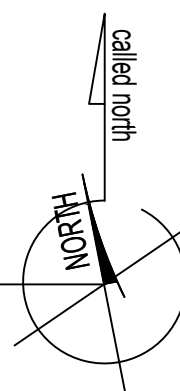


**SUPPLEMENTAL  
A. SITE PLAN**

1/10" = 1'-0"

- △ = WALL SEGMENT TAG FOR HEIGHT CALCULATION
- - - = EAVE/ROOF LINE
- = REVISED TOPO (FINISHED GRADES)
- - - = EXISTING TOPOGRAPHY (SURVEY SHOWN IN BACKGROUND)



- = TREE FENCING, CHAIN LINK
- ▨ = GROUND PROTECTION AREA - (9" OF WOOD CHIPS)
- SEE ARBORISTS REPORT FOR FULL TREE PROTECTION NOTES
- ⊙ = TREE NUMBER PER ARBORISTS REPORT
- ⊕ = EXCEPTIONAL TREE
- ⋯ = TREE LIMIT OF DISTURBANCE

TOTAL REMOVED LOT COVERAGE

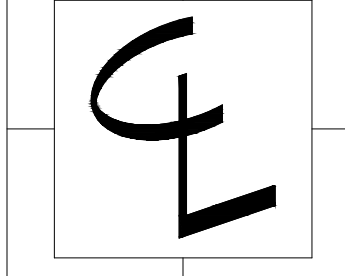
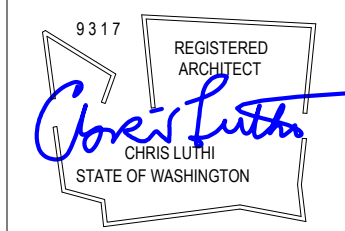
DRIVE = 939.6 sf
MISC. roof = 232.68 sf
<b>PROJECT TOTAL = 1172.28 sf</b>

TOTAL F.A.R. CALC.

MAIN FLOOR L.A. PROPOSED = 3086.4 sf
LOWER FLOOR L.A. PROPOSED = 1831.9 sf (w/o below grade exception)
LOWER FLOOR GARAGE PROPOSED = 908.48 sf (w/o below grade exception)
F.A.R. PROJECT TOTAL = 5826.8 sf
allow. = 16963 sf x .4 = 6785.2 MAX

TOTAL HARDSCAPE CALC

REMAINING HARDSCAPE	PROPOSED NEW HARDSCAPE	PROJECT TOTAL
patios = 500 sf	decks = 57.6 sf	REMAINING + NEW
walks = 450 sf	walks = 71.6 sf	
stairs = 115 sf	stairs = 64.74 sf	
rockeries = 182.5 sf	rockeries = 31.5 sf	
<b>TOTAL remaining = 1257.5 sf</b>	<b>TOTAL new = 225.44 sf</b>	<b>1257.5 + 225.44 = 1482.94</b>
TOTAL ALLOWABLE = 16963 sf x .09 = 1526.67 sf		



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Site Plan

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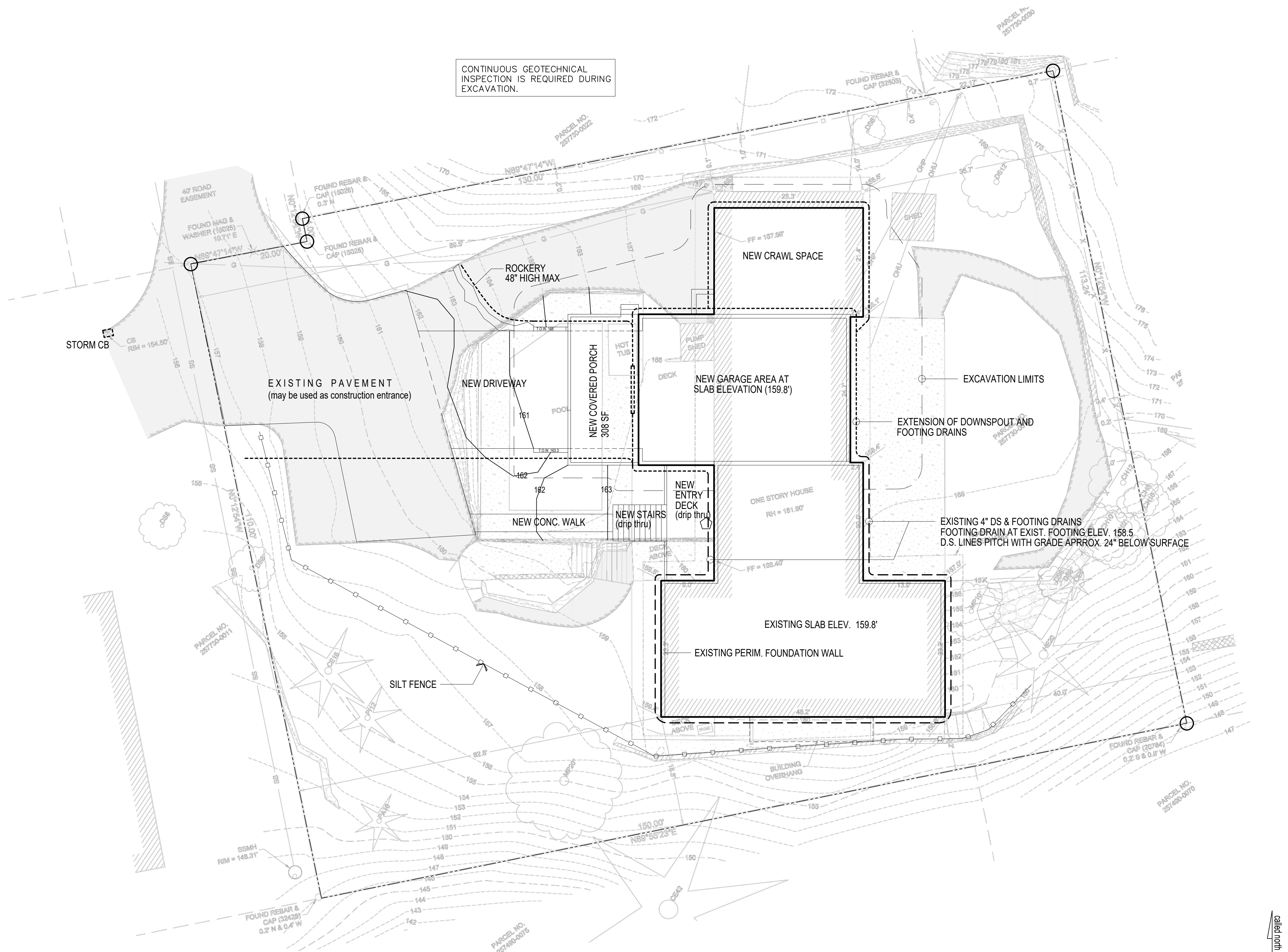
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**DATE**

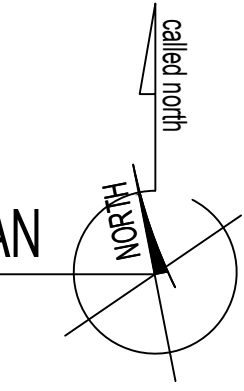
4.1.21  
10.7.21

**1B**



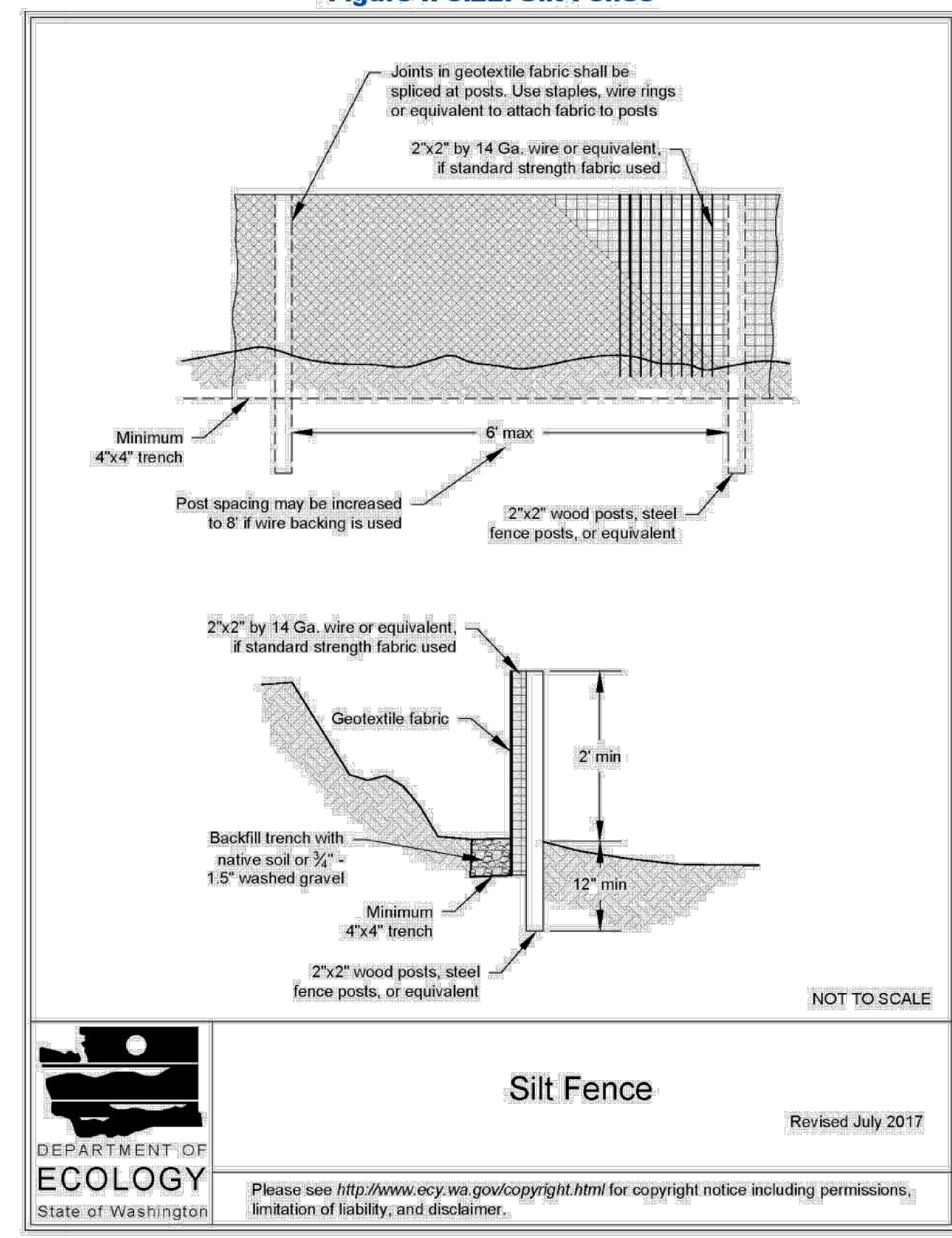


A. DRAINAGE AND TEMP. EROSION CONTROL PLAN  
1/10" = 1'-0"

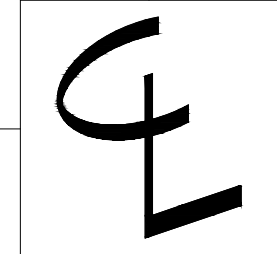
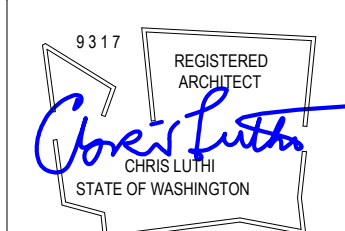


SILT FENCE DETAIL DOE

Figure II-3.22: Silt Fence



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Drainage and TESC Plan

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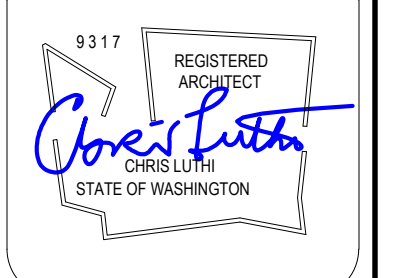
1d

**NOTES**

- SD = SMOKE DETECTOR, HARDWIRE, INTERCONNECTED w/ BATTERY BACK-UP
- CO = CARBON MONOXIDE DETECTOR, HARDWIRE w/ BATTERY BACK-UP

DOORS ARE 3-0 x 6-8 (r.o. = 3'-2" x 6'-10") unless otherwise indicated  
 ☉ = FAN, 50 CFM UNLESS OTHERWISE INDICATED  
 FOR SHEAR WALL INFORMATION SEE STRUCTURAL PLANS  
 ALL INTERIOR WALLS TO BE 2x4, EXTERIOR WALLS 2x6, EXCEPT AS INDICATED, OR EXISTING

E = EGRESS WINDOWS  
 Contractor shall verify to Inspector all guards and railings shall be capable of resisting 200 lb load on top rail acting in any direction as required by IRC Table R301.5.  
 ALL WALLS FULL HEIGHT UNLESS OTHERWISE INDICATED  
T = TEMPER/SAFETY GLAZE WINDOWS  
 ALL GAS F.P. TO BE APPROVED DIRECT VENT

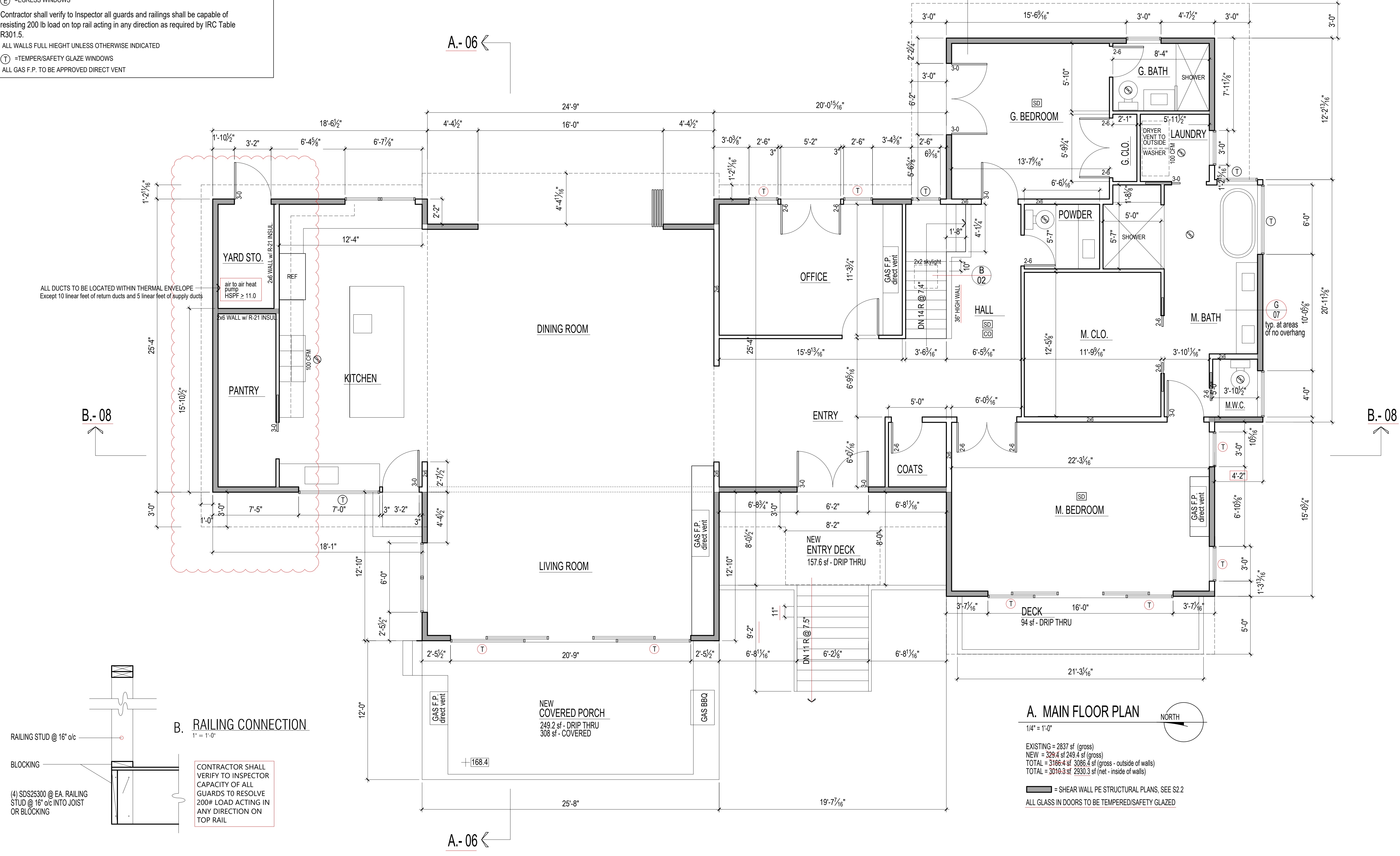


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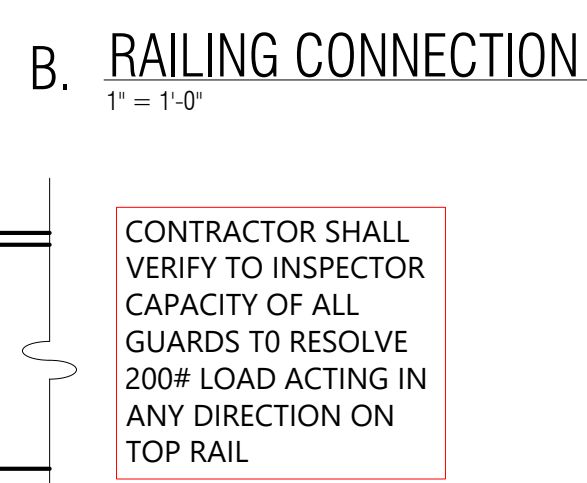
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 Main Floor Plan

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**A. MAIN FLOOR PLAN**  
 1/4" = 1'-0"  
 NORTH  
 EXISTING = 2837 sf (gross)  
 NEW = 329.4 sf (249.4 sf (gross))  
 TOTAL = 3166.4 sf (3086.4 sf (gross - outside of walls))  
 TOTAL = 3010.3 sf (2930.3 sf (net - inside of walls))  
 ——— = SHEAR WALL PER STRUCTURAL PLANS, SEE S2.2  
 ALL GLASS IN DOORS TO BE TEMPERED/SAFETY GLAZED



RAILING STUD @ 16" o/c  
 BLOCKING  
 (4) SDS25300 @ EA. RAILING STUD @ 16" o/c INTO JOIST OR BLOCKING

**NOTES**

- SD = SMOKE DETECTOR, HARDWIRE, INTERCONNECTED w/ BATTERY BACK-UP
- CO = CARBON MONOXIDE DETECTOR, HARDWIRE w/ BATTERY BACK-UP

DOORS ARE 3-0 x 6-8 (r.o. = 3'-2" x 6'-10") unless otherwise indicated  
 Ⓢ = FAN, 50 CFM UNLESS OTHERWISE INDICATED  
 FOR SHEAR WALL INFORMATION SEE STRUCTURAL PLANS  
 ALL INTERIOR WALLS TO BE 2x4, EXTERIOR WALLS 2x6, EXCEPT AS INDICATED, OR EXISTING

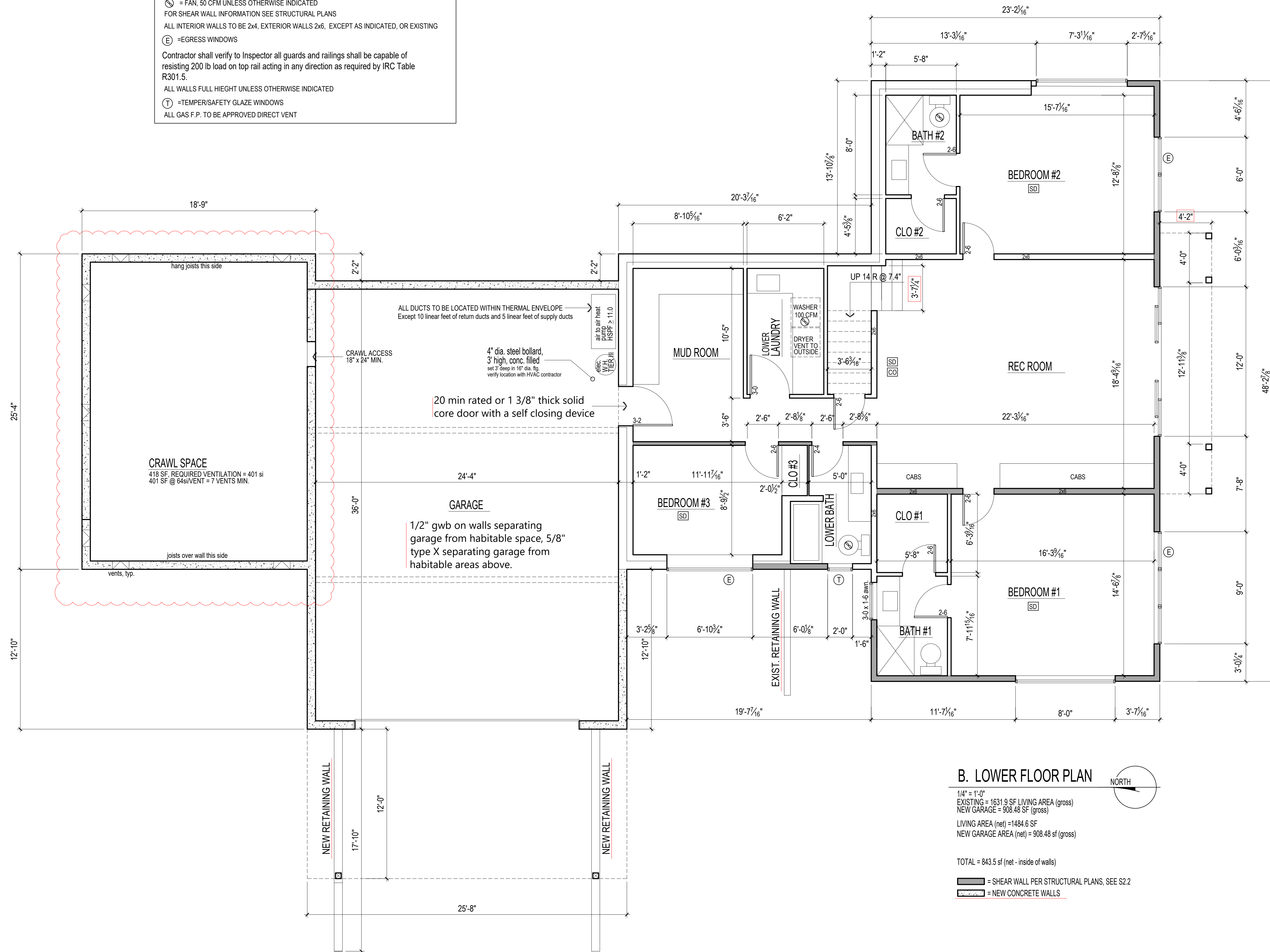
E = EGRESS WINDOWS

Contractor shall verify to Inspector all guards and railings shall be capable of resisting 200 lb load on top rail acting in any direction as required by IRC Table R301.5.

ALL WALLS FULL HEIGHT UNLESS OTHERWISE INDICATED

T = TEMPER/SAFETY GLAZE WINDOWS

ALL GAS F.P. TO BE APPROVED DIRECT VENT

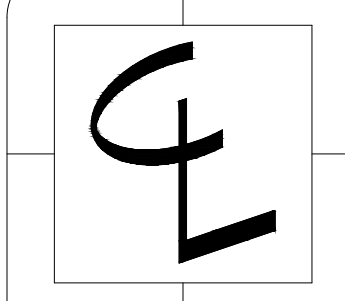
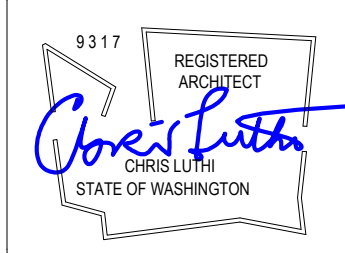


**B. LOWER FLOOR PLAN**

1/4" = 1'-0"  
 EXISTING = 1631.9 SF LIVING AREA (gross)  
 NEW GARAGE = 908.48 SF (gross)  
 LIVING AREA (net) = 1484.6 SF  
 NEW GARAGE AREA (net) = 908.48 sf (gross)

TOTAL = 843.5 sf (net - inside of walls)

- = SHEAR WALL PER STRUCTURAL PLANS, SEE S2.2
- = NEW CONCRETE WALLS



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Lower Floor Plan

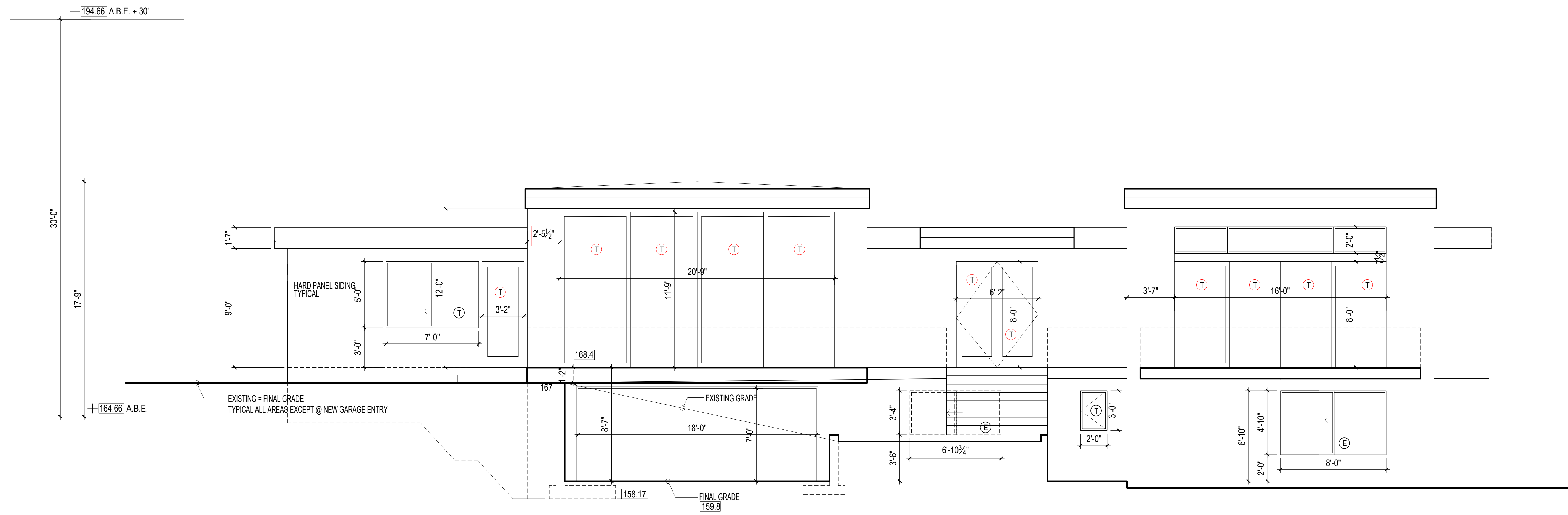
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DATE

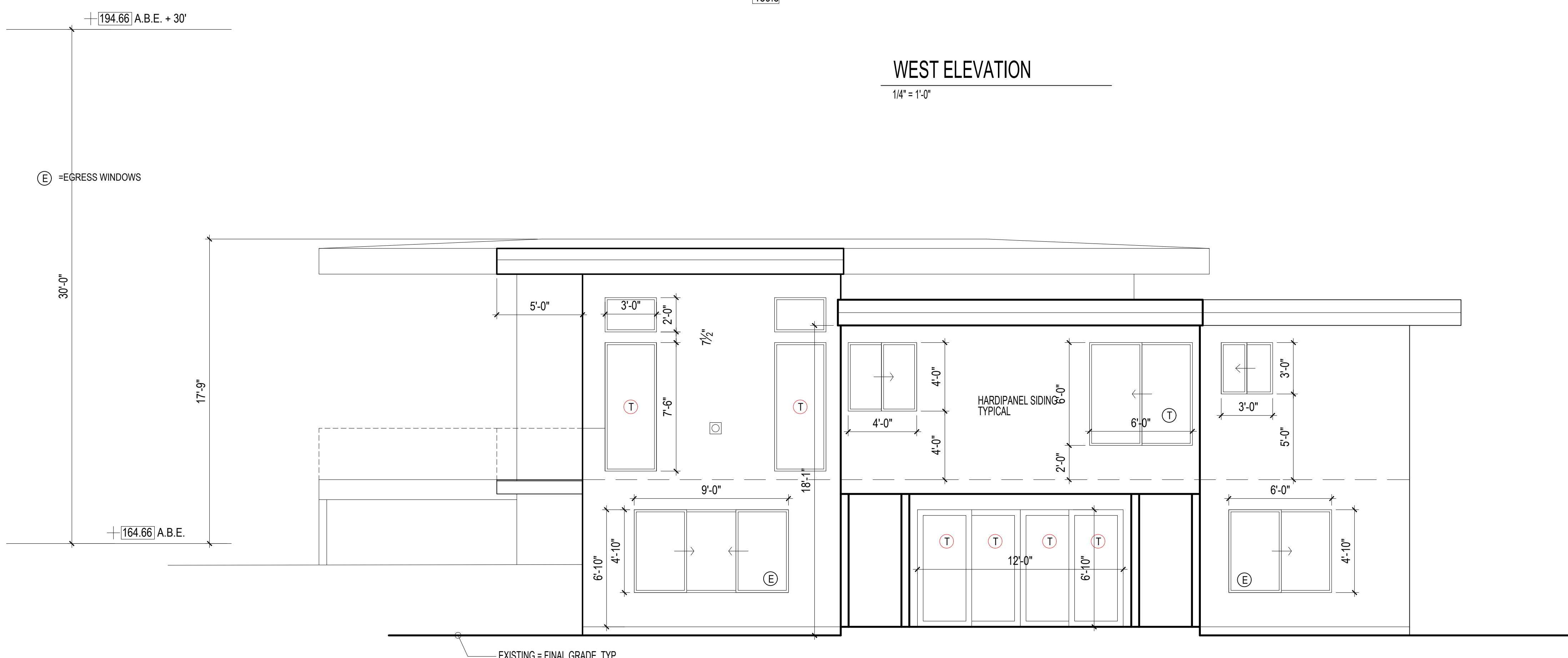
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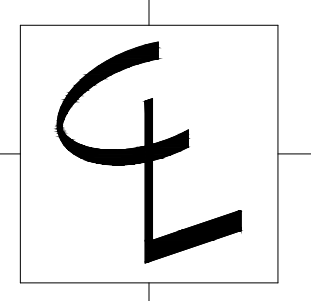
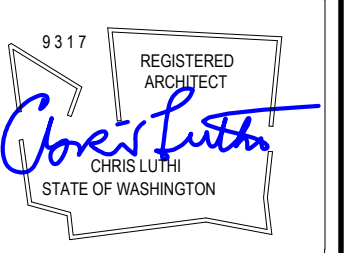
WEST ELEVATION

