



SEAL:

OF WASHING

DESIGNER:
KJ

DRAWN BY:
KJ

APPROVED BY:
MG

DATE:
OCTOBER 2020

OCTOBER 2020

JOB No:
2551

DRAWING FILE No:

DRAWING No:

LA2.5

SHEET No:

7 OF 10

THIS SHEET HAS BEEN PROVIDED IN RESPONSE TO FIRST REVIEW COMMENTS

(The following apply unless shown otherwise on the plan)

1. CODE REQUIREMENTS:

ALL MATERIALS, WORKMANSHIP, DESIGN, AND CONSTRUCTION SHALL CONFORM TO THE DRAWINGS, SPECIFICATIONS, THE 2015 IBC, AND THE LATEST EDITION OF THE PTI DOCUMENT, "RECOMMENDATIONS FOR PRE-STRESSED ROCK AND SOIL ANCHORS".

2. REFERENCE DOCUMENTS:

TOPOGRAPHIC AND BOUNDARY SURVEY INFORMATION BY SCJ STUDIO. REPORT ON GEOTECHNICAL INVESTIGATION BY EARTH SOLUTIONS NW, ES-6510 DATED 10-3-19.

3. GENERAL REQUIREMENTS:

CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES REQUIRED TO PERFORM THE CONTRACTOR'S WORK. THE STRUCTURAL ENGINEER HAS NO OVERALL OR ACTUAL SUPERVISORY AUTHORITY AND/OR DIRECT RESPONSIBILITY FOR THE SPECIFIC WORKING CONDITIONS AT THE SITE AND/OR FOR ANY HAZARDS RESULTING FROM THE ACTIONS OF ANY TRADE CONTRACTOR. THE STRUCTURAL ENGINEER HAS NO DUTY TO INSPECT, SUPERVISE, NOTE, CORRECT, OR REPORT ANY HEALTH OR SAFETY DEFICIENCIES TO THE OWNER, CONTRACTORS, OR OTHER ENTITIES AT THE PROJECT SITE.

UTILITY LOCATION: THE SHORING CONTRACTOR SHALL DETERMINE THE HORIZONTAL AND VERTICAL LOCATION OF ALL ADJACENT UNDERGROUND UTILITIES PRIOR TO DRIVING PILES OR TIEBACK ANCHORS. THE UTILITIES INFORMATION SHOWN ON THE PLANS MAY NOT BE COMPLETE. THIS INCLUDES CALLING UTILITY LOCATE AT 1-800-424-5555 AND THEN POTHOLING ALL UTILITIES PRIOR TO 6. INSPECTIONS: CONSTRUCTION TO CONFIRM DEPTHS AND LOCATIONS AND TO VERIFY THAT THERE ARE NO CONFLICTS WITH THE PILE AND TIEBACK CROSSING ELEVATIONS. PILES AND TIEBACKS SHALL MAINTAIN A MINIMUM OF 12" CLEARANCE TO ANY EXISTING UTILITIES TO REMAIN. CONTRACTOR SHALL RESOLVE ANY PROBLEMS PRIOR TO PROCEEDING WITH CONSTRUCTION.

CONTRACTOR SHALL VERIFY ALL DIMENSIONS OF EXISTING STRUCTURES IN THE FIELD AND SHALL NOTIFY THE ENGINEER OF ALL FIELD CHANGES PRIOR TO FABRICATION AND INSTALLATION OF ANY STRUCTURAL MEMBER.

ANY DISCREPANCIES FOUND AMONG THE DRAWINGS, THE SPECIFICATIONS, THESE GENERAL NOTES AND THE SITE CONDITIONS SHALL BE REPORTED TO THE ENGINEER AND ARCHITECT, WHO SHALL CORRECT SUCH DISCREPANCY IN WRITING. SHOULD ANY DISCREPANCIES BE FOUND IN THE PROJECT DOCUMENTS, THE CONTRACTOR WILL BE DEEMED TO HAVE INCLUDED IN THE PRICE THE MOST EXPENSIVE WAY OF COMPLETING THE WORK, UNLESS PRIOR TO SUBMISSION OF THE PRICE THE CONTRACTOR ASKS FOR A DECISION FROM THE ENGINEER AND ARCHITECT AS TO WHICH SHALL GOVERN. ANY WORK DONE BY THE GENERAL CONTRACTOR AFTER DISCOVERY OF SUCH DISCREPANCY SHALL BE DONE AT THE GENERAL CONTRACTOR'S RISK.

4. GEOTECHNICAL INFORMATION AND CRITERIA:

INSTALLATION OF PILES AND TIEBACKS, SUBGRADE PREPARATION INCLUDING DRAINAGE, EXCAVATION, COMPACTION AND FILLING REQUIREMENTS SHALL CONFORM WITH THE RECOMMENDATIONS CONTAINED IN THE SOILS REPORT AND/OR AS DIRECTED BY THE GEOTECHNICAL ENGINEER. THE SUBSURFACE CHARACTERIZATIONS USED TO DESIGN THE SHORING ARE CONTAINED IN THE SOILS REPORT AS REFERENCED ABOVE.

