SPEED (3-SECOND GUST)	MPI
WIND IMPORTANCE FACTOR (Iw)	1.
WIND EXPOSURE	
TOPOGRAPHICAL FACTOR (Kzt)	60
EARTHQUAKE:	
LAT. / LONG	22

LAT. / LONG
SEISMIC IMPORTANCE FACTOR (Ie)
SEISMIC USE GROUP
MAPPED SPECTRAL RESPONSE (Ss/S1)
SPECTRAL RESPONSE COEF. (SDS/SD1) 1. 11g/0. 48g
SEISMIC FORCE RESISTING SYSTEM: PLYWOOD SHEAR WALLS
DESIGN BASE SHEAR
SEISMIC RESPONSE COEFICIENT (Cs)
SEISMIC DESIGN CATEGORY
RESPONSE MODIFICATION FACTOR (R) 6.5
ANALYSIS PROCEDURE

REFERENCE: USGS NATIONAL SEISMIC HAZARD MAPPING PROJECT, 2008 DATA

- 3. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL DRAWINGS FOR BIDDING AND CONSTRUCTION. CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS FOR COMPATIBILITY AND SHALL NOTIFY ARCHITECT OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.
- 4. CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS, MEMBER SIZES, AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS ARE INTENDED AS GUIDELINES ONLY AND MUST VERIFIED. CONTRACTOR SHALL DETERMINE THE LOCATION OF ALL ADJACENT UNDERGROUND UTILITIES PRIOR TO COMMENCING EXCAVATION. THE CONTRACTOR SHALL BRING ALL CONFLICTS AND DISCREPANICES TO THE ATTENTION OF THE ARCHITECT AND STRUCTURAL ENGINEER.
- 5. CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS BEFORE COMMENCING ANY DEMOLITION. SHORING SHALL BE INSTALLED TO SUPPORT EXISTING CONSTRUCTION AS REQUIRED AND IN A MANNER SUITABLE TO THE WORK SEQUENCES. EXISTING REINFORCING SHALL BE RETAINED UNDAMAGED WHERE NOTED ON THE PLANS. DEMOLITION DEBRIS SHALL NOT BE ALLOWED TO DAMAGE OR OVERLOAD THE EXISTING STRUCTURE. LIMIT CONSTRUCTION LOADING (INCLUDING DEMOLITION DEBRIS) ON EXISTING FLOOR SYSTEMS TO 40 PSF. ALL NEW OPENINGS THROUGH EXISTING CONCRETE OR MASONRY WALLS, SLABS AND BEAMS SHALL BE ACCOMPLISHED BY SAW CUTTING WHEREVER POSSIBLE.
- 6. CONTRACTOR SHALL PROVIDE TEMPORARY BRACING FOR THE STRUCTURE AND STRUCTURAL COMPONENTS UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE PLANS.
- 7. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES REQUIRED TO PERFORM THE WORK. THE STRUCTURAL ENGINEER HAS NO OVERALL SUPERVISORY AUTHORITY OR ACTUAL AND/OR DIRECT RESPONSIBILITY FOR THE SPECIFIC WORKING CONDITIONS AT THE SITE AND/OR FOR ANY HAZARDS RESULTING FROM THE ACTIONS OF ANY TRADE CONTRACTOR. THE STRUCTURAL ENGINEER HAS NO DUTY TO INSPECT, SUPERVISE, NOTE, CORRECT, OR REPORT ANY HEALTH OR SAFETY DEFICIENCIES OF THE OWNER, CONCONTRACTORS, OR OTHER ENTITIES OR PERSONS AT THE PROJECT SITE.
- 8. SPECIAL INSPECTION OF EPOXY GROUTED INSTALLATIONS SHALL BE PROVIDED IN ACCORDANCE WITH SECTIONS 109 AND 1704 OF THE INTERNATIONAL BUILDING CODE AND THE PROJECT SPECIFICATIONS BY A QUALIFIED TESTING AGENCY DESIGNATED BY THE ARCHITECT, AND RETAINED BY THE BUILDING OWNER. THE ARCHITECT, STRUCTURAL ENGINEER, AND BUILDING DEPARTMENT SHALL BE FURNISHED WITH COPIES OF ALL INSPECTION AND TEST RESULTS.
- 9. SHOP DRAWINGS FOR THE FOLLOWING ITEMS SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION OF THESE ITEMS.

A. STRUCTURAL STEEL

APPROVED SETS OF ALL SHOP DRAWINGS SHALL ALSO BE SUBMITTED TO THE BUILDING DEPARTMENT.

GEOTECHNICAL

10. FOUNDATION NOTES: SUBGRADE PREPARATION INCLUDING DRAINAGE, EXCAVATION, COMPACTION, AND FILLING REQUIREMENTS, SHALL CONFORM STRICTLY WITH RECOMMENDATIONS GIVEN IN THE SOILS REPORT OR AS DIRECTED BY THE SOILS ENGINEER. FOOTINGS SHALL BEAR ON SOLID UNDISTURBED EARTH (CONTROLLED, COMPACTED STRUCTURAL FILL OR BOTH) AT LEAST 18" BELOW LOWEST ADJACENT FINISHED GRADE. FOOTING DEPTHS/ELEVATIONS SHOWN ON PLANS (OR IN DETAILS) ARE MINIMUM AND FOR GUIDANCE ONLY; THE ACTUAL ELEVATIONS OF FOOTINGS MUST BE ESTABLISHED BY THE CONTRACTOR IN THE FIELD WORKING WITH THE TESTING LAB AND SOILS ENGINEER. BACKFILL BEHIND ALL RETAINING WALLS WITH FREE DRAINING GRANULAR FILL AND PROVIDE FOR SUBSURFACE DRAINAGE AS NOTED IN THE SOILS REPORT.

SOILS REPORT REFERENCE: PANGEO INCORPORATED, FILE NO. 19-150

11. PIPE PILE INSTALLATION SHALL CONFORM STRICTLY WITH THE RECOMMENDATIONS GIVEN IN THE SOILS REPORT OR AS DIRECTED BY THE SOILS ENGINEER. THE GEOTECHNICAL SPECIAL INSPECTOR SHALL CONTINUOUSLY OBSERVE INSTALLATION OF THE PILES. PIPE PILES SHALL BE DRIVEN TO REFUSAL, WHERE REFUSAL IS DEFINED AS THE MINIMUM NUMBER OF SECONDS REQUIRED TO ACHIEVE ONE INCH OF PENETRATION, AS INDICATED BELOW:

HAMMER MODEL	HAMMER WEIGHT	BLOWS PER MINUTE	SECONDS PER INCH
TB225	650 LB	1000	12
TB325	850 LB	900	10
TB425	1100 LB	900	6

PIPE PILE AXIAL CAPACITY IS 6 TONS (12,000 LB).

PIPE PILES SHALL BE 3" DIAMETER, SCHEDULE 40 (0.216" WALL), AND SHALL CONFORM TO ASTM A53, GRADE A, FY = 35 KSI. THREE INCH PILES SHALL BE TESTED IN GENERAL ACCORDANCE WITH ASTM D1143-81. USE OF THE "QUICK LOAD TEST METHOD" IN THE ASTM STANDARD IS THE MINIMUM REQUIRED.

CONCRETE

12. CONCRETE SHALL BE MIXED, PROPORTIONED, CONVEYED AND PLACED IN ACCORD—ANCE WITH ACI 301. CONCRETE SHALL ATTAIN A 28-DAY STRENGTH OF F'C = 2,500 PSI AND MIX SHALL CONTAIN NOT LESS THAN 5-1/2 SACKS OF CEMENT PER CUBIC YARD AND SHALL BE PROPORTIONED TO PRODUCE A SLUMP OF 5" OR LESS.

THE MINIMUM AMOUNTS OF CEMENT AND MAXIMUM AMOUNTS OF WATER MAY BE CHANGED IF A CONCRETE PERFORMANCE MIX IS SUBMITTED TO THE STRUCTURAL ENGINEER AND THE BUILDING DEPARTMENT FOR APPROVAL TWO WEEKS PRIOR TO PLACING ANY CONCRETE. THE CONCRETE PERFORMANCE MIX SHALL INCLUDE THE AMOUNTS OF CEMENT, FINE AND COARSE AGGREGATE, WATER AND ADMIXTURES AS WELL AS THE WATER CEMENT RATIO, SLUMP, CONCRETE YIELD AND SUBSTANTIATING STRENGTH DATA IN ACCORDANCE WITH ACI 301. REVIEW OF MIX SUBMITTALS BY THE ENGINEER OF RECORD INDICATES ONLY THAT INFORMATION PRESENTED CONFORMS GENERALLY WITH CONTRACT DOCUMENTS. CONTRACTOR OR SUPPLIER MAINTAINS FULL RESPONSIBILITY FOR SPECIFIED PERFORMANCE.

ALL CONCRETE WITH SURFACES EXPOSED TO STANDING WATER SHALL BE AIR-ENTRAINED WITH AN AIR-ENTRAINING AGENT CONFORMING TO ASTM C260, C494, AND C618. TOTAL AIR CONTENT SHALL BE IN ACCORDANCE WITH TABLE 19.3.2.1 OF THE ACI 318.

- 13. REINFORCING STEEL SHALL CONFORM TO ASTM A615 (INCLUDING SUPPLEMENT S1), GRADE 40, FY = 40,000 PSI. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185.
- 14. REINFORCING STEEL SHALL BE DETAILED (INCLUDING HOOKS AND BENDS) IN ACCORDANCE WITH ACI 318. LAP ALL CONTINUOUS REINFORCEMENT 40 BAR DIAMETERS OR 2'-0" MINIMUM. PROVIDE CORNER BARS AT ALL WALL AND FOOTING INTERSECTIONS. LAP CORNER BARS 40 BAR DIAMETERS OR 2'-0" MINIMUM. LAP ADJACENT MATS OF WELDED WIRE FABRIC A MINIMUM OF 8" AT SIDES AND ENDS.
- 16. NON-SHRINK GROUT SHALL BE FURNISHED BY AN APPROVED MANUFACTURER AND SHALL BE MIXED AND PLACED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED RECOMMENDATIONS. GROUT STRENGTH SHALL BE AT LEAST EQUAL TO THE MATERIAL ON WHICH IT IS PLACED (3000 PSI MINIMUM).

ANCHORAGE

- 17. EXPANSION BOLTS INTO CONCRETE AND GROUTED MASONRY UNITS SHALL BE "STRONG-BOLT" ANCHORS AS MANUFACTURED BY THE SIMPSON COMPANY AND INSTALLED IN STRICT ACCORDANCE WITH ICC ESR 1771, INCLUDING MINIMUM EMBEDMENT REQUIREMENTS.
- 18. EPOXY-GROUTED ITEMS SPECIFIED ON THE DRAWINGS SHALL BE GROUTED WITH "SET-XP" HIGH STRENGTH EPOXY AS MANUFACTURED BY THE SIMPSON COMPANY AND INSTALLED IN STRICT ACCORDANCE WITH ICC ESR 2508.

STEEL

- 19. STRUCTURAL STEEL DESIGN, FABRICATION, AND ERECTION SHALL BE BASED ON THE LATEST EDITIONS OF THE AISC SPECIFICATIONS AND CODES:
 - A. SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (AISC 360)
 - B. CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES (AISC 303)
 - C. SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. BOLTS IN SHEAR OR BEARING TYPE CONNECTIONS NEED ONLY BE TIGHTENED TO THE SNUG TIGHT CONDITION PER SECTION 8(C).
- 20. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING MINIMUM STANDARDS. PLATES, ANGLES, AND CHANNELS SHALL CONFORM TO ASTM A36, FY = 36 KSI. WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992, FY = 50 KSI. STEEL PIPE SHALL CONFORM TO ASTM A53, TYPE E OR S, GRADE B, FY = 35 KSI. SQUARE OR RECTANGULAR STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, FY = 46 KSI. ANCHOR BOLTS AND CONNECTION BOLTS SHALL CONFORM TO ASTM A36.
- 21. ALL WELDING SHALL BE IN CONFORMANCE WITH AISC AND AWS STANDARDS AND SHALL BE PERFORMED BY WABO CERTIFIED WELDERS USING E70XX ELECTRODES. ONLY PREQUALIFIED WELDS (AS DEFINED BY AWS) SHALL BE USED.

WOOD

22. FRAMING LUMBER SHALL BE KILN DRIED OR MC-15, AND GRADED AND MARKED IN CON-FORMANCE WITH WCLIB STANDARD GRADING RULES FOR WEST COAST LUMBER NO. 17, LATEST EDITION. FURNISH TO THE FOLLOWING MINIMUM STANDARDS:

	(2X MEMBERS) (3X & 4X MEMBERS)	HEM-FIR NO. 2 MINIMUM BASE VALUE, FB = 850 PSI DOUGLAS FIR NO. 1
	(on a in memberso)	MINIMUM BASE VALUE, FB = 1000 PSI
STRUCTURA	L LIGHT FRAMING:	DOUGLAS FIR NO. 2

(INCL. 3X AND 4X POSTS)

MINIMUM BASE VALUE, FB = 900 PSI

BEAMS AND STRINGERS:

DOLICIAS FIR NO. 1

BEAMS AND STRINGERS:

(INCL. 6X AND LARGER)

DOUGLAS FIR NO. 1

MINIMUM BASE VALUE, FB = 1350 PSI

POSTS AND TIMBERS:

DOUGLAS FIR NO. 1

(6X6 AND LARGER) MINIMUM BASE VALUE, FC = 1000 PSI

STUDS, PLATES & MISC. FRAMING: DOUGLAS FIR OR HEM-FIR STANDARD GRADE

2X6 STUDS AND PLATES: HEM-FIR NO. 3/ STUD GRADE

2X AND 3X T & G DECKING HEM-FIR COMMERICAL DEX,

23. ENGINEERED LUMBER MEMBERS SHALL BE MANUFACTURED UNDER A PROCESS BY THE NATIONAL RESEARCH BOARD. EACH PIECE SHALL BEAR A STAMP OR STAMPS NOTING THE NAME AND PLANT NUMBER OF THE MANUFACTURER, THE GRADE, THE NATIONAL RESEARCH BOARD NUMBER, AND THE QUALITY CONTROL AGENCY. ALL LUMBER SHALL BE MANUFACTURED IN ACCORDANCE WITH THE APPROPRIATE NER REPORT AND GLUED WITH A WATERPROOF ADHESIVE MEETING THE REQUIREMENTS OF ASTM D2559 WITH ALL GRAIN PARALLEL WITH THE LENGTH OF THE MEMBER.