1/4" = 1'-0"



SOUTH ELEVATION

1/4" = 1'-0"



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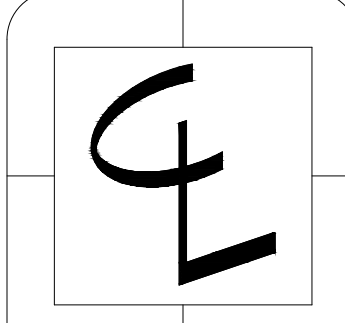
S and W Elevations

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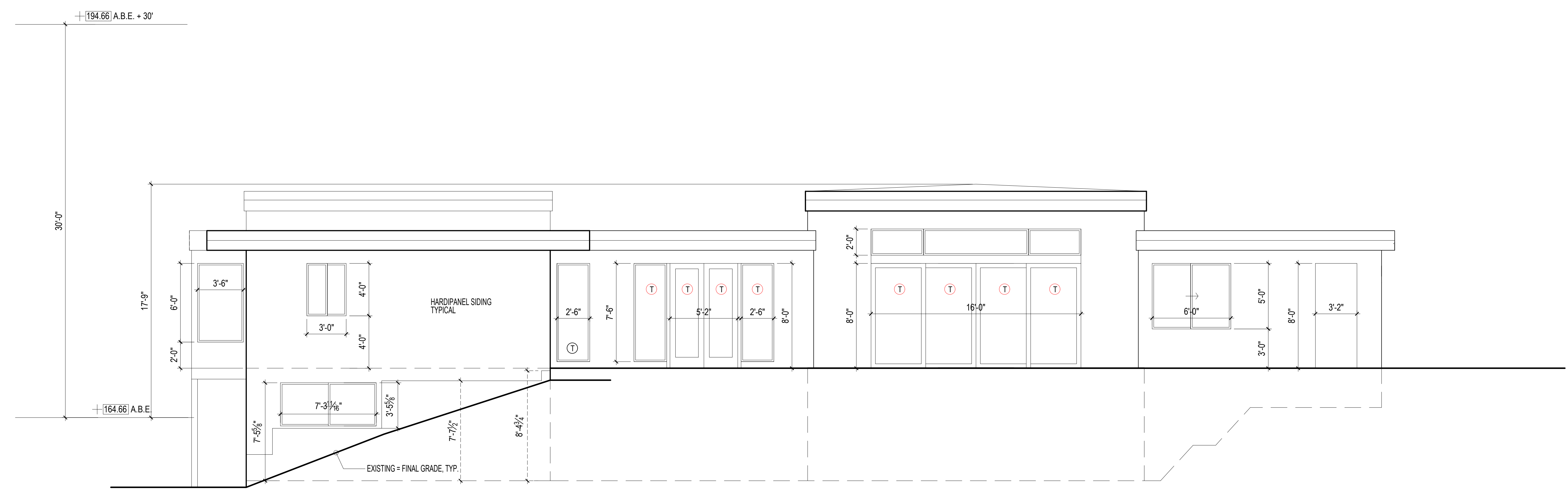
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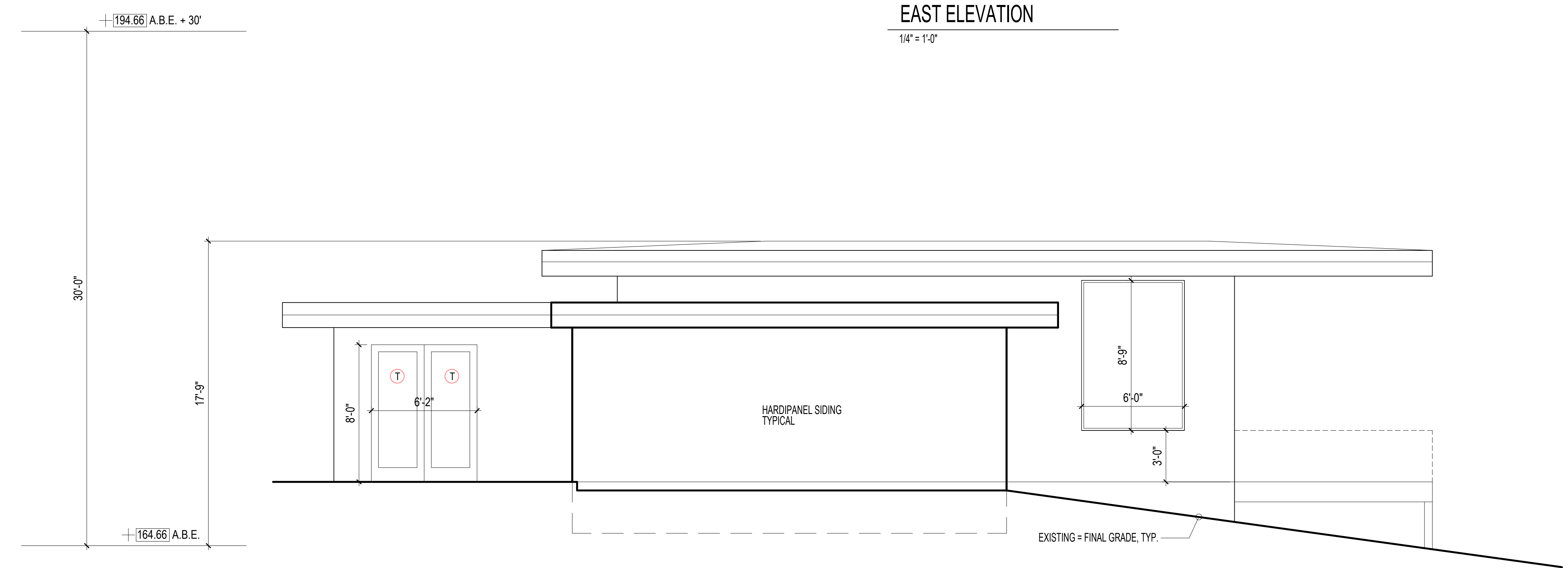
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CONTENTS  
 N and E Elevations

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EAST ELEVATION  
 1/4" = 1'-0"



NORTH ELEVATION  
 1/4" = 1'-0"



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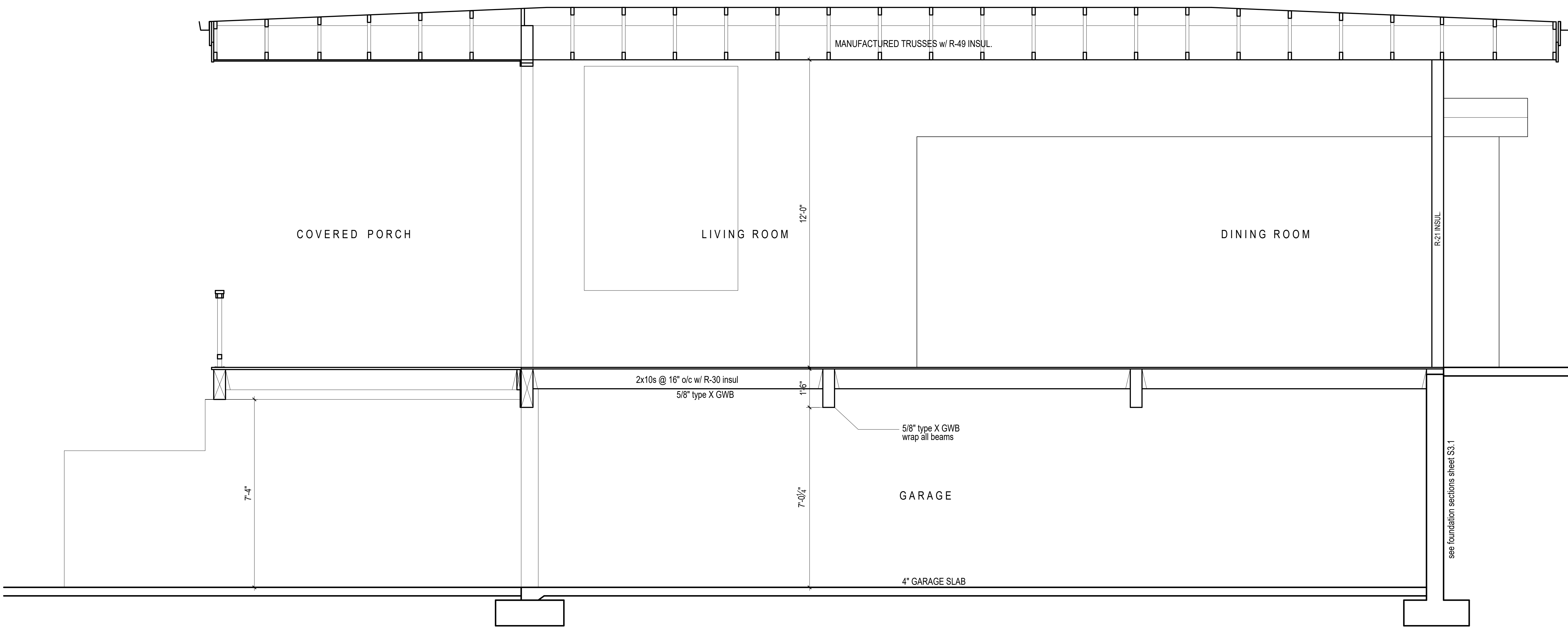
LR Section

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DATE

4.1.21



SECTION @ GARAGE/LIVING ROOM  
 1/2" = 1'-0"

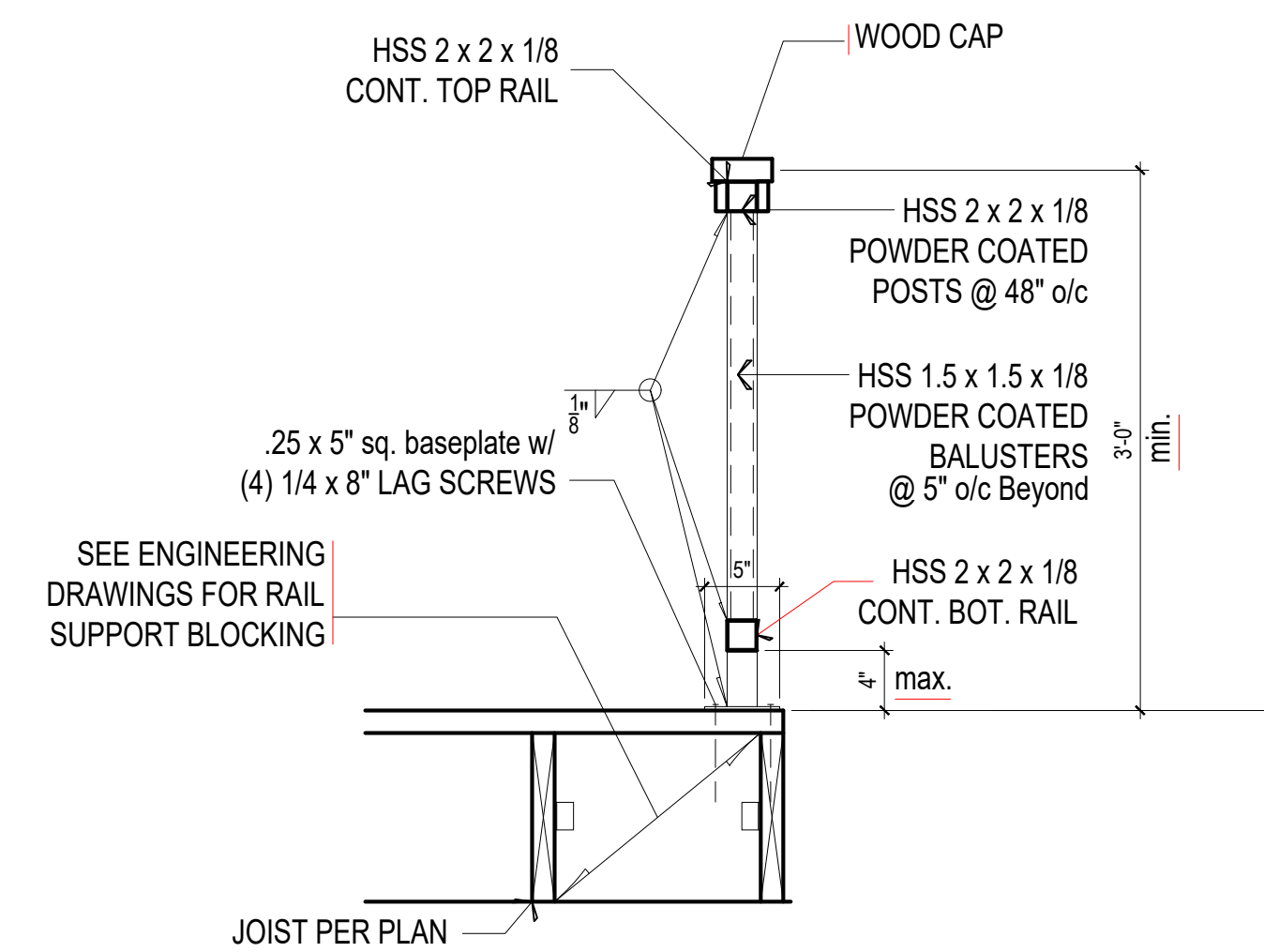
MIN. STAIRWAY WIDTH = 3'-0" CLEAR  
 STAIR RISE, RUN AND NOSING CANNOT VARY BY MORE THAN 3/8"  
 HANDRAIL TERMINATIONS MUST RETURN TO WALL

MIN. STAIRWAY WIDTH = 3'-0" CLEAR  
 STAIR RISE, RUN AND NOSING CANNOT VARY BY MORE THAN 3/8"  
 HANDRAIL TERMINATIONS MUST RETURN TO WALL

**C. STAIR SECTION**

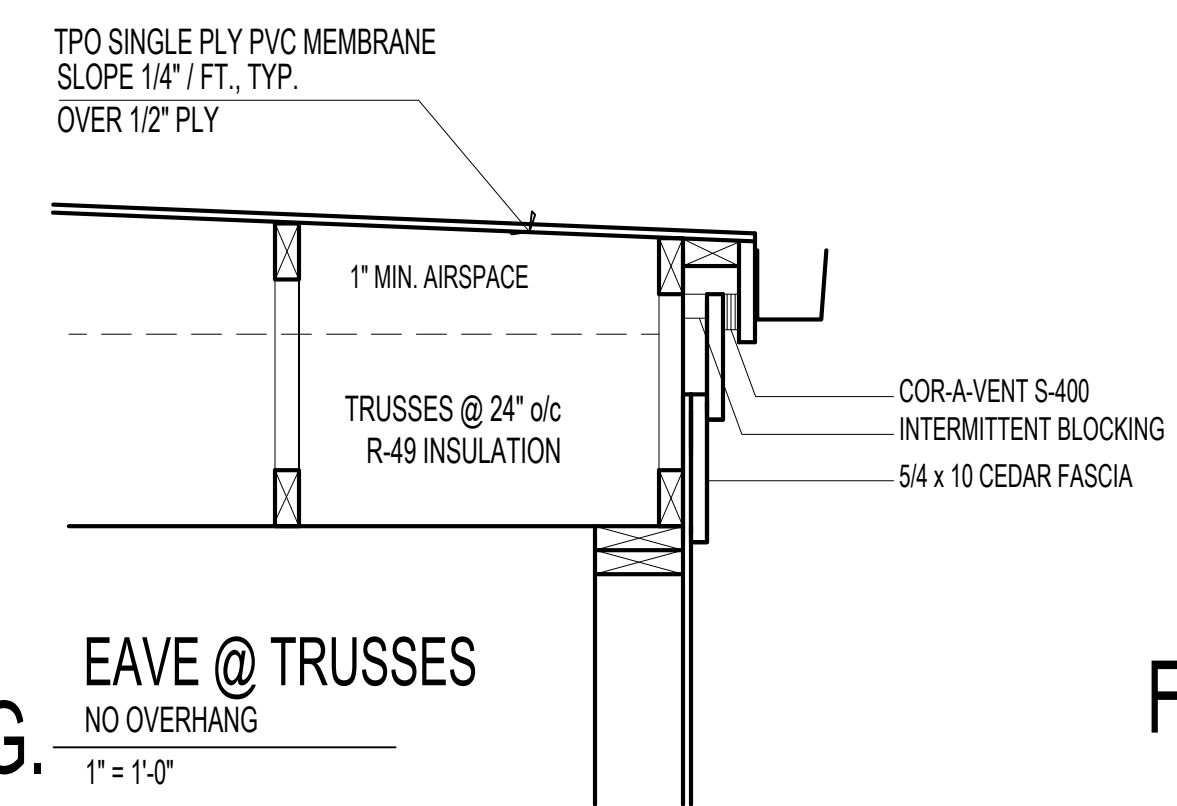
1" = 1'-0"

Enclosed accessible space under stairs shall have walls, under-stair surface and any soffits protected on the enclosed side with 1/2-inch (12.7 mm) gypsum board.



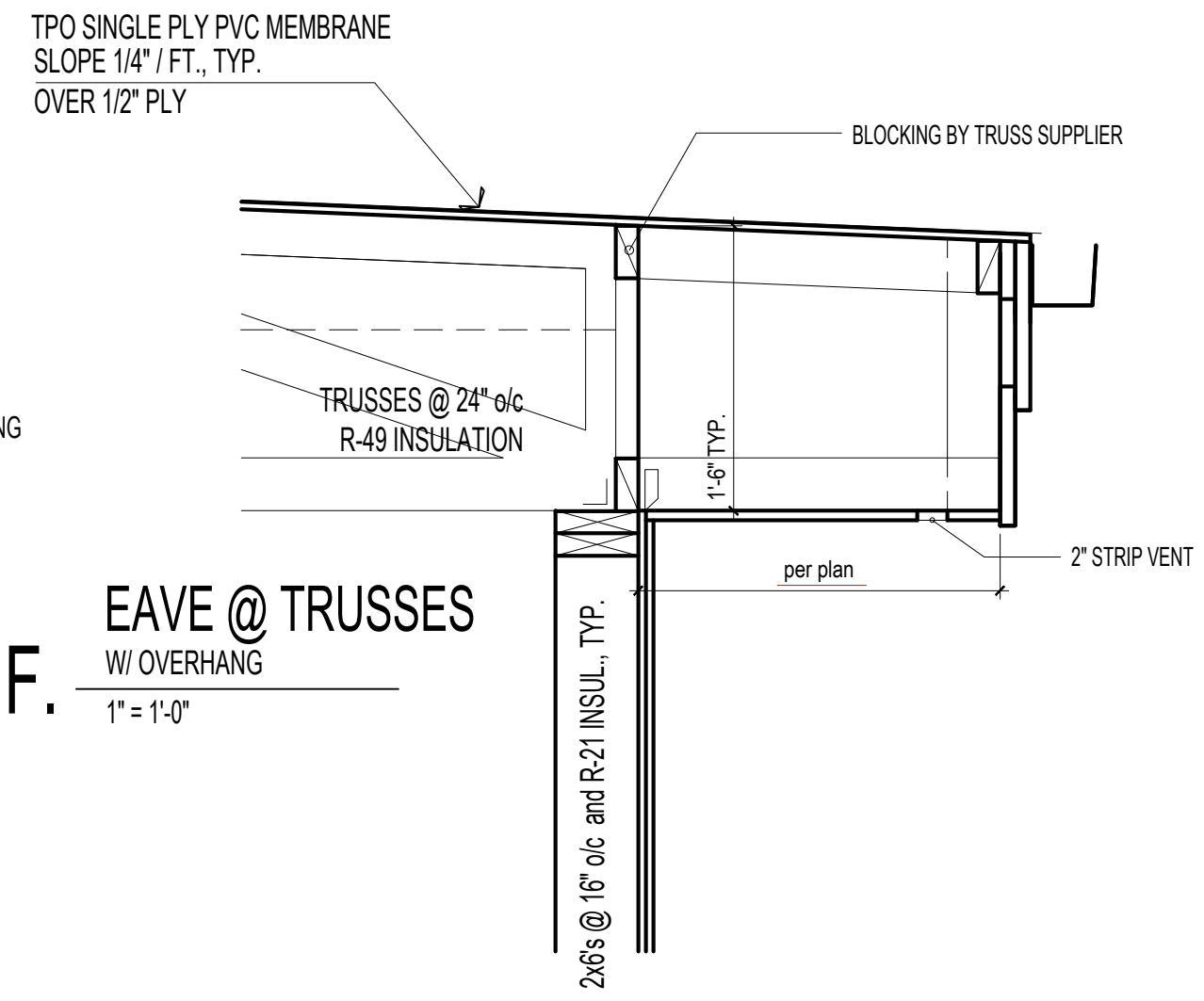
**D. RAILING DETAIL**

NO OVERHANG, perp to jsts.  
 1" = 1'-0"



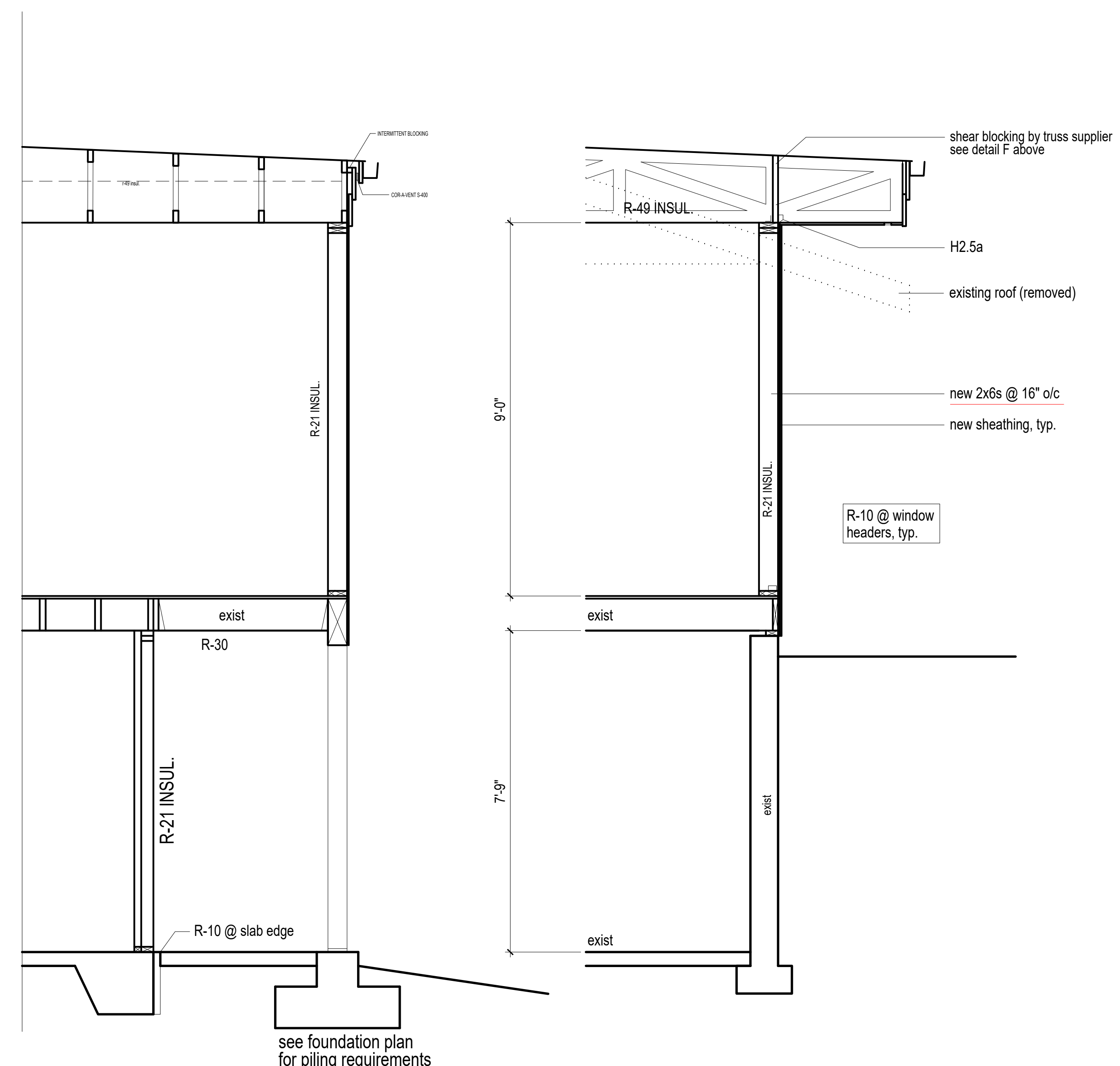
**G. EAVE @ TRUSSES**

NO OVERHANG  
 1" = 1'-0"



**F. EAVE @ TRUSSES**

W/ OVERHANG  
 1" = 1'-0"



**SECTION @ SOUTH BUMP-OUT**

1/2" = 1'-0"

**TYPICAL SECTION**

1/2" = 1'-0"

**CONTENTS**

Arch Details

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# Energy Code Info

2018 WA STATE PRESCRIPTIVE PATH FOR ALL CLIMATE ZONES  
 ENERGY CREDIT OPTIONS =  
 1.7(.5),2(1),2.1(.5),3.6(2),5.5(2) = 6 CREDITS  
 Vertical fenestration U = 0.30  
 Floor R-30

## PRIMARY RESIDENCE HVAC NOTES

DUCTED HEAT PUMP (HSPF>9.0) INT. AIR HANDLER  
 INTEGRATED VENTILATION  
 REQUIRED VENTILATION = CONTINUOUS 120CFM  
 SET TO OPERATE AT 240 CFM FOR 2 HOURS IN EA. 4 HR PERIOD (50%)  
 PROVIDED BY VARIABLE SPEED HIGH EFF. FAN (MAX .35 WATTS/CFM)  
 CONTROLLED TO OPERATE AT LOW SPEED IN VENTILATION MODE ONLY.

design professional or builder shall complete and post an "Insulation Certificate for Residential Construction" within 3' of the electrical panel prior to final inspection.

A minimum of 75 percent of permanently installed lamps in lighting fixtures shall be high-efficacy lamps.

Maximum flow rates for shower heads and kitchen sink - 1.75 GPM or less. All other lavatory faucets - 1.0 GPM or less.

Air leakage shall not exceed 3 air changes/ hour and shall be tested as such. A written report of the test results, shall be signed by the testing party and provided to the building inspector, prior to call for final inspection.

Per WSEC R402.4, The building thermal Envelope shall be constructed to limit air leakage to 3.0 air changes per hour maximum. The results of the test shall be signed by the party conducting the test and provided to the code official (R402.4.1.2). Per WSEC R403.1.1, at least one thermostat per dwelling unit shall be capable of controlling the heating and cooling system on a daily schedule. Per WSEC R403.2.2, Ducts, air handlers, and filter boxes shall be sealed. Per WSEC R404.1, A minimum of 75 percent of the lamps in permanently installed lighting fixtures shall be high-efficacy lamps.

All Climate Zones (Table R402.1.1)		
	R-Value <sup>a</sup>	U-Factor <sup>a</sup>
Fenestration U-Factor <sup>b</sup>	n/a	0.30
Skylight U-Factor <sup>b</sup>	n/a	0.50
Glazed Fenestration SHGC <sup>b,e</sup>	n/a	n/a
Ceiling <sup>e</sup>	49	0.026
Wood Frame Wall <sup>g,h</sup>	21 int	0.056
Floor	30	0.029
Below Grade Wall <sup>c,h</sup>	10/15/21 int + TB	0.042
Slab <sup>d,f</sup> R-Value & Depth	10, 2 ft	n/a

<sup>a</sup> R-values are minimums. U-factors and SHGC are maximums. When insulation is installed in a cavity that is less than the label or design thickness of the insulation, the compressed R-value of the insulation from Appendix Table A101.4 shall not be less than the R-value specified in the table.

<sup>b</sup> The fenestration U-factor column excludes skylights.

<sup>c</sup> "10/15/21 +5TB" means R-10 continuous insulation on the exterior of the wall, or R-15 continuous insulation on the interior of the wall, or R-21 cavity insulation plus a thermal break between the slab and the basement wall at the interior of the basement wall. "10/15/21 +5TB" shall be permitted to be met with R-13 cavity insulation on the interior of the basement wall plus R-5 continuous insulation on the interior or exterior of the wall. "5TB" means R-5 thermal break between floor slab and basement wall.

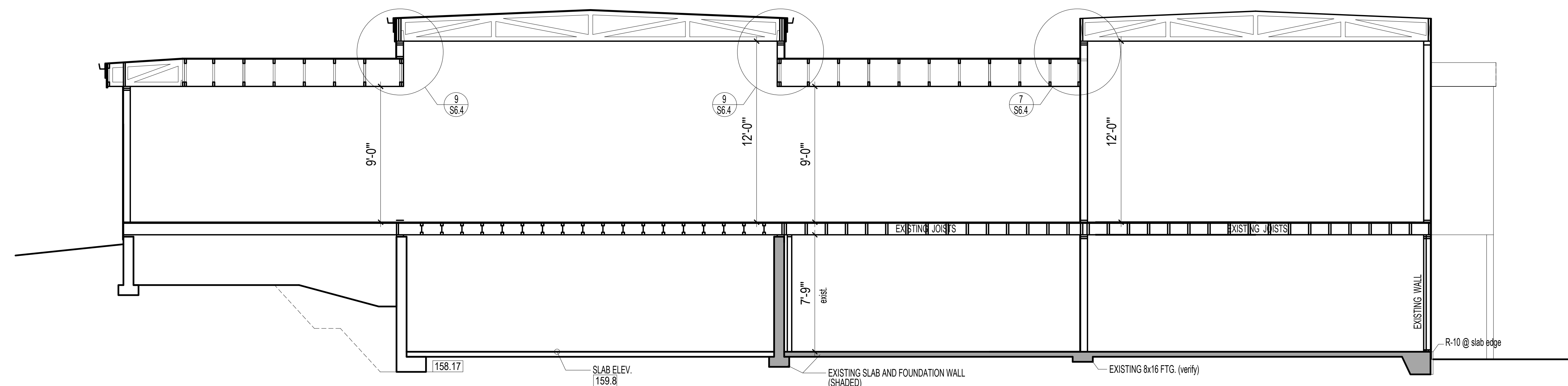
<sup>d</sup> R-10 continuous insulation is required under heated slab on grade floors. See Section R402.2.9.1.