EXCAVATIONS FOR FOUNDATIONS SHALL BE PER PLAN DOWN PER THE GEOTECHNICAL ENGINEERING RECOMMENDATIONS. OVER EXCAVATED AREAS SHALL BE BACKFILLED WITH LEAN CONCRETE OR PER GEOTECHNICAL RECOMMENDATIONS AT THE CONTRACTOR'S EXPENSE. EXCAVATION SLOPES SHALL BE SAFE AND SHALL NOT BE GREATER THAN THE LIMITS SPECIFIED BY LOCAL, STATE, AND NATIONAL SAFETY REGULATIONS. CONTRACTOR SHALL PROTECT CUT SLOPES AS NECESSARY IF CONSTRUCTION OCCURS DURING WET WEATHER, AND SHALL CONTROL AND MANAGE RUNOFF TO MINIMIZE EFFECTS ON CONSTRUCTION.

DESIGN LOADS ARE DETERMINED BY THE GEOTECHNICAL ENGINEER. THE SOIL PRESSURES INDICATED ON THE SOIL PRESSURE DIAGRAM WERE USED FOR DESIGN, IN ADDITION TO THE DEAD AND LIVE LOADS. SEE REPORT OF GEOTECHNICAL INVESTIGATION FOR MORE COMPLETE INFORMATION, INCLUDING RECOMMENDATIONS FOR PILES AND TIEBACKS IN GENERAL, MONITORING, EXCAVATION, AND DRAINAGE.

DESIGN PARAMETERS AS APPROVED BY THE GEOTECHNICAL ENGINEER ARE AS FOLLOWS:

ACŤIVE ĚARŤH PŘESŠURE (YIEĽDIŇG) ACTIVE EARTH PRESSURE WITH 3' WIDTH OF GEOFOAM SEISMIC SURCHARGE PRESSURE (UNIFORM LOAD) 6H PSF 250 PCF/100 PCF PASSIVE EARTH PRESSURE (4:1SLOPE/2:1SLOPE)

HELICAL ANCHORS: REFER SECTION ON HELICAL ANCHORS PILE CAPACITY: 2IN DIAMETER SCHEDULE 80 PIPE

PILE AND TIEBACK DURATION: THE PILES AND TIEBACKS ARE PERMANENT.

PILES SHALL BE GALVANIZED

5. SHOP DRAWINGS:

SHOP DRAWINGS ARE REQUIRED FOR THE FOLLOWING ITEMS:

STRUCTURAL STEEL MISCELLANEOUS METALS HELICAL ANCHORS

CONTRACTOR SHALL ALSO COORDINATE APPROVED SHORING SUBMITTALS WITH BUILDING DEPARTMENT REQUIREMENTS.

SHOP DRAWING REVIEW: DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE ENGINEER OF RECORD, THEREFORE MUST BE VERIFIED BY THE CONTRACTOR. CONTRACTOR SHALL REVIEW AND STAMP DRAWINGS PRIOR TO REVIEW BY ENGINEER OF RECORD. CONTRACTOR SHALL REVIEW DRAWINGS FOR CONFORMANCE WITH THE MEANS, METHODS, TECHNIQUES, SEQUENCES AND OPERATIONS OF CONSTRUCTION, AND ALL SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO. SUBMITTALS SHALL INCLUDE A REPRODUCIBLE AND ONE COPY: REPRODUCIBLE WILL BE MARKED AND RETURNED WITHIN TWO WEEKS OF RECEIPT WITH A NOTATION INDICATING THAT THE SUBMITTAL HAS BEEN FOUND TO BE IN GENERAL CONFORMANCE WITH THE STRUCTURAL DESIGN. THE SUBMITTED ITEMS SHALL NOT BE INSTALLED UNTIL THEY HAVE BEEN APPROVED BY THE DESIGN TEAM AND/OR THE OWNER'S REPRESENTATIVE. ELECTRONIC SUBMISSIONS MAY ALSO BE DEEMED TO MEET THE SUBMITTAL REQUIREMENTS NOTED

SHOP DRAWING SUBMITTALS PROCESSED BY THE ENGINEER ARE NOT CHANGE ORDERS. THE PURPOSE OF SHOP DRAWING SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE ENGINEER THAT THE CONTRACTOR UNDERSTANDS THE DESIGN CONCEPT, BY INDICATING WHICH MATERIAL IS INTENDED TO BE FURNISHED AND INSTALLED AND BY DETAILING THE INTENDED FABRICATION AND INSTALLATION METHODS.

THE CONTRACTOR SHALL BE RESPONSIBLE TO COORDINATE ALL INSPECTIONS REQUIRED BY THE LOCAL BUILDING DEPARTMENT. IN ADDITION TO INSPECTIONS REQUIRED BY THE LOCAL BUILDING DEPARTMENT, THE OWNER OR A REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE ACTING AS THE OWNER'S AGENT SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTIONS FOR ITEMS NOTED IN THE SPECIFICATIONS AND IBC SECTIONS 108 AND 1704. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF THE DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL AND THE STRUCTURAL ENGINEER IMMEDIATELY AND PRIOR TO COMPLETION OF THAT PHASE OF WORK.