MINIMUM BASE VALUE, FB = 1350 PSI

PSL	FB = 2900 PSI	E = 2000 KSI	FV = 290 PSI	NER-292
LSL	FB = 2250 PSI	E = 1500 KSI	FV = 285 PSI	NER-481
LVL	FB = 2600 PSI	E = 1800 KSI	FV = 285 PSI	NER-126

DESIGN SHOWN ON PLANS IS BASED ON LUMBER MANUFACTURED BY THE WEYERHAUSER CORPORATION. ALTERNATE MANUFACTURERS MAY BE USED SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER. ALTERNATE JOIST HANGERS AND OTHER HARDWARE MAY BE SUBSTITUTED FOR ITEMS SHOWN PROVIDED THEY HAVE ICC APPROVAL FOR EQUAL OR GREATER LOAD CAPACITIES. ALL JOIST HANGERS AND OTHER HARDWARE SHALL BE COMPATIBLE IN SIZE WITH MEMBERS PROVIDED.

ALL PROPOSED HOLE SIZES AND LOCATIONS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL TWO WORKING DAYS PRIOR TO DRILLING HOLES.

PREFABRICATED PLYWOOD WEB JOIST DESIGN SHOWN ON PLANS IS BASED ON JOISTS MANUFACTURED BY THE WEYERHAUSER CORPORATION AND SHALL BE FURNISHED AND INSTALLED IN CONFORMANCE WITH THE MANUFACTURER'S PUBLISHED SPECIFICATIONS. ALL NECESSARY BRIDGING, BLOCKING, BLOCKING PANELS, STIFFENERS, ETC., SHALL BE DETAILED AND FURNISHED BY THE MANUFACTURER. SUBMIT SHOP DRAWINGS TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION. ALTERNATE PLYWOOD WEB JOIST MANUFACTURERS MAY BE USED SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND STRUCTURAL ENGINEER. ALTERNATE JOIST HANGERS AND OTHER HARDWARE MAY BE SUBSTITUTED FOR ITEMS SHOWN PROVIDED THEY HAVE ICC APPROVAL FOR EQUAL OR GREATER LOAD CAPACITIES. ALL JOIST HANGERS AND OTHER HARDWARE SHALL BE COMPATIBLE IN SIZE WITH PLYWOOD WEB JOIST PROVIDED.

ALL HOLES SHALL CONFORM TO THE MANUFACTURERS SPECIFICATIONS. IF THREE OR FEWER HOLES ARE PROPOSED FOR A SINGLE JOIST, HOLES SHALL CONFORM TO THE WEYERHAUSER ILEVEL TJI ALLOWABLE HOLE CHART. IF MORE THEN THREE HOLES ARE PROPOSED FOR ONE SINGLE JOIST, ALL HOLE SIZES AND LOCATIONS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR APPROVAL TWO WORKING DAYS PRIOR TO DRILLING HOLES.

25. PLYWOOD SHEATHING SHALL BE GRADE C-D, EXTERIOR GLUE OR STRUCTURAL II, EXTERIOR GLUE IN CONFORMANCE WITH APA STANDARDS. ORIENTED STRAND BOARD OF EQUIVALENT THICKNESS, EXPOSURE RATING AND SPAN RATING MAY BE USED IN LIEU OF PLYWOOD.

A. ROOF SHEATHING SHALL BE 1/2" (NOM.) WITH SPAN RATING 24/0.

B. FLOOR SHEATHING SHALL BE 3/4" (NOM.) WITH SPAN RATING 40/20.

C. WALL SHEATHING SHALL BE 1/2" (NOM.) WITH SPAN RATING 24/0.

REFER TO WOOD FRAMING NOTES BELOW FOR TYPICAL NAILING.

- 26. ALL WOOD IN DIRECT CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE—
 TREATED WITH AN APPROVED PRESERVATIVE OR (2) LAYERS OF ASPHALT IMPREGNATED
 BUILDING PAPER SHALL BE PROVIDED BETWEEN UNTREATED WOOD AND CONCRETE OR
 MASONRY. ALL WOOD EXPOSED TO WEATHER WITHOUT THE ADEQUATE PROTECTION OF
 A ROOF OR EAVE SHALL BE AN APPROVED WOOD OF NATURAL RESISTANCE TO DECAY
 OR PRESSURE TREATED. SUCH MEMBERS INCLUDE HORIZONTAL MEMBERS SUCH AS
 GIRDERS, JOISTS, AND DECKING; OR VERTICAL MEMBERS SUCH AS POSTS, POLES,
 AND COLUMNS.
- 27. TIMBER CONNECTORS CALLED OUT BY LETTERS AND NUMBERS SHALL BE "STRONG-TIE" BY SIMPSON COMPANY, AS SPECIFIED IN THEIR MOST RECENT CATALOG. EQUIVALENT DEVICES BY OTHER MANUFACTURERS MAY BE SUBSTITUTED, PROVIDED THEY HAVE ICC APPROVAL FOR EQUAL OR GREATER LOAD CAPACITIES. PROVIDE NUMBER AND SIZE OF FASTENERS AS SPECIFIED BY MANUFACTURER. CONNECTORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. WHERE CONNECTOR STRAPS CONNECT TWO MEMBERS, PLACE ONE—HALF OF THE NAILS OR BOLTS IN EACH MEMBER. ALL BOLTS IN WOOD MEMBERS SHALL CONFORM TO ASTM A307. PROVIDE WASHERS UNDER THE HEADS AND NUTS OF ALL BOLTS AND LAG SCREWS BEARING ON WOOD. UNLESS NOTED OTHERWISE, ALL NAILS SHALL BE COMMON. ALL SHIMS SHALL BE SEASONED AND DRIED AND THE SAME GRADE (MINIMUM) AS MEMBERS CONNECTED. HANGERS IN DIRECT CONTACT WITH PRESERVATIVE—TREATED WOOD SHALL BE EITHER STAINLESS STEEL (SST300), POST HOT—DIPPED GALVANIZED(HDG) OR GALVANIZED WITH A MINIMUM OF 1.850Z ZINC PER SQUARE INCH (ZMAX). UNLESS NOTED OTHERWISE, ALL

LUMBER JOISTS SHALL BE CONNECTED TO FLUSH BEAMS WITH "LUS" SERIES JOIST HANGERS, AND ALL TJI JOISTS SHALL BE CONNECTED TO FLUSH BEAMS WITH "ITT" OR "IUT" SERIES JOIST HANGERS.

28. NAILS - NAIL SIZES SPECIFIED ON DRAWINGS ARE BASED ON THE FOLLOWING SPECIFICATIONS:

SIZE	LENGTH	DIAMETI
6D	2"	0. 113"
8D	2-1/2"	0. 131"
10D	3"	0. 148"
12D	3-1/4"	0. 148"
16D	3-1/2"	0. 162"

IF CONTRACTOR PROPOSES THE USE OF ALTERNATE NAILS, THEY SHALL SUBMIT NAIL SPECIFICATIONS TO THE STRUCTURAL ENGINEER (PRIOR TO CONSTRUCTION) FOR REVIEW AND APPROVAL. NAILS SHALL BE DRIVEN FLUSH TO FACE OF SHEATHING WITH NO COUNTERSINKING PERMITTED.

29. TONGUE AND GROOVE STRUCTURAL ROOF AND FLOOR DECKING SHALL BE INSTALLED AS FOLLOWS: 2X DECKING SHALL BE TOENAILED THROUGH THE TONGUE AND FACENAILED WITH ONE 16D NAIL PER PIECE PER SUPPORT. 3X AND 4X DECKING SHALL BE

TOENAILED WITH ONE 40D NAIL AND FACENAILED WITH ONE 60D NAIL PER SUPPORT. COURSES SHALL BE SPIKED TOGETHER WITH 8" SPIKES AT 30" O.C. (MAXIMUM) AND AT 10" (MAXIMUM) FROM EACH END OF EACH PIECE. SPIKES SHALL BE INSTALLED IN PREDRILLED EDGE HOLES.

- 30. WOOD FRAMING NOTES--THE FOLLOWING APPLY UNLESS OTHERWISE SHOWN:
 - A. ALL WOOD FRAMING DETAILS NOT SHOWN OTHERWISE SHALL BE CONSTRUCTED TO THE MINIMUM STANDARDS OF THE INTERNATIONAL BUILDING CODE. MINIMUM NAILING, UNLESS OTHERWISE NOTED, SHALL CONFORM TO TABLE 2304.10.1 OF THE INTERNATIONAL BUILDING CODE. UNLESS NOTED OTHERWISE, ALL NAILS SHALL BE COMMON. COORDINATE THE SIZE AND LOCATION OF ALL OPENINGS WITH MECHANICAL AND ARCHITECTURAL DRAWINGS. PROVIDE WASHERS UNDER THE HEADS AND NUTS OF ALL BOLTS AND LAG SCREWS BEARING ON WOOD.

WALL FRAMING: ALL STUD WALLS SHOWN AND NOT OTHERWISE NOTED SHALL BE

- 2X4 STUDS @ 16"O.C. AT INTERIOR WALLS AND 2X6 @ 16"O.C. AT EXTERIOR WALLS. TWO STUDS MINIMUM SHALL BE PROVIDED AT THE END OF ALL WALLS AND AT EACH SIDE OF ALL OPENINGS. TWO 2X8 HEADERS SHALL BE PROVIDED OVER ALL OPENINGS NOT OTHERWISE NOTED. SOLID BLOCKING FOR WOOD COL-UMNS SHALL BE PROVIDED THROUGH FLOORS TO SUPPORTS BELOW. WALLS SHALL HAVE A SINGLE BOTTOM PLATE AND A DOUBLE TOP PLATE. END NAIL TOP PLATE TO EACH STUD WITH TWO 16D NAILS, AND TOENAIL OR END NAIL EACH STUD TO BOTTOM PLATE WITH TWO 16D NAILS. FACE NAIL DOUBLE TOP PLATE WITH 16D AT 12" O.C. AND LAP MINIMUM 4'-0" AT JOINTS AND PROVIDE SIX 16D NAILS AT 4" O.C. EACH SIDE OF JOINT. ALL STUD WALLS SHALL HAVE THEIR LOWER WOOD PLATES ATTACHED TO WOOD FRAMING BELOW WITH 16D NAILS AT 12" O.C. STAGGERED OR BOLTED TO CONCRETE WITH 5/8" DIAMETER ANCHOR BOLTS (WITH 7" MINIMUM EMBEDMENT)@ 4'-0" O.C. UNLESS INDICATED OTHERWISE. INDIVI-DUAL MEMBERS OF BUILT-UP POSTS SHALL BE NAILED TO EACH OTHER WITH 16D @ 12" O.C. STAGGERED. REFER TO THE PLANS AND SHEAR WALL SCHEDULE FOR REQUIRED SHEATHING AND NAILING. WHEN NOT OTHERWISE NOTED, PROVIDE GYPSUM WALLBOARD ON INTERIOR SURFACES NAILED TO ALL STUDS, TOP AND BOTTOM PLATES AND BLOCKING WITH NAILS AT 7" O.C. USE 5D COOLER NAILS FOR 1/2" GWB AND 6D COOLER NAILS FOR 5/8" GWB. WHEN NOT OTHERWISE NOTED, PROVIDE 1/2" (NOM.) APA RATED SHEATHING (SPAN RATING 24/0) ON EXTERIOR SURFACES NAILED AT ALL PANEL EDGES (BLOCK UNSUPPORTED EDGES). TOP AND BOTTOM PLATES WITH 8D @ 6" O.C. AND TO ALL INTERMEDIATE STUDS AND BLOCKING WITH 8D @ 12" O.C. ALLOW 1/8" SPACING AT ALL PANEL EDGES AND ENDS.
- C. FLOOR AND ROOF FRAMING: PROVIDE DOUBLE JOISTS AROUND ALL OPENINGS IN FLOORS OR ROOFS UNLESS OTHERWISE NOTED. PROVIDE SOLID BLOCKING AT ALL BEARING POINTS. TOENAIL JOISTS TO SUPPORTS WITH TWO 16D NAILS. ATTACH TIMBER JOISTS TO FLUSH HEADERS OR BEAMS WITH METAL JOIST HANGERS IN ACCORDANCE WITH TIMBER CONNECTOR NOTE. NAIL ALL MULTI-JOIST BEAMS TO-GETHER WITH 16D @ 12" O.C. STAGGERED. UNLESS OTHERWISE NOTED ON THE PLANS, ROOF AND FLOOR SHEATHING SHALL BE LAID UP WITH STRENGTH AXIS PERPENDICULAR TO SUPPORTS AND NAILED WITH 8D NAILS @ 6" O.C. TO FRAMED PANEL EDGES AND OVER STUD WALLS AS SHOWN ON PLANS AND @ 12" O.C. TO INTERMEDIATE SUPPORTS. PROVIDE APPROVED PLYWOOD EDGE CLIPS CENTERED BETWEEN JOISTS/TRUSSES AT UNBLOCKED ROOF SHEATHING EDGES. ALL FLOOR SHEATHING EDGES SHALL HAVE APPROVED TONGUE-AND-GROOVE JOINTS OR SHALL BE SUPPORTED WITH SOLID BLOCKING. ALLOW 1/8" SPACING AT ALL PANEL EDGES AND ENDS OF ALL ROOF AND FLOOR SHEATHING. TOENAIL BLOCKING TO SUPPORTS WITH 16D @ 12" O.C. UNLESS OTHERWISE NOTED. AT BLOCKED FLOOR AND ROOF DIAPHRAGMS PROVIDE FLAT 2X BLOCKING AT ALL UNFRAMED PLYWOOD PANEL EDGES AND NAIL WITH EDGE NAILING SPECIFIED.