<sup>e</sup> For single rafter- or joist-vaulted ceilings, the insulation may be reduced to R-38 if the full insulation depth extends over the top plate of the exterior wall.

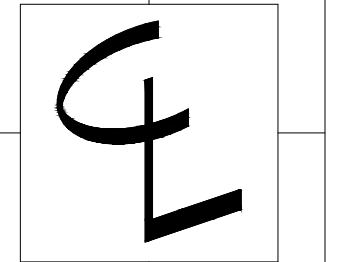
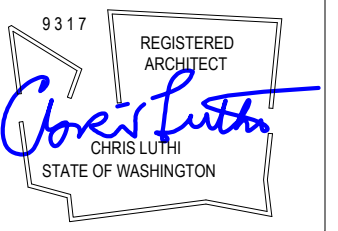
<sup>f</sup> R-7.5 continuous insulation installed over an existing slab is deemed to be equivalent to the required perimeter slab insulation when applied to existing slabs complying with Section R503.1.1. If foam plastic is used, it shall meet the requirements for thermal barriers protecting foam plastics.

<sup>g</sup> For log structures developed in compliance with Standard ICC 400, log walls shall meet the requirements for *climate zone 5* of ICC 400.

<sup>h</sup> Int. (intermediate framing) denotes framing and insulation as described in Section A103.2.2 including standard framing 16 inches on center, 78% of the wall cavity insulated and headers insulated with a minimum of R-10 insulation.



**B.** LONGITUDINAL SECTION B  
 1/4" = 1'-0"



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Shoring Layouts

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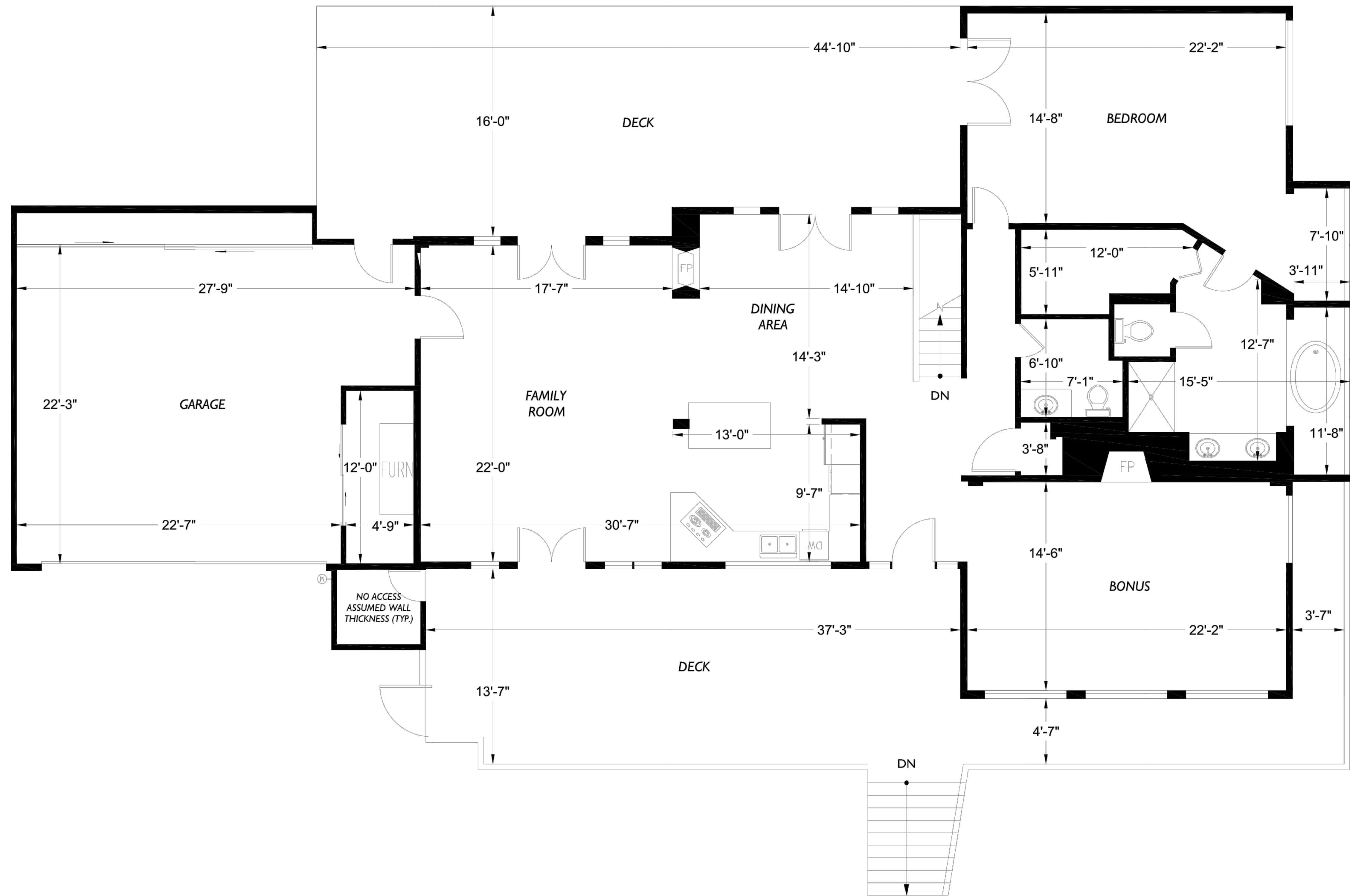
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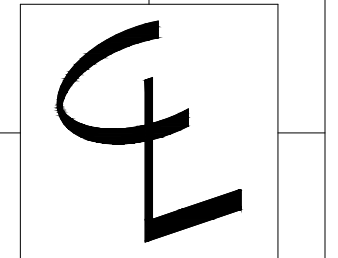
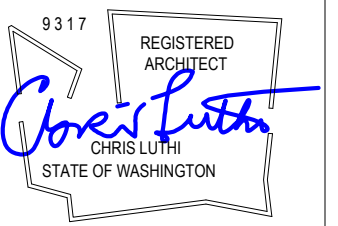
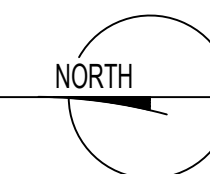
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08



A. EXISTING MAIN FLOOR PLAN

1/4" = 1'-0"



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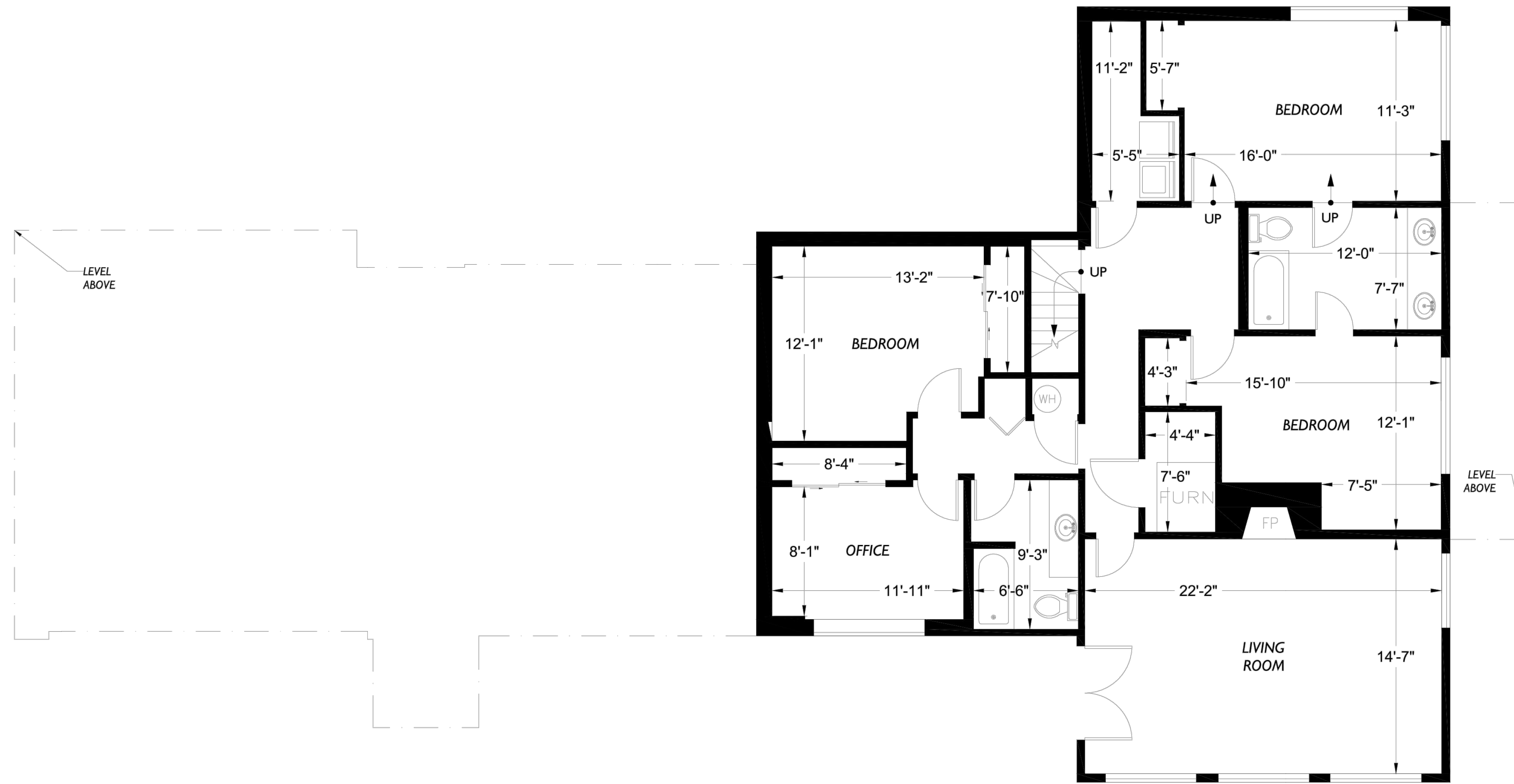
Existing  
Main Floor Plan

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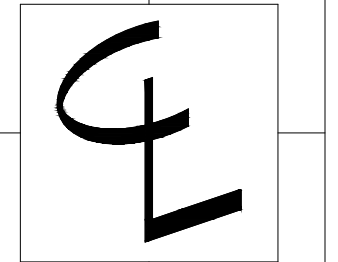
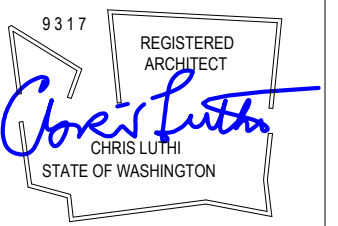
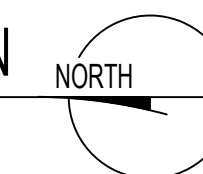
DATE

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A. EXISTING LOWER FLOOR PLAN

1/4" = 1'-0"



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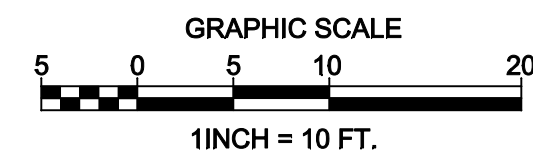
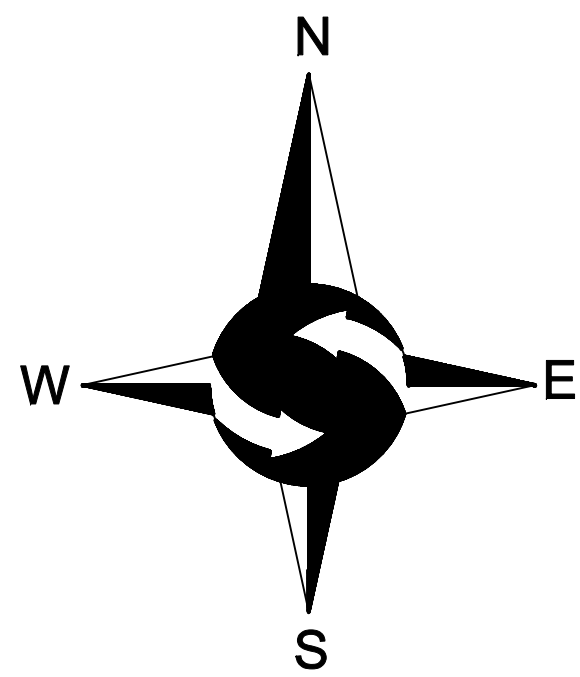
Existing  
Lower Floor Plan

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## DATE

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**LEGEND**

	FOUND MONUMENT AS DESCRIBED		OHP - OVERHEAD POWER
	FOUND REBAR AS DESCRIBED		OHU - OVERHEAD UTILITIES
	FOUND MAG & WASHER		-X- CHAINLINK FENCE
	SET 5/8" X 24" IRON ROD W/1" YELLOW PLASTIC CAP		-□- WOOD FENCE
	SET MAG NAIL AS DESCRIBED		CONCRETE WALL
	GUY WIRE		ROCKERY
	POWER METER		ASPHALT SURFACE
	UTILITY POLE		CONCRETE SURFACE
	GAS METER		GRAVEL SURFACE
	SANITARY SEWER MANHOLE		CE CEDAR
	CATCH BASIN		CH CHERRY
	APPROXIMATE LOCATION UNDERGROUND GAS LINE		DS DECIDUOUS
	APPROXIMATE LOCATION SANITARY SEWER LINE		HE HEMLOCK
	APPROXIMATE LOCATION STORM DRAIN LINE		MP MAPLE
	TIMBER WALL		PA PALM
			PI PINE
			* INDICATES MULTI-TRUNK

**LEGAL DESCRIPTION**

THAT PORTION OF GOVERNMENT LOT 7, SECTION 24, TOWNSHIP 24 NORTH, RANGE 4 EAST, WILLAMETTE MERIDIAN, IN KING COUNTY, WASHINGTON, DESCRIBED AS FOLLOWS: BEGINNING AT THE NORTHEAST CORNER OF GOVERNMENT LOT 7 IN SAID SECTION 24, THENCE SOUTH 0°00'35" WEST ALONG THE EAST LINE THEREOF, 98 FEET; THENCE NORTH 89°33'45" WEST PARALLEL WITH THE NORTH LINE OF SAID SECTION, 208 FEET TO THE TRUE POINT OF BEGINNING; THENCE CONTINUING NORTH 89°33'45" WEST 130 FEET; THENCE SOUTH 0°00'35" WEST 4 FEET; THENCE NORTH 89°33'45" WEST 20 FEET; THENCE SOUTH 0°00'35" WEST 110.05 FEET TO A POINT BEARING NORTH 89°51'58" WEST FROM A POINT ON THE EAST LINE TO SAID SECTION 208.4 FEET SOUTH OF THE NORTHEAST CORNER THEREOF; THENCE SOUTH 89°51'08" EAST TO A POINT BEARING SOUTH 0°00'35" WEST FROM THE TRUE POINT OF BEGINNING; THENCE NORTH 0°00'35" EAST 113.00 FEET, MORE OR LESS, TO THE TRUE POINT OF BEGINNING; TOGETHER WITH AN EASEMENT FOR ROAD PURPOSES OVER THE WEST 40 FEET OF THE EAST 378 FEET OF THE NORTH 106 FEET OF SAID SECTION 24.

**BASIS OF BEARINGS**

RECORD OF SURVEY FOR HELEN SCHWEDENBERG BY NORTH POINTE SURVEYING AS RECORDED UNDER RECORDING NUMBER 20111108900002, RECORDS OF KING COUNTY, WASHINGTON.

**PROJECT INFORMATION**

**SURVEYOR:** SITE SURVEYING, INC.  
21923 NE 11TH ST  
SAMMAMISH, WA 98074  
PHONE: 425.298.4412

**PROPERTY OWNER:** KAM DERAKSHANI  
8151 SE 48TH STREET  
MERCER ISLAND, WA 98040

**TAX PARCEL NUMBER:** 257730-0010

**PROJECT ADDRESS:** 8151 SE 48TH STREET  
MERCER ISLAND, WA 98040

**ZONING:** R-15

**JURISDICTION:** CITY OF MERCER ISLAND

**PARCEL ACREAGE:** 16,963 S.F. (0.389 ACRES) AS SURVEYED

**GENERAL NOTES**

- THIS SURVEY WAS COMPLETED WITHOUT BENEFIT OF A CURRENT TITLE REPORT. EASEMENTS AND OTHER ENCUMBRANCES MAY EXIST ON THIS PROPERTY THAT ARE NOT SHOWN HEREON.
- INSTRUMENTATION FOR THIS SURVEY WAS A 3-SECOND SPECTRAPRECISION FOCUS 38 TOTAL STATION. PROCEDURES USED IN THIS SURVEY MEET OR EXCEED STANDARDS SET BY WAC 332-130-020.
- THE INFORMATION ON THIS MAP REPRESENTS THE RESULTS OF A SURVEY MADE IN AUGUST 2020 AND CAN ONLY BE CONSIDERED AS INDICATING THE GENERAL CONDITIONS EXISTING AT THAT TIME.
- UTILITIES SHOWN ON THIS SURVEY ARE BASED UPON ABOVE GROUND OBSERVATIONS AND AS-BUILT PLANS WHERE AVAILABLE. ACTUAL LOCATIONS OF UNDERGROUND UTILITIES MAY VARY AND UTILITIES NOT SHOWN ON THIS SURVEY MAY EXIST ON THIS SITE.
- ALL MONUMENTS WERE LOCATED DURING THIS SURVEY UNLESS OTHERWISE NOTED.

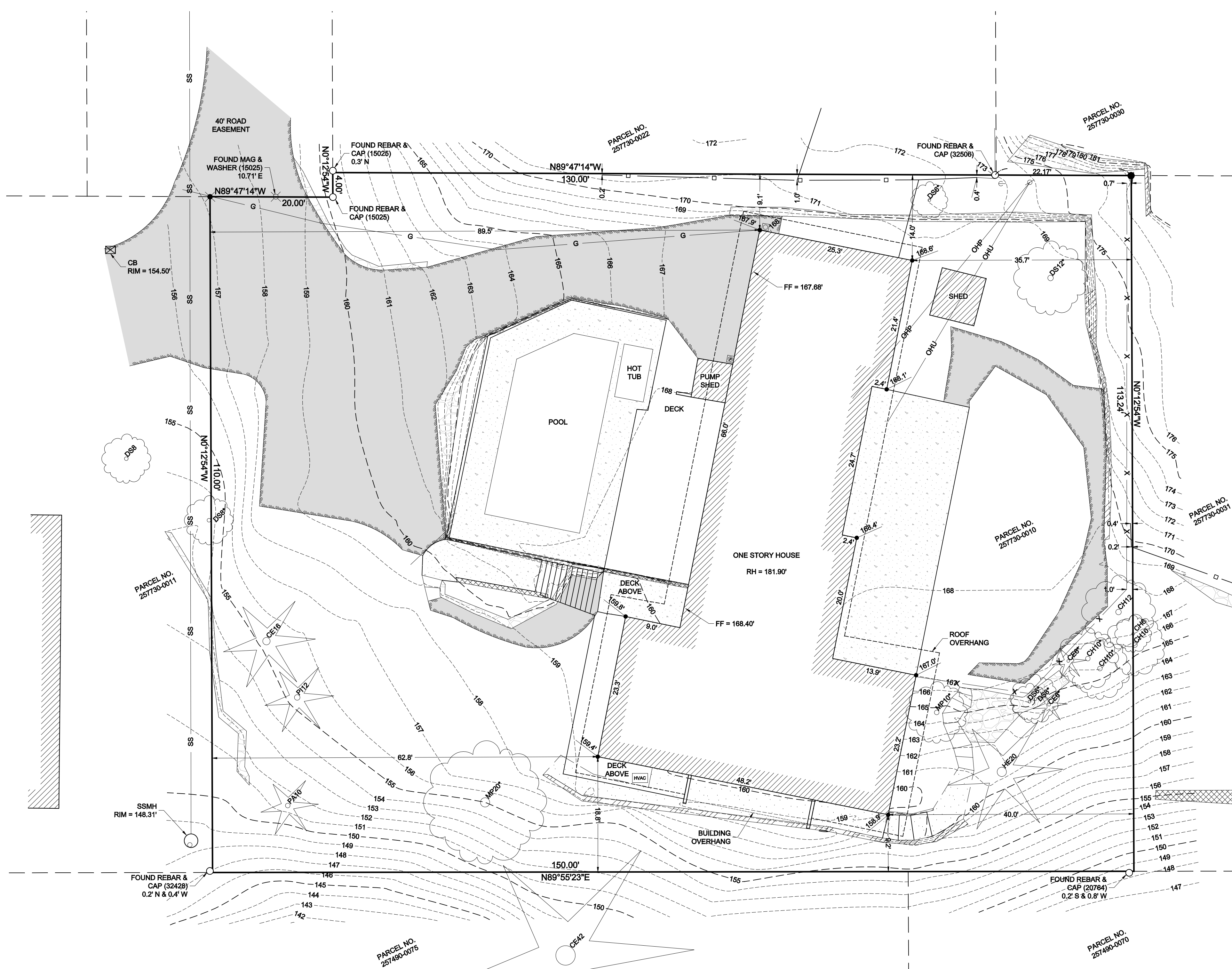
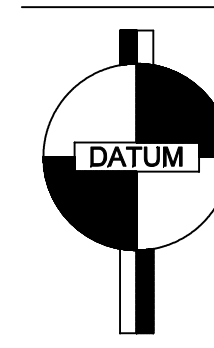
**VERTICAL DATUM & CONTOUR INTERVAL**

ELEVATIONS SHOWN ON THIS DRAWING WERE DERIVED FROM INFORMATION PROVIDED BY WCCS SURVEY CONTROL DATABASE.

THE MARK IS A MONUMENT IN CASE AT THE NE CORNER OF SECTION 24.

POINT ID NO. 8;  
ELEVATION: 202.49 FEET NAVD 88

1.0' CONTOUR INTERVAL - THE EXPECTED VERTICAL ACCURACY IS EQUAL TO 1/2 THE CONTOUR INTERVAL OR PLUS / MINUS 0.5' FOR THIS PROJECT.



VICINITY MAP  
NTS



NE 1/4, NE 1/4, SEC 24, TWP 24N, RNG 4E, W.M.

**TOPOGRAPHIC SURVEY**  
FRANK IMANI  
8151 SE 48TH STREET  
MERCER ISLAND, WA 98040

PROJECT NO. 20-346  
DRAWN BY: MTS  
CHECKED BY: TNW  
DATE: 8/18/2020  
SHEET 1 OF 1

## General Structural Notes (GSN's)

### CRITERIA:

1. ALL MATERIALS, WORKMANSHIP, DESIGN, AND CONSTRUCTION SHALL CONFORM TO THE DRAWINGS, SPECIFICATIONS, AND THE INTERNATIONAL BUILDING CODE (IBC) WITH WASHINGTON STATE ADMINISTRATIVE CODE AMENDMENTS, 2018 EDITION.

2. DESIGN LOADING CRITERIA

RISK CATEGORY IBC TABLE 1604.5	II
ROOF SNOW LOAD	25 PSF ( $s_g = 1.0$ )
ROOF RAIN ON SNOW SURCHARGE	5 PSF <sup>1</sup>
ROOF DEAD LOAD	20 PSF
LIVE LOAD	40 PSF
DECK LIVE LOAD	60 PSF
FLOOR DEAD LOAD	25 PSF

EARTHQUAKE

SEISMIC DESIGN CATEGORY D

$S_g = 1.443$ ,  $S_1 = 0.501$ ,  $S_{0.5} = 1.155$ ,  $S_{0.1} = 0.601$

EQUIVALENT LATERAL FORCE PROCEDURE

LIGHT FRAME (WOOD) WALLS AND ROOFS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR

$R = 6.5$ ,  $C_d = 1.0$ ,  $C_e = 4$ ,  $C_s = 0.178$

BASE SHEAR  $V = 2\% A_s W$  OR LRFD

110 MPH, EXPOSURE "C",  $K_{zt} = 1.0$

COMPONENTS & CLADDING

-34.4/-20.7 PSF MAX. AT WALLS (LRFD/ASD)

-58.3/-35.0 CROSS UP/LIFT AT ROOF (LRFD/ASD)

WIND PRESSURES BASED ON LESS THAN 10 SQUARE FOOT TRIANGULAR AREAS NEAR WALL CORNERS OR ROOF EDGES (INCLUDING ZONE ZONES AT ROOF). REDUCED DESIGN PRESSURES MAY BE CALCULATED IN ACCORDANCE WITH ASCE 7-10 CHAPTER 30.

3. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ALL OTHER CONTRACT DOCUMENTS FOR BIDDING AND CONSTRUCTION. CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS FOR COMPATIBILITY AND SHALL NOTIFY ENGINEER OF ALL DISCREPANCIES PRIOR TO CONSTRUCTION. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE BUILDING LAYOUT DIMENSIONS (GRID LAYOUTS, SITE COORDINATES, ETC.) AMONGST ALL TRADES, INCLUDING SHOP FABRICATED ITEMS.

4. CONTRACTOR SHALL PROVIDE TEMPORARY BRACING, BOTH FOR VERTICAL LOADS AND LATERAL STABILITY, FOR THE STRUCTURE AND STRUCTURAL COMPONENTS UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE DRAWINGS.

5. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES REQUIRED TO PERFORM THE WORK.

6. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER.

7. ALL STRUCTURAL SYSTEMS COMPOSED OF COMPONENTS TO BE FIELD ERECTED SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH INSTRUCTIONS PREPARED BY THE SUPPLIER.

8. SEISMIC BRACING AND/OR GRAVITY SUPPORT AND ANCHORAGE OF ALL MECHANICAL OR ELECTRICAL EQUIPMENT SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF WASHINGTON. EXCEPT FOR ELEMENTS SPECIFICALLY SHOWN AND DETAILED ON THE STRUCTURAL DRAWINGS, THE MECHANICAL/ELECTRICAL CONTRACTOR MUST HIRE THE ENGINEER AND IS RESPONSIBLE FOR ALL COSTS RELATED TO THE PURCHASE AND INSTALLATION OF NECESSARY SUPPORTS, BRACING AND ANCHORAGE. SEISMIC BRACING AND ANCHORAGE DESIGN AND CONSTRUCTION SHALL COMPLY WITH CHAPTER 13 OF ASCE 7-10.

9. SHOP DRAWING REVIEW RECORDS FOR TRUSSES SHALL BE SUBMITTED TO THE CONTRACTOR, ARCHITECT, AND ENGINEER OF RECORD FOR REVIEW PRIOR TO FABRICATION OF THESE ITEMS. DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE ENGINEER OF RECORD, AND THEREFORE MUST BE VERIFIED BY THE CONTRACTOR. THE CONTRACTOR SHALL REVIEW AND STAMP DRAWINGS PRIOR TO REVIEW BY ENGINEER OF RECORD. SUBMITTALS SHALL INCLUDE A REPRODUCIBLE AND ONE COPY. THE REPRODUCIBLE SHALL BE MARKED AND RETURNED. SHOP DRAWING SUBMITTALS PROCESSED BY THE ENGINEER ARE NOT CHANGE ORDERS. THE PURPOSE OF SHOP DRAWING SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE ENGINEER THAT THE CONTRACTOR UNDERSTANDS THE DESIGN CONCEPT BY INDICATING WHICH MATERIAL IS INTENDED TO BE FURNISHED AND INSTALLED AND BY DETAILING THE INTENDED FABRICATION AND INSTALLATION METHODS. IF DEVIATIONS, DISCREPANCIES, OR CONFLICTS BETWEEN SHOP DRAWING SUBMITTALS AND THE CONTRACT DOCUMENTS ARE DISCOVERED EITHER PRIOR TO OR AFTER SHOP DRAWING SUBMITTALS ARE PROCESSED BY THE ENGINEER, THE DESIGN DRAWINGS AND SPECIFICATIONS SHALL CONTROL AND SHALL BE FOLLOWED.