SOILS INSPECTION: INSPECTION BY THE SOILS ENGINEER SHALL BE PERFORMED FOR PILE PLACEMENT, EXCAVATION, AND TIEBACK PLACING AND STRESSING. THE GEOTECHNICAL ENGINEER SHALL ALSO ADVISE ON WATER CONTROL AND SLAB ON GRADE

PILE AND ANCHOR INSTALLATION AS WELL AS REQUIRED TESTING SHALL BE PERFORMED UNDER DIRECT AND CONTINUOUS OBSERVATION OF THE GEOTECHNICAL SPECIAL INSPECTOR.

TESTING AND SPECIAL INSPECTION REPORTS ARE TO BE DISTRIBUTED TO THE ARCHITECT, OWNER, BUILDING DEPARTMENT AND STRUCTURAL ENGINEER WITHIN TWO WEEKS OF COMPLETION OF EACH PHASE OF WORK UNLESS DISCREPANCIES ARE NOT CORRECTED AS NOTED ABOVE.

THE FOLLOWING ITEMS REQUIRE SPECIAL INSPECTIONS:

CONSTRUCTION TYPE TYPE OF INSPECTION

CONCRETE CONSTRUCTION PER TABLE 1704.4 STRUCTURAL STEEL FABRICATION AND ERECTION _PER TABLE 1704.3 DŘIVĚN PILE INŠTAĽLAŤIOŇ CONTINUOUS CONTINUOUS $\sqrt{2}$ TIEBACK CONSTRUCTION

PERIODIC INSPECTION ALLOWS INSPECTION AT INTERVALS NECESSARY TO CONFIRM THAT WORK REQUIRING SPECIAL INSPECTION IS IN COMPLIANCE WITH REQUIREMENTS. CONTINUOUS SPECIAL INSPECTION REQUIRES THAT THE INSPECTOR BE ONSITE AT ALL TIMES THAT WORK REQUIRING SPECIAL INSPECTION IS PERFORMED.

PERIODIC

EXCAVATION AND GRADING

CONCRETE CONSTRUCTION SHALL CONFORM TO ALL REQUIREMENTS OF IBC CHAPTER 19 AND THE ACI STANDARD 318-02 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE".

CONCRETE SHALL BE MIXED, PROPORTIONED, CONVEYED AND PLACED IN ACCORDANCE WITH ACI 301, INCLUDING TESTING PROCEDURES. CONCRETE SHALL ATTAIN A 28-DAY STRENGTH OF f'c = 3,000 PSI AND MIX SHALL CONTAIN NOT LESS THAN 5-1/2 SACKS OF CEMENT PER CUBIC YARD AND SHALL BE PROPORTIONED TO PRODUCE A SLUMP OF 5" OR LESS. REQUIRED CONCRETE STRENGTH IS BASED ON THE DURABILITY REQUIREMENTS OF IBC SECTION 1904, DESIGN STRENGTH IS fc = 2.500 PSI.

8. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, FY = 60,000 PSI.

9. STEEL:

STEEL SPECIFICATIONS: DESIGN, FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE AISC MANUAL, AISC 360 AND IBC SECTION 2205.

STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:

A307

ASTM SPECIFICATION

OTHER SHAPES, PLATES, AND RODS A36 36 KSI PIPE COLUMNS A53 (E OR S, GR.B) 35 KSI

ALL WELDING SHALL BE IN CONFORMANCE WITH A.I.S.C. AND A.W.S. STANDARDS AND SHALL BE PERFORMED BY W.A.B.O. CERTIFIED WELDERS USING E70 XX ELECTRODES. ONLY PREQUALIFIED WELDS (AS DEFINED BY A.W.S.) SHALL BE USED. ALL COMPLETE JOINT PENETRATION GROOVE WELDS SHALL BE MADE WITH A FILLER MATERIAL THAT HAS A MINIMUM CVN TOUGHNESS OF 20 FT-LBS AT 20 DEGREES F AND 40 FT-LBS AT 70 DEGREES F, AS DETERMINED BY AWS CLASSIFICATION OR MANUFACTURER CERTIFICATION.

10. HELICAL ANCHORS:

TYPE OF MEMBER

CONNECTION BOLTS

HELICAL ANCHORS REMOVED FROM PLANS

11. PROOF TESTS:

ALL ANCHORS NOT PERFORMANCE TESTED SHALL BE INSTALLED TO 200 PERCENT OF THE DESIGN LOAD AS PERFORMED BY THE TORQUE METHOD WHEN APPROVED BY THE STRUCTURAL AND GEOTECHNICAL ENGINEERS.

12. TORQUE METHOD INSTALLATION:

ANCHORS INSTALLED BY THE TORQUE METHOD MUST FIRST MEET THE EMBEDMENT REQUIREMENTS NOTED ON THE STRUCTURAL DRAWINGS OR APPROVED BY BOTH THE STRUCTURAL AND GEOTECHNICAL ENGINEER IN WRITING. ONCE THE REQUIRED MINIMUM EMBEDMENT HAS BEEN MET, THE CONTRACTOR SHALL PROVIDE GAUGES OR SHEAR BOLTS TO DETERMINE THAT THE ANCHOR HAS BEEN TORQUED TO A MINIMUM OF 200 PERCENT OF THE DESIGN LOAD BASED ON THE EMPIRICAL RELATIONSHIP THAT THE 200 PERCENT LOAD MEASURED IN POUNDS IS APPROXIMATELY EQUAL TO 10 TIMES THE INSTALLATION TORQUE AS MEASURED IN FOOT POUNDS. THE CONTRACTOR SHALL BE PREPARED TO PROVIDE EXTENSIONS TO ADVANCE HELICALS DEEPER INTO THE SOILS UNTIL THE REQUIRED INSTALLATION TORQUE HAS BEEN MET. INSTALLATION SHALL BE CONDUCTED UNDER THE OBSERVATION OF THE GEOTECHNICAL SPECIAL INSPECTOR.