HV

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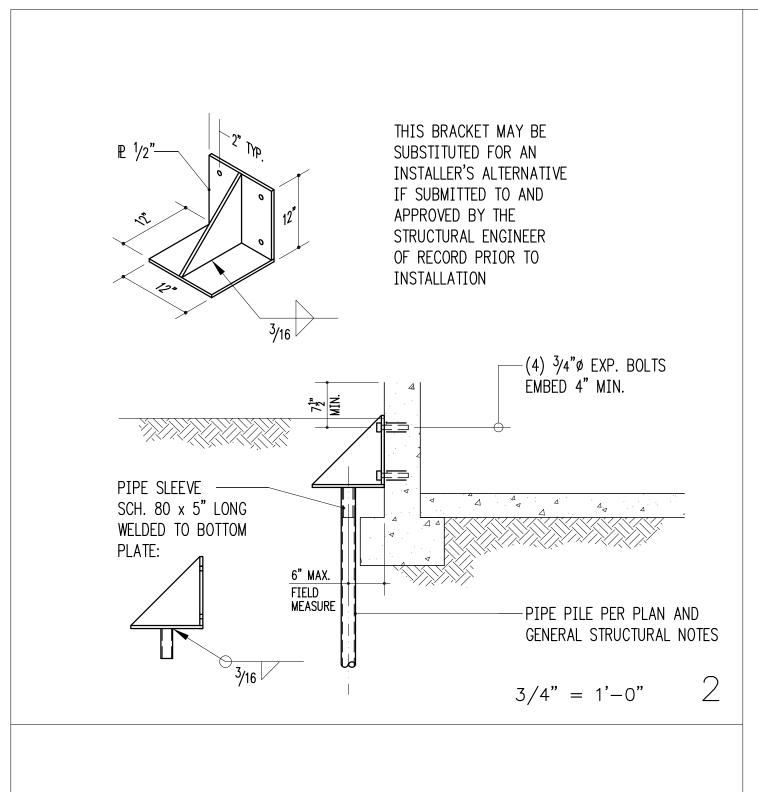
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7/16/19	Framing Revisions	
7/26/19	Framing Revisions	
1/17/20	Corrections #1	
4/3/20	Framing Revisions	
4/17/20	Construction	
4/24/20	Trellis Revisions	
6/1/20	Corrections #2	
8/17/20	Revisions	
8/31/20	Foundation Revisions	
10/8/20	Framing Revisions	
10/14/20	Pin Pile Revision	
11/13/20	Revisions	
11/30/20	Revisions	

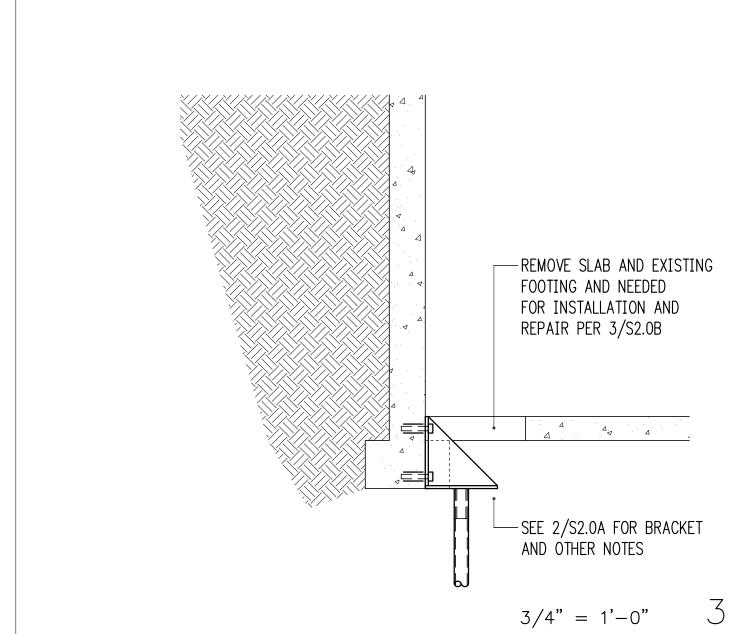
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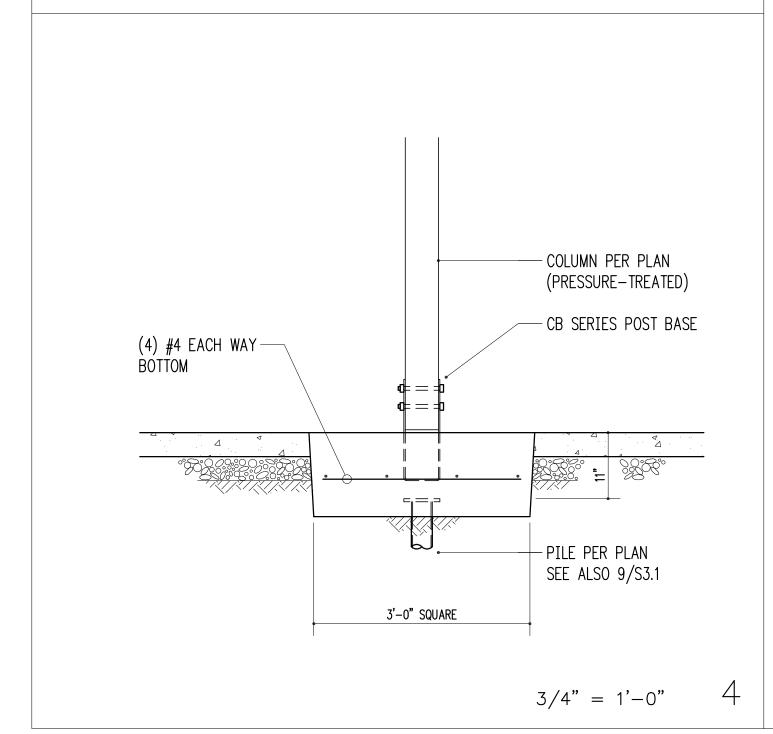
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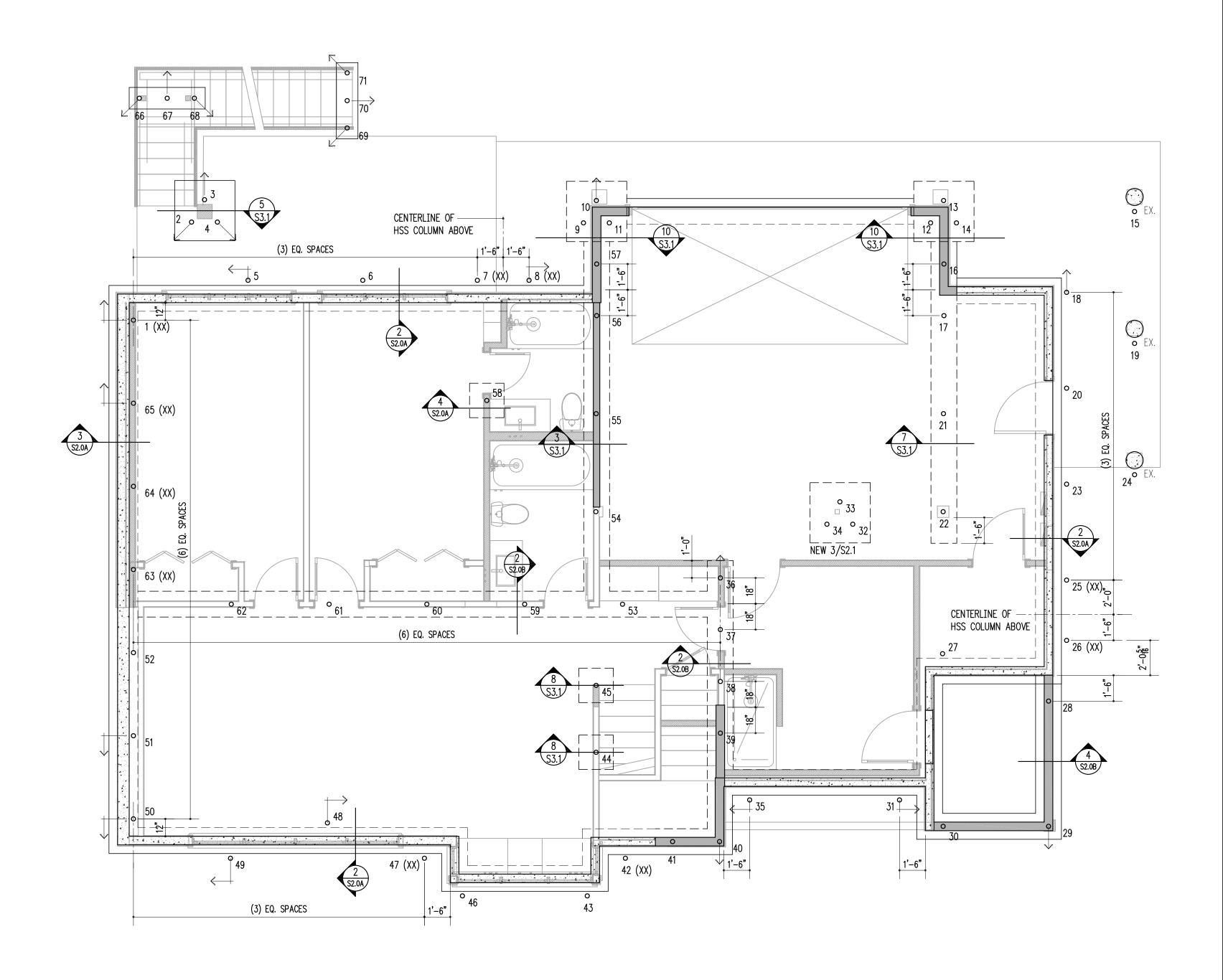
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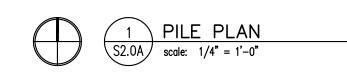
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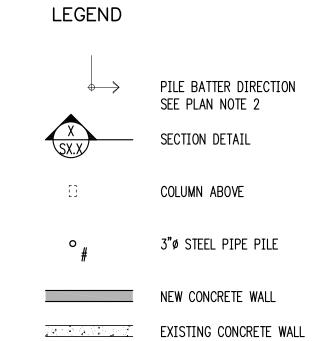












PILE PLAN NOTES

- ALL PILES 3"Ø PER GENERAL NOTES PILES MARKED AS (XX) SHALL BE DOUBLE-EXTRA STRONG PIPE
- 2. WHERE MARKED WITH \$\rightarrow\$ BATTER PILE AT 15 DEGREES FROM VERTICAL IN THE DIRECTION INDICATED
- 3. GEOTECHNICAL ENGINEER OF RECORD MUST BE PRESENT FOR PILE DRIVING.
- 4. SEE GENERAL STRUCTURAL NOTES FOR HAMMER AND DRIVING REQUIREMENTS
- 5. UNLESS NOTED OTHERWISE, ALL PILES SHALL BE CENTERED UNDER THE STEMWALL DIRECTLY ABOVE
- 6. SEE X/SX.0 FOR PILE EMBEDMENT INTO FOOTING REQUIREMENTS

THIS SHEET ENTIRELY NEW



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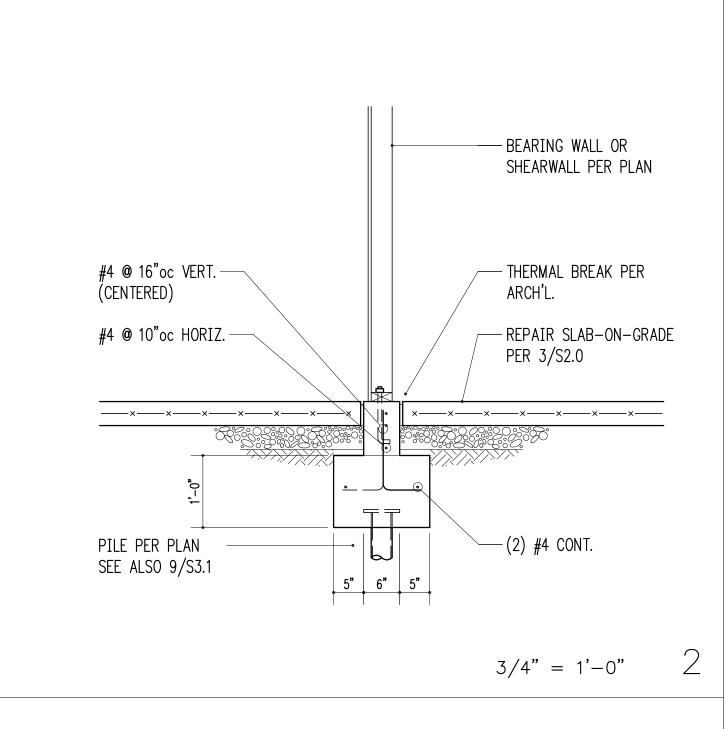
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8/31/20	Foundation Revisions	
10/8/20	Framing Revisions	
10/14/20	Pin Pile Revision	
11/13/20	Revisions	
11/30/20	Revisions	