10. DEFERRED SUBMITTALS SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE OF WASHINGTON. THE COMPONENT DESIGNER SHALL BE A REGISTERED STRUCTURAL ENGINEER IF REQUIRED BY THE BUILDING OFFICIAL OF THE LOCAL JURISDICTION. BUILDING COMPONENT SUBMITTALS SHALL INCLUDE THE DESIGNING PROFESSIONAL ENGINEER'S STAMP AND SHALL BE APPROVED BY THE COMPONENT DESIGNER PRIOR TO CURSORY REVIEW BY THE ENGINEER OF RECORD FOR LOADS IMPOSED ON THE BASIC STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE INCLUDING ACCOMMODATION FOR STRUCTURAL DISPLACEMENT PER ASCE 7-10 SECTION 13.3.2. AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. DEFERRED SUBMITTALS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON BASIC STRUCTURE. DESIGN CALCULATIONS SHALL BE INCLUDED IN THE SUBMITTAL. THE CONTRACTOR SHALL FORWARD DEFERRED SUBMITTALS TO THE BUILDING OFFICIAL AND HAVE THE DEFERRED SUBMITTALS ON SITE FOR THE GOVERNING JURISDICTIONS INSPECTORS USE AND REFERENCE. THE FOLLOWING BUILDING COMPONENTS SHALL BE DEFERRED SUBMITTALS FOR THIS PROJECT:

PREFABRICATED CONNECTOR PLATE WOOD ROOF TRUSSES (SEE NOTE 23)

### GEOTECHNICAL:

11. FOUNDATION NOTES: SUBGRADE PREPARATION INCLUDING DRAINAGE, EXCAVATION, COMPACTION, AND FILLING REQUIREMENTS, SHALL CONFORM STRICTLY WITH THE RECOMMENDATIONS GIVEN IN THE SPECIFICATIONS OR AS DIRECTED BY THE OWNER APPOINTED GEOTECHNICAL ENGINEER. FOOTINGS SHALL BEAR ON FIRM, UNDISTURBED EARTH OR CONTROLLED, COMPACTED STRUCTURAL FILL AT LEAST 18" BELOW LOWEST ADJACENT FINISHED GRADE. THE OWNER APPOINTED GEOTECHNICAL ENGINEER SHALL APPROVE FOOTING EXCAVATION/PREPARATION PRIOR TO PLACEMENT OF ALL FOOTINGS. BACKFILL BEHIND ALL RETAINING WALLS WITH FREE DRAINING, GRANULAR FILL AND PROVIDE FOR SUBSURFACE DRAINAGE AS NOTED IN THE SPECIFICATIONS OR AS DIRECTED BY THE OWNER APPOINTED GEOTECHNICAL ENGINEER

ALLOWED SOIL PRESSURE	3,000 PSF
LATERAL EARTH PRESSURE (RESTRAINED / UNRESTRAINED)	45 PCF / 35 PCF
PASSIVE EARTH PRESSURE	300 PCF (ULTIMATE)
SEISMIC SURCHARGE	8H PSF (UNIFORM)
BASE COEFFICIENT OF FRICTION	0.50 (ULTIMATE)
FACTOR OF SAFETY FOR SLIDING AND OVERTURNING	1.50
SOIL PROFILE TYPE	SITE CLASS D

GEOTECHNICAL REPORT REFERENCE: "Geotech Consultants, Inc. ... Geotechnical Engineering Study and Critical Area Study...Project #21062...March 29, 2021"

11b. PIPE PILES SHALL BE 2" EXTRA-STRONG STEEL WITH AN ALLOWABLE COMPRESSIVE LOAD OF 3-TONS. INSTALLATION, FINAL PENETRATION RATE, FINISH, CONNECTION, ETC. SHALL CONFORM STRICTLY WITH THE RECOMMENDATIONS GIVEN IN THE ABOVE GEOTECHNICAL REPORT REFERENCE. PILES SHALL BE DRIVEN TO REFUSAL USING A METHOD APPROVED BY THE PROJECT GEOTECHNICAL ENGINEER. ACTUAL LENGTH OF PILES TO ACHIEVE RECOMMENDED REFUSAL RATE SHALL NOT BE LESS THAN 7 FEET BELOW THE EXISTING GRADE PER GEOTECHNICAL REPORT. PIPE PILE DEPTHS ARE SUBJECT TO ON-SITE VERIFICATION AND APPROVAL BY THE PROJECT GEOTECHNICAL ENGINEER. BATTERED PILES SHALL BE BATTERED DOWN TOWARD THE SOUTH AT A 1:5 (H:V) INCLINATION. DUE TO THE GROUND SURFACE SLOPING AWAY FROM THE SOUTHERN EDGE OF THE RESIDENCE, NO PASSIVE PRESSURE WAS ACCOUNTED FOR AGAINST THE PILE CAPS/GRADE BEAMS FOR THE SOUTHERN BUMP OUT ADDITION. THE LATERAL CAPACITY OF A BATTERED PILE IS EQUAL TO ONE-HALF OF THE LATERAL COMPONENT OF THE ALLOWABLE COMPRESSIVE LOAD, WITH A MAXIMUM ALLOWABLE LATERAL CAPACITY OF 500 POUNDS. THE ALLOWABLE VERTICAL CAPACITY OF BATTERED PILES DOES NOT NEED TO BE REDUCED IF THE PILES ARE BATTERED STEEPER THAN 1:5 (HORIZONTAL:VERTICAL).

### ANCHORAGE:

12. DRIVE PINS AND OTHER POWDER-ACTUATED FASTENERS SHALL BE ONE OF THE FOLLOWING INSTALLED IN STRICT ACCORDANCE WITH THE ICC-ES REPORTS INDICATED AND MANUFACTURER'S INSTRUCTIONS INCLUDING MINIMUM EMBEDDED REQUIREMENTS: "E SERIES" (0.157" DIAMETER) AS MANUFACTURED BY ITW RAMSET (ICC-ES NO. 1799); OR "X-10" (0.157" DIAMETER) AS MANUFACTURED BY HILTI, INC. (ICC-ES NO. 2269); OR "STRONG-TIE PDPA" (0.157" DIAMETER) AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC. (ICC-ES NO. 2138); OR "CSI PIN" (0.157" DIAMETER) AS MANUFACTURED BY DEWALT/POWERS (ICC-ES NO. 2024); OR AN APPROVED EQUIVALENT IN STRENGTH AND EMBEDMENT. MINIMUM EMBEDMENT IN CONCRETE SHALL BE 1" UNLESS OTHERWISE NOTED. MAINTAIN AT LEAST 3-1/2" TO NEAREST CONCRETE EDGE.

### CONCRETE:

13. CONCRETE SHALL BE MIXED, PROPORTIONED, CONVEYED AND PLACED IN ACCORDANCE WITH ACI 318-14 CHAPTER 26 AND ACI 301. CONCRETE SHALL ATTAIN A 28-DAY STRENGTH OF  $f'_c = 4,000$  PSI (4,500 PSI AT ALL CONCRETE EXPOSED TO WEATHER). MAXIMUM WATER-CEMENTITIOUS MATERIAL RATIO FOR INTERIOR SLABS SHALL BE BETWEEN 0.40 AND 0.44. ALL CONCRETE SHALL BE EXPOSURE CLASSES FO, SO, W0, AND CO PER ACI 318-14 TABLES 19.3.1.1 AND 19.3.2.1 EXCEPT AS NOTED BELOW.

ALL CONCRETE EXPOSED TO EARTH (FOUNDATIONS, ETC.): (FO, SO, W0, C1)

ALL CONCRETE EXPOSED TO WEATHER: (F1, SO, W0, C1)

SEE SPECIFICATIONS FOR SHRINKAGE REDUCING CONCRETE MIX CRITERIA WHERE INDICATED ON DRAWINGS. CONCRETE MIXES SHALL MEET OR EXCEED THE REQUIREMENTS SPECIFIED ABOVE. MIXES SHALL BE SUBMITTED TO THE ENGINEER AND BUILDING OFFICIAL FOR APPROVAL TWO WEEKS PRIOR TO PLACING ANY CONCRETE AND SHALL INCLUDE THE AMOUNTS OF CEMENT, CEMENTITIOUS MATERIAL, FINE AND COARSE AGGREGATE, WATER AND ADMIXTURES, AS WELL AS THE WATER-CEMENT RATIO, SLUMP, CONCRETE YIELD AND SUBSTANTIATING STRENGTH DATA IN ACCORDANCE WITH ACI 318-14, CHAPTER 26 AND 27. REVIEW OF MIX SUBMITTALS BY THE ENGINEER OF RECORD INDICATES ONLY THAT INFORMATION PRESENTED CONFORMS GENERALLY WITH CONTRACT DOCUMENTS. CONTRACTOR OR SUPPLIER MAINTAINS FULL RESPONSIBILITY FOR SPECIFIED PERFORMANCE.

14. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60,  $f_y = 60,000$  PSI. GRADE 60 REINFORCING BARS WHICH ARE TO BE WELDED SHALL CONFORM TO ASTM A706. REINFORCEMENT COMPLYING WITH ASTM A615(S1) MAY BE WELDED UNIF. MATERIAL PROPERTY REPORTS INDICATING CONFORMANCE WITH WELDING PROCEDURES SPECIFIED IN A.W.S. DATA ARE SUBMITTED. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064.

15. REINFORCING STEEL SHALL BE DETAILED (INCLUDING HOOKS AND BENDS) IN ACCORDANCE WITH ACI 318-99 AND 318-14. LAP ALL CONTINUOUS REINFORCEMENT IN ACCORDANCE WITH "REINFORCEMENT SPLICE AND DEVELOPMENT LENGTH SCHEDULE" OF 10/53.1. PROVIDE CORNER BARS AT ALL WALL AND FOOTING INTERSECTIONS. LAP ADJACENT MATS OF WELDED WIRE FABRIC A MINIMUM OF 12" AT SIDES AND ENDS. NO BARS PARTIALLY EMBEDDED IN HARDENED CONCRETE SHALL BE FIELD BENT UNLESS OTHERWISE NOTED ON THE DRAWINGS OR APPROVED BY THE STRUCTURAL ENGINEER.

16. CONCRETE PROTECTION (COVER) FOR REINFORCING STEEL SHALL BE AS FOLLOWS:

FOOTINGS AND OTHER UNFORMED SURFACES	3"
CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3"
FORMED SURFACES EXPOSED TO EARTH	1 1/2"

(i.e. WALLS BELOW GROUND/OR WEATHER (#5 BARS OR SMALLER)).

17. BONDING AGENT SHALL BE "MASTEREMCO ADH 326" BY BASF CORPORATION, OR EQUIVALENT, AND SHALL BE USED WHERE NEW CONCRETE IS PLACED AGAINST HARDENED CONCRETE. PLACE IN STRICT ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS, INCLUDING PREPARATION OF EXISTING SURFACES. CONCRETE SHALL BE CONSIDERED HARDENED AFTER 56 DAYS.

18. NON-SHRINK GROUT SHALL BE FURNISHED BY AN APPROVED MANUFACTURER AND SHALL BE MIXED AND PLACED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. GROUT STRENGTH SHALL BE AT LEAST EQUAL TO THE MATERIAL ON WHICH IT IS PLACED (6,000 PSI MINIMUM).

### WOOD:

19. FRAMING LUMBER SHALL BE KILN DRIED OR MC-19, AND GRADED AND MARKED IN CONFORMANCE WITH W.C.L.L.B. STANDARD GRADING RULES FOR WEST COAST LUMBER NO. 17 OR W.N.P.A. WESTERN LUMBER GRADING RULES. FURNISH TO THE FOLLOWING MINIMUM STANDARDS:

PLATES, LEDGERS & MISC. LIGHT FRAMING:	DOUGLAS FIR NO. 3 OR STUD GRADE	$F_x = 525$ PSI, $E = 1,400$ KSI
MIN. BASIC DESIGN STRESS, $F_b = 525$ PSI, $E = 1,400$ KSI		
DOUGLAS FIR NO. 3 OR STUD GRADE		$F_x = 775$ PSI, $F_y = 325$ PSI
MIN. BASIC DESIGN STRESS, $F_b = 1,000$ PSI, $E = 1,700$ KSI		
DOUGLAS FIR NO. 1		$F_x = 1,500$ PSI, $F_y = 1,000$ PSI
MIN. BASIC DESIGN STRESS, $F_b = 1,000$ PSI, $E = 1,700$ KSI		

20. MANUFACTURED LUMBER SHALL BE AS MANUFACTURED BY TRUS JOIST OR APPROVED EQUAL. REQUESTS FOR APPROVAL AS EQUAL WILL REQUIRE SUBMITTAL OF ICC REPORT EQUIVALENT TO ESR-1387 FOR LAMINATED VENER LUMBER (LVL), LAMINATED STRAND LUMBER (LSL), OR PARALLEL STRAND LUMBER (PSL). THE MINIMUM ALLOWABLE DESIGN VALUES ARE AS FOLLOWS:

LVL - $F_b = 2,600$	$F_x = 290$ PSI	$E = 2,000,000$ PSI
LSL - $F_b = 1,900$	$F_x = 150$ PSI	$E = 1,300,000$ PSI

21. ENGINEERED WOOD I-JOISTS SHALL BE FURNISHED AND INSTALLED IN CONFORMANCE WITH THE MANUFACTURER'S INSTRUCTIONS. ALL NECESSARY BRIDGING, BLOCKING, BLOCKING PANELS, STIFFENERS, ETC., SHALL BE DETAILED AND FURNISHED BY THE MANUFACTURER. PERMANENT AND TEMPORARY BRIDGING SHALL BE INSTALLED IN CONFORMANCE WITH MANUFACTURER'S INSTRUCTIONS. ALL JOIST HANGERS AND OTHER HARDWARE SHALL BE COMPATIBLE IN SIZE WITH ENGINEERED WOOD I-JOISTS PROVIDED. DESIGN SHOWN ON THE DRAWINGS IS BASED ON RESIDENTIAL JOISTS MANUFACTURED BY WEINHAUSER IN ACCORDANCE WITH ICC-ES REPORT NO. ESR-1153. ALTERNATE ENGINEERED WOOD I-JOISTS MAY BE USED SUBJECT TO REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER OF RECORD

22. GLUED LAMINATED MEMBERS SHALL BE FABRICATED IN CONFORMANCE WITH ASTM AND A.I.T.C. STANDARDS IN ACCORDANCE WITH IBC SECTION 2303.1.3. EACH MEMBER SHALL BEAR AN A.I.T.C. IDENTIFICATION MARK AND SHALL BE ACCOMPANIED BY AN A.I.T.C. CERTIFICATE OF CONFORMANCE. HORIZONTAL MEMBERS AND INCLUDED MEMBERS OF LESS THAN 1:1 SLOPE SHALL HAVE A RADIIUSED CAMBER OF 3.500 FT. UNLESS OTHERWISE NOTED.

SIMPLE SPAN BEAMS DOUGLAS FIR COMBINATION 24F-VR

$F_b = 2,400$  PSI;  $F_x = 265$  PSI;  $E = 1,800,000$  PSI

GLUED LAMINATED MEMBERS EXPOSED TO WEATHER OR MOISTURE SHALL BE TREATED WITH A NON-CORROSIVE, APPROVED PRESERVATIVE.

23. PREFABRICATED CONNECTOR PLATE WOOD ROOF TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH ANS/TP1 I-2007 AND IBC SECTION 2303.4 FOR THE SPANS AND CONDITIONS SHOWN ON THE DRAWINGS.

DESIGN LOADS SHALL BE AS FOLLOWS:

TOP CHORD LIVE LOAD	25 PSF, SNOW + 5 PSF, RAIN ON SNOW SURCHARGE
BOTTOM CHORD LIVE LOAD	0 PSF
TOP CHORD DEAD LOAD	15 PSF
BOTTOM CHORD DEAD LOAD	5 PSF
WIND UPLIFT (TOP CHORD)	SEE NOTE#2 COMPONENTS & CLADDING ROOF LOADS

THE TRUSS MANUFACTURER SHALL COORDINATE LOCATIONS AND SUPPORT CONFIGURATIONS OF PLUMBING, MECHANICAL UNITS, DUCTS, AND/OR OTHER MISCELLANEOUS ITEMS WITH THE CONTRACTOR PRIOR TO TRUSS FABRICATION. THE TRUSS MANUFACTURER SHALL DESIGN TRUSSES TO SUPPORT ALL LOADS ASSOCIATED WITH SUCH ITEMS. THE TRUSS SHOP DRAWINGS SHALL INCLUDE ALL DESIGN LOADS AND APPROVED HANGER CONNECTION DETAILS TO TRUSS CHORDS FOR SUPPORT OF HUNG MECHANICAL SYSTEM COMPONENTS AS APPLICABLE.

WOOD TRUSSES SHALL UTILIZE APPROVED CONNECTOR PLATES (GANGNAIL OR EQUAL). SHOP DRAWINGS AND CALCULATIONS SHALL BE PROVIDED AS A DEFERRED SUBMITTAL TO THE CONTRACTOR AND STRUCTURAL ENGINEER OF RECORD PER GENERAL STRUCTURAL NOTE 13. SHOP DRAWINGS SHALL INDICATE SHAPES, BEARING POINTS, INTERSECTIONS, HIPS, VALLEYS, ETC. EXACT COMPOSITION OF SPECIAL HIP, VALLEY, AND INTERSECTION AREAS (USE OF ORDER TRUSSES, JACK TRUSSES, STEP-DOWN TRUSSES, ETC.) SHALL BE DETERMINED BY THE MANUFACTURER UNLESS OTHERWISE NOTED ON THE DRAWINGS. THE TRUSS MANUFACTURER SHALL PROVIDE ALL TRUSS-TO-TRUSS BEAM/JOIST CONNECTION DETAILS AND REQUIRED CONNECTION MATERIALS. THE TRUSS MANUFACTURER SHALL DESIGN AND PROVIDE DETAILS FOR ALL TEMPORARY AND PERMANENT TRUSS BRACING AND BRIDGING.

24. ROOF & WALL SHEATHING SHALL BE APA RATED, EXTERIOR OR EXPOSURE 1 PLYWOOD OR ORIENTED STRAND BOARD (OSB) IN CONFORMANCE WITH IBC SECTION 2303.1.5. SHEATHING SHALL BE MANUFACTURED UNDER THE PROVISIONS OF VOLUNTARY PRODUCT STANDARDS DOC PS-1-09, PS-2-10, OR APA PRP-108 PERFORMANCE STANDARDS AND POLICES FOR STRUCTURAL USE PANELS. SEE DRAWINGS FOR THICKNESS, SPAN RATING, AND NAILING REQUIREMENTS.

25. AT NON-SHEAR WALL EXTERIOR WALLS, UNLESS OTHERWISE NOTED, WALL SHEATHING SHALL BE 1/2" (NOMINAL) WITH SPAN RATING OF 3/8" WITH 8d @ 6" oc PANEL NAILING (APPLIES TO ALL SHEATHING PANEL EDGES); AND 8d @ 12" oc TO INTERMEDIATE FRAMING.

26. ALL PRESSURE-TREATED (P-T) WOOD MEMBERS SPECIFIED ON THE DRAWINGS THAT OCCUR ABOVE GROUND AND CONTINUOUSLY PROTECTED FROM MOISTURE (INTERIOR LOCATIONS) SHALL BE PRESURE-TREATED WITH DOT SODIUM BORATE (SBX) WITHOUT  $Na_2SO_3$  AT LOCATIONS PERMANENTLY EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND. WOOD MEMBERS SHALL BE PRESURE-TREATED WITH ALKALINE COPPER QUAT (ACQ-C FOR DOUGLAS-FIR) PRESERVATIVE UNLESS OTHERWISE NOTED. AMMONIACAL COPPER ZINC ARSENATE (ACZA) PRESERVATIVE OR OTHER PRESERVATIVES WITH AMMONIA CARRIERS, SHALL NOT BE USED. GLUED LAMINATED MEMBERS EXPOSED TO WEATHER OR MOISTURE SHALL BE TREATED WITH A NON-CORROSIVE, APPROVED PRESERVATIVE.

SEE NOTE #27 FOR MATERIAL REQUIREMENTS OF CONNECTORS AND FASTENERS IN CONTACT WITH PRESURE-TREATED MEMBERS.

27. TIMBER CONNECTORS CALLED OUT BY LETTERS AND NUMBERS SHALL BE "STRONG-TIE" BY SIMPSON COMPANY, AS SPECIFIED IN THEIR WOOD CONSTRUCTION CONNECTORS CATALOG NO. C-C-2017-18. INSTALL NUMBER AND SIZE OF FASTENERS AS SPECIFIED BY MANUFACTURER. CONNECTORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. WHERE CONNECTOR STRIPS CONNECT TWO MEMBERS, CENTER STRIP ON JOIST AND INSTALL NUMBER AND SIZE OF FASTENERS AS SPECIFIED BY MANUFACTURER, WITH EQUAL NUMBER AND SIZE OF FASTENERS IN EACH MEMBER. ALL BOLTS IN WOOD MEMBERS SHALL CONFORM TO ASTM A307. INSTALL WASHERS UNDER THE HEADS AND NUTS OF ALL BOLTS AND LAG SCREWS BEARING ON WOOD. ALL SHIMS SHALL BE SEASONED AND DRIED AND THE SAME GRADE (MINIMUM) AS MEMBERS CONNECTED.

ALL TIMBER CONNECTORS IN CONTACT WITH PRESSURE-TREATED WOOD THAT USED PRESERVATIVE CHEMICALS OTHER THAN DOT SODIUM BORATE (SBX) WITHOUT  $Na_2SO_3$  SHALL BE MANUFACTURED FROM Z<sub>MAX</sub> STEEL BY SIMPSON (G185 STEEL PER ASTM A653), OR TYPE 304 OR 316 STAINLESS STEEL. ALTERNATIVELY, CONNECTORS CAN BE POST HOT DIP GALVANIZED PER ASTM A123 OR MECHANICALLY GALVANIZED PER ASTM B695, CLASS 55 OR GREATER. STAINLESS STEEL FASTENERS SHALL BE USED WITH STAINLESS STEEL CONNECTORS, AND HOT DIP GALVANIZED FASTENERS PER ASTM A153 SHALL BE USED WITH GALVANIZED CONNECTORS.

28. WOOD FRAMING NOTES: THE FOLLOWING SHALL APPLY UNLESS OTHERWISE NOTED ON THE DRAWINGS:

A. ALL WOOD FRAMING DETAILS SHALL BE CONSTRUCTED TO THE MINIMUM STANDARDS OF THE IBC. MINIMUM NAILING SHALL CONFORM TO IBC TABLE 2304.9.1 OR CURRENT ICC-ES REPORT NER-272. COORDINATE THE SIZE AND LOCATION OF ALL OPENINGS WITH MECHANICAL AND ARCHITECTURAL DRAWINGS. INSTALL WASHERS UNDER THE HEADS AND NUTS OF ALL BOLTS AND LAG SCREWS BEARING ON WOOD. INSTALLATION OF LAG SCREWS SHALL CONFORM TO 2012 NDS SECTION 11.1.4, AND INSTALLATION OF BOLTS SHALL CONFORM TO 2012 NDS SECTION 11.1.3.

B. WALL FRAMING: TWO STUDS MINIMUM SHALL BE INSTALLED AT THE ENDS OF ALL WALLS, UNLESS NOTED OTHERWISE. INSTALL SOLID BLOCKING FOR WOOD COLUMN THROUGH FLOOR SPACES TO SUPPORTS BELOW.

ALL STUD WALLS SHALL HAVE THEIR LOWER WOOD PLATES ATTACHED TO WOOD FRAMING BELOW WITH 16d NAILS @ 12" oc STAGGERED OR BOLTED TO CONCRETE WITH 3/8" ANCHOR BOLTS @ 4'-0" oc PER IBC SECTION 2308.6 (EMBED 7"), UNLESS OTHERWISE NOTED. 3" x 3" x 0.229" PLATE WASHERS SHALL BE USED WITH ALL SILL PLATE ANCHOR BOLTS AND INSTALLED PER AFAPA SDFWS-2008 SECTION 4.3.6.4.3. INDIVIDUAL MEMBERS OF BUILT-UP STUD POSTS SHALL BE NAILED TO EACH OTHER WITH 16d @ 12" oc STAGGERED.

C. FLOOR AND ROOF FRAMING: INSTALL SOLID BLOCKING AT ALL BEARING POINTS. TOENAIL JOISTS TO SUPPORTS WITH (2)16d NAILS. ATTACH TIMBER JOISTS TO FLUSH HEADERS OR BEAMS WITH SIMPSON METAL JOIST HANGERS IN ACCORDANCE WITH NOTES ABOVE. NAIL ALL MAIN-JOIST BEAMS TOGETHER WITH 16d@12" oc STAGGERED.

ROOF AND FLOOR SHEATHING SHALL BE LAID UP WITH GRAN PERPENDICULAR TO SUPPORTS AND NAILED AS SHOWN ON THE DRAWINGS. INSTALL APPROVED PANEL EDGE CLIPS CENTERED BETWEEN JOISTS/TRUSSES AT UNLOCKED ROOF SHEATHING EDGES. ALL FLOOR SHEATHING EDGES SHALL HAVE APPROVED TONGUE-AND-GROOVE JOINTS OR SHALL BE SUPPORTED WITH SOLID BLOCKING ALLOW 1/2" SPAcing AT ALL PANEL EDGES AND ENDS OF LOOR AND ROOF SHEATHING. TOENAIL BLOCKING TO SUPPORTS WITH 16d@12" oc. IN ACCORDANCE WITH IBC SECTION 1604.8.1, DECKS SHALL BE POSITIVELY ANCHORED TO THE STRUCTURE BY MEANS OTHER THAN NAILS SUBJECT TO WITHDRAGAL. WOOD WITH MINIMUM (1) C16 STRIP AT EACH END ATTACHED TO DECK JOISTS AND TO A SOLID BLOCKING MEMBER WITHIN THE BUILDING.

### POST-INSTALLED ANCHORS AND EPOXY ADHESIVE:

29. EPOXY-GROUTED RODS OR REBAR TO CONCRETE SPECIFIED ON THE DRAWINGS SHALL BE ONE OF THE FOLLOWING INSTALLED IN STRICT ACCORDANCE WITH THE ICC-ES REPORTS INDICATED AND MANUFACTURER'S INSTRUCTIONS INCLUDING MINIMUM EMBED REQUIREMENTS: "SET-10" AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC. (ICC-ES NO. 2508); OR "HIT-HY 200" AS MANUFACTURED BY HILTI, INC. (ICC-ES NO. 3167), "SAFE-SET" INSTALLATION WITH HOLLOW CARBIDE DRILL BIT IS PERMITTED, OR "PURE110+" AS MANUFACTURED BY DEWALT/POWERS (ICC-ES NO. 3298). SUBSTITUTES PROPOSED BY CONTRACTOR SHALL BE SUBMITTED FOR REVIEW WITH ICC-ES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. IN ADDITION, SUBSTITUTIONS SHALL MEET ICC-ES ACCEPTANCE CRITERIA AC308. SPECIAL INSPECTION OF EPOXY-GROUTED ANCHOR INSTALLATION IS REQUIRED. EPOXY GROUTED RODS OR REBAR SHALL NOT BE USED AS SUBSTITUTES FOR CAST-IN-PLACE ANCHOR BOLTS OR REINFORCING STEEL UNLESS SPECIFICALLY APPROVED BY THE STRUCTURAL ENGINEER. NOTIFY ENGINEER IF ANCHOR LOCATIONS CONFLICT WITH REINFORCING STEEL - DO NOT CUT REINFORCING OR REDUCE EMBEDMENT DEPTHS WITHOUT PRIOR APPROVAL. INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCLINED TO SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED BY CERTIFIED PERSONNEL IN CONFORMANCE TO ACI 318-14 SECTION 17.8.2.2. HOLES SHALL BE HAMMER DRILLED AND DRY.

30. EXPANSION ANCHORS SHALL BE ONE OF THE APPROVED PRODUCTS BELOW:

- KWIK BOLT TZ ANCHORS AS MANUFACTURED BY HILTI, INC. AND INSTALLED IN STRICT ACCORDANCE WITH ICC-ES REPORT NO. 1917. OR
- STRONG-BOLT Z AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC. AND INSTALLED IN STRICT ACCORDANCE WITH ICC-ES REPORT NO. 3037

AND INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

REQUIRED?	VERIFICATION & INSPECTION	CONTINUOUS/PERIODIC	REF. STD.	IBC REF.
N*	1. INSPECT REINFORCEMENT, INCLUDING TENDONS AND VERIFY PLACEMENT.	---	X ACI 318 CH. 20, 25.2, 25.3, 26.2.1-26.2.3	1908.4
N	2. REINFORCING BAR WELDING: A. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A 706. B. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16", AND C. INSPECT ALL OTHER WELDS.	---	X AWS D1.4 ACI 318 26.2.4	---
YES	3. INSPECT ANCHORS CAST IN CONCRETE.	---	X ACI 318: 17.8.2	---
YES	4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS. A. ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESS. B. SUSTAINED TENSION LOADS C. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.4.	X	X ACI 318: 17.8.2.4	---
N*	5. VERIFY USE OF REQUIRED DESIGN MIX.	---	X ACI 318: CH. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
N*	6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	---	ASTM C 172 ASTM C 31 ACI 318: 26.4.3, 26.4.12
N*	7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	---	ACI 318: 26.4.5
N*	8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	---	X ACI 318: 26.4.7-26.4.9	1908.6, 1908.7, 1908.8
N	9. INSPECT PRESTRESSED CONCRETE FOR: A. APPLICATION OF PRESTRESSING FORCES; AND B. GROUPING OF BONDED PRESTRESSING TENDONS	X	---	ACI 318: 26.9.2.1 ACI 218: 26.9.2.3
N	10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	---	X ACI 318: CH. 26.8	---
N*	11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	---	X ACI 318: 26.10.2	---
N*	12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	---	X ACI 318: 26.10.1(b)	---

\* EXEMPTIONS 2 PER IBC SECTION 1705.3 APPLIES TO CONCRETE WORK ON THIS PROJECT.