13. <u>PIN PILES:</u>

PIN PILES SHOWN ON THE PLAN SHALL BE 2" DIAMETER SCHEDULE 80 AND GALVANIZED. THE MAXIMUM CAPACITY OF 2" PILES SHALL BE 3 TONS. ALL PILES SHALL BE DRIVEN TO REFUSAL N ACCORDANCE WITH THE GEOTECHNICAL REPORT. AS A MINIMUM, PILE REFUSAL SHALL BE DEFINED AS 1 INCH OF PENETRATION IN 60 SECONDS DURING CONTINUOUS DRIVING OF A 90 LB JACK HAMMER UNDER THE FULL WEIGHT AND EFFORT OF THE OPE**RATOR OF THE CONVENTIONAL DRIV**ING OF A RHINO PD-140. PILES SHALL HAVE THE ADDITIONAL REQUIREMENT OF BEING EMBEDDED A MINIMUM OF 10 FEET BELOW RETAINED GRADE. THE MAXIMUM PILE ECCENTRICITY SHALL BE 2 INCHES. GEOTECHNICAL SPECIAL INSPECTION IS REQUIRED.

14. ANCHORAGE

EXPANSION BOLTS INTO CONCRETE SHALL BE "STRONG-BOLT 2" WEDGE ANCHORS AS MANUFACTURED BY THE SIMPSON STRONG TIE COMPANY AND INSTALLED IN STRICT CONFORMANCE TO ICC-ES REPORT NUMBER ESR-3037. PERIODIC SPECIAL INSPECTION IS REQUIRED TO VERIFY ANCHOR TYPE, ANCHOR DIMENSIONS, ANCHOR LOCATION, TIGHTENING TORQUE, HOLE DIMENSIONS, ANCHOR EMBEDMENT, AND ADHERENCE TO THE INSTALLATION INSTRUCTIONS.

15. PILE TESTING

LOAD TESTS TO 200 PERCENT OF THE DESIGN CAPACITY ARE REQUIRED TO SUBSTANTIATE THE ALLOWABLE PILE LOAD. THE APPROPRIATE NUMBER OF LOAD TEST SHALL BE DETERMINED BY THE GEOTECHNICAL ENGINEER AT THE TIME THE CONTRACTOR AND INSTALLATION METHOD ARE CHOSEN. AS A MINIMUM, LOAD TESTS SHALL BE REQUIRED ON AT LEAST 3 PERCENT OF ALL PILES INSTALLED AT THE SITE, WITH A MINIMUM OF FIVE TESTS. ALL TESTS MUST CONFORM TO THE QUICKLOAD TEST METHOD ACCORDING TO THE AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM) D-1143.

DETAILING OF REIFORCING STEEL (INCLUDING HOOKS AND BENDS SHALL BE IN ACCORDANCE WITH ACI 315-99 AND 318-11, LAP ALL CONTINUOUS RENFORCEMENT 2'-0" MINIMUM. PROVIDE CORNER BARS AT ALL WALL AND FOOTING INTERSECTIONS AND LAP 2'-0" MINIMUM.