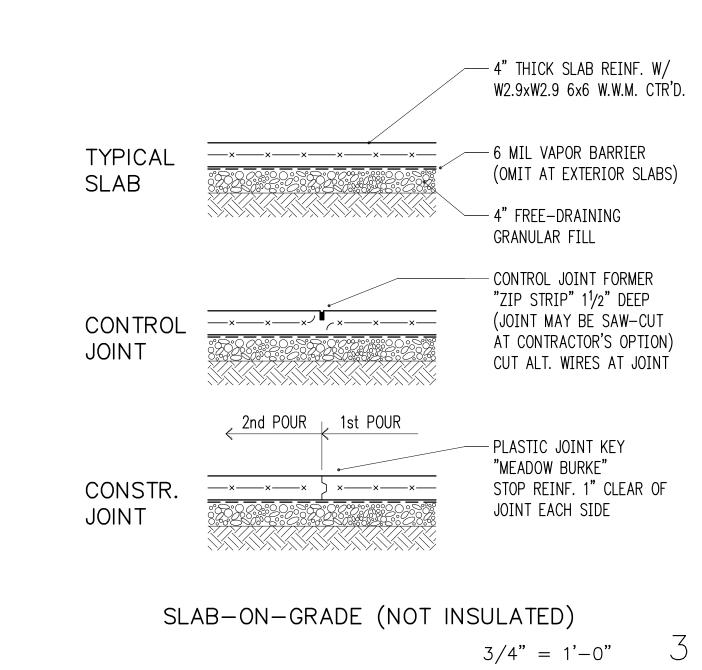
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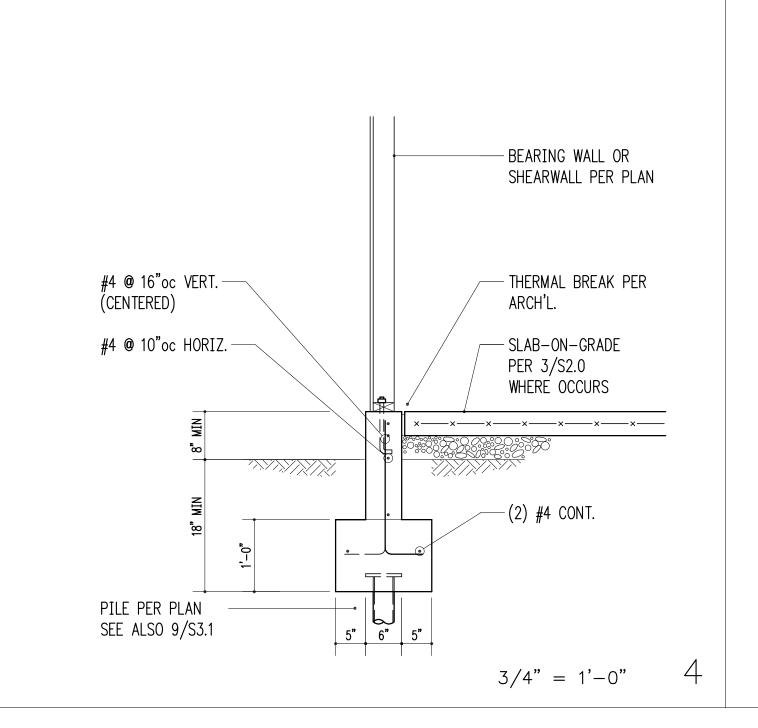
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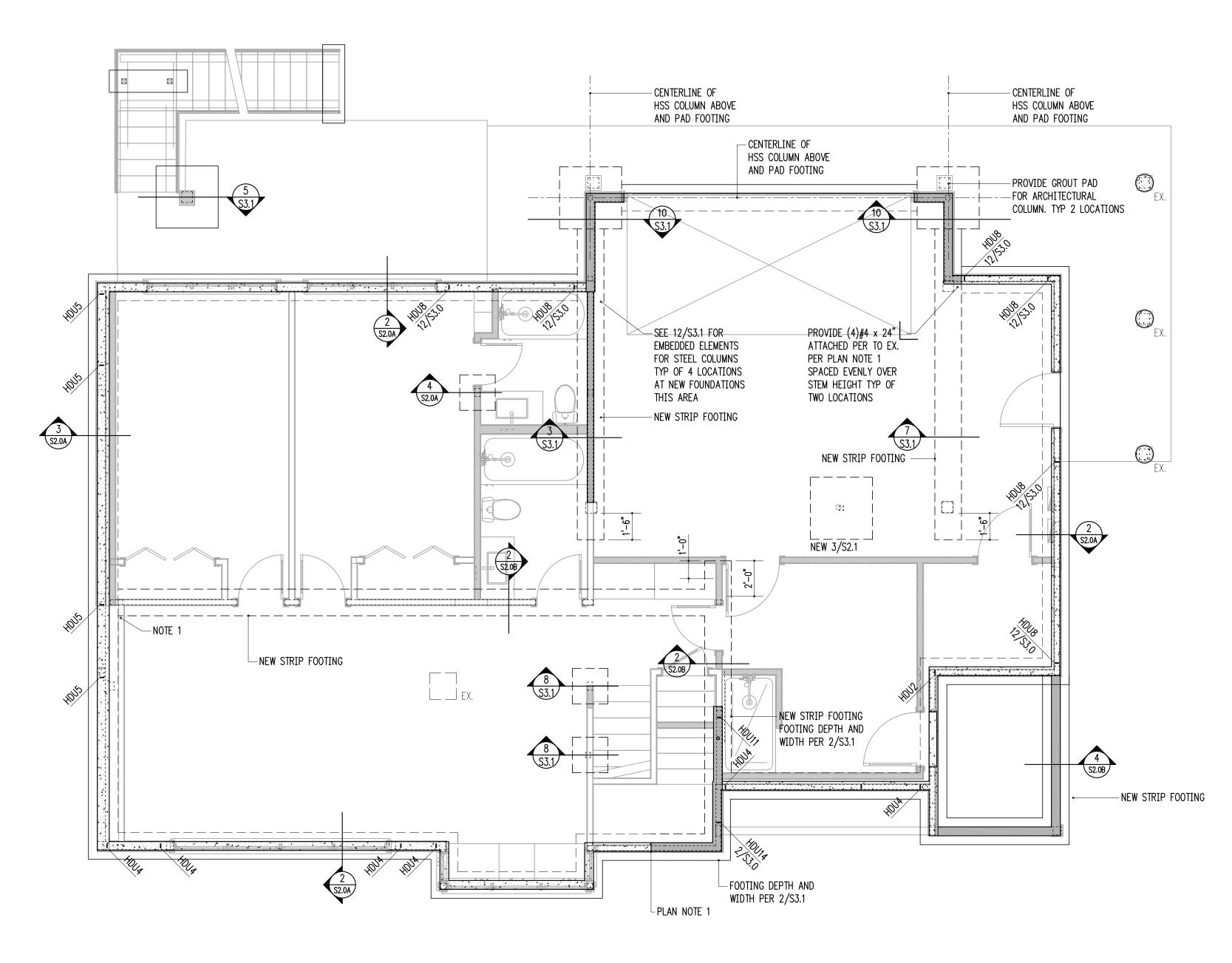
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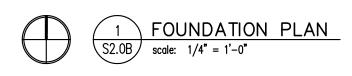
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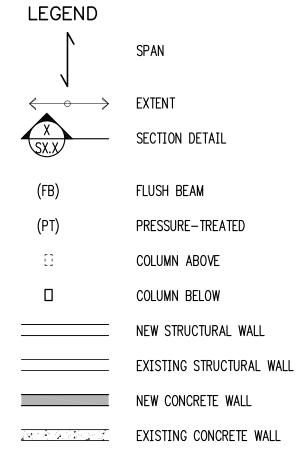




HANGER SCHEDULE

HANGER	FACE NAILING	CAPACITY (Cd = 1.0)
LUS210	10d COMMON	1275 lb
HUS210-2	0.162x3 ¹ / ₂	2110 lb
HUC410	0.162x3 ¹ / ₂	2680 lb
IUS3.56/11.88	10d COMMON	1405 lb
HUC410	0.162x3 ¹ /2	2680 lb
	LUS210 HUS210-2 HUC410 IUS3.56/11.88	LUS210 10d COMMON HUS210-2 0.162x3½ HUC410 0.162x3½ IUS3.56/11.88 10d COMMON

NOTE!! EXISTING FRAMING MEMBERS AS INDICATED ON THIS PLAN ARE ASSUMED FOR DESIGN PURPOSES ONLY. HARRIOTT VALENTINE ENGINEERS SHALL NOT BE HELD LIABLE FOR LOCATION/ SIZE OF EXISTING MEMBERS AS CALLED ON THIS PLAN. EXISTING MEMBERS SHALL BE VERIFIED AND REPORTED TO STRUCTURAL ENGINEER OF RECORD PRIOR TO CONSTRUCTION.



ALL-THREAD HOLDOWN AT END OF SHEARWALL ABOVE STRAP HOLDOWN AT

END OF SHEARWALL ABOVE

FOUNDATION PLAN NOTES

- 1. WHERE NEW CONCRETE WALLS OR FOOTING ABUT EX. CONCRETE, PROVIDE DOWELS #4 x 2'-0" TO MATCH HORIZ. REINFORCING, EMBED 5" IN EPOXY GROUT.
- 2. SEE 10/S4.0 FOR TYPICAL HOLDOWN REQUIREMENTS AT CONCRETE WALLS AND FOOTINGS.
- 3. SLAB-ON-GRADE SHALL BE PLACED AND CURED FOR A MINIMUM OF SEVEN DAYS BEFORE RETAINING WALLS ARE BACKFILLED. SEE RETAINING WALL DETAILS FOR SPECIFIC CONFIGURATION.
- 4. SEE 11/S3.0 FOR CORNER REINFORCING AT NEW CONCRETE STEMS AND FOOTINGS

THIS SHEET ENTIRELY REVISED

FOUNDATION PLAN

S2.0B



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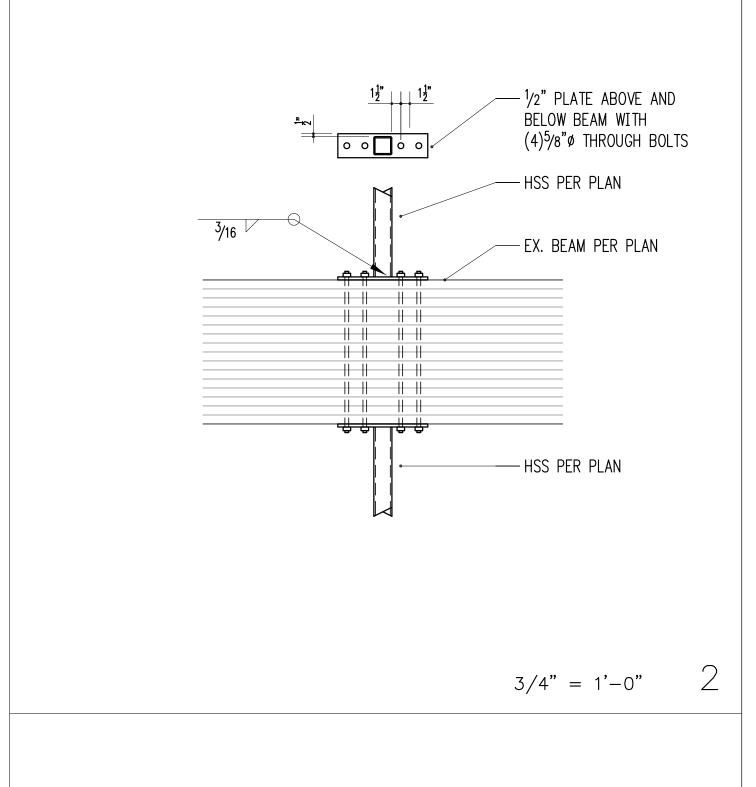
Werelius Residence 8452 North Mercer Way Mercer Island, WA 98040

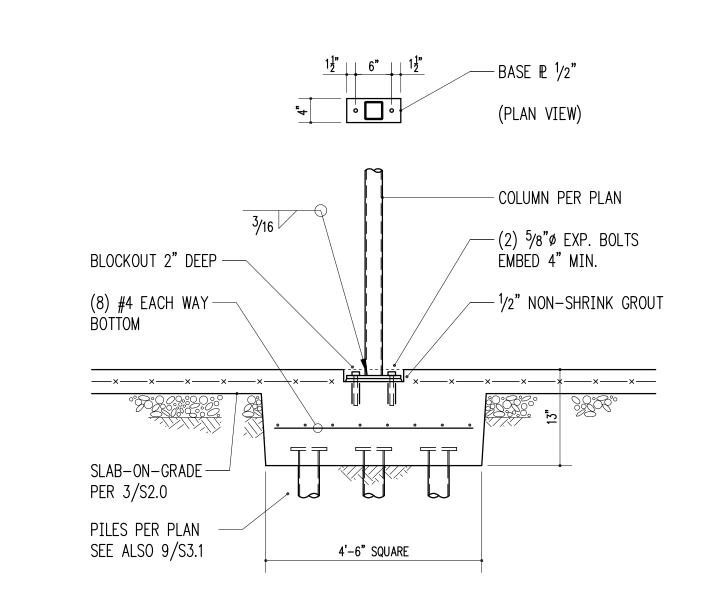
Issue Date

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10/8/20	Framing Revisions	
10/14/20	Pin Pile Revision	
11/13/20	Revisions	
11/30/20	Revisions	

Building Department Approval

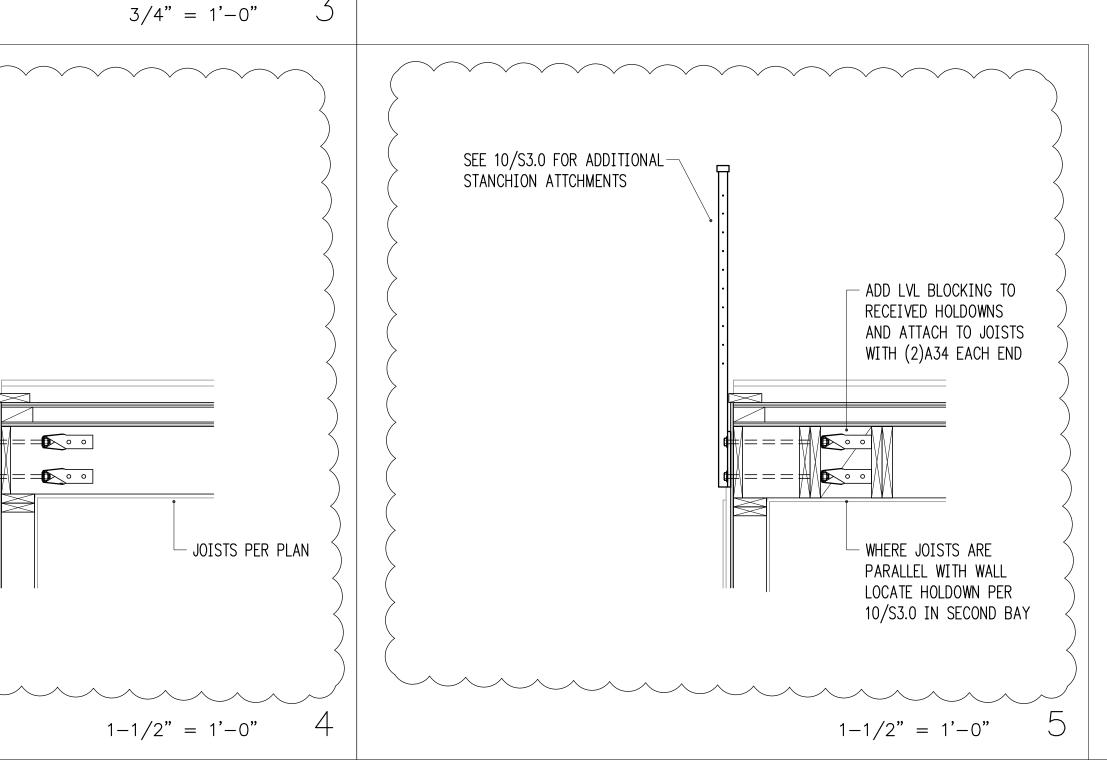
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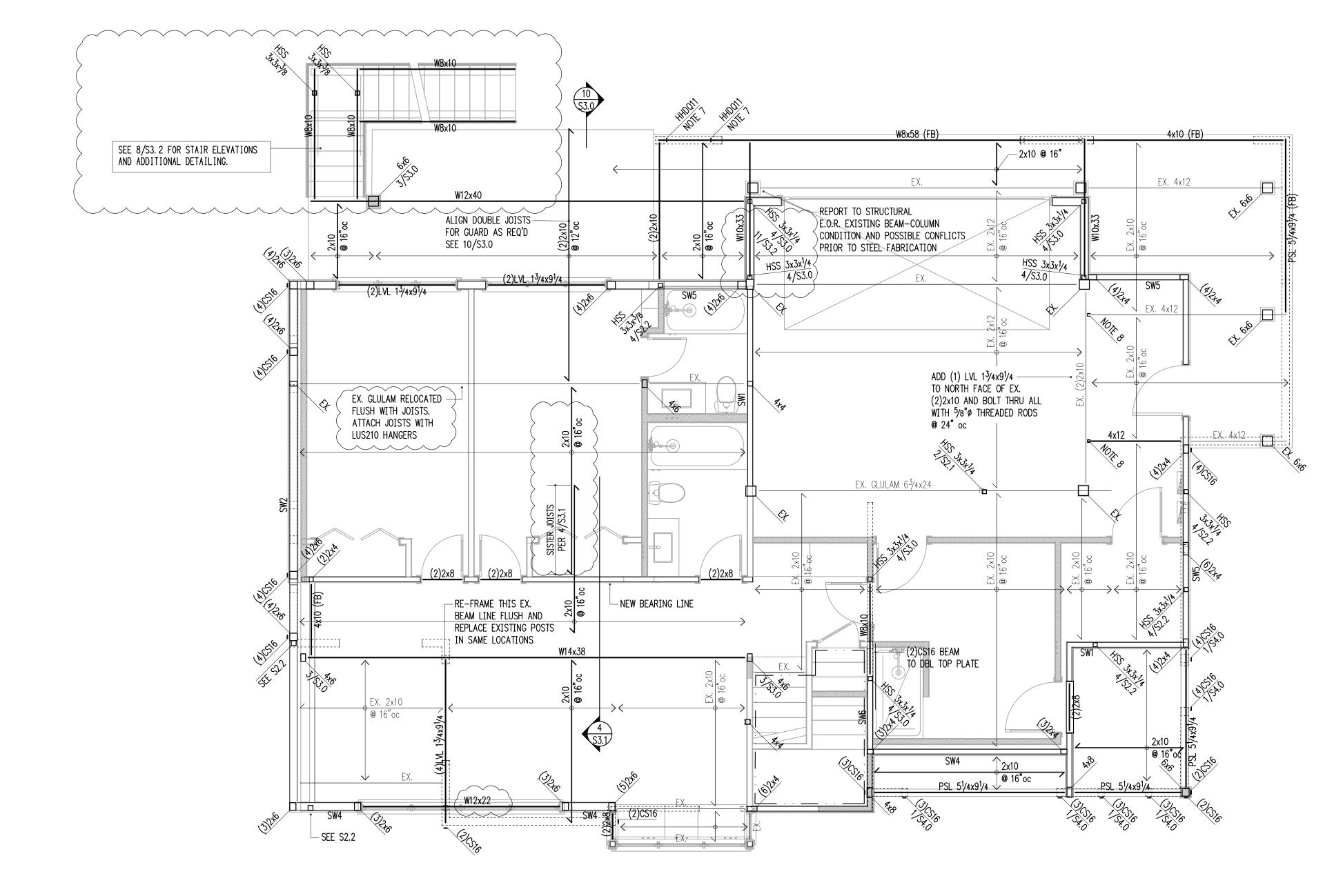