## Minimum Connectors and Fasteners for Wood Members per IBC 2018

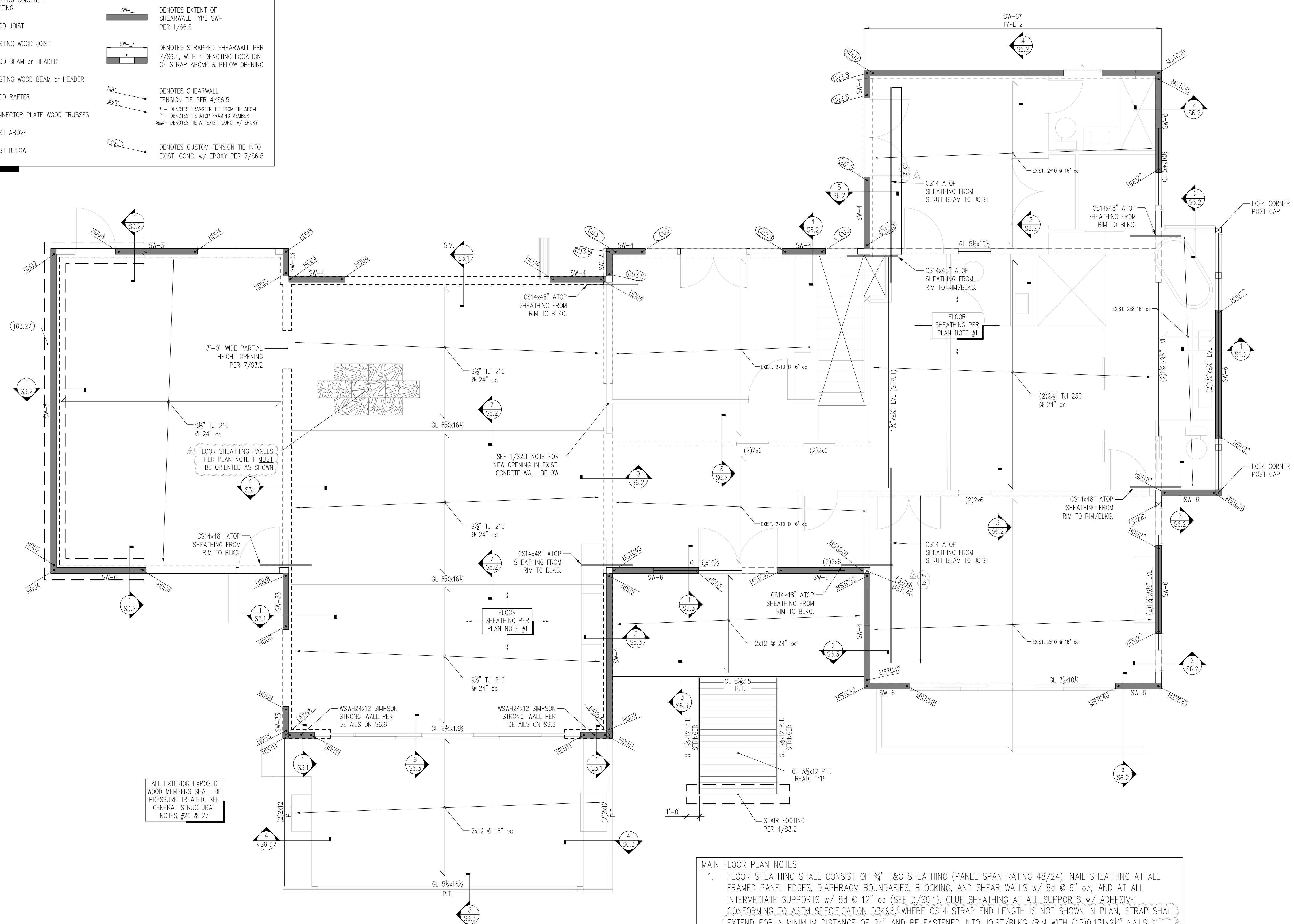
DESCRIPTION OF BUILDING ELEMENT	NUMBER AND TYPE OF FASTENERS	SPACING & LOCATION
ROOF		
1. BLOCKING BETWEEN CEILING JOISTS, RAFTERS, OR TRUSSES TO TOP PLATE OR OTHER FRAMING BELOW	3-8d COMMON (2 1/2" x 0.131"); or 3-10d BOX (3" x 0.128"); or 3-5" x 0.131" NAILS; or 3-5" x 14 GAGE STAPLES, 3/16" CROWN	EACH END, TOENAIL
BLOCKING BETWEEN RAFTERS OR TRUSS NOT AT THE WALL TOP PLATE, TO RAFTER OR TRUSS	2-8d COMMON (2 1/2" x 0.131") 2-3" x 0.131" NAILS 2-3" x 14 GAGE STAPLES	EACH END, TOENAIL
FLAT BLOCKING TO TRUSS AND WEB FILLER	2-16d COMMON (3 1/2" x 0.162") 3-3" x 0.131" NAILS	END NAIL
2. CEILING JOISTS TO TOP PLATE	16d COMMON (3 1/2" x 0.162") @ 6" oc 3" x 0.131" NAILS @ 6" oc	FACE NAIL
3. CEILING JOIST NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITION (NO THRUST) (SEE 2308.7.3.1, TABLE 2308.7.3.1)	3-8d COMMON (2 1/2" x 0.131"); or 3-10d BOX (3" x 0.128"); or 3-3" x 0.131" NAILS; or 3-3" x 14 GAGE STAPLES, 3/16" CROWN	EACH JOIST, TOENAIL
4. CEILING JOIST ATTACHED TO PARALLEL RAFTER (HEEL JOINT)	3-16d COMMON (3 1/2" x 0.162"); or 4-10d BOX (3" x 0.128"); or 4-3" x 0.131" NAILS; or 4-3" x 14 GAGE STAPLES, 3/16" CROWN	FACE NAIL
5. COLLAR TIE TO RAFTER	PER TABLE 2308.7.3.1	FACE NAIL
6. RAFTER OR ROOF TRUSS TO TOP PLATE (SEE 2308.7.5, TABLE 2308.7.5)	3-10d COMMON (3" x 0.148"); or 3-16d BOX (3 1/2" x 0.135"); or 4-10d BOX (3" x 0.128"); or 4-3" x 0.131" NAILS; or 4-3" x 14 GAGE STAPLES, 3/16" CROWN	FACE NAIL
7. ROOF RAFTERS TO RIDGE VALLEY OR HIP RAFTERS; OR ROOF RAFTER TO 2" RIDGE BEAM	3-10d COMMON (3" x 0.148"); or 3-16d BOX (3 1/2" x 0.135"); or 4-10d BOX (3" x 0.128"); or 4-3" x 0.131" NAILS; or 4-3" x 14 GAGE STAPLES, 3/16" CROWN	TOENAIL
8. STUD TO STUD (NOT AT SHEARWALL CHORDS)	16d COMMON (3 1/2" x 0.162")	24" oc FACE NAIL
9. STUD TO STUD AND ABUTTING STUDS AT INTERSECTION WALL CORNERS	10d BOX (3" x 0.128"); or 3" x 0.131" NAILS; or 	





LEGEND

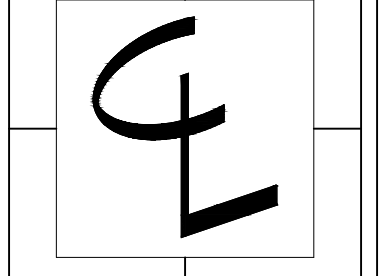
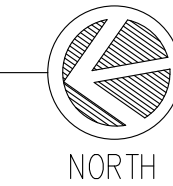
	REINFORCED CONCRETE WALL		EXISTING CONCRETE FOOTING		DENOTES STRAP TYPE BY LENGTH, CENTERED ON ABUTTING ELEMENTS
	REINFORCED CONCRETE WALL BELOW		WOOD JOIST		DENOTES EXTENT OF SHEARWALL TYPE SW- PER 1/S6.5
	EXISTING CONCRETE WALL BELOW		EXISTING WOOD JOIST		DENOTES STRAPPED SHEARWALL PER 7/S6.5, WITH * DENOTING LOCATION OF STRAP ABOVE & BELOW OPENING
	STRUCTURAL WOOD STUDWALL ABOVE		WOOD BEAM or HEADER		DENOTES SHEARWALL TENSION TIE PER 4/S6.5
	STRUCTURAL WOOD STUDWALL BELOW		EXISTING WOOD BEAM or HEADER		* -- DENOTES TENSION TIE FROM TIE ABOVE ^ -- DENOTES TIE AT TOP FRAMING MEMBER @ -- DENOTES TIE AT EXIST. CONC. w/ EPOXY
	EXISTING STRUCTURAL WOOD STUDWALL BELOW		WOOD RAFTER		DENOTES SHEARWALL TENSION TIE PER 4/S6.5
			CONNECTOR PLATE WOOD TRUSSES		* -- DENOTES TENSION TIE FROM TIE ABOVE ^ -- DENOTES TIE AT TOP FRAMING MEMBER @ -- DENOTES TIE AT EXIST. CONC. w/ EPOXY
			POST ABOVE		DENOTES CUSTOM TENSION TIE INTO EXIST. CONC. w/ EPOXY PER 7/S6.5
			POST BELOW		



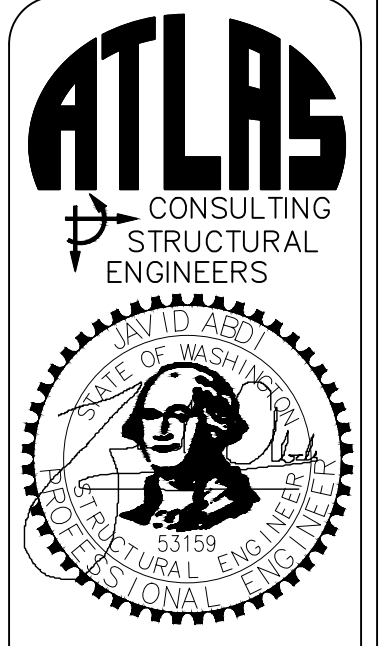
ALL EXTERIOR EXPOSED WOOD MEMBERS SHALL BE PRESSURE TREATED, SEE GENERAL STRUCTURAL NOTES #26 & 27

- MAIN FLOOR PLAN NOTES**
- FLOOR SHEATHING SHALL CONSIST OF 3/4" T&G SHEATHING (PANEL SPAN RATING 48/24). NAIL SHEATHING AT ALL FRAMED PANEL EDGES, DIAPHRAGM BOUNDARIES, BLOCKING, AND SHEAR WALLS w/ 8d @ 6" oc; AND AT ALL INTERMEDIATE SUPPORTS w/ 8d @ 12" oc (SEE 3/S6.1). GLUE SHEATHING AT ALL SUPPORTS w/ ADHESIVE CONFORMING TO ASTM SPECIFICATION D3498, WHERE CS14 STRAP END LENGTH IS NOT SHOWN IN PLAN, STRAP SHALL EXTEND FOR A MINIMUM DISTANCE OF 24" AND BE FASTENED INTO JOIST/BLKG./RIM WITH (15)0.131x2 1/2" NAILS. EXTERIOR STUDWALLS SHALL BE 2x6 STUDS @ 16" oc. SEE 6/6.1, 5/S6.1, AND 2/S6.1 FOR ALLOWABLE HOLES & NOTCHES IN STUDWALL STUDS AND TOP & BOTTOM PLATES.
  - SEE 8/S6.1 FOR CONNECTION DETAILS OF INTERIOR NON-STRUCTURAL PARTITION WALLS.
  - SOLID WALLS AND SHEARWALLS SHOWN IN PLAN ARE ABOVE FRAMING (i.e. FROM MAIN FLOOR LEVEL TO UPPER FLOOR/ROOF LEVEL). DASHED WALLS SHOWN IN PLAN ARE LOAD-BEARING ELEMENTS BELOW FRAMING (i.e. FROM FOUNDATION/LOWER FLOOR TO MAIN FLOOR). JOISTS AND BEAMS SHOWN IN PLAN ARE AT MAIN FLOOR ELEVATION.
  - SEE GENERAL STRUCTURAL NOTE #20 ON S1.0 FOR ENGINEERED LUMBER REQUIREMENTS.
  - ALL HEADERS/BEAMS SHALL HAVE A MINIMUM OF (2)2x POSTS AND (1)FULL HEIGHT TRIMMER STUD, U.N.O. IN PLAN (STUD DEPTH SHALL MATCH DEPTH OF THE WALL)

1 MAIN FLOOR FRAMING PLAN  
S2.2 1/4" = 1'-0"



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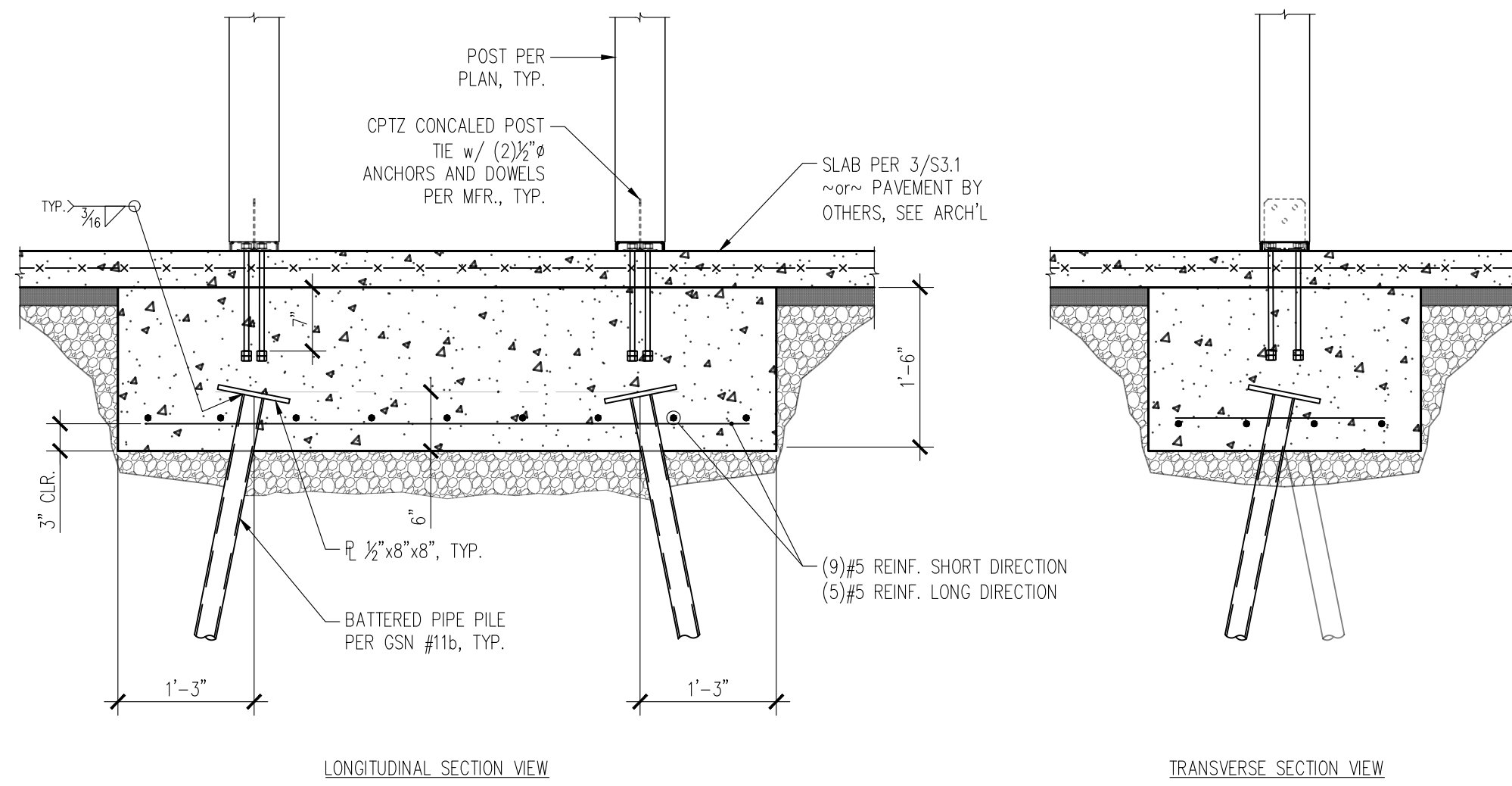
Derakshani Residence  
8151 SE 48th St  
Mercer Island, WA - 98040

CONTENTS  
Main Floor Framing Plan

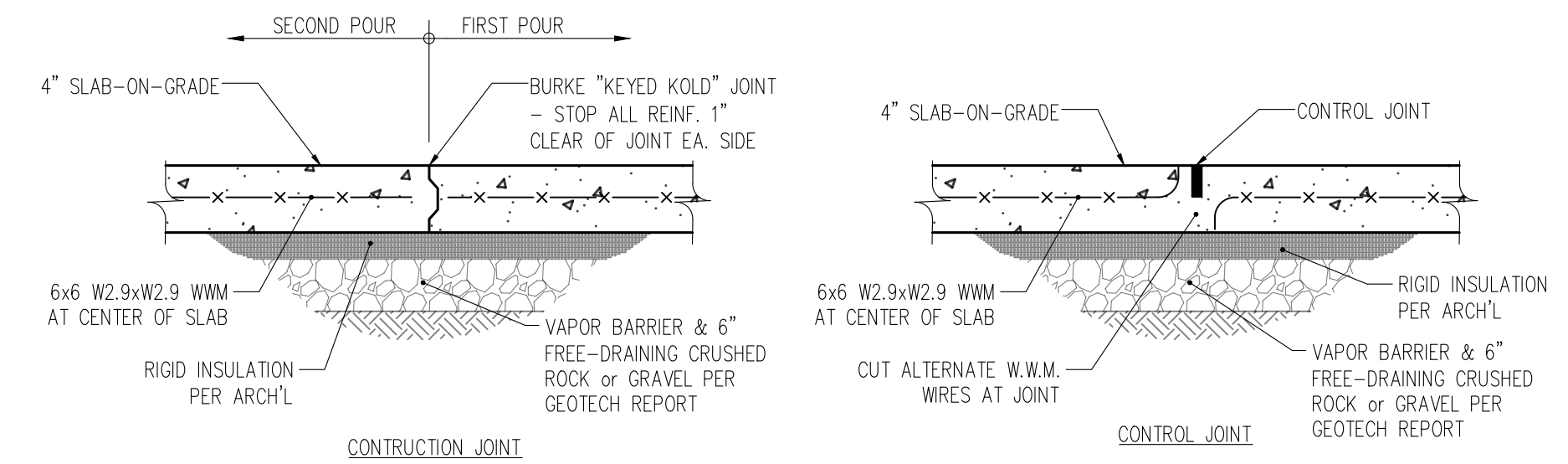
DRAWN BY  
JDA  
DATE  
04.01.21  
09.29.21  
12.03.21

S2.2





9 PIPE SUPPORTED PILE CAP  
S3.1 3/4" = 1'-0"

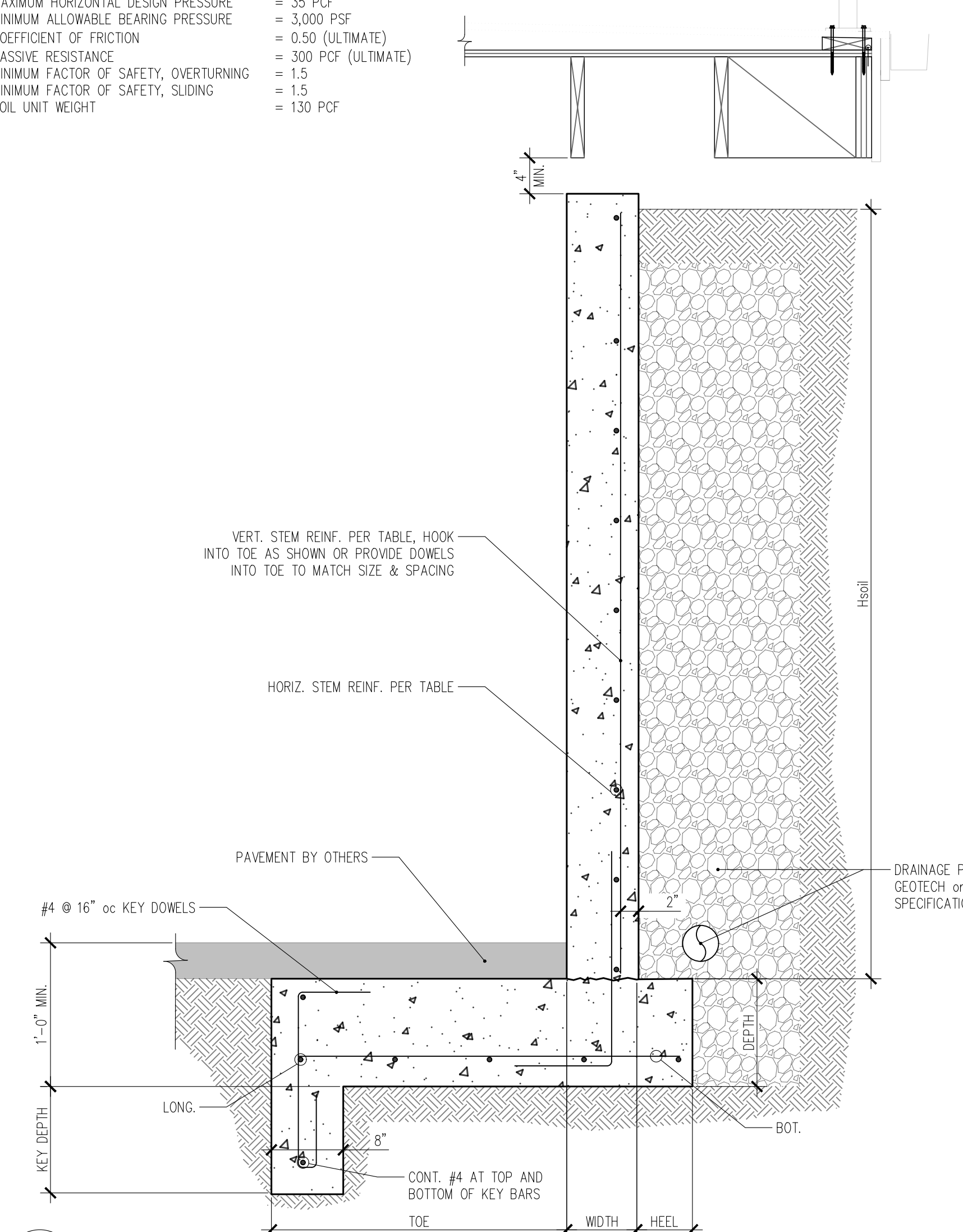


3 TYPICAL SLAB-ON-GRADE JOINTING  
S3.1 1" = 1'-0"

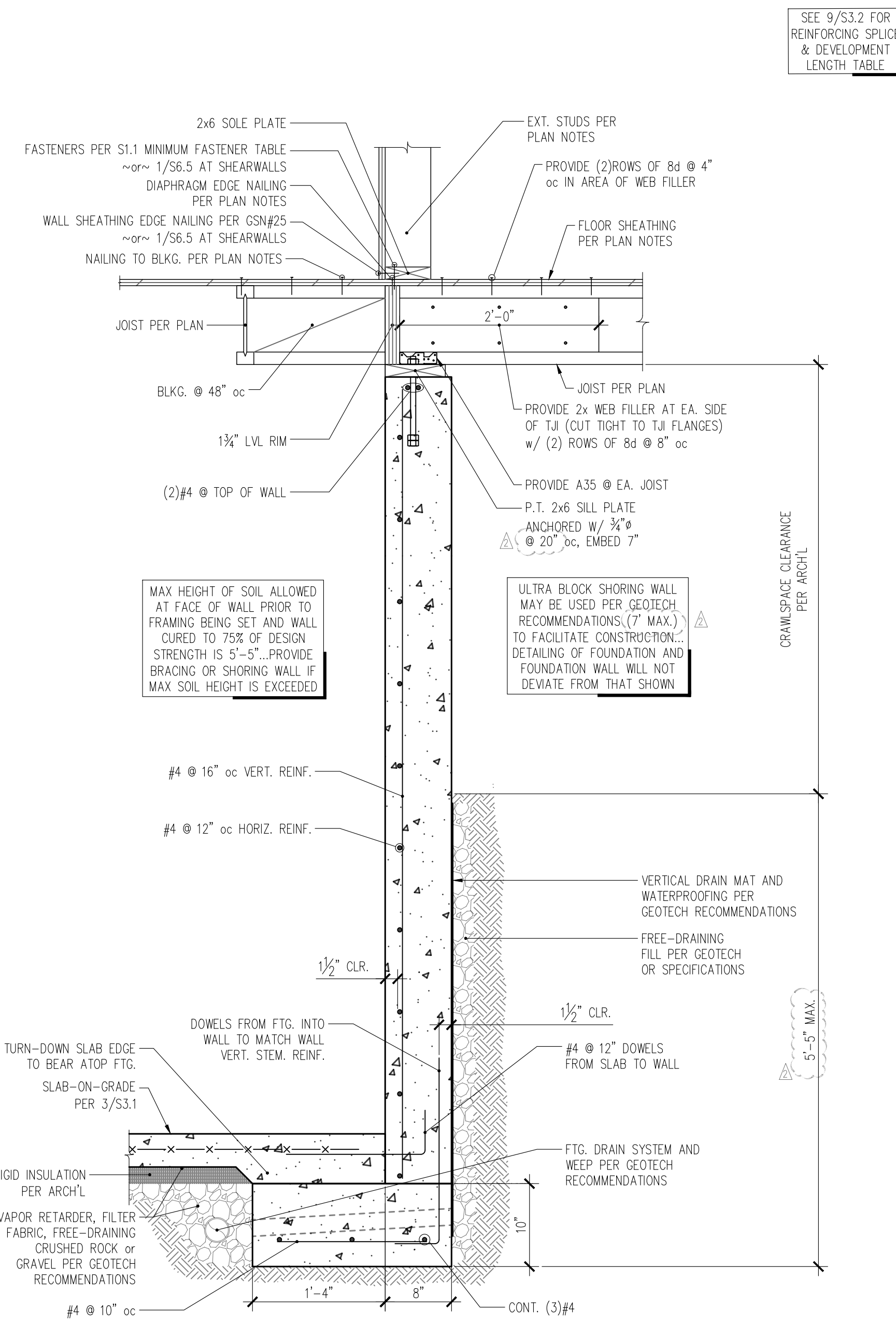
Hsoil	TOE	WIDTH	HEEL	DEPTH	STEM REINF.		FTG. REINF.		KEY DEPTH
					VERT.	HORIZ.	LONG.	BOT.	
$H_{soil} \leq 4'-6"$	2'-0"	8"	-	10"	#4 @ 16" oc	#4 @ 12" oc	(3) #4	#4 @ 12" oc	0'-6"
$4'-6" < H_{soil} \leq 5'-6"$	3'-3"	8"	-	10"	#4 @ 10" oc	#4 @ 12" oc	(5) #4	#4 @ 12" oc	1'-0"
$5'-6" < H_{soil} \leq 6'-6"$	4'-3"	8"	-	10"	#5 @ 10" oc	#4 @ 12" oc	(5) #5	#5 @ 16" oc	1'-6"
$6'-6" < H_{soil} \leq 7'-6"$	5'-6"	8"	-	10"	#5 @ 8" oc	#4 @ 12" oc	(5) #5	#5 @ 10" oc	1'-6"
$H_{soil} \leq 8'-6"$	6'-6"	8"	-	12"	#5 @ 6" oc	#4 @ 12" oc	(5) #5	#5 @ 8" oc	2'-0"

NOTE:  
 MAXIMUM HORIZONTAL DESIGN PRESSURE = 35 PCF  
 MINIMUM ALLOWABLE BEARING PRESSURE = 3,000 PSF  
 COEFFICIENT OF FRICTION = 0.50 (ULTIMATE)  
 PASSIVE RESISTANCE = 300 PCF (ULTIMATE)  
 MINIMUM FACTOR OF SAFETY, OVERTURNING = 1.5  
 MINIMUM FACTOR OF SAFETY, SLIDING = 1.5  
 SOIL UNIT WEIGHT = 130 PCF

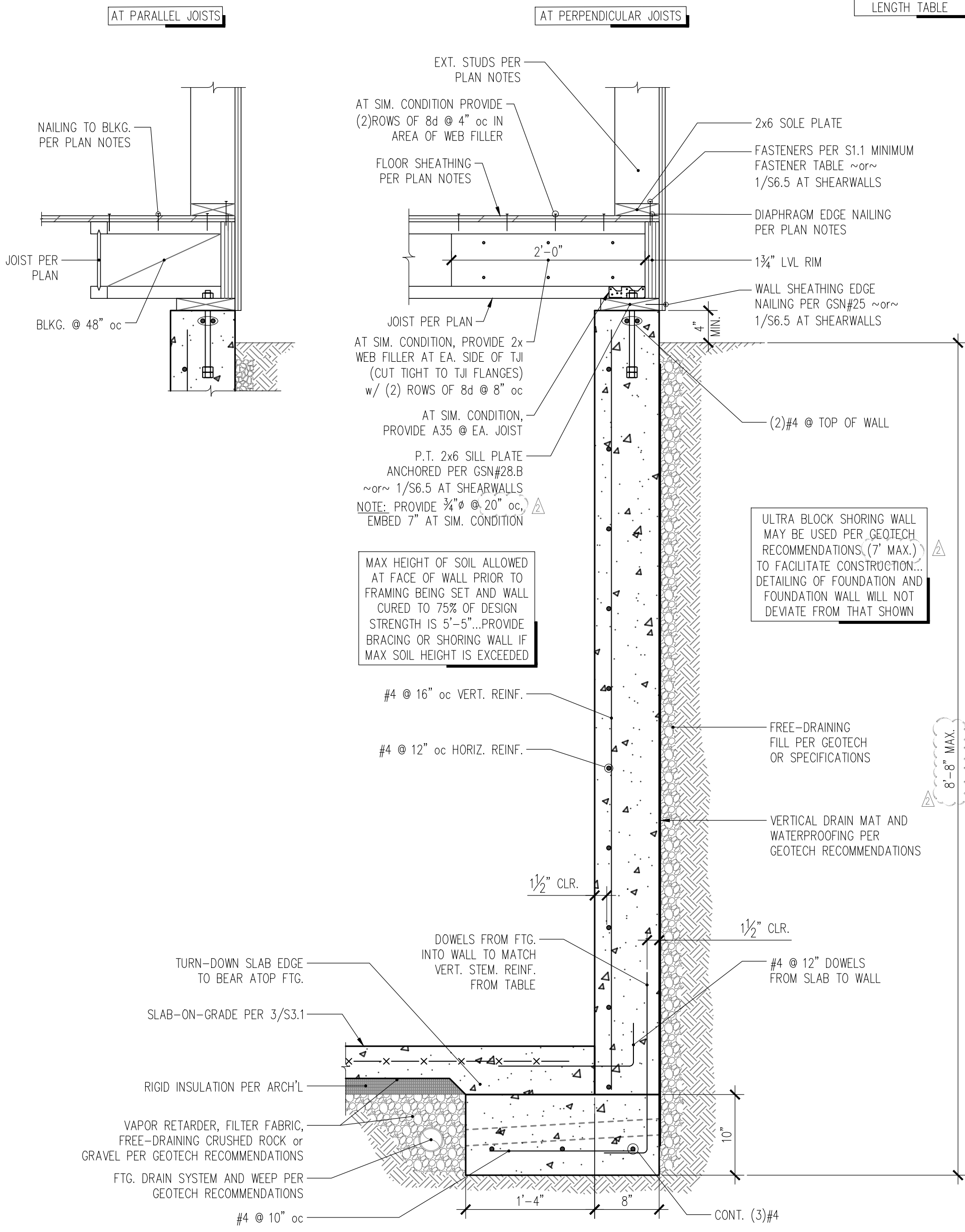
SEE 9/S3.2 FOR REINFORCING SPLICE & DEVELOPMENT LENGTH TABLE  
 SEE DETAIL 4/S3.3 FOR DECK FRAMING AND RAILING DETAIL