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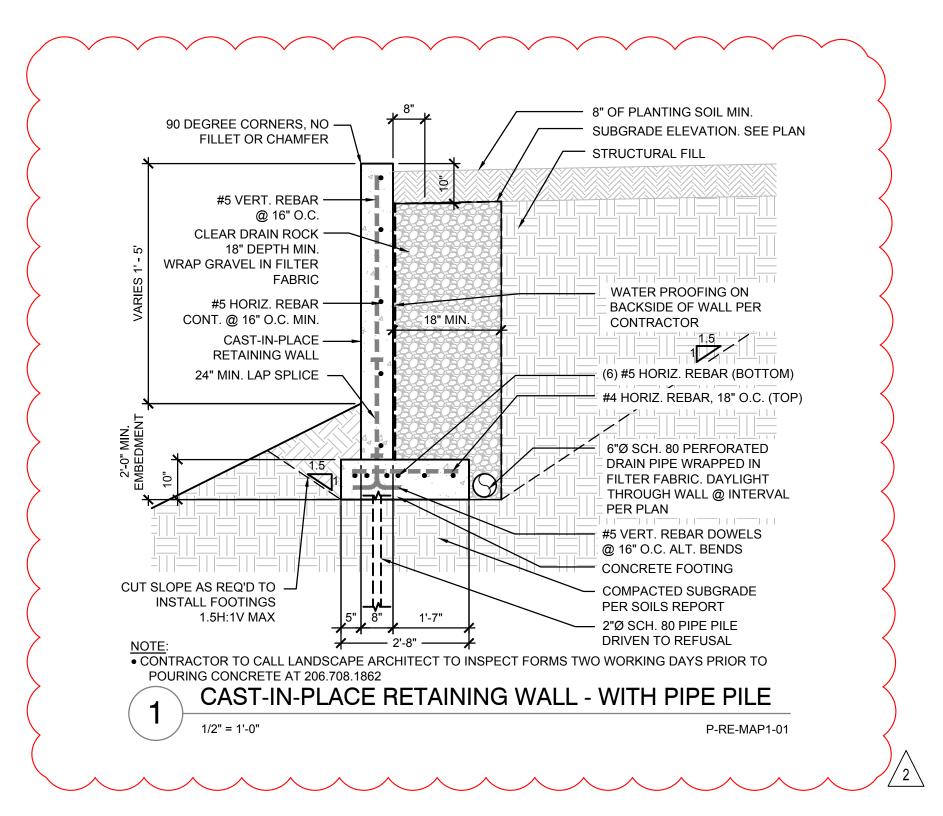
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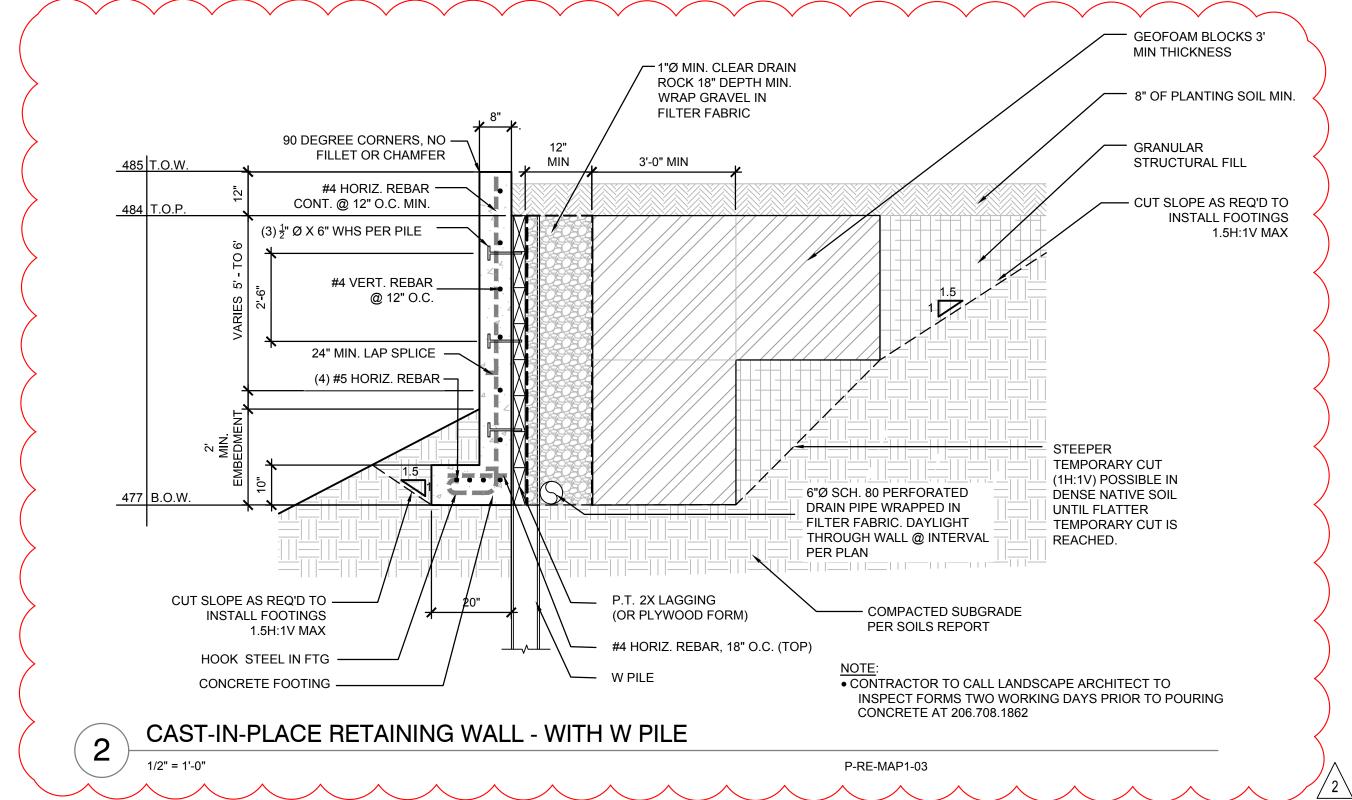
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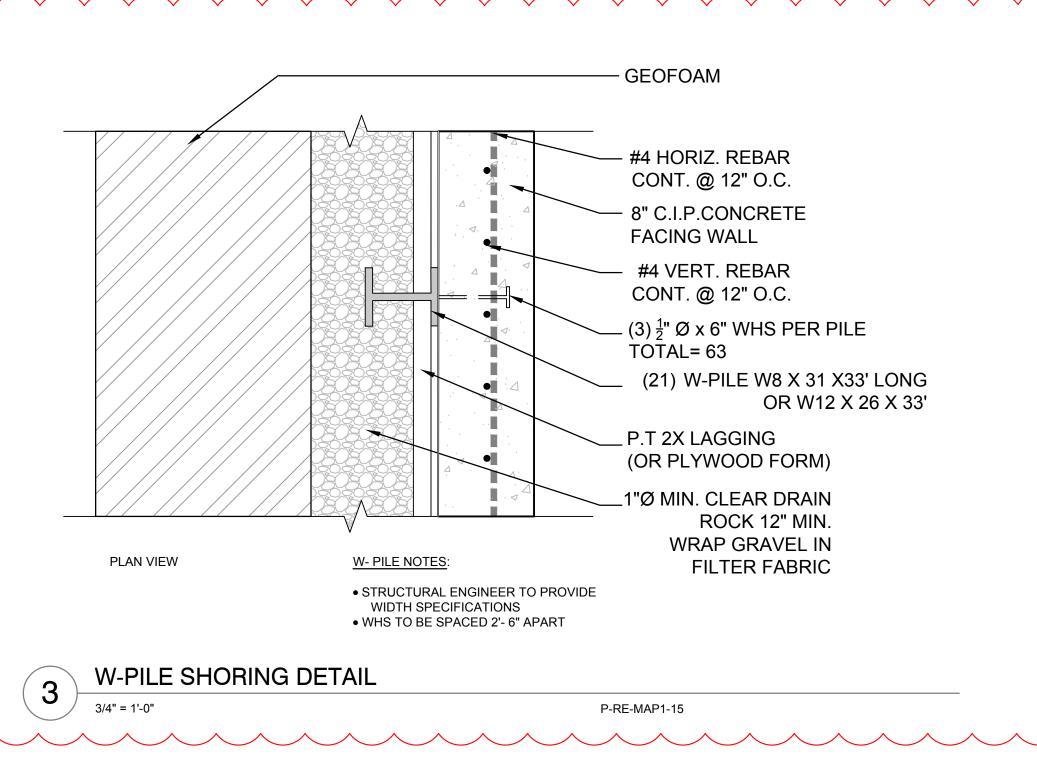