SEE 10/S3.0 FOR ADDITIONAL

STANCHION ATTCHMENTS



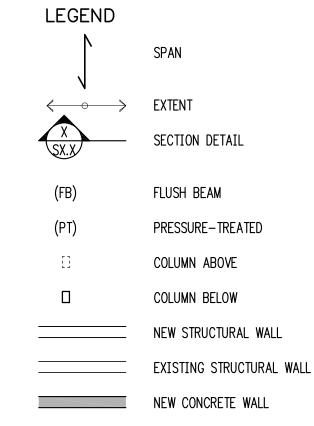




HANGER SCHEDULE

MEMBER (FLAT ONLY)	HANGER	FACE NAILING	CAPACITY (Cd = 1.0)
2x10 or 2x12	LUS210	10d COMMON	1275 lb
(2)2x10	HUS210-2	0.162x3 ¹ /2	2110 lb
4x10	HUC410	0.162x3 ¹ /2	2680 lb
11 ⁷ /8" TJI 560	IUS3.56/11.88	10d COMMON	1405 lb
(2) 1 ³ /4x9 ¹ /4 LVL	HUC410	0.162x3 ¹ /2	2680 lb

NOTE!! EXISTING FRAMING MEMBERS AS INDICATED ON THIS PLAN ARE ASSUMED FOR DESIGN PURPOSES ONLY. HARRIOTT VALENTINE ENGINEERS SHALL NOT BE HELD LIABLE FOR LOCATION/ SIZE OF EXISTING MEMBERS AS CALLED ON THIS PLAN. EXISTING MEMBERS SHALL BE VERIFIED AND REPORTED TO STRUCTURAL ENGINEER OF RECORD PRIOR TO CONSTRUCTION.



ALL—THREAD HOLDOWN AT END OF SHEARWALL ABOVE

STRAP HOLDOWN AT END OF SHEARWALL ABOVE

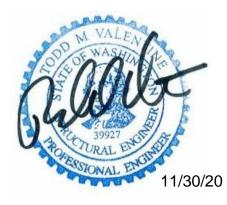
FRAMING PLAN NOTES

- 1. SW__ INDICATES SHEARWALL TYPE PER SCHEDULE 8/S4.0. REFER TO DETAILS FOR TYPICAL SHEARWALL CONSTRUCTION. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL WALL INFORMATION.
- 2. REFER TO GENERAL STRUCTURAL NOTES FOR FLOOR OR ROOF SHEATHING TYPE, THICKNESS, AND NAILING.
- 3. COLUMNS SHALL BE DOUBLE STUD MINIMUM, UNLESS NOTED OTHERWISE. SEE 11/S4.0.
- 4. AT ALL SHEARWALLS PROVIDE DOUBLE TOP PLATES AND SPLICE PER 12/S4.0.
- 5. CS__ INDICATES COILED STRAP TYPE PER SCHEDULE 6/S4.0. REFER TO DETAILS FOR TYPICAL STRAP ASSEMBLY.
- 6. POSTS □, INCLUDING ENDS OF WALL OPENINGS, SHALL BE (2)2x6 UNLESS NOTED OTHERWISE.
- 7. WELD THREADED RODS FOR HOLD DOWNS AND BOTTOM PLATE ATTACHMENT TO STEEL BEAMS BELOW
- 8. WHERE NOTED ON PLAN ATTACH 4x12 TO BEAM ABOVE WITH SIMPSON THA426 TRUSS HANGER



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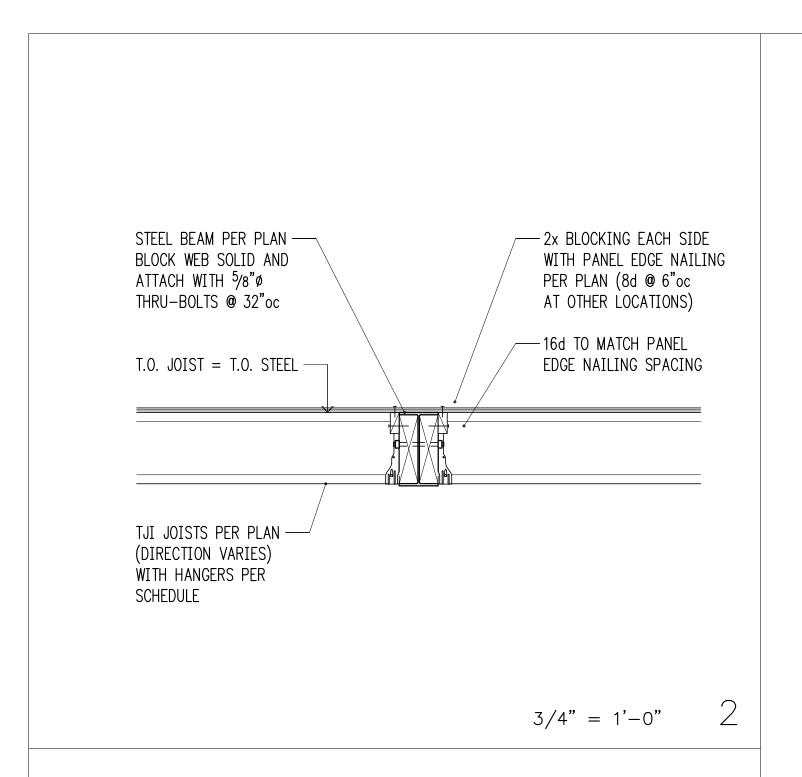
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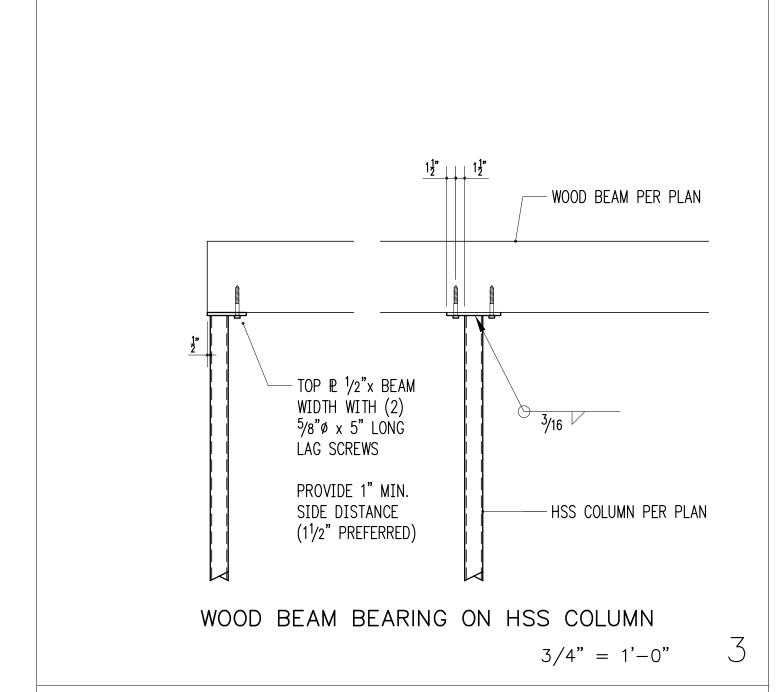
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MAIN FLOOR
FRAMING PLAN

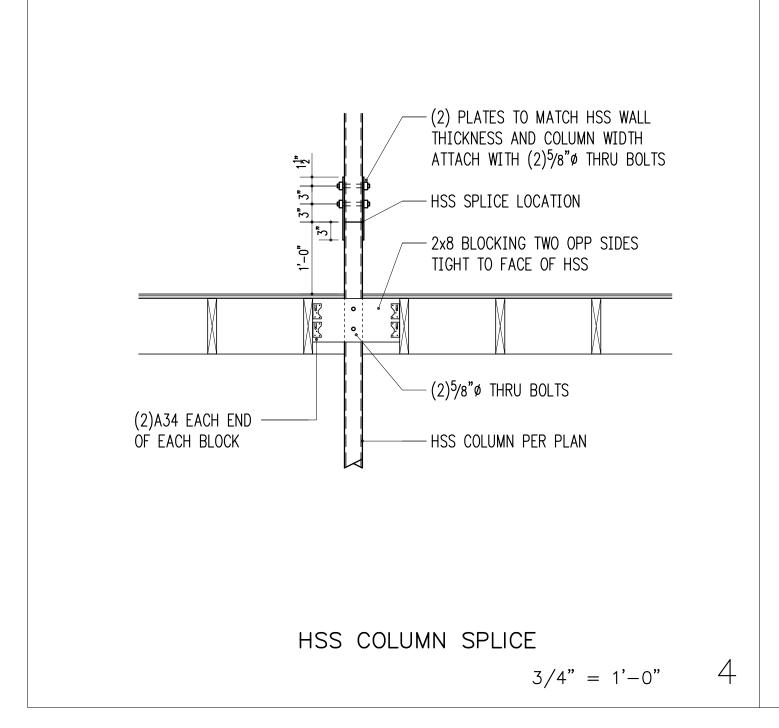
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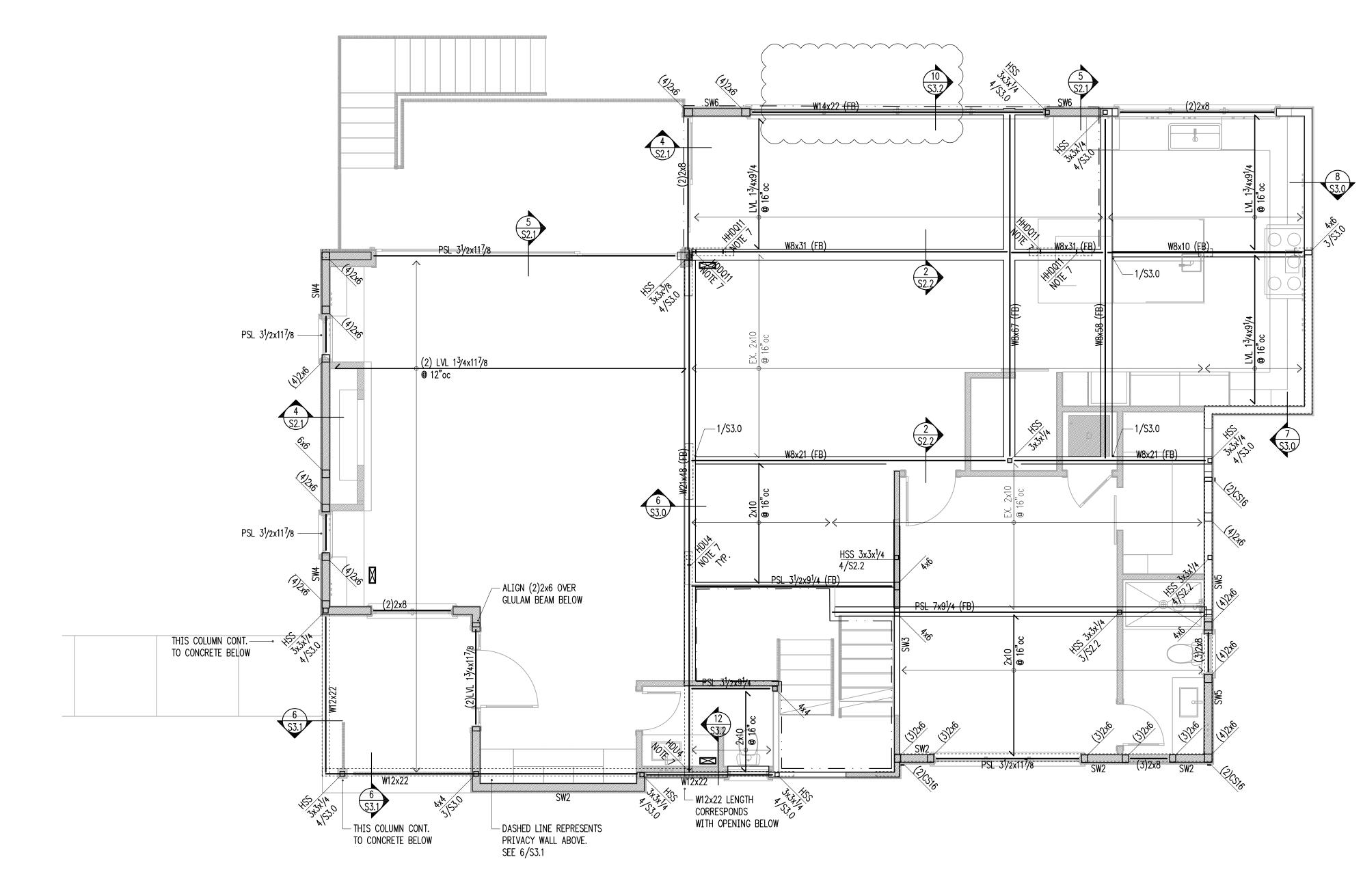
S2.1

VERELIUS RESIDENCE







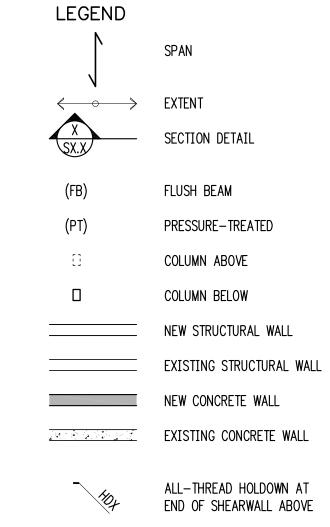




HANGER SCHEDULE

MEMBER (FLAT ONLY)	HANGER	FACE NAILING	CAPACITY (Cd = 1.0)
2x10 or 2x12	LUS210	10d COMMON	1275 lb
(2)2x10	HUS210-2	0.162x3 ¹ /2	2110 lb
4x10	HUC410	0.162x3 ¹ /2	2680 lb
11 ⁷ /8" TJI 560	IUS3.56/11.88	10d COMMON	1405 lb
(2) 1 ³ /4x9 ¹ /4 LVL	HUC410	0.162x3 ¹ /2	2680 lb

NOTE!! EXISTING FRAMING MEMBERS AS INDICATED ON THIS PLAN ARE ASSUMED FOR DESIGN PURPOSES ONLY. HARRIOTT VALENTINE ENGINEERS SHALL NOT BE HELD LIABLE FOR LOCATION/ SIZE OF EXISTING MEMBERS AS CALLED ON THIS PLAN. EXISTING MEMBERS SHALL BE VERIFIED AND REPORTED TO STRUCTURAL ENGINEER OF RECORD PRIOR TO CONSTRUCTION.