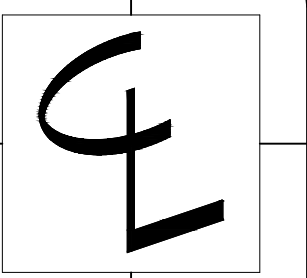
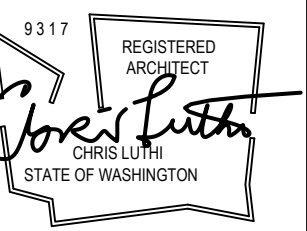
7 RETAINING WALL  
S3.1 1" = 1'-0"



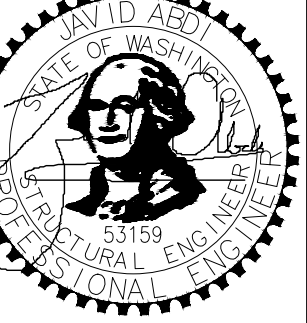
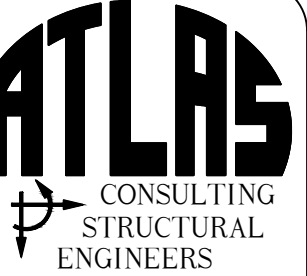
4 SECTION THROUGH FOUNDATION WALL AT BASEMENT SLAB, CRAWLSPACE AND MAIN FLOOR JOISTS  
S3.1 1" = 1'-0"



1 SECTION THROUGH FOUNDATION WALL AT BASEMENT SLAB AND MAIN FLOOR JOISTS  
S3.1 1" = 1'-0"



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CONTENTS

Foundation Details

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JDA

DATE

04.01.21

09.29.21

12.03.21

S3.1

MIN. STRAIGHT DEVELOPMENT LENGTH			MIN. LAP SPLICE LENGTH (CLASS B)		
BAR SIZE	TOP BARS	OTHER BARS	BAR SIZE	TOP BARS	OTHER BARS
#4	25"	19"	#4	33"	25"
#5	31"	24"	#5	41"	31"

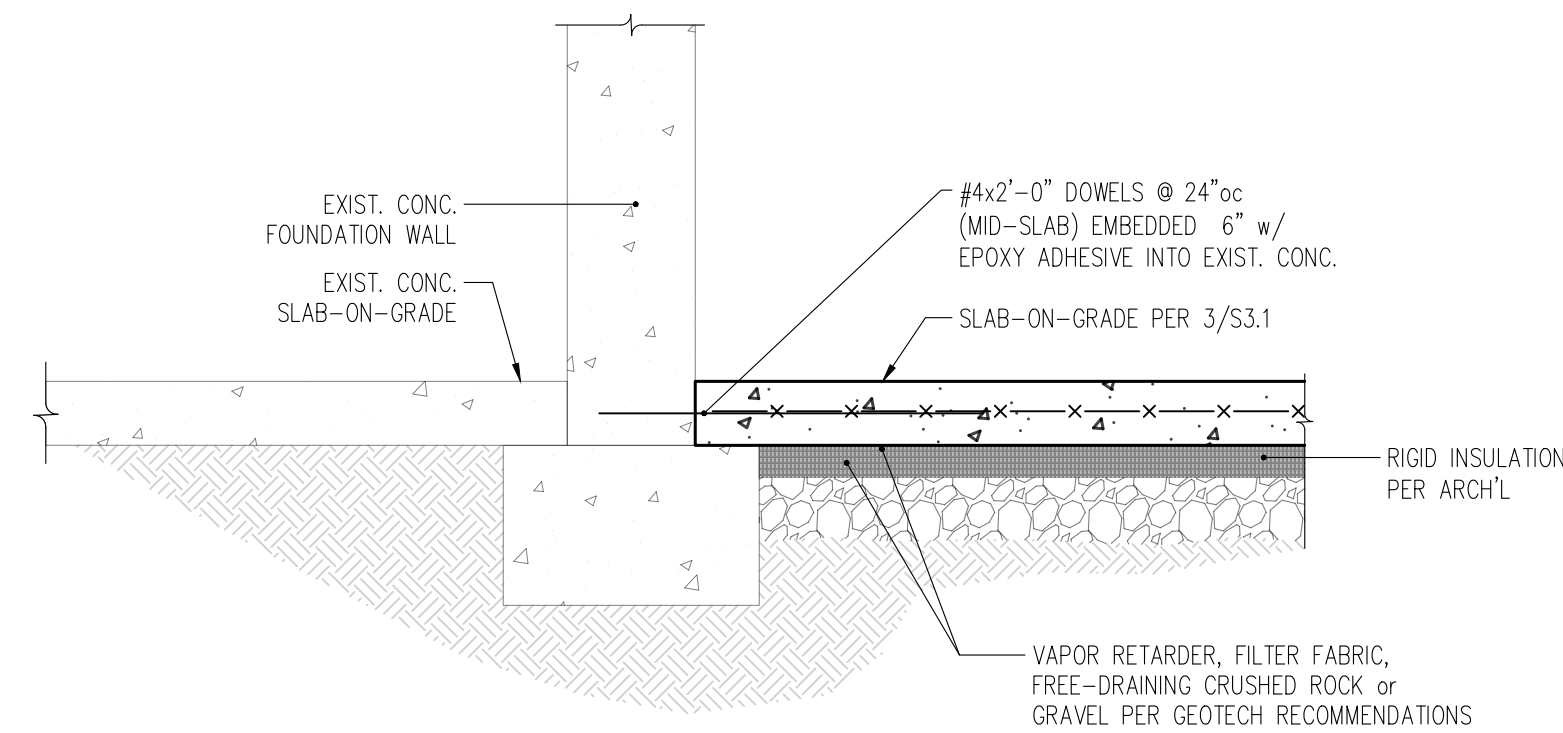
\*TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12" DEPTH OF CONCRETE CAST BELOW THEM

IF CLEAR CONCRETE COVER IS LESS THAN 1x THE DIAMETER OF THE BAR OR THE CENTER-TO-CENTER SPACING IS LESS THAN (3) BAR DIAMETERS, THEN VALUES SHALL BE INCREASED BY 50%

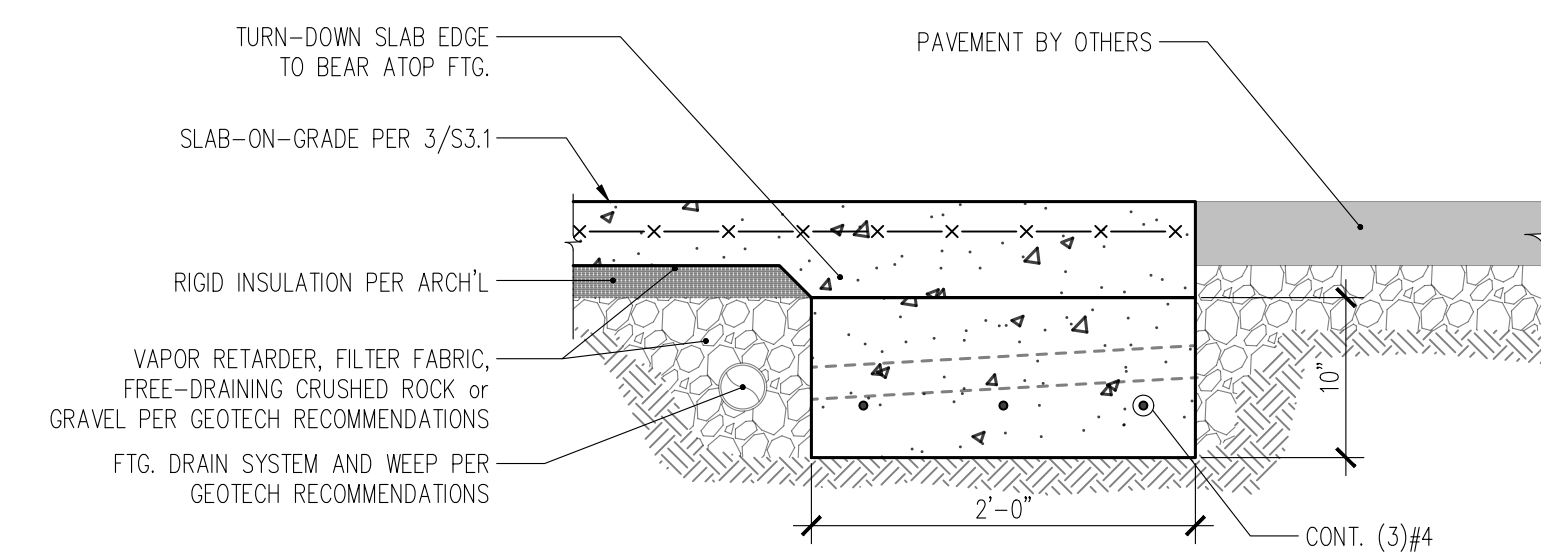
MIN. EMBEDMENT LENGTH FOR STANDARD END HOOKS	
BAR SIZE	LENGTH
#4	7"
#5	9"

- SIDE COVER MUST BE EQUAL TO OR GREATER THAN 2"
- END COVER FOR 90° HOOKS MUST BE EQUAL TO OR GREATER THAN 2"

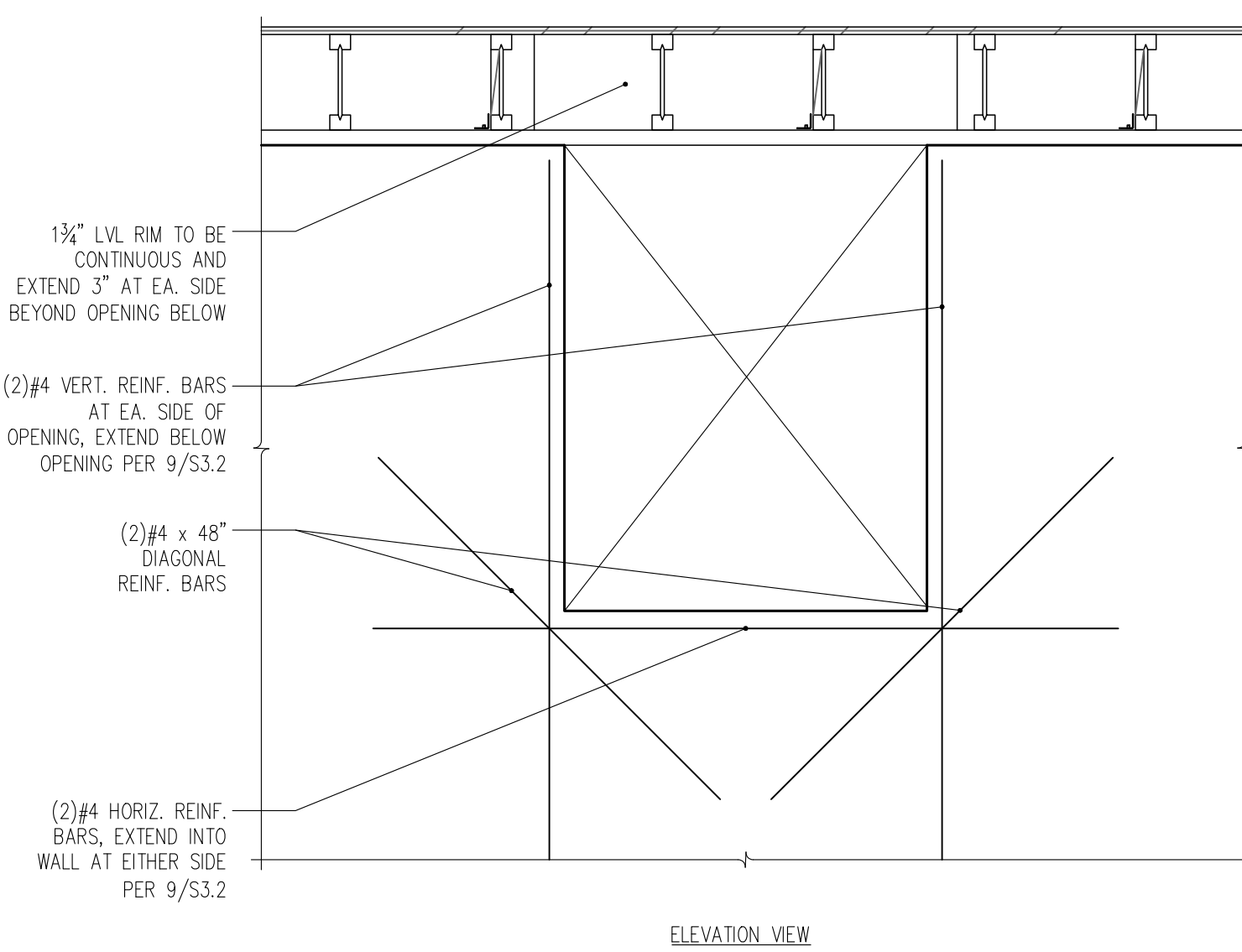
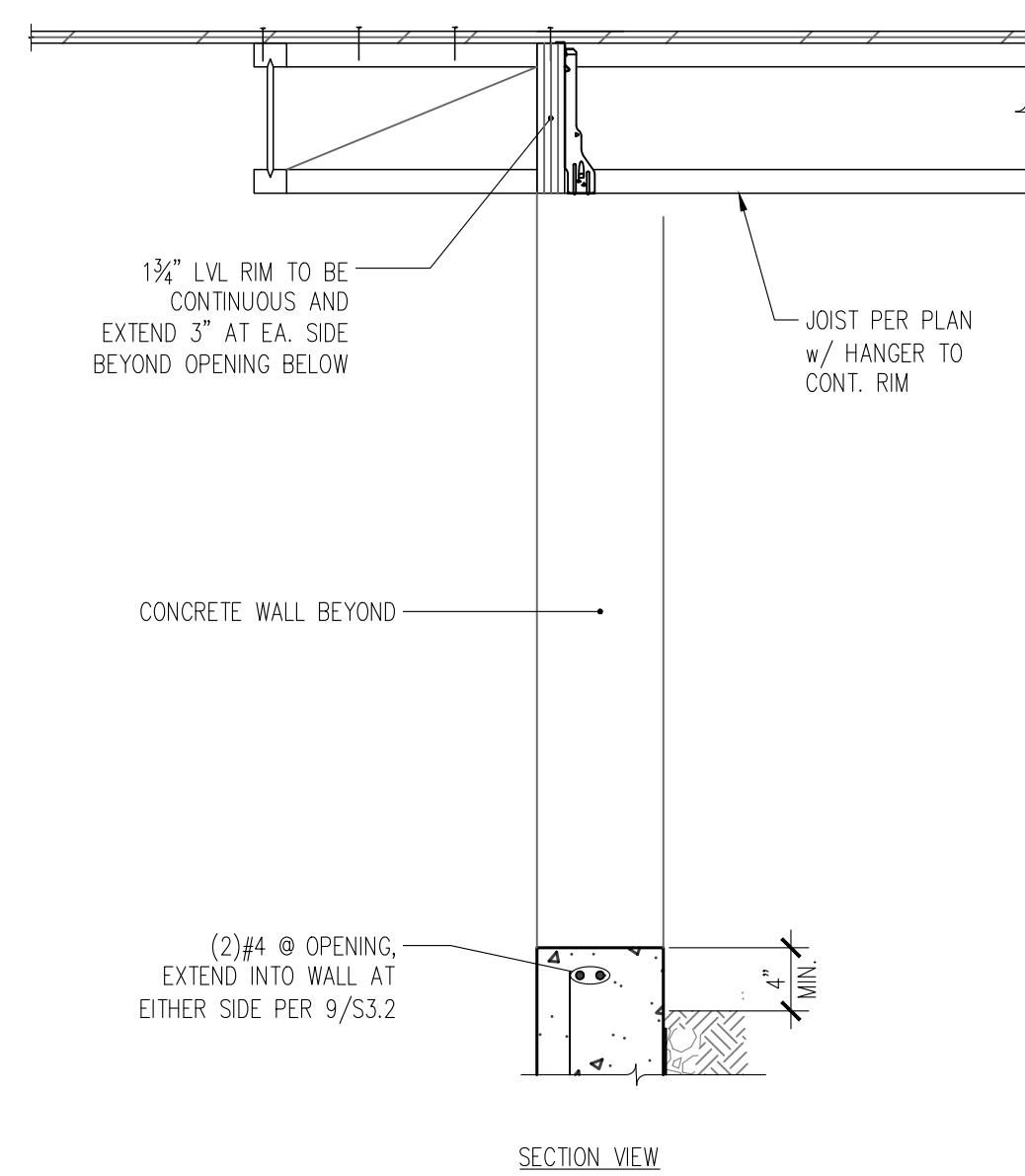
9 CONCRETE REINFORCING DEVELOPMENT AND SPLICE LENGTH TABLES  
S3.2 N/A



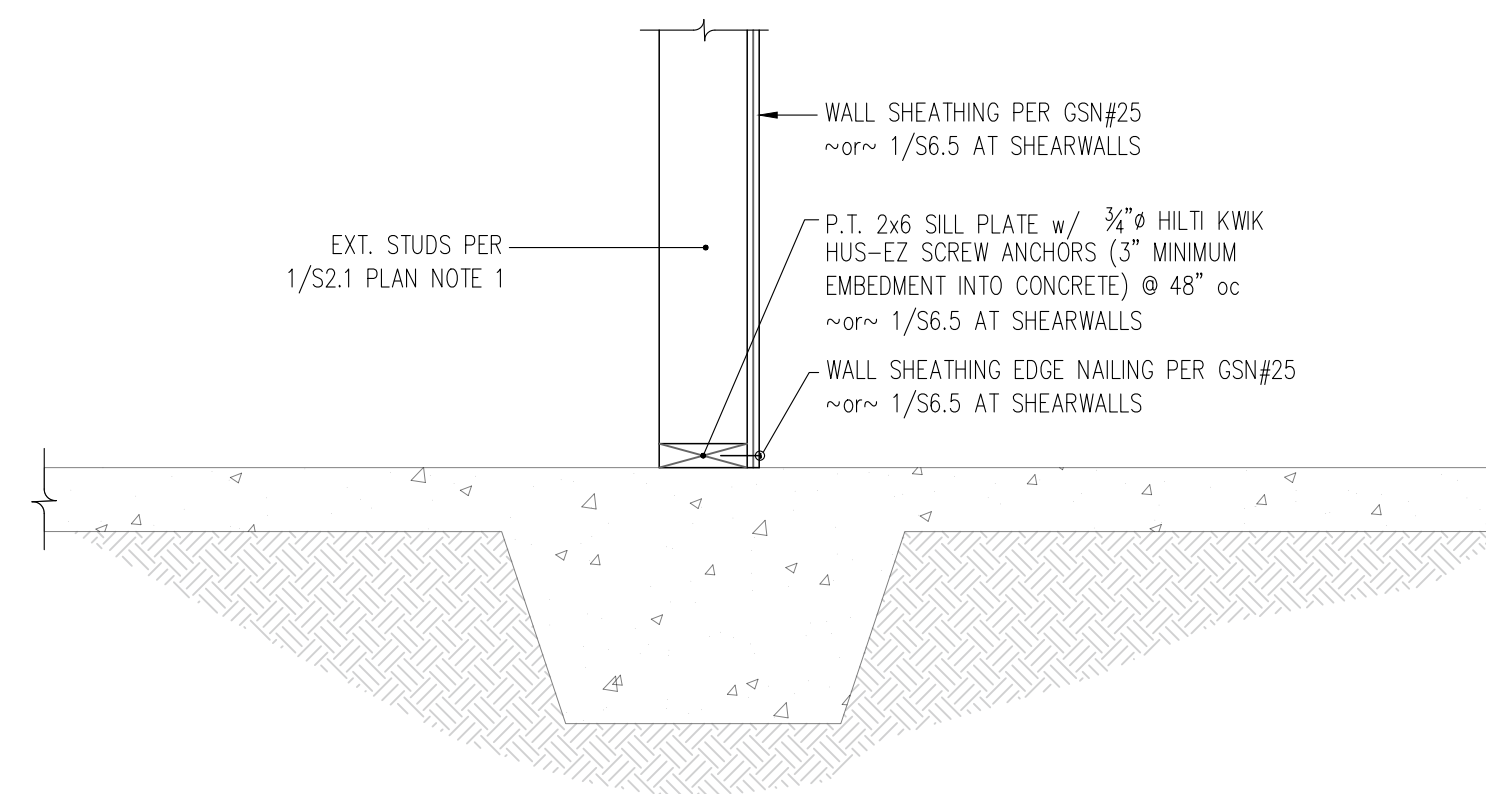
6 SECTION AT NEW SLAB AND EXISTING FOUNDATION WALL  
S3.2 1" = 1'-0"



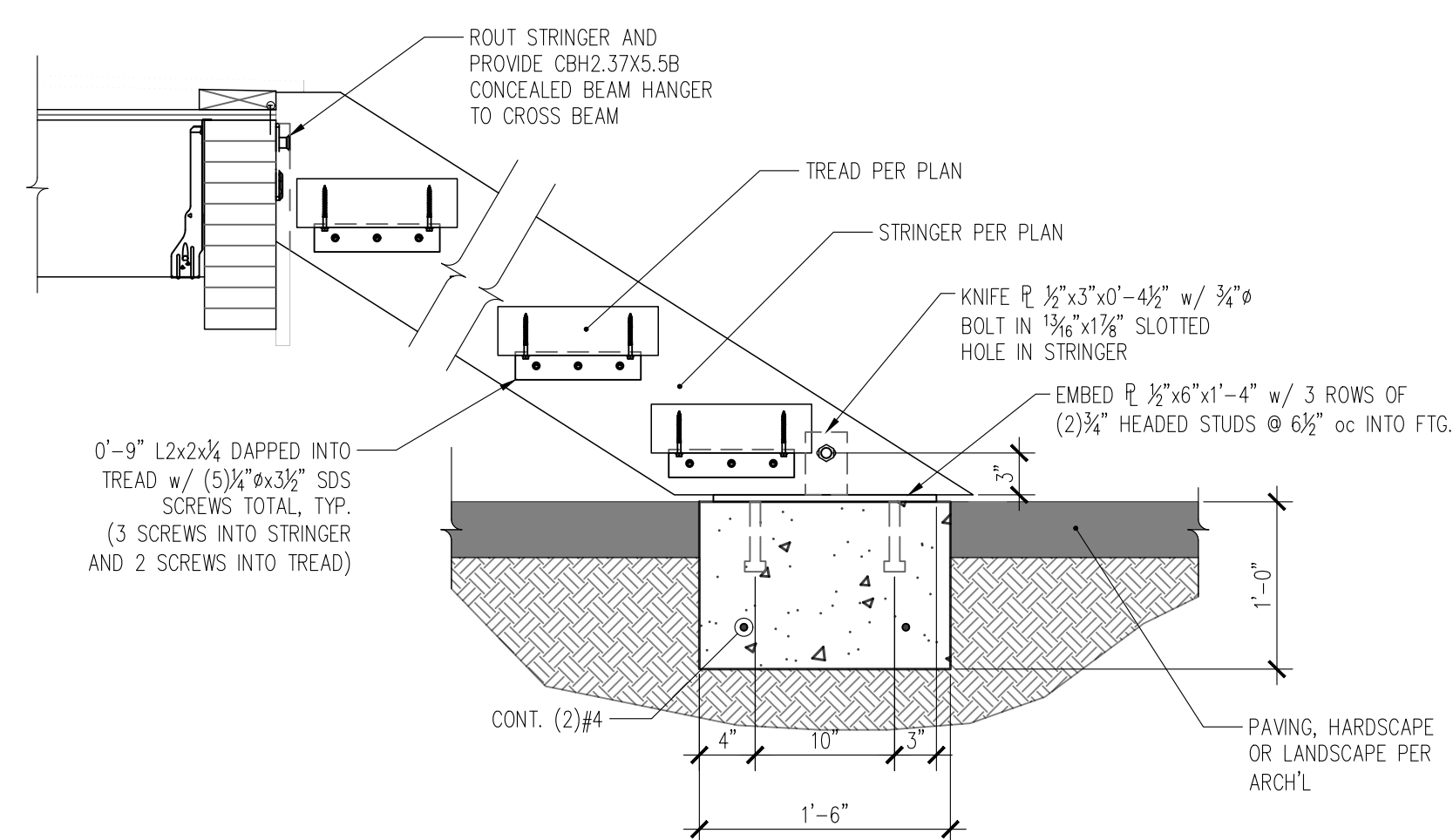
3 SECTION AT GARAGE ENTRANCE  
S3.2 1" = 1'-0"



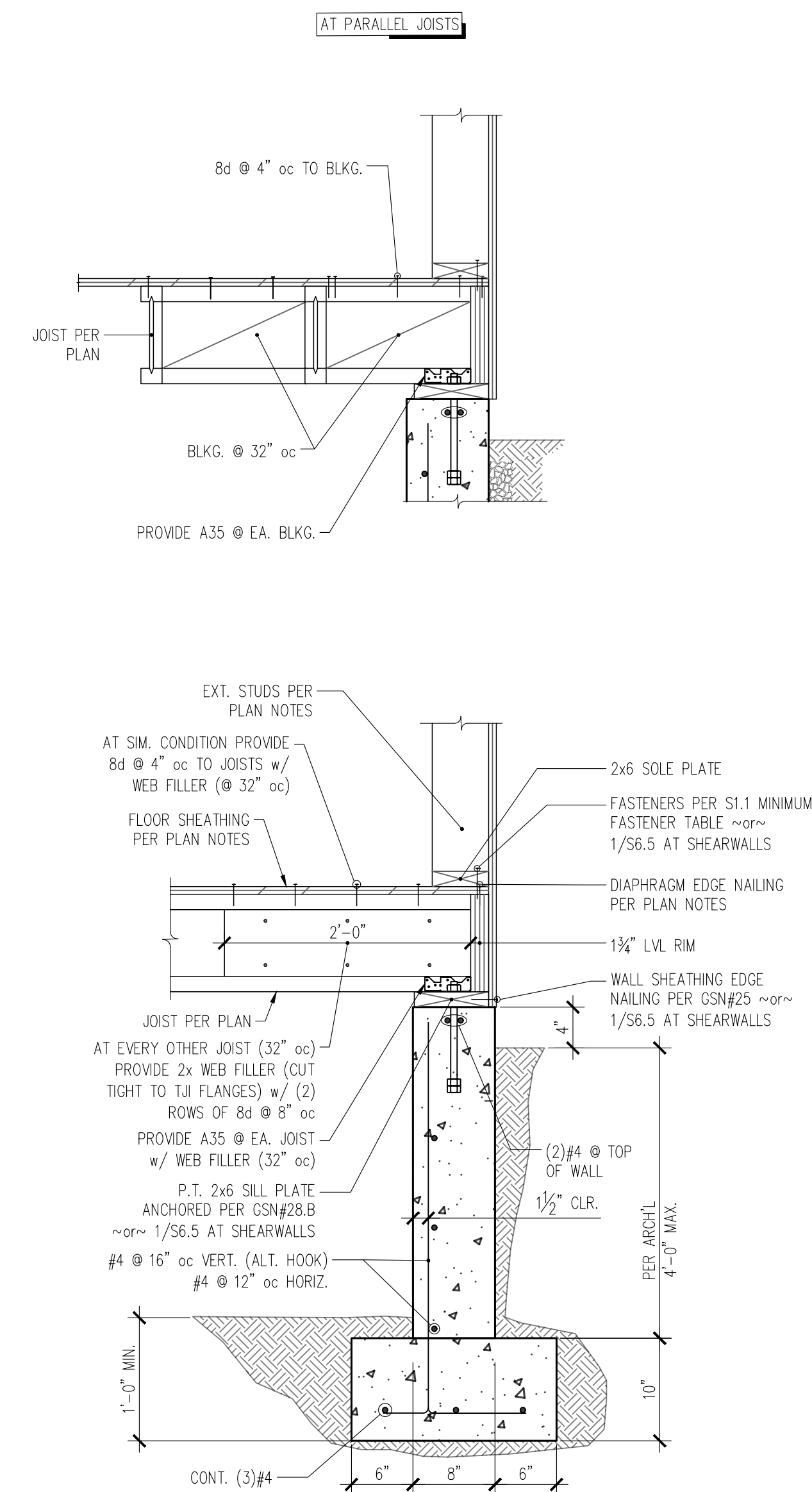
7 PARTIAL HEIGHT OPENING IN NEW FOUNDATION WALL  
S3.2 N.T.S.