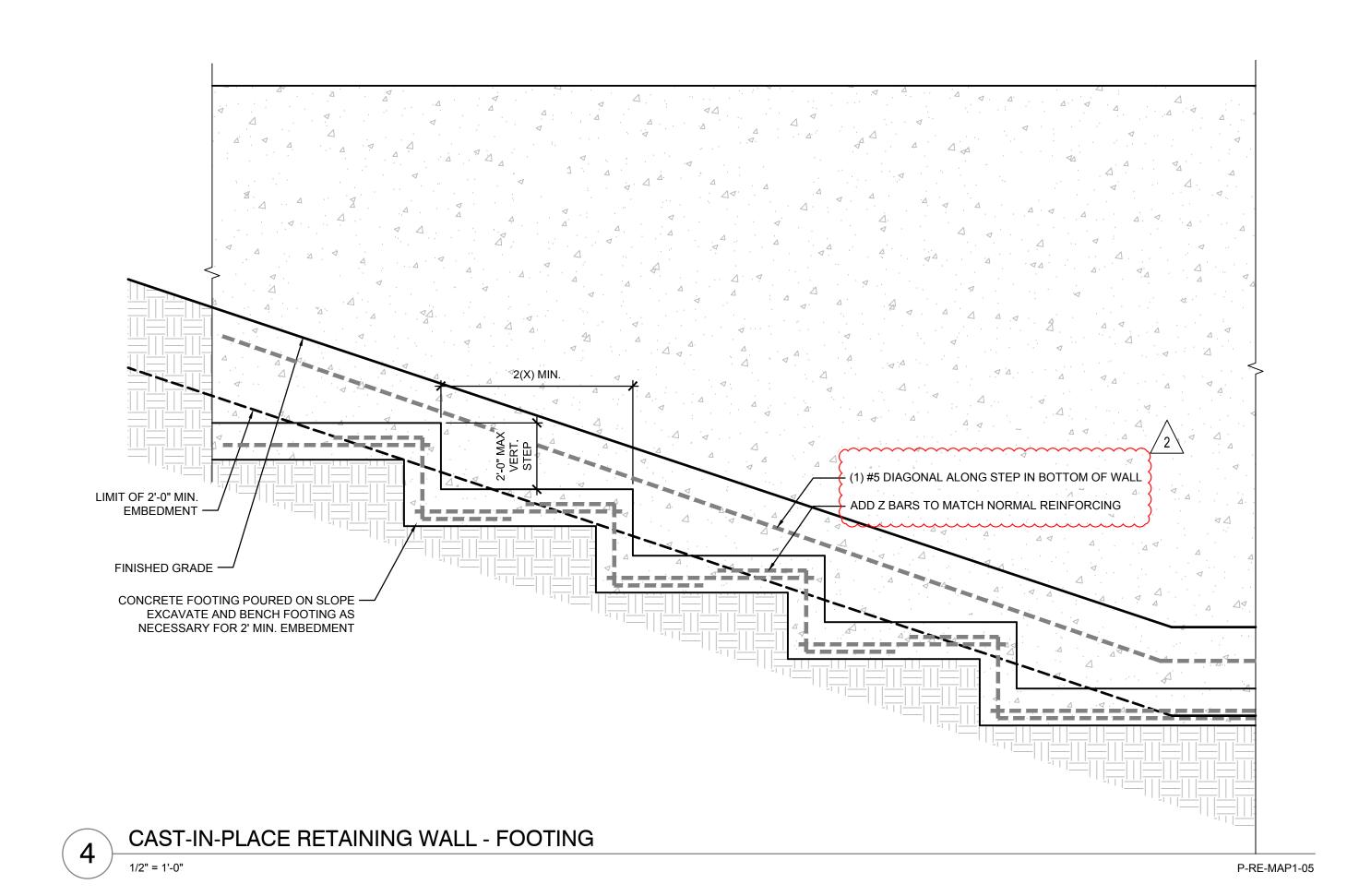
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LA3.1 SHEET No: 8 OF 10