STRAP HOLDOWN AT END OF SHEARWALL ABOVE

FRAMING PLAN NOTES 1 SW INDICATES SHEARWALL TYPE

- SW__ INDICATES SHEARWALL TYPE PER SCHEDULE 8/S4.0. REFER TO DETAILS FOR TYPICAL SHEARWALL CONSTRUCTION. SEE ARCHITECTURAL DRAWINGS FOR ADDITIONAL WALL INFORMATION.
- 2. REFER TO GENERAL STRUCTURAL NOTES FOR FLOOR OR ROOF SHEATHING TYPE, THICKNESS, AND NAILING.
- 3. COLUMNS SHALL BE DOUBLE STUD MINIMUM, UNLESS NOTED OTHERWISE. SEE 11/S4.0.
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- 5. CS__ INDICATES COILED STRAP TYPE PER SCHEDULE 6/S4.0. REFER TO DETAILS FOR TYPICAL STRAP ASSEMBLY.
- 6. POSTS □, INCLUDING ENDS OF WALL OPENINGS, SHALL BE (2)2x6 UNLESS NOTED OTHERWISE.
- 7. WELD THREADED RODS FOR HOLD DOWNS AND BOTTOM PLATE ATTACHMENT TO STEEL BEAMS BELOW

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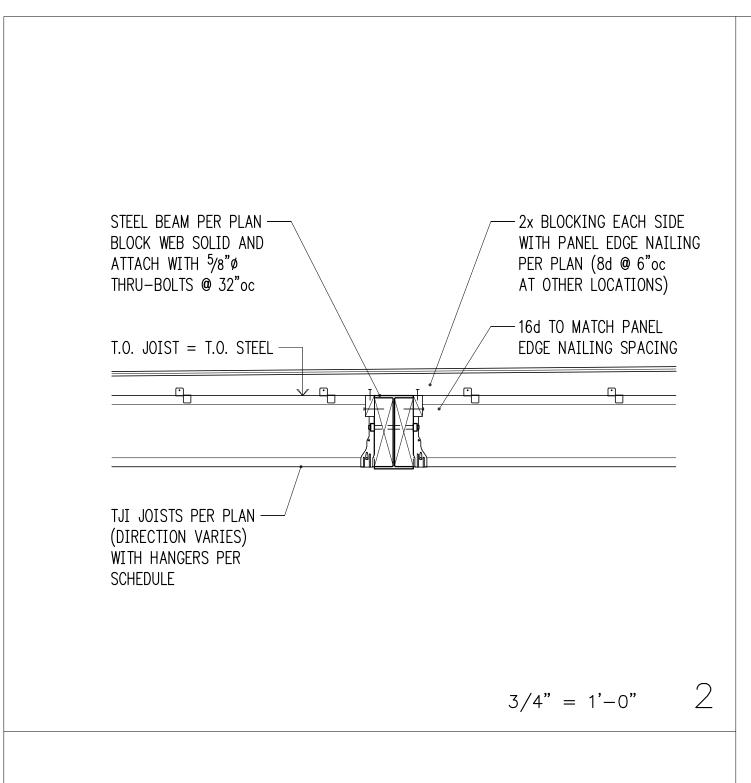
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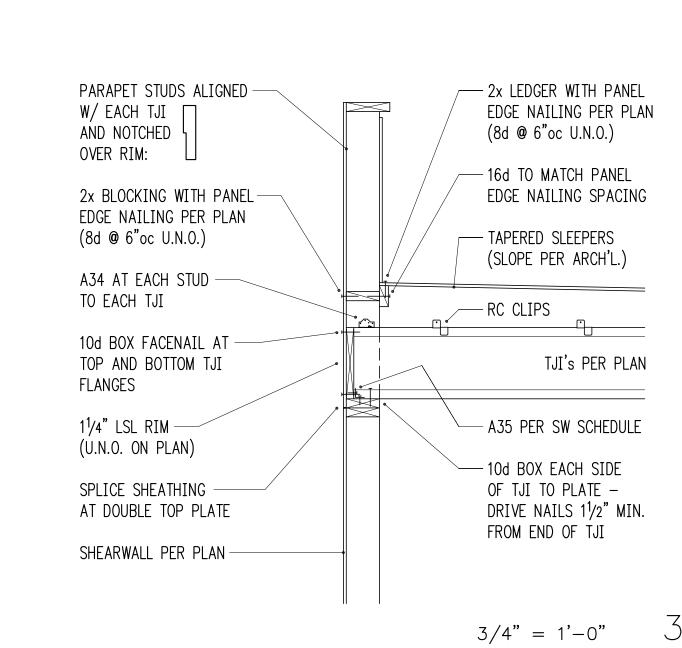
Drawing Title
SECOND FLOOR
FRAMING PLAN

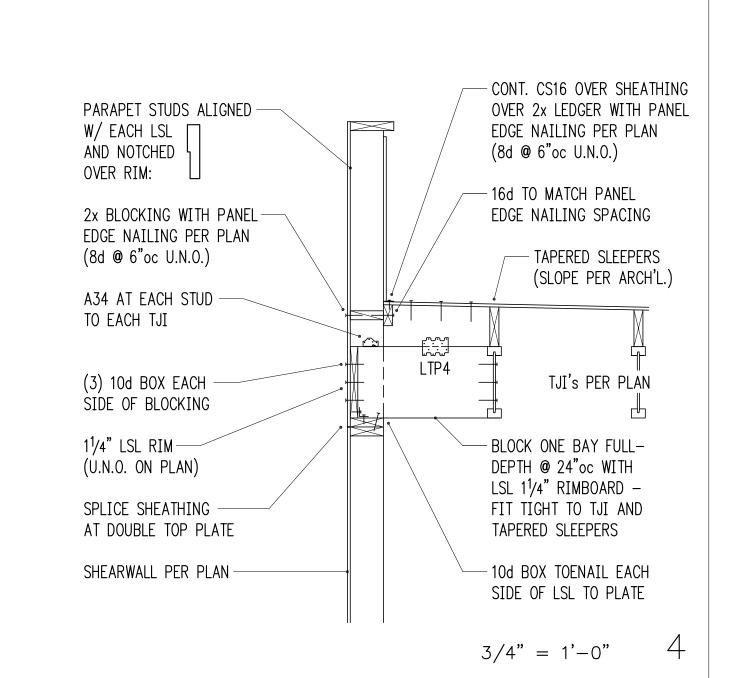
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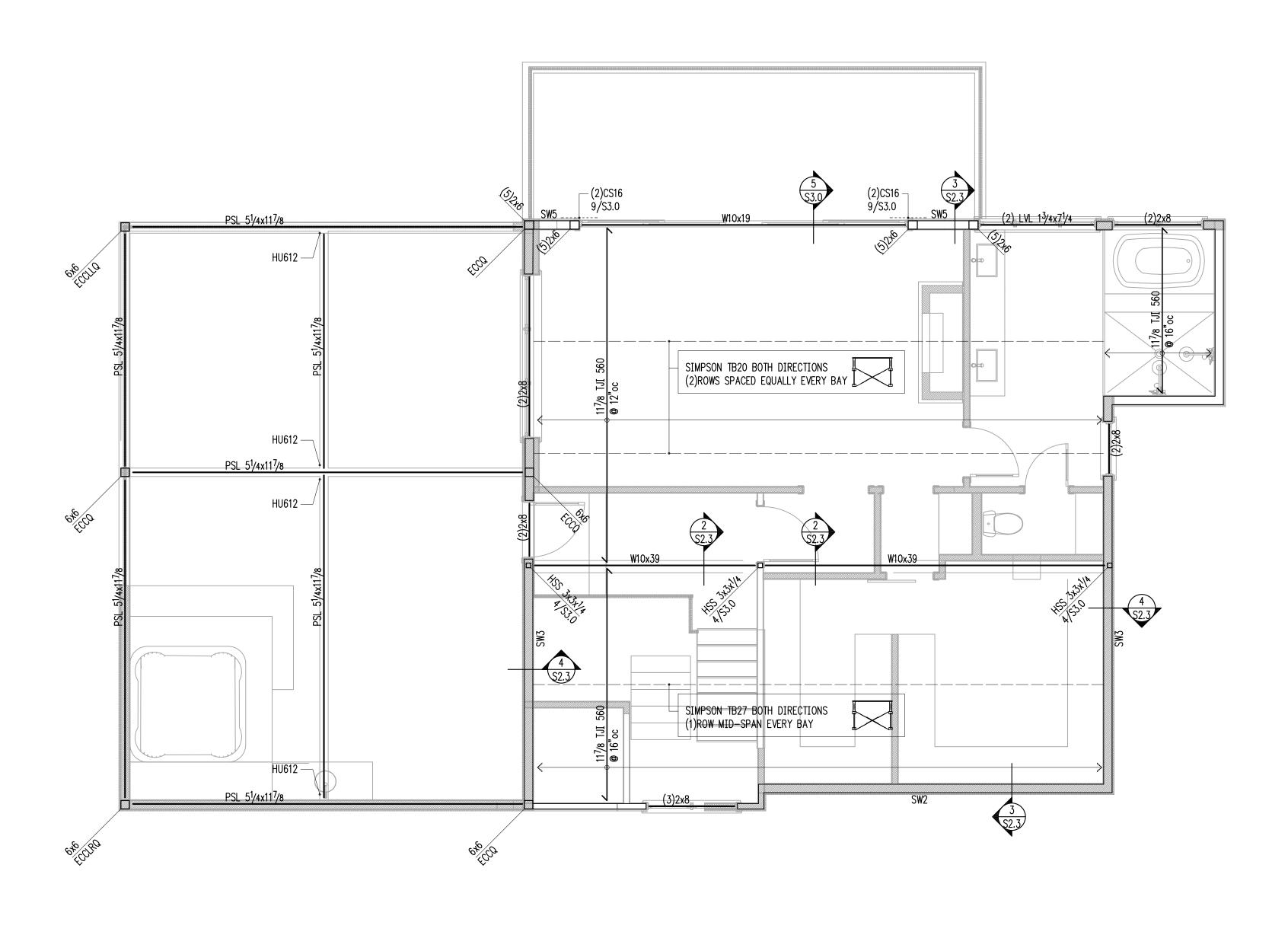
S2.2

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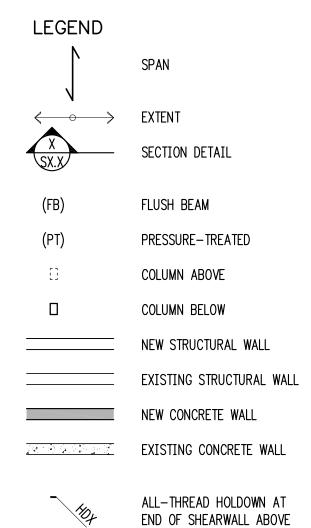




HANGER SCHEDULE

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2x10 or 2x12	LUS210	10d COMMON	1275 lb
(2)2x10	HUS210-2	0.162x3 ¹ /2	2110 lb
4x10	HUC410	0.162x3 ¹ /2	2680 lb
11 ⁷ /8" TJI 560	IUS3.56/11.88	10d COMMON	1405 lb
(2) 1 ³ /4x9 ¹ /4 LVL	HUC410	0.162x3 ¹ /2	2680 lb

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STRAP HOLDOWN AT

END OF SHEARWALL ABOVE

FRAMING PLAN NOTES

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- REFER TO GENERAL STRUCTURAL NOTES FOR FLOOR OR ROOF SHEATHING TYPE, THICKNESS, AND NAILING.
- 3. COLUMNS SHALL BE DOUBLE STUD MINIMUM, UNLESS NOTED OTHERWISE. SEE 11/S4.0.
- 4. AT ALL SHEARWALLS PROVIDE DOUBLE TOP PLATES AND SPLICE PER 12/S4.0.
- CS__ INDICATES COILED STRAP TYPE PER SCHEDULE 6/S4.0. REFER TO DETAILS FOR TYPICAL STRAP ASSEMBLY.
- 6. POSTS □, INCLUDING ENDS OF WALL OPENINGS, SHALL BE (2)2x6 UNLESS NOTED OTHERWISE.