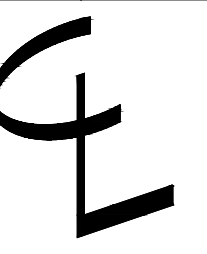
5 NEW SHEAR/BEARING STUD WALL AT EXISTING THICKENED SLAB  
S3.2 1" = 1'-0"



4 SECTION THROUGH FOUNDATION AT STAIR STRINGER  
S3.2 N.T.S.

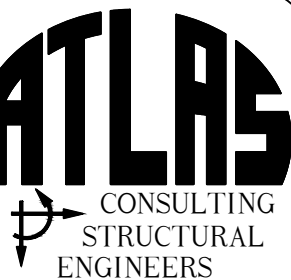


1 SECTION THROUGH FOUNDATION WALL AT CRAWLSPACE  
S3.2 1" = 1'-0"



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SEATTLE  
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Derakshani Residence  
8151 SE 48th St  
Mercer Island, WA - 98040

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Foundation Details

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DATE

04.01.21

09.29.21

S3.2

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Wood Typical Details

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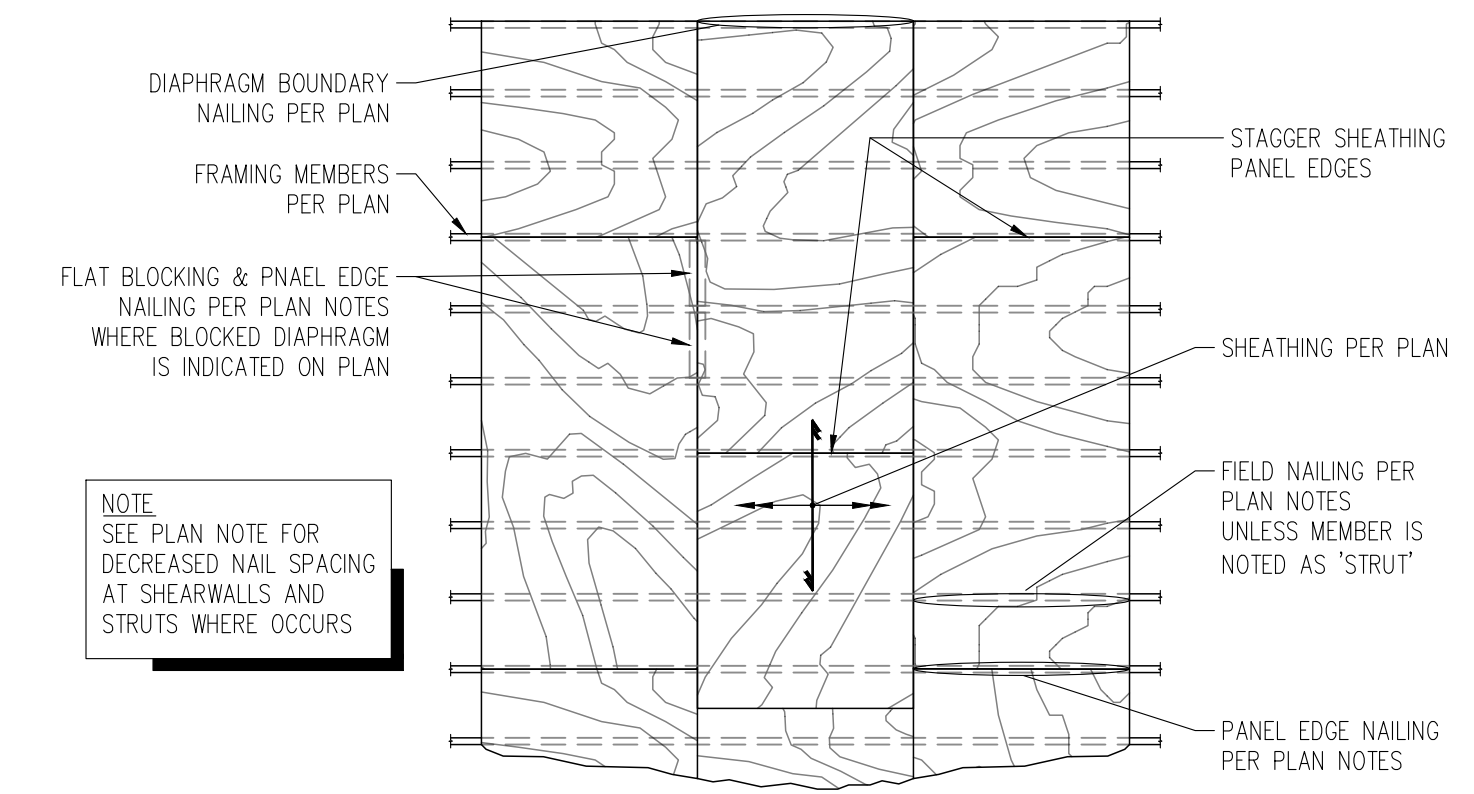
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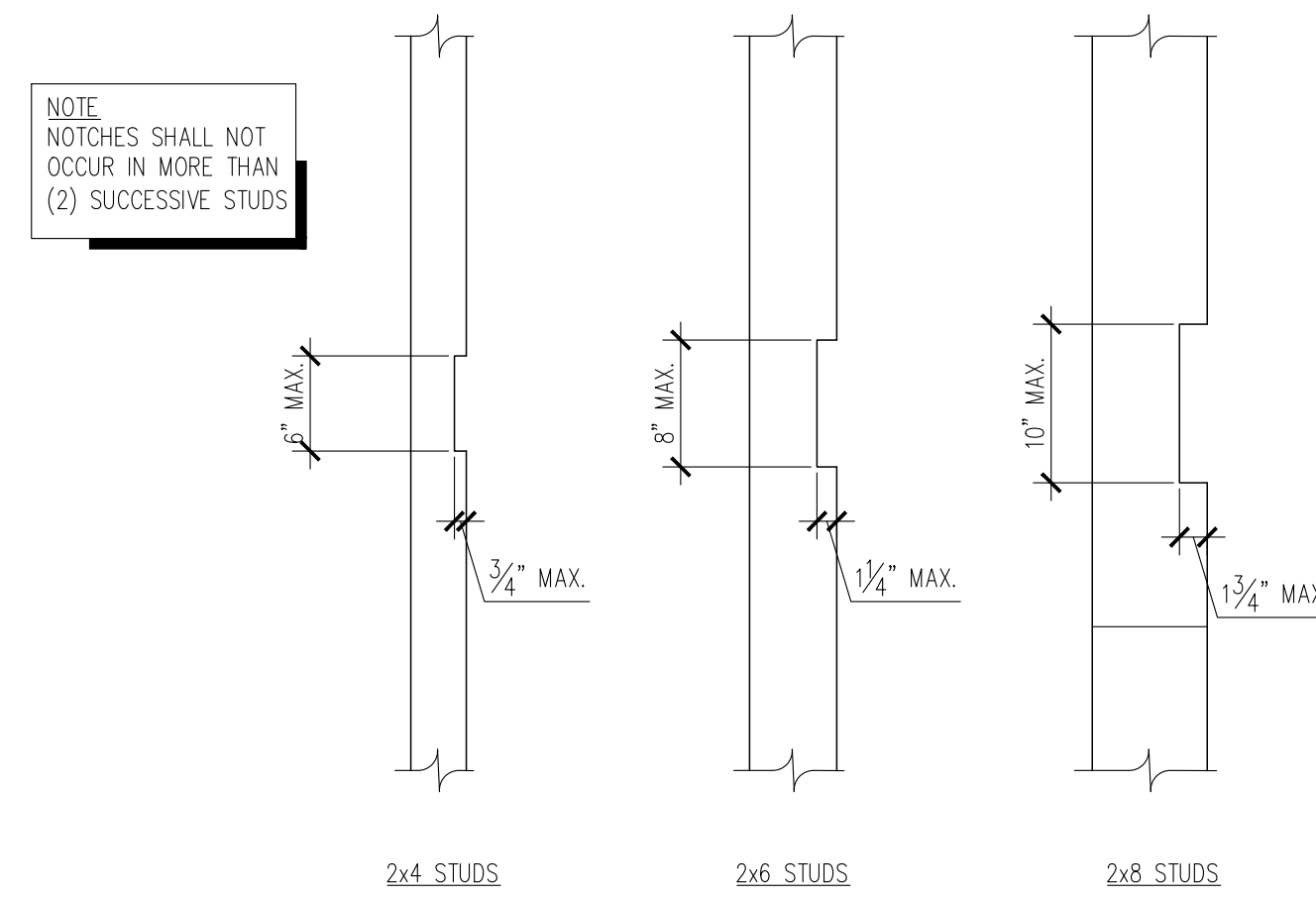
04.01.21

09.29.21

S6.1



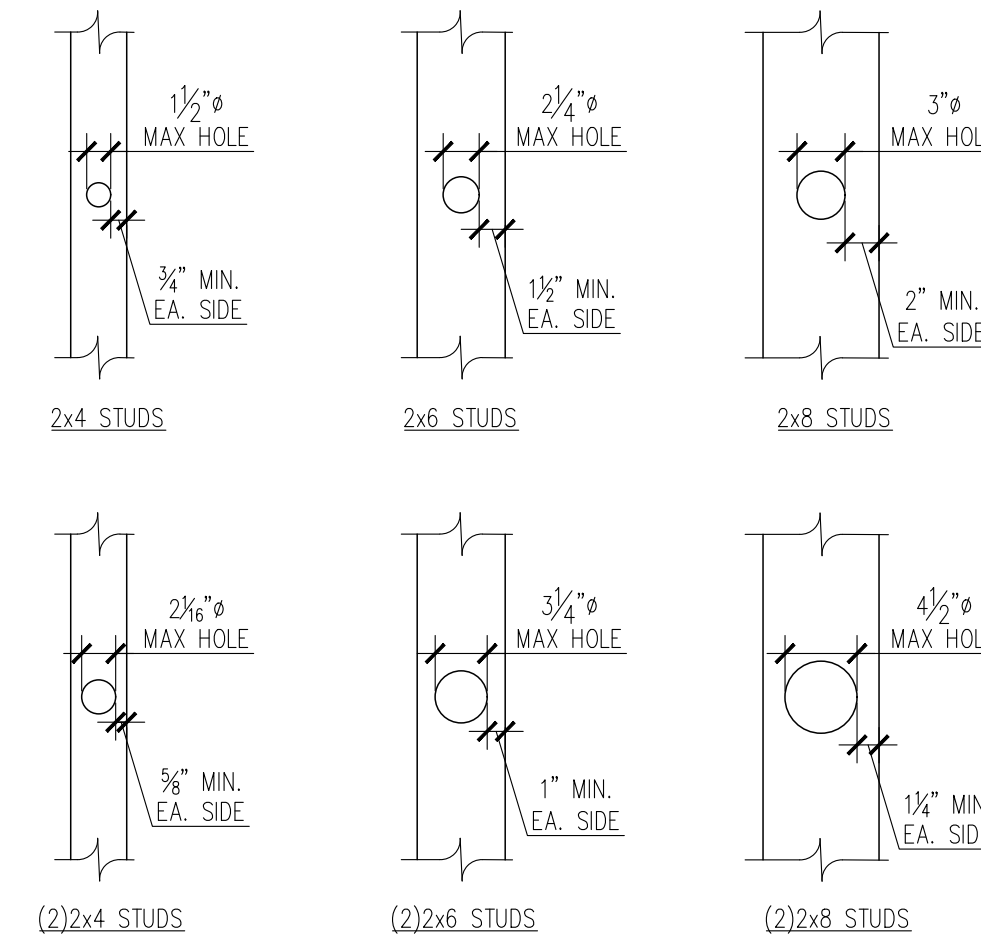
3 TYPICAL DIAPHRAGM NAILING  
S6.1 NTS



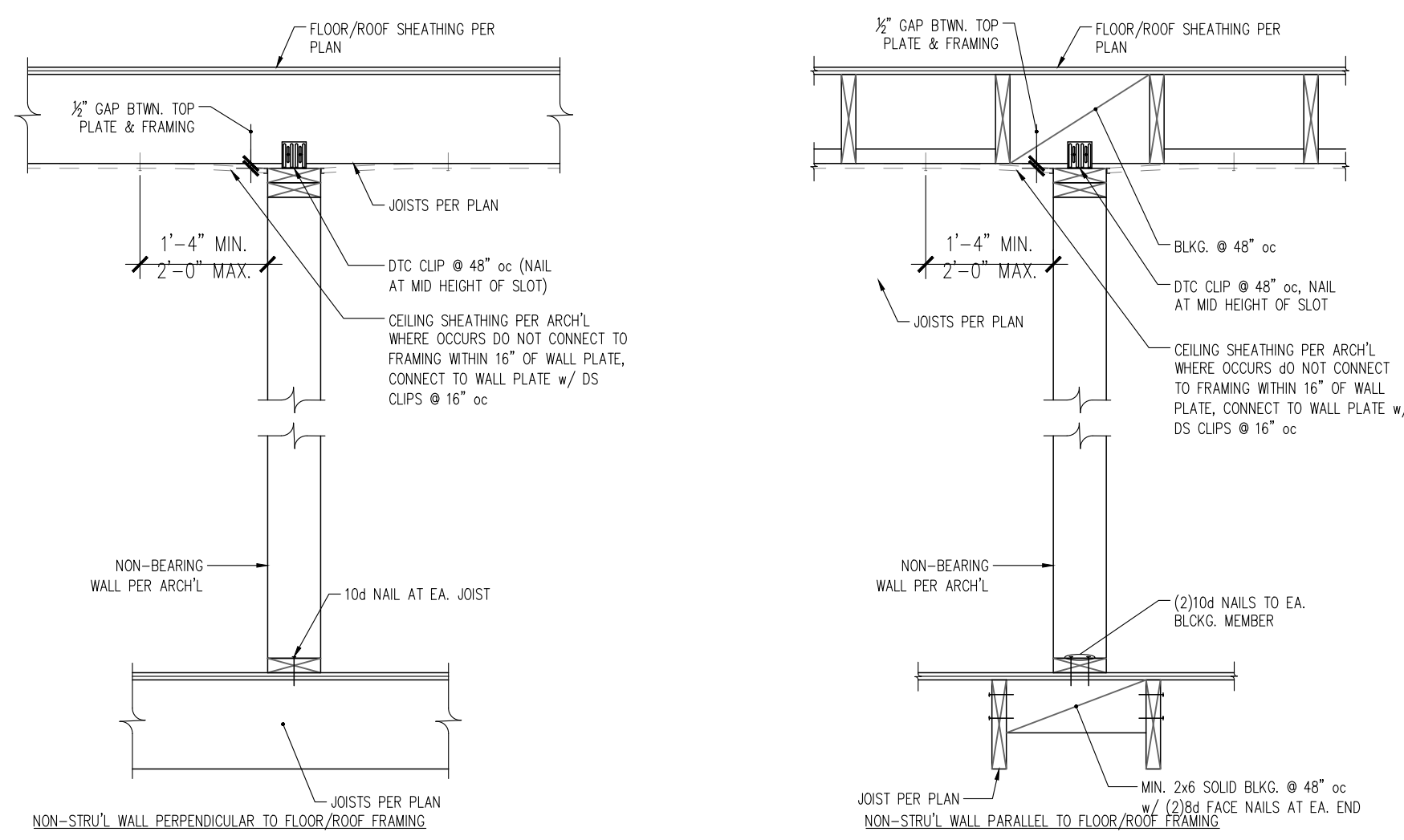
6 ALLOWABLE HOLES IN STUDWALL STUDS  
S6.1 NTS

	NO REINF. REQUIRED	STRAP REINF. REQUIRED
2x4 PLATES	1 1/2" MAX. HOLE 3/8" MIN. EA. SIDE	2 5/8" MAX. HOLE CMSTC16x3'-0" (CS16x2'-0" AT BOT. PLATES) 3/8" MIN. EA. SIDE
2x6 PLATES	2 1/4" MAX. HOLE 1 1/2" MIN. EA. SIDE	3 3/4" MAX. HOLE CMSTC16x3'-0" (CS16x2'-0" AT BOT. PLATES) 3/4" MIN. EA. SIDE
2x8 PLATES	3 1/4" MAX. HOLE 2" MIN. EA. SIDE	5" MAX. HOLE CMSTC16x3'-0" (CS16x2'-0" AT BOT. PLATES) 1 1/2" MIN. EA. SIDE

2 ALLOWABLE HOLES THROUGH TOP PLATES  
S6.1 NTS



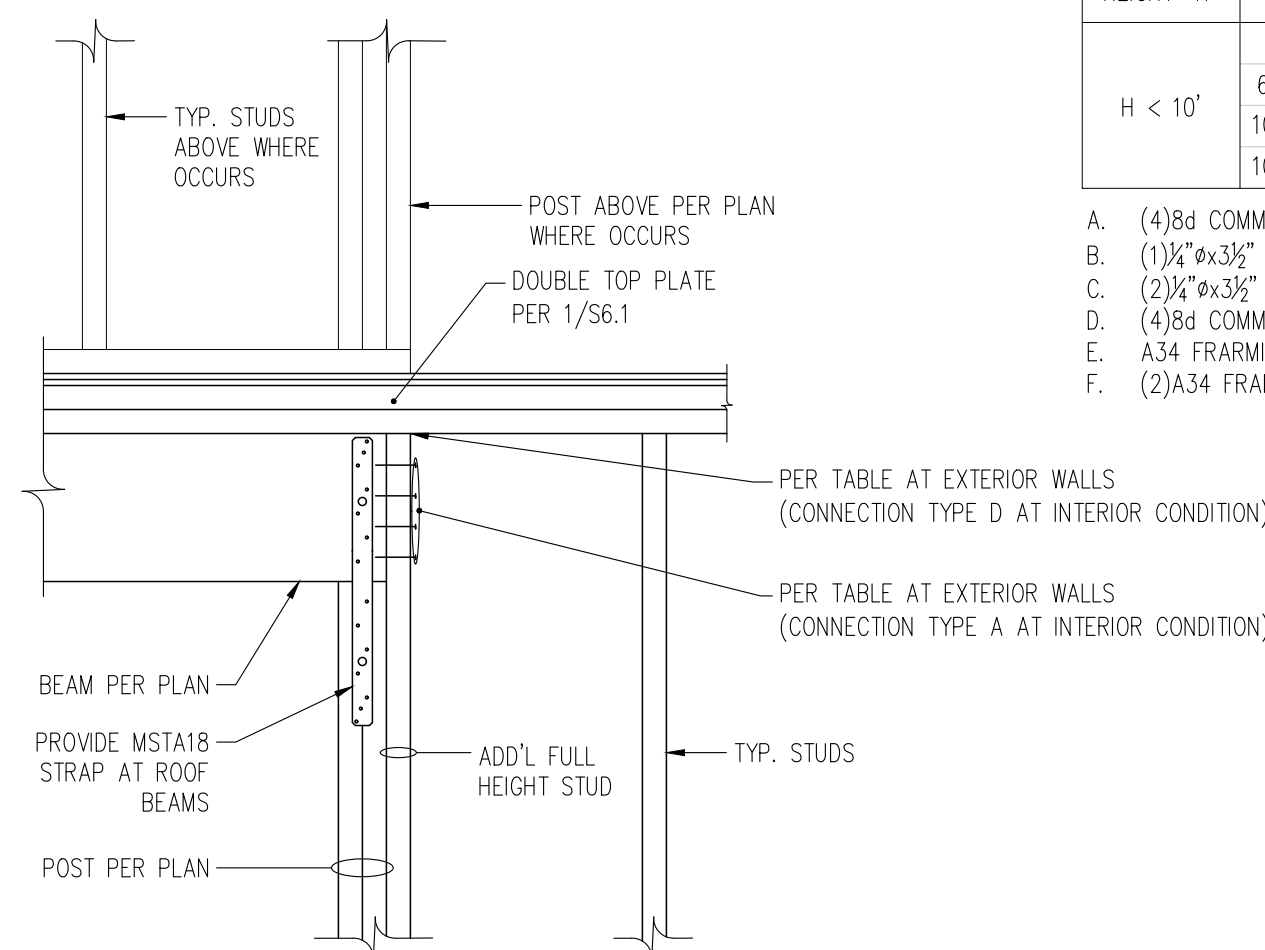
5 ALLOWABLE HOLES IN STUDWALL STUDS  
S6.1 NTS



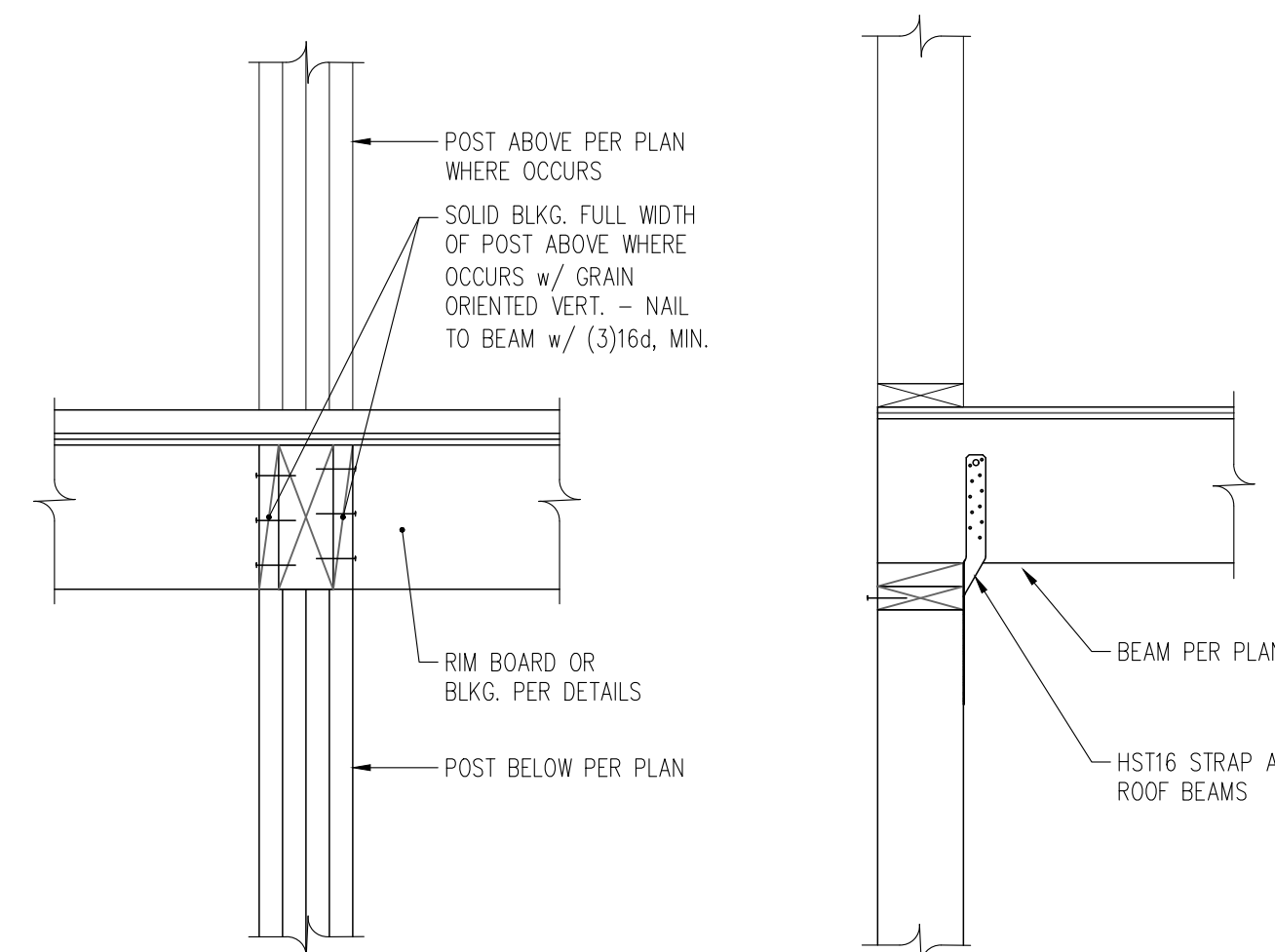
8 CONNECTION OF NON-STRUC'L PARTITION WALL TO STRUCTURE  
S6.1 NTS

TYPICAL EXTERIOR WALL OPENING FRAMING SCHEDULE			
CLEAR HEIGHT "H"	OPENING WIDTH "L"	FULL HEIGHT STUD TO BEAM CONNECTION	FULL HEIGHT STUD TO TOP PLATE CONNECTION
H < 10'	L ≤ 6'-0"	A	D
	6' < L < 10'	A + B	E
	10' ≤ L ≤ 15'	A + B	D + E
	10' ≤ L ≤ 21'	A + C	D + F

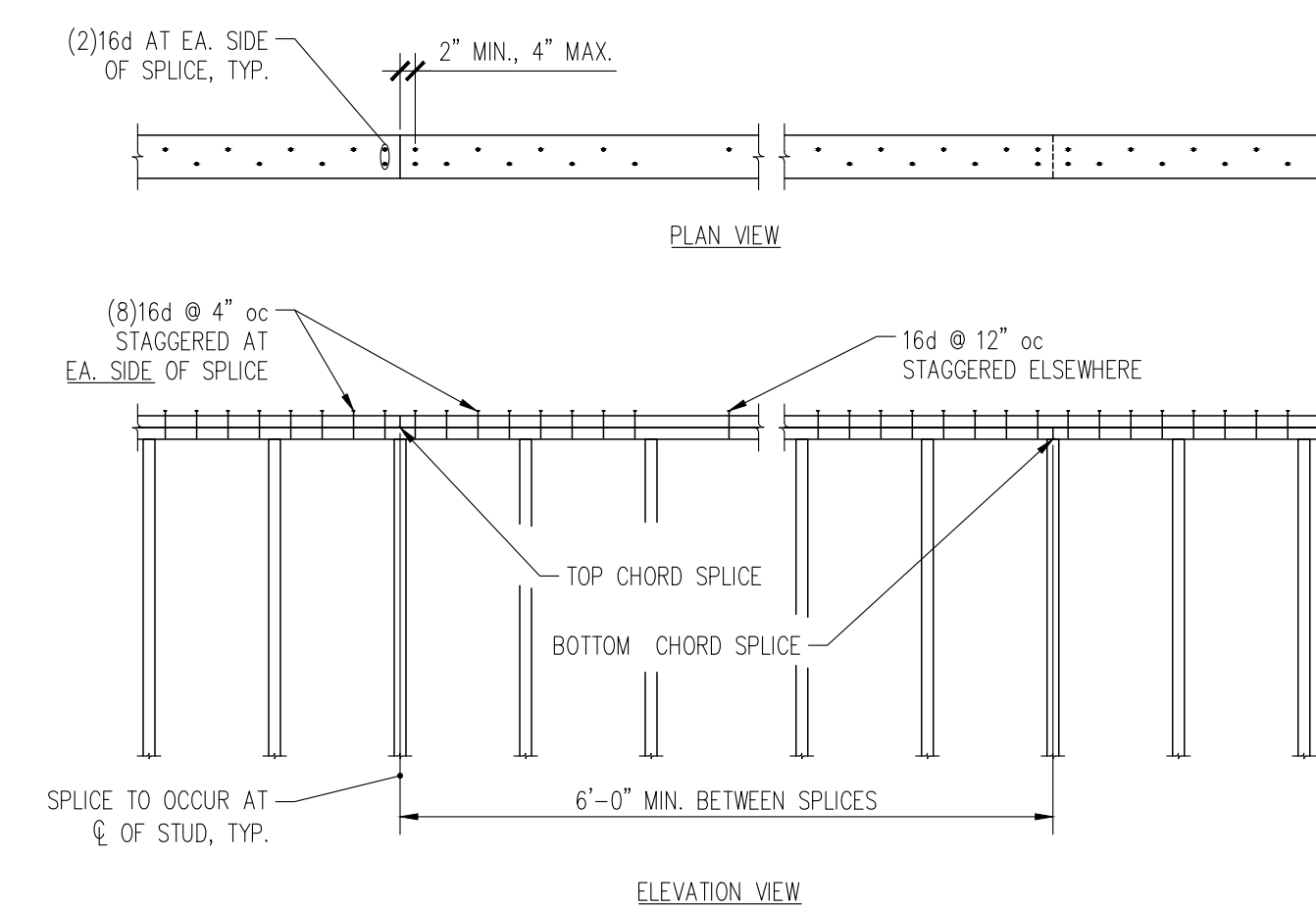
- A. (4)8d COMMON or (4)10d BOX
- B. (1)1/2" x 3 1/2" SDS SCREW
- C. (2)1/2" x 3 1/2" SDS SCREWS
- D. (4)8d COMMON or (4)10d BOX (TOENAILED)
- E. A34 FRAMING ANGLES
- F. (2)A34 FRAMING ANGLES



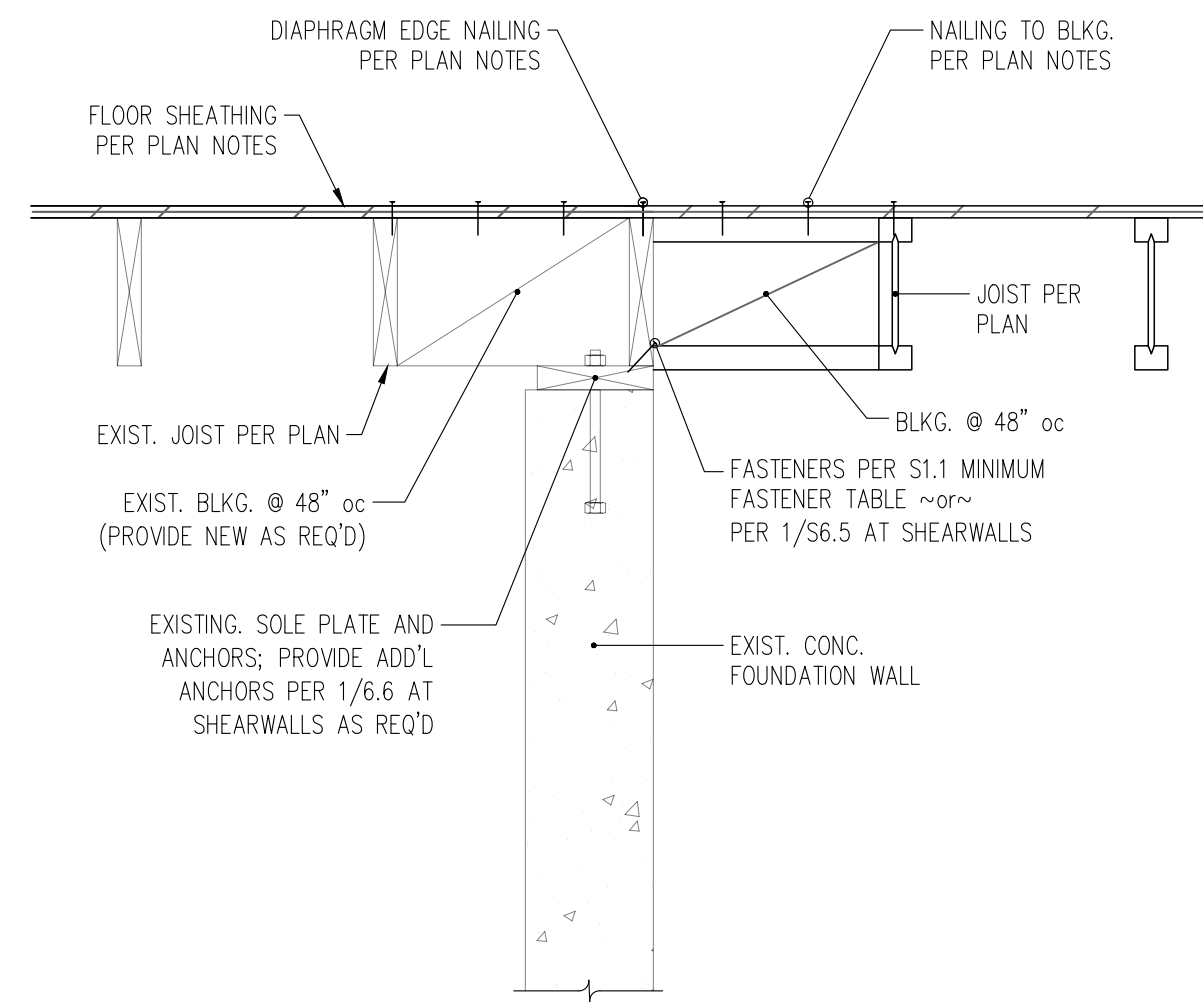
7 TYPICAL BEAM PARALLEL TO WALL  
S6.1 NTS