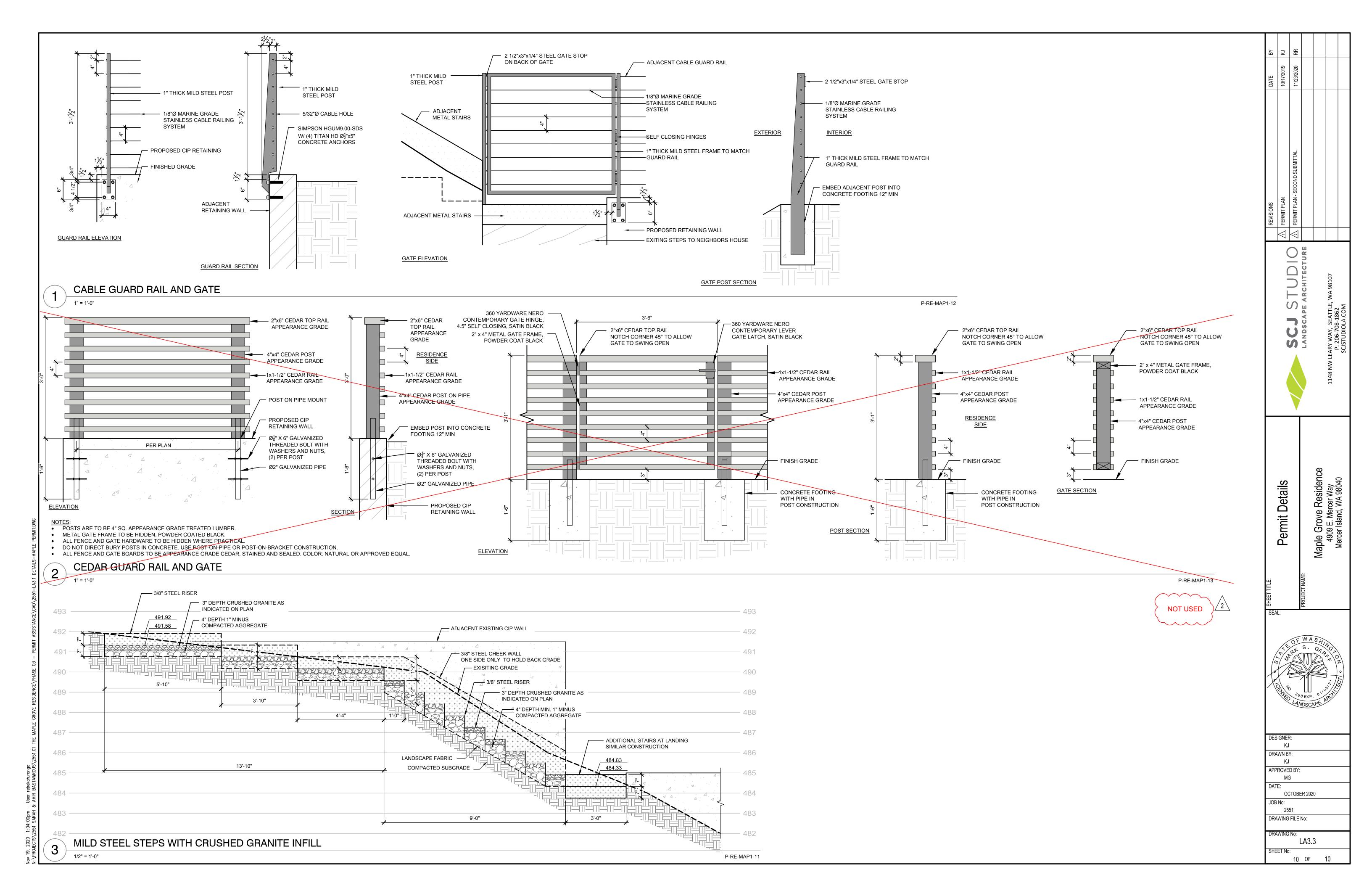


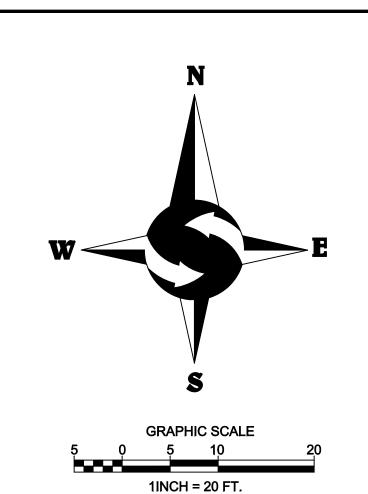






— F S Details Structural Maple Grove F 4909 E. Merc Mercer Island, ¹ DESIGNER: DRAWN BY: KJ APPROVED BY: MG OCTOBER 2020 JOB No: 2551 DRAWING FILE No: LA3.2 SHEET No: 9 OF 10