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11/13/20	Revisions
11/30/20	Revisions

Drawing Title

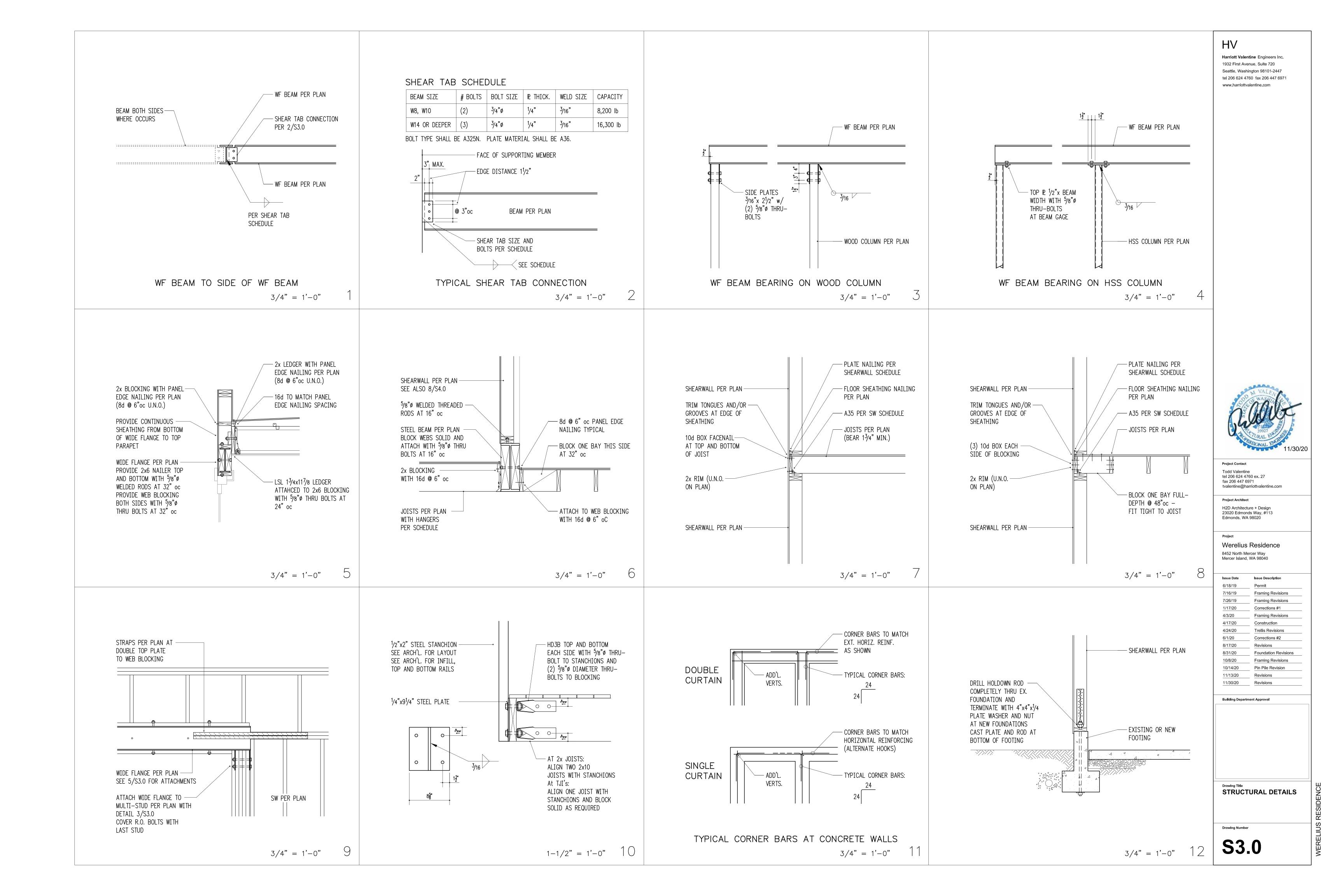
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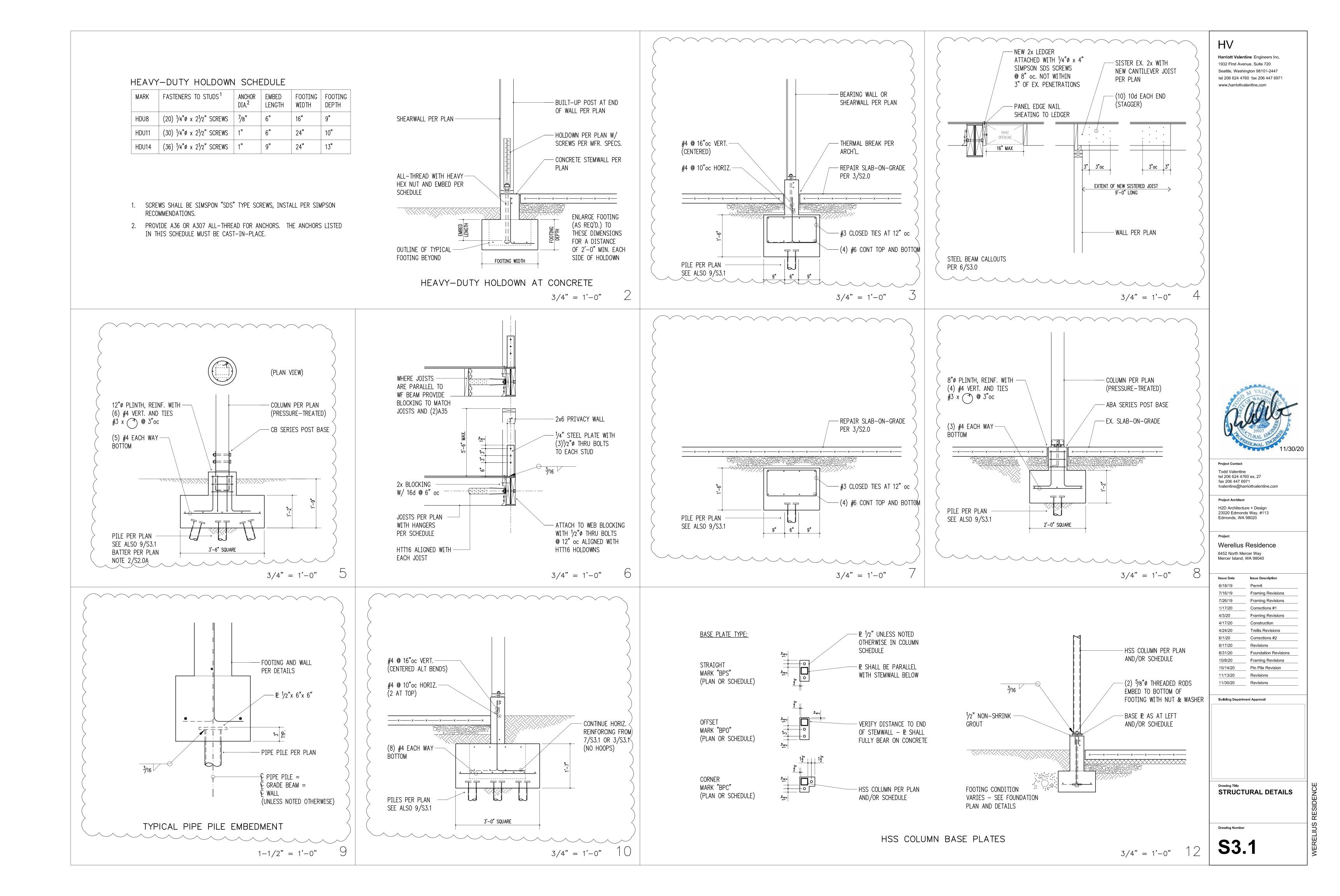
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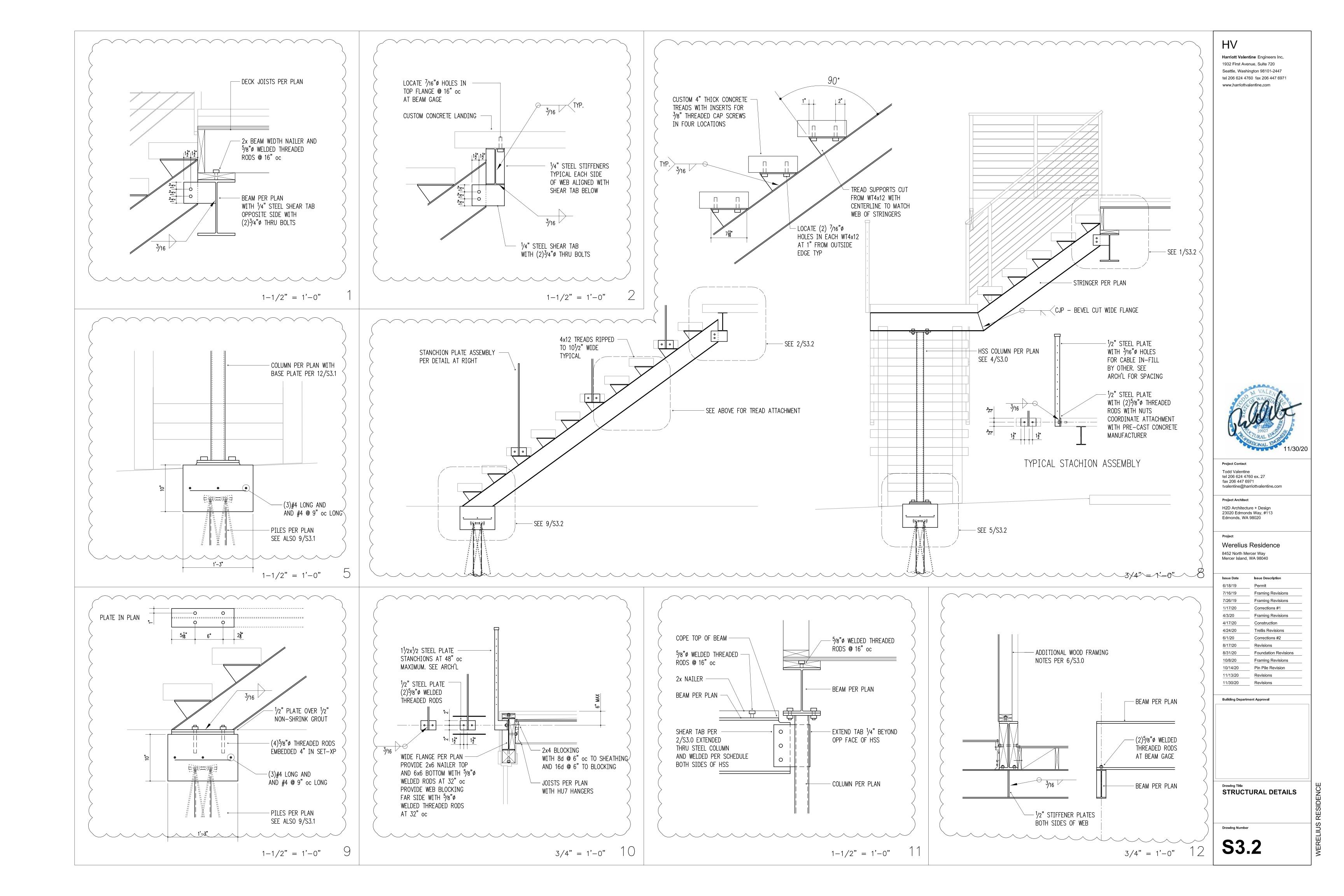
FRAMING PLAN

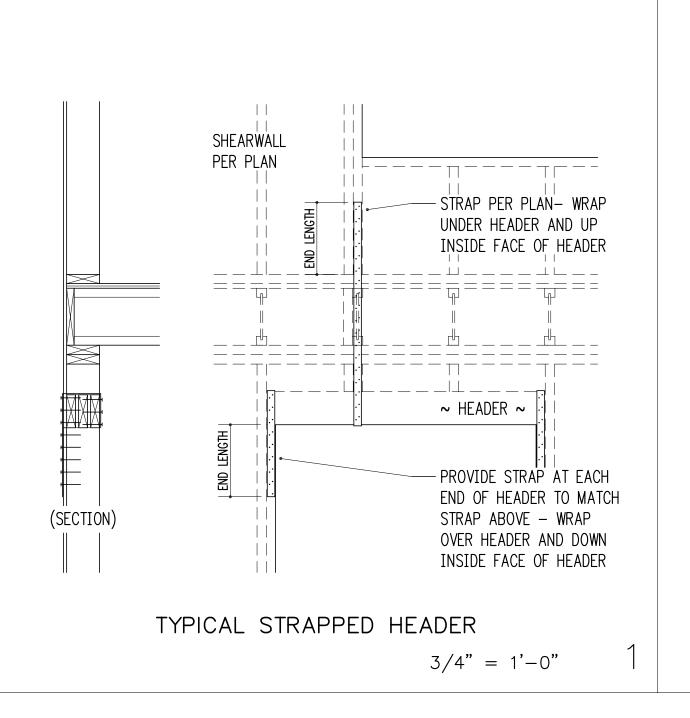
S2.3

WEREI IIS RESIDENCE

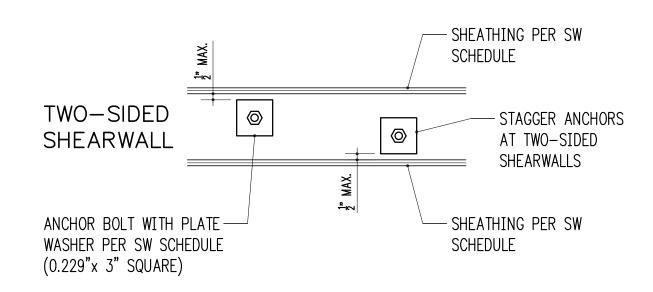








ONE—SIDED SHEARWALL

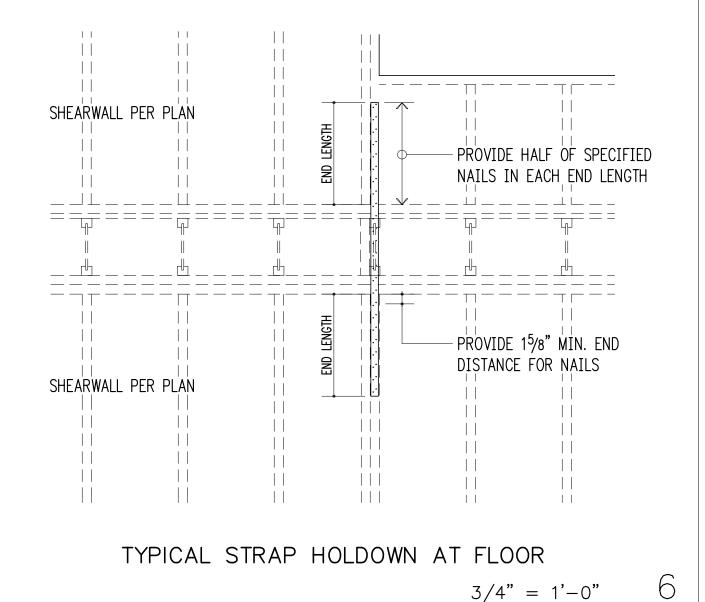


TYPICAL SHEARWALL ANCHOR BOLT PLACEMENT 1-1/2" = 1'-0"

STRAP SCHEDULE (NOT ALL USED)

		`	,
MARK	END LENGTH	NAILS	NAIL SPACING
CMST12	44"	(98) 10d x 3"	1 ³ /4"
CMST14	34"	(76) 10d x 3"	1 ³ /4"
CMSTC16	25"	(58) 12d x 3 ¹ /4"	11/2"
CS14	19"	(36) 8d x 2 ¹ /2"	21/16"
CS16	14"	(26) 8d x 2 ¹ /2"	21/16"
CS18	12"	(22) 8d x 2 ¹ /2"	21/16"
CS20	9"	(16) 8d x 2 ¹ /2"	21/16"
CS22	8"	(14) 8d x 2 ¹ /2"	21/16"

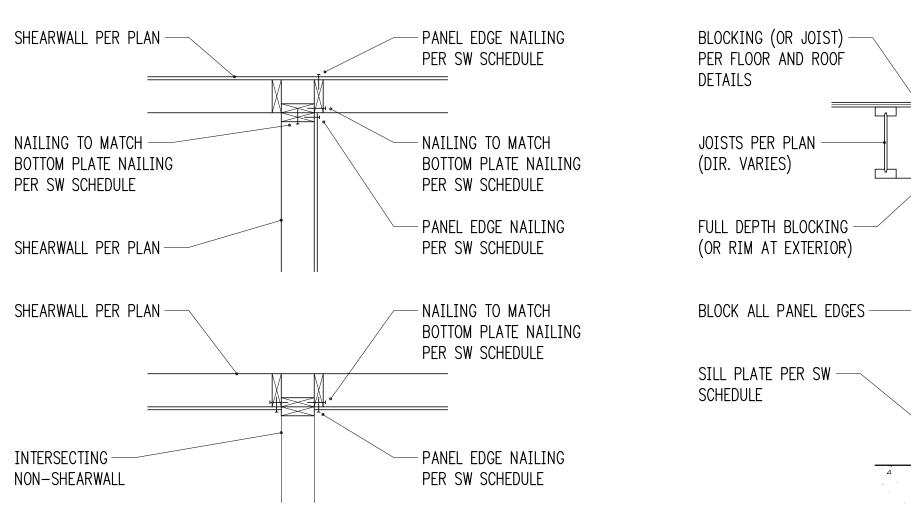
- 1. 10d AND 12d DIAMETER = 0.148"; 8d DIAMETER = 0.131".
- 2. USE HALF OF THE REQUIRED NAILS IN EACH MEMBER BEING CONNECTED (i.e. IN EACH END LENGTH).