LEGEND

lacktriangle	FOUND MONUMENT AS DESCRIBED	-OHP $-$	OVERHEAD POWER	
0	FOUND REBAR AS DESCRIBED	—они—	OVERHEAD UTILITIE	
X	TACK IN LEAD FOUND	— X —	CHAINLINK FENCE	
•	SET 5/8" X 24" IRON ROD	——	WOOD FENCE	
	W/1" YELLOW PLASTIC CAP	××××××××××××××××××××××××××××××××××××××	CONCRETE WALL	
P	POWER METER			
Ø	UTILITY POLE		ROCKERY	
0	GAS METER		ASPHALT SURFACE	
	SANITARY SEWER CLEANOUT			
	SANITARY SEWER MANHOLE		CONCRETE SURFAC	
\bowtie	WATER VALVE		GRAVEL SURFACE	
Q	FIRE HYDRANT		0,0 (722 00) (1,762	
\blacksquare	WATER METER	CE	CEDAR	
-0-	SIGN	DS	DECIDUOUS	
— ss —	APPROXIMATE LOCATION SANITARY	MP	MAPLE	
	SEWER LINE	DF	DOUGLAS FIR	
—sd—	APPROXIMATE LOCATION STORM DRAIN LINE	PI	PINE	
		* INDICA	* INDICATES MULTI-TRUNK	

LEGAL DESCRIPTION

THAT PORTION OF THE SOUTH 100 FEET OF THE NORTH 700 FEET OF GOVERNMENT LOT 1, SECTION 19, TOWNSHIP 24 NORTH, RANGE 5 EAST, W.M., IN KING COUNTY,

WASHINGTON LYING WEST OF E. MERCER WAY EXCEPT THE PORTION THEREOF LYING WEST OF A LINE BEGINNING AT A POINT ON THE SOUTH LINE OF THE NORTH 700 FEET OF SAID GOVERNMENT LOT 1, WHICH POINT BEARS S 89°46'00" E ON SAID SOUTH LINE A DISTANCE OF 543.09 FEET FROM THE WEST LINE OF

SITUATE IN THE CITY OF MERCER ISLAND, COUNTY OF KING, STATE OF WASHINGTON.

BASIS OF BEARINGS

SURVEYOR:

ACCEPTED A BEARING OF S 02°38'45" E ALONG THE CENTER LINE OF EAST MERCER WAY BETWEEN FOUND MONUMENTS IN CASE PER RECORD OF SURVEY UNDER KING COUNTY

SITE SURVEYING, INC.

PROJECT INFORMATION

21923 NE 11TH ST SAMMAMISH, WA 98074 PHONE: 425,298,4412 AMIR & SARAH BASTAWROUS PROPERTY OWNER: 4909 E MERCER WAY MERCER ISLAND, WA 98040 TAX PARCEL NUMBER: 216200-0230 PROJECT ADDRESS: 4909 E MERCER WAY MERCER ISLAND, WA 98040 ZONING: JURISDICTION: CITY OF MERCER ISLAND 17,399 S.F. (± 0.399 ACRES) AS SURVEYED PARCEL ACREAGE:

GENERAL NOTES

- THIS SURVEY WAS COMPLETED WITHOUT BENEFIT OF A CURRENT TITLE REPORT. EASEMENTS AND OTHER ENCUMBRANCES MAY EXIST ON THIS PROPERTY THAT ARE NOT SHOWN HEREON.
- INSTRUMENTATION FOR THIS SURVEY WAS A 5-SECOND SPECTRA PRECISION FOCUS 30 TOTAL STATION. PROCEDURES USED IN THIS SURVEY MEET OR EXCEED STANDARDS SET BY WAC 332-130-090.
- THE INFORMATION ON THIS MAP REPRESENTS THE RESULTS OF A SURVEY MADE IN MAY 2018 AND CAN ONLY BE CONSIDERED AS INDICATING THE GENERAL CONDITIONS EXISTING AT THAT TIME. UTILITIES SHOWN ON THIS SURVEY ARE BASED UPON ABOVE GROUND OBSERVATIONS AND AS-BUILT PLANS WHERE AVAILABLE. ACTUAL LOCATIONS OF
- UNDERGROUND UTILITIES MAY VARY AND UTILITIES NOT SHOWN ON THIS SURVEY MAY EXIST ON THIS SITE.
- ALL MONUMENTS WERE LOCATED DURING THIS SURVEY UNLESS OTHERWISE

VERTICAL DATUM & CONTOUR INTERVAL

ELEVATIONS SHOWN ON THIS DRAWING ARE ON AN ASSUMED



2.0' CONTOUR INTERVAL - THE EXPECTED VERTICAL ACCURACY IS EQUAL TO 1/2 THE CONTOUR INTERVAL OR PLUS / MINUS 1.0' FOR THIS PROJECT.