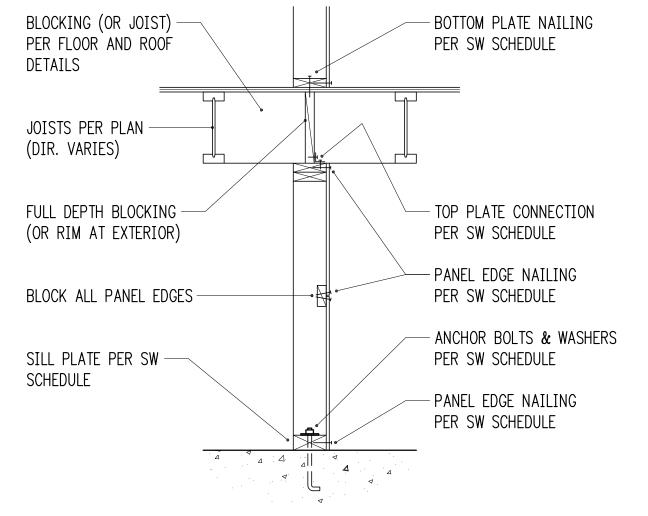


SHEARWALL SCHEDULE (NOT ALL USED ON PLANS)

MARK	MARK SHEATHING ¹	STUDS AT ABUTTING PANEL EDGES ²	PANEL EDGE NAILING ^{3,4}	RIM JOIST OR BLOCKING TO TOP PLATE		BOTTOM PLATE ATTACHMENT		
				SOLID RIM	TJI RIM	BOTTOM PLATE TO RIM JOIST BELOW 4	ANCHOR BOLT TO CONCRETE 5	SILL PLATE AT FOUND.
SW1	15/32" CDX PLYWOOD	2x	8d @ 6"oc	A35 @ 24"oc	16d @ 6"oc	16d @ 6"oc	⁵ /8"ø @ 48"oc	2x
SW2	15/32" CDX PLYWOOD	2x	8d @ 4"oc	A35 @ 15"oc	16d @ 4"oc	16d @ 4"oc	⁵ /8"ø @ 32"oc	2x
SW3	15/32" CDX PLYWOOD	3x	8d @ 3"oc	A35 @ 12"oc	N/A - USE SOLID RIM	16d @ 3"oc	⁵ /8"ø @ 16"oc	2x
SW4	15/32" CDX PLYWOOD	3x	8d @ 2"oc	A35 @ 9"oc	N/A - USE SOLID RIM	16d @ 2"oc	⁵ /8"ø @ 12"oc	2x
SW5	15/32" CDX PLYWOOD BOTH SIDES	3x	8d @ 3"oc	A35 @ 6"oc	N/A - USE SOLID RIM	(2) ROWS 16d @ 3"oc	⁵ /8"ø @ 12"oc	3x
SW6	15/32" CDX PLYWOOD BOTH SIDES	3x	8d @ 2"oc	A35 @ 4 ¹ /2"oc	N/A - USE SOLID RIM	(2) ROWS 16d @ 2"oc	5∕8"ø @ 12"oc	3x

- 1. WALL SHEATHING SHALL CONSIST OF APA RATED PLYWOOD WITH SPAN RATING 24/0. ALLOW 1/8" SPACING AT ALL PANEL EDGES AND ENDS OF PANELS. 7/16" APA RATED SHEATHING (OSB) MAY BE USED IN PLACE OF 15/32" CDX.
- 2. STUDS AT ABUTTING PANEL EDGES MAY CONSIST OF (2)2x STUDS IN PLACE OF 3x STUDS NAIL (2)2x STUDS TOGETHER WITH BOTTOM PLATE ATTACHMENT NAILING.
- 3. BLOCK ALL PANEL EDGES W/ 2x4 FLAT, ATTACH W/ PANEL EDGE NAILING. TWO STUDS MINIMUM ARE REQUIRED AT EACH END OF ALL SHEARWALLS. END STUDS SHALL RECEIVE PANEL EDGE NAILING. INTERMEDIATE STUDS SHALL BE 2x STUDS. NAIL SHEATHING TO INTERMEDIATE FRAMING MEMBERS WITH 8d @ 12"oc.
- 4. 8d NAILS SHALL BE 0.131" DIAMETER x $2\frac{1}{2}$ " (COMMON). 16d NAILS SHALL BE 0.135" DIAMETER x $3\frac{1}{2}$ " (BOX).
- 5. ANCHORS TO CONCRETE SHALL CONSIST OF CAST—IN—PLACE ANCHOR BOLTS, EXPANSION BOLTS, EPOXY GROUTED ALL—THREADS, OR TITEN HD HEAVY DUTY SCREW ANCHORS. CAST—IN—PLACE ANCHOR BOLTS HAVE A 7" EMBED AND SHALL BE J—BOLTS OR SHALL HAVE A HEX NUT AT THE BOTTOM END. EXPANSION BOLTS SHALL HAVE 5" EMBED AND SHALL NOT BE USED AT STEM WALL LOCATIONS WITH EDGE DISTANCE LESS THAN 5" (INSTEAD, USE EPOXY GROUTED ALL—THREADS OR TITEN HD ANCHORS). EPOXY GROUTED ANCHORS SHALL HAVE 5" EMBED AND 2½" MIN. EDGE DISTANCE. TITEN HD ANCHORS SHALL HAVE 3½" EMBED AND 1¾4" MIN. EDGE DISTANCE. AT ALL ANCHOR BOLTS, PROVIDE STEEL PLATE WASHERS THAT ARE A MINIMUM OF 0.229" (3 GAUGE) x 3"x 3" (SIMPSON BP5/8—3 OR SIMILAR). PLACE BOLTS PER ANCHOR BOLT PLACEMENT DETAIL.





TYPICAL SHEARWALL SECTION

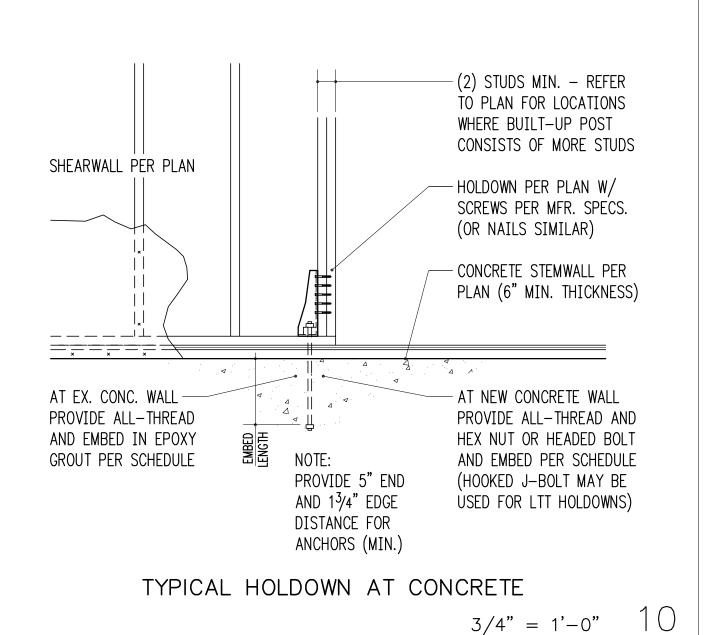
3/4" = 1'-0"

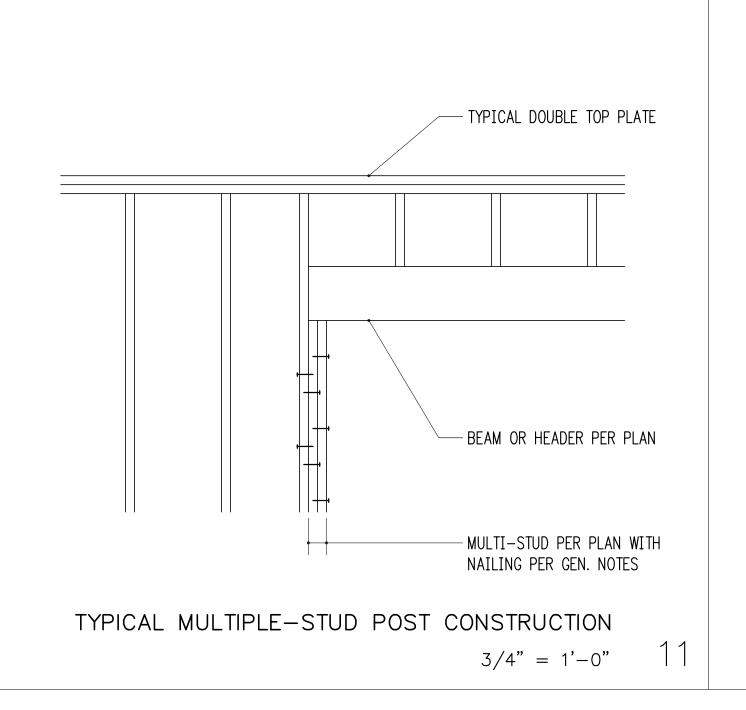
3/4" = 1'-0"

HOLDOWN SCHEDULE (NOT ALL USED ON PLANS)

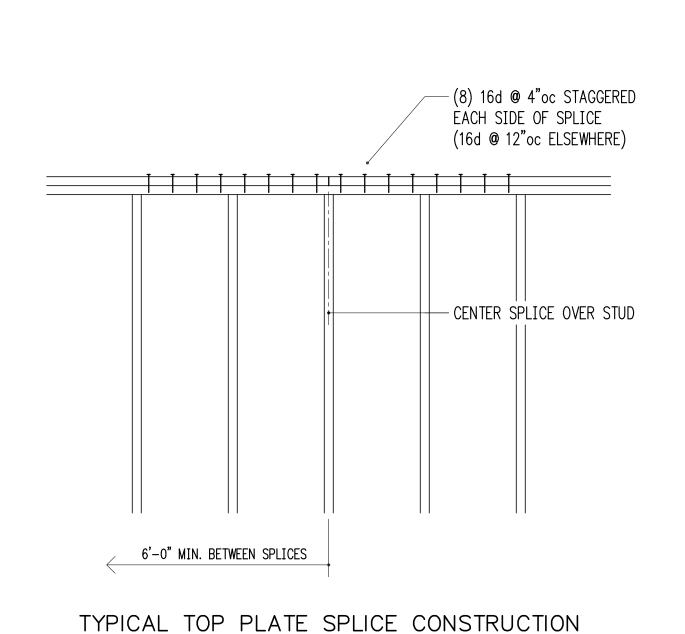
MARK	FASTENERS TO STUDS ¹	ANCHOR	EMBEDMENT LENGTH		SSTB ⁵
		DIA. ²	EP0XY ³	CAST-IN ⁴	
HDU2	(6) 1/4"ø x 21/2" SCREWS	5/8"	12"	11"	SSTB16
HDU4	(10) ¹ /4"ø x 2 ¹ /2" SCREWS	5/8"	24"	24"	SSTB20
HDU5	(14) ¹ /4"ø x 2 ¹ /2" SCREWS	5/8"	N/A	37"	SSTB24
HDU8	(20) ¹ /4"ø x 2 ¹ /2" SCREWS	7/8"	N/A - SEE 12/S3.0 & 2/S3.1		
HDU11	(30) ¹ /4"ø x 2 ¹ /2" SCREWS	1"	N/A - SEE 12/S3.0 & 2/S3.1		
HDU14	(36) ¹ / ₄ "ø x 2 ¹ / ₂ " SCREWS	1"	N/A - S	SEE 12/S3.0 8	c 2/S3.1

- 1. 10d AND 12d DIAMETER = 0.148"; 16d DIAMETER = 0.162". SCREWS SHALL BE SIMPSON "SDS" TYPE SCREWS, INSTALL PER SIMPSON RECOMMENDATIONS.
- 2. PROVIDE A36 OR A307 ALL-THREAD AT EPOXY AND CAST-IN ANCHORS.
- 3. PROVIDE SIMPSON "SET-XP" EPOXY PER GENERAL STRUCTURAL NOTES. SPECIAL INSPECTION IS REQUIRED.
- 4. AT CAST-IN ANCHORS PROVIDE HEAVY HEX NUT AT BOTTOM OF ALL-THREAD. HOOKED J-BOLT MAY BE USED FOR LTT HOLDOWNS.
- 5. AT 3x SILL PLATES, PROVIDE LONGER SSTBL MODELS.





TYPICAL SHEARWALL INTERSECTIONS



8452 North Mercer Way Mercer Island, WA 98040 Issue Description 7/16/19 Framing Revisions 7/26/19 Framing Revisions Framing Revisions 4/17/20 Construction 4/24/20 Trellis Revisions 6/1/20 Corrections #2 Revisions Foundation Revisions Framing Revisions 10/14/20 Pin Pile Revision 11/13/20 Revisions 11/30/20 **Building Department Approval** STRUCTURAL DETAILS

S4.0

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