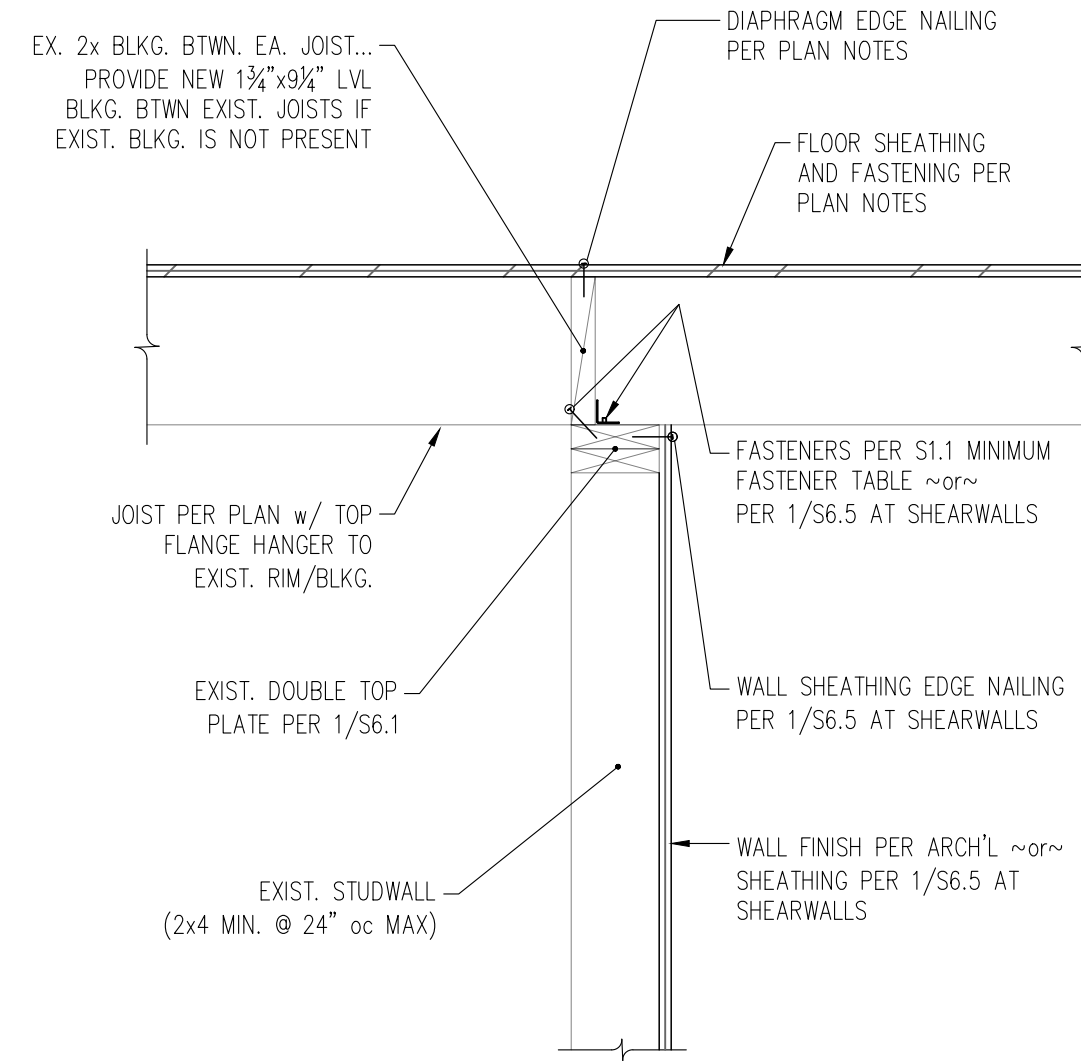
4 TYPICAL BEAM PERPENDICULAR TO WALL  
S6.1 NTS



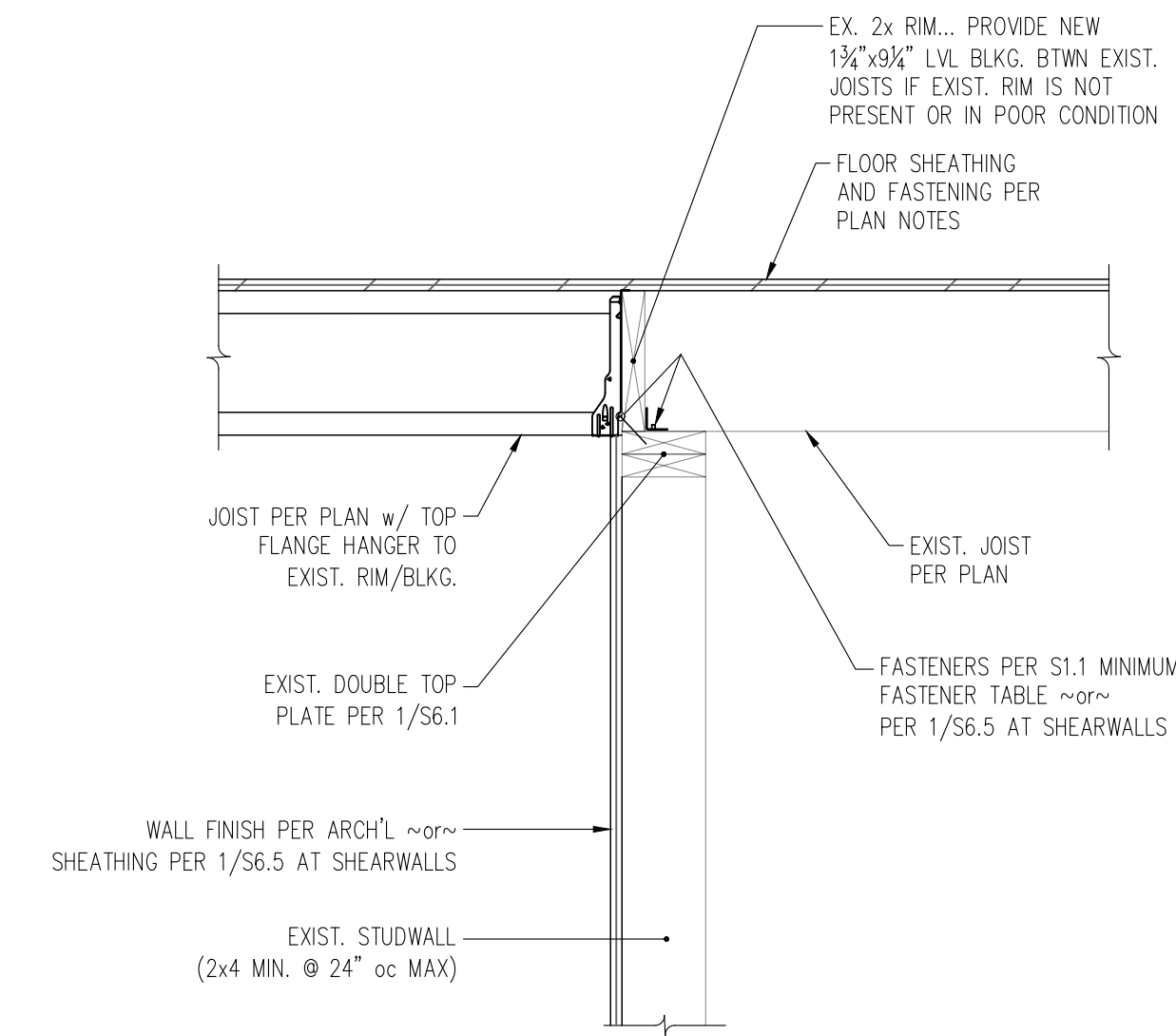
1 TOP PLATE SPLICE  
S6.1 NTS



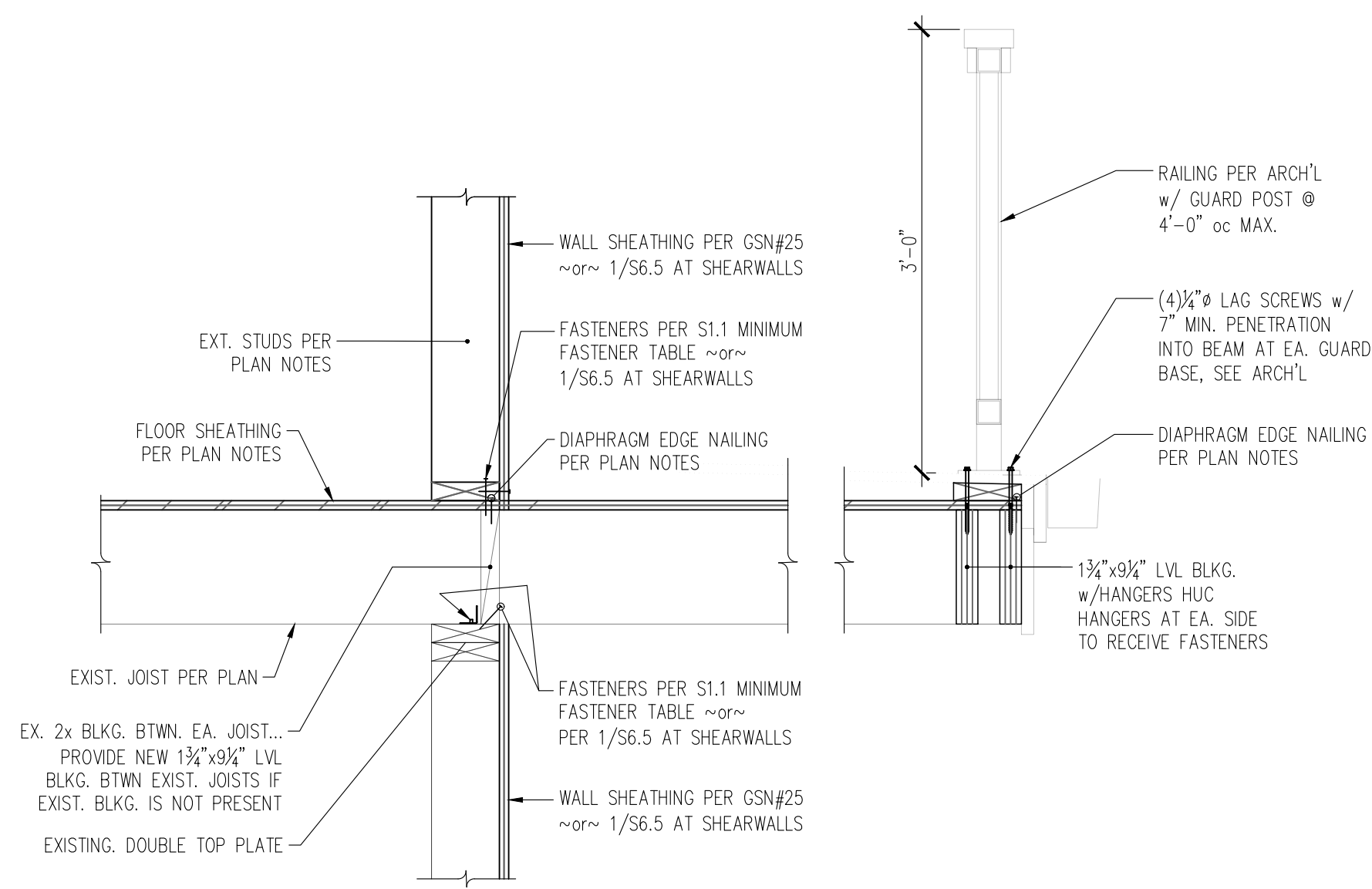
9 SECTION THROUGH INTERIOR SHEAR WALL w/ PERPENDICULAR JOISTS AT EA. SIDE  
S6.2 1" = 1'-0"



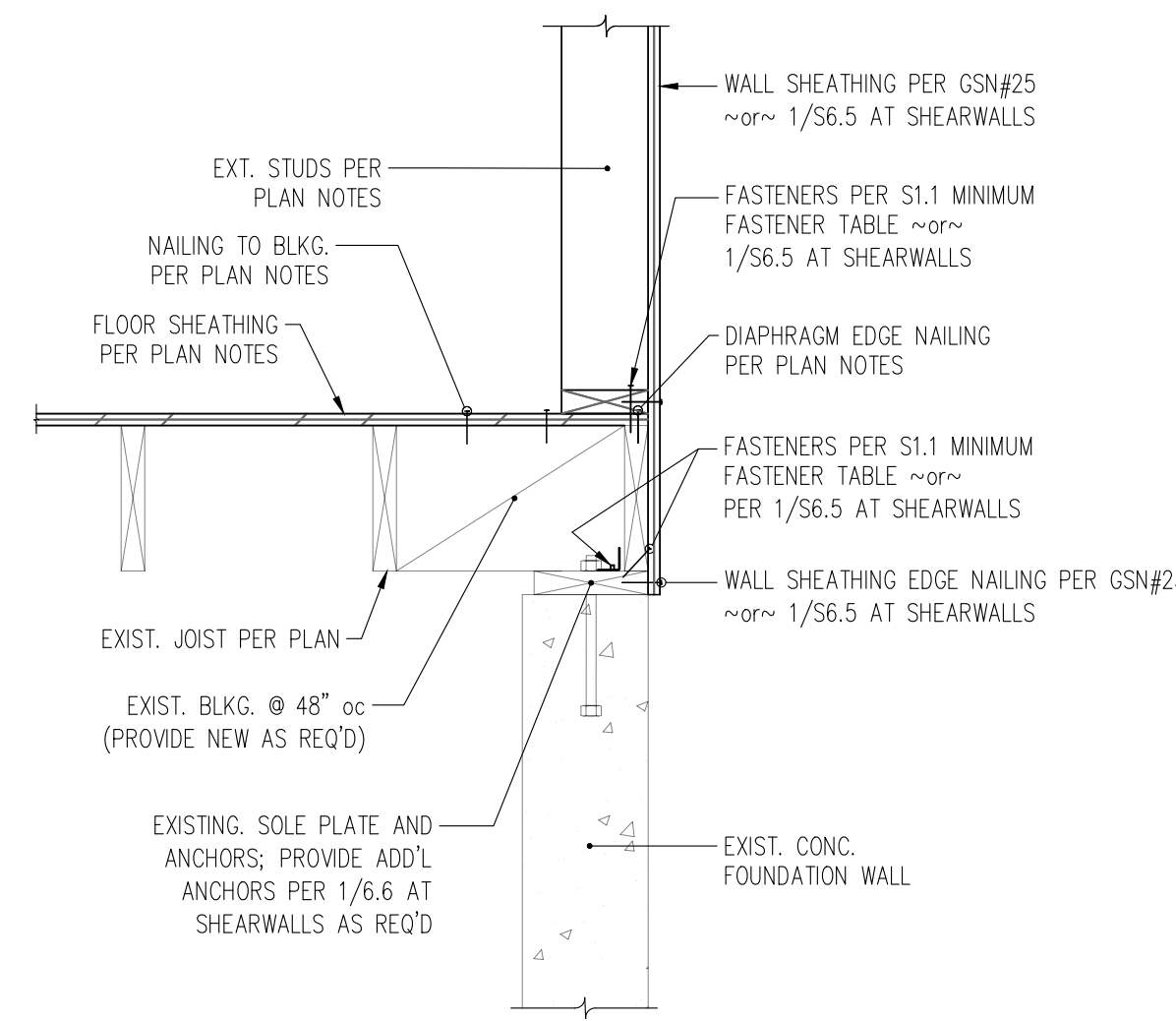
6 SECTION THROUGH INTERIOR SHEAR WALL w/ PERPENDICULAR JOISTS AT EA. SIDE  
S6.2 1" = 1'-0"



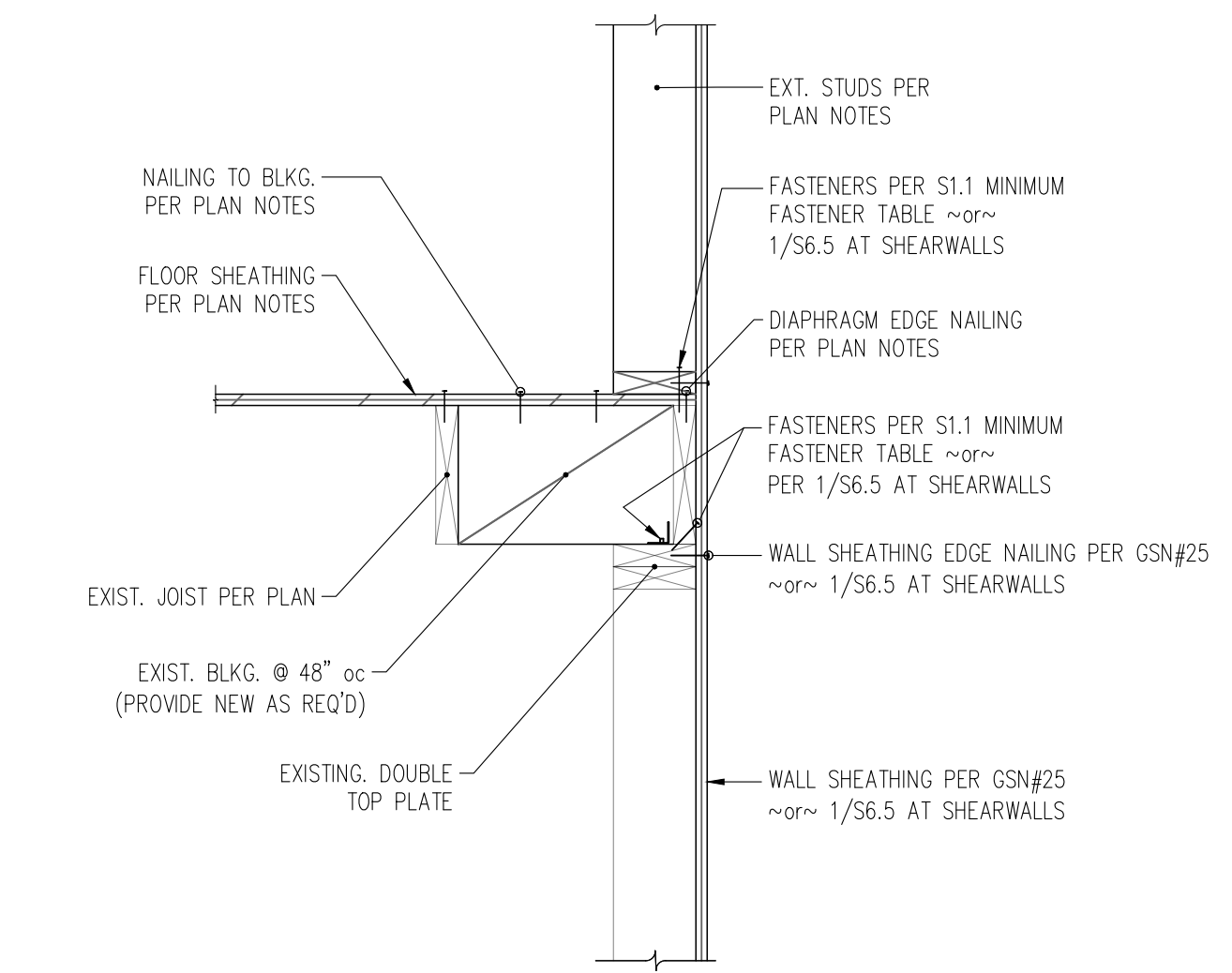
3 SECTION THROUGH INTERIOR BEARING WALL w/ PERPENDICULAR JOISTS AT EA. SIDE  
S6.2 1" = 1'-0"



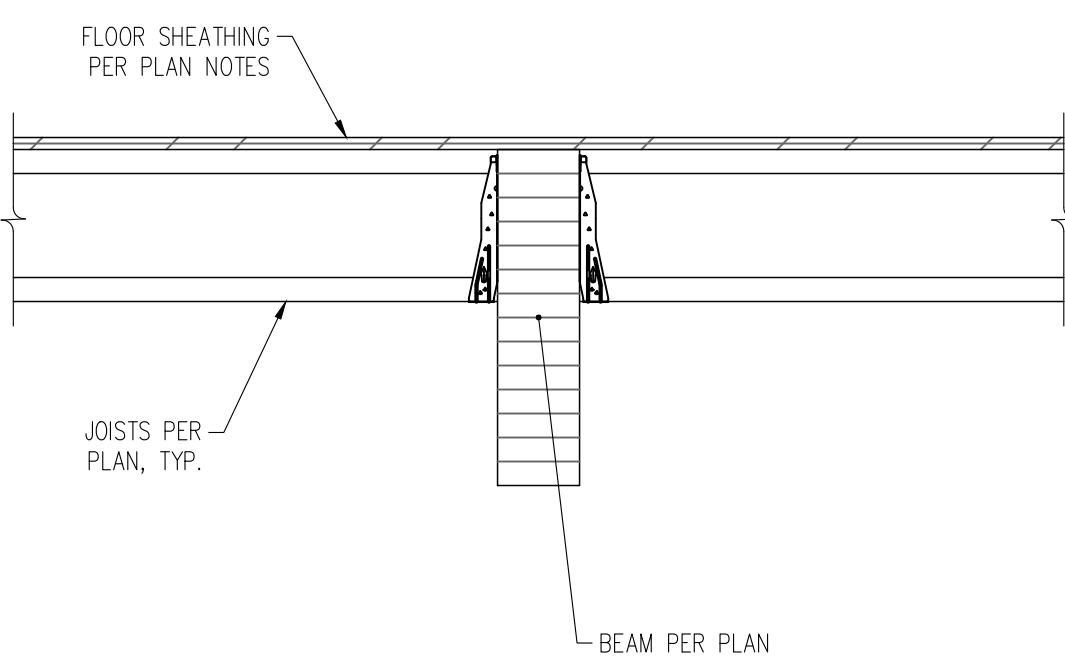
8 SECTION THROUGH BEAM SUPPORTING SHEARWALL ABOVE  
S6.2 1" = 1'-0"



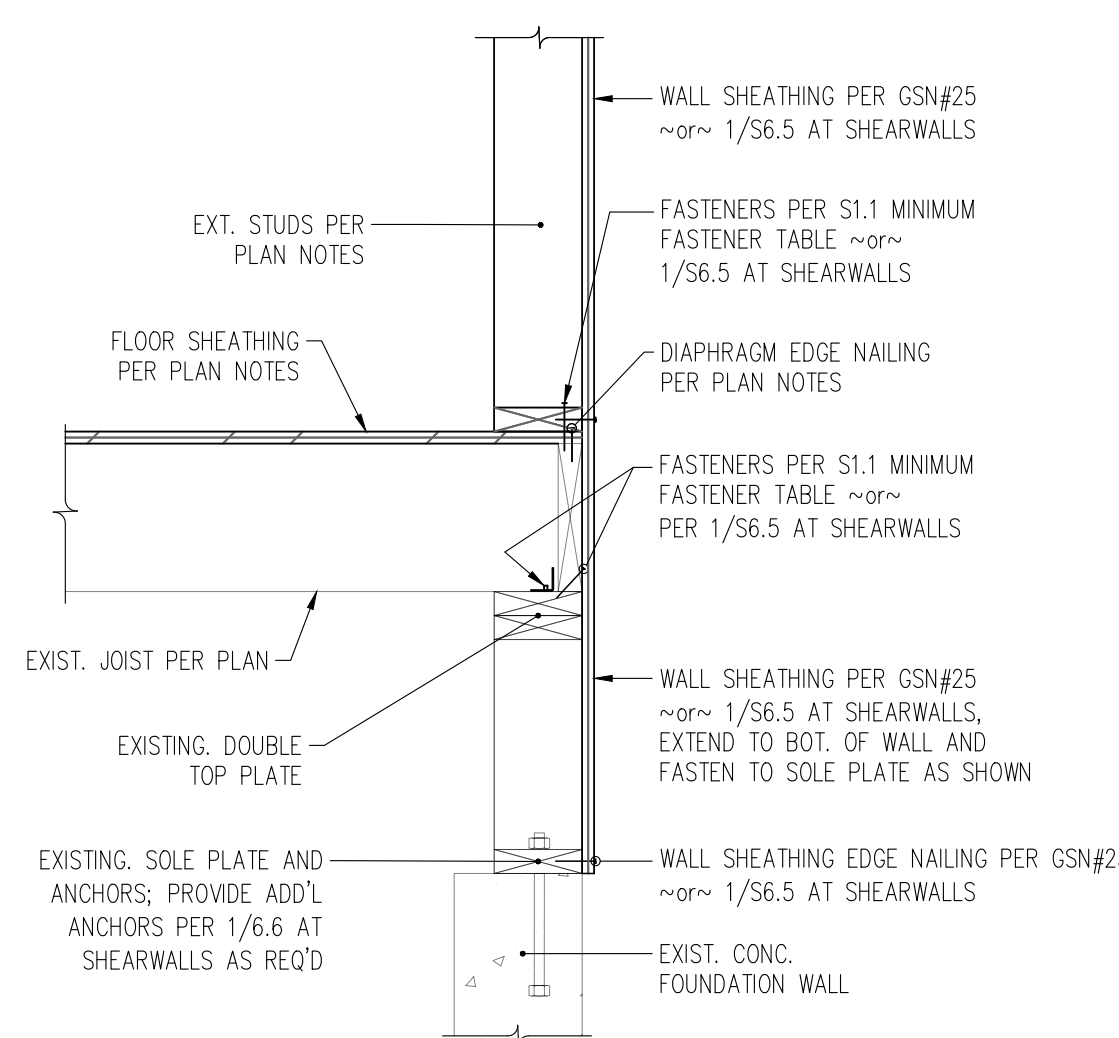
5 SECTION THROUGH EXTERIOR FOUNDATION WALL AT EXISTING PARALLEL JOISTS  
S6.2 1" = 1'-0"



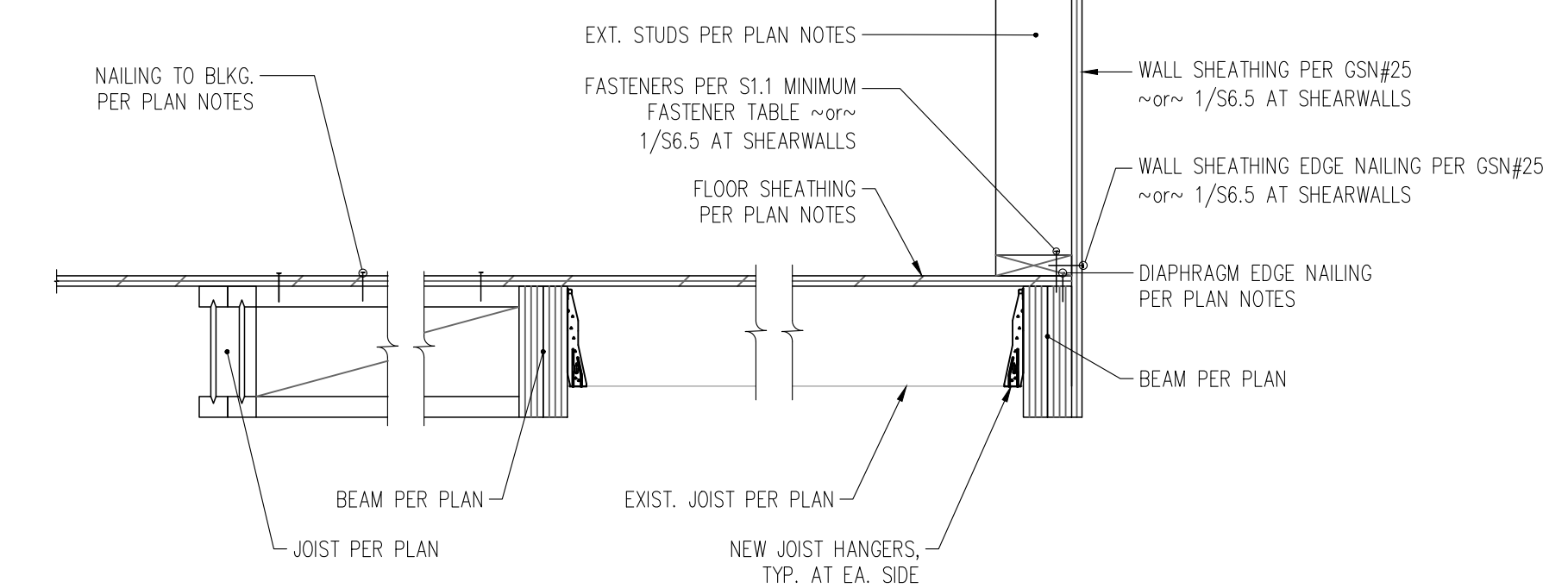
2 SECTION THROUGH EXTERIOR WALL AT EXISTING PARALLEL JOISTS  
S6.2 1" = 1'-0"



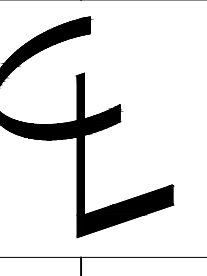
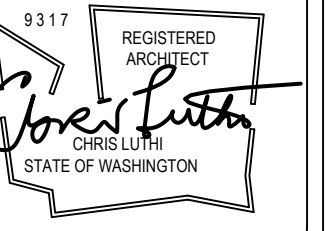
7 SECTION THROUGH BEAM SUPPORTING JOISTS  
S6.2 1" = 1'-0"



4 SECTION THROUGH EXTERIOR FOUNDATION WALL AT EXISTING PERPENDICULAR JOISTS  
S6.2 1" = 1'-0"

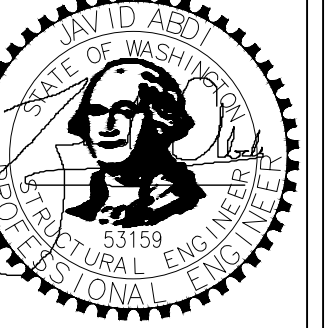
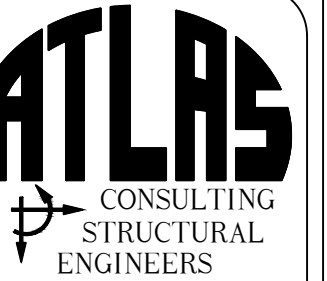


1 SECTION THROUGH EXTERIOR WALL AT EXISTING PERPENDICULAR JOISTS  
S6.2 1" = 1'-0"



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Wood Floor Framing Details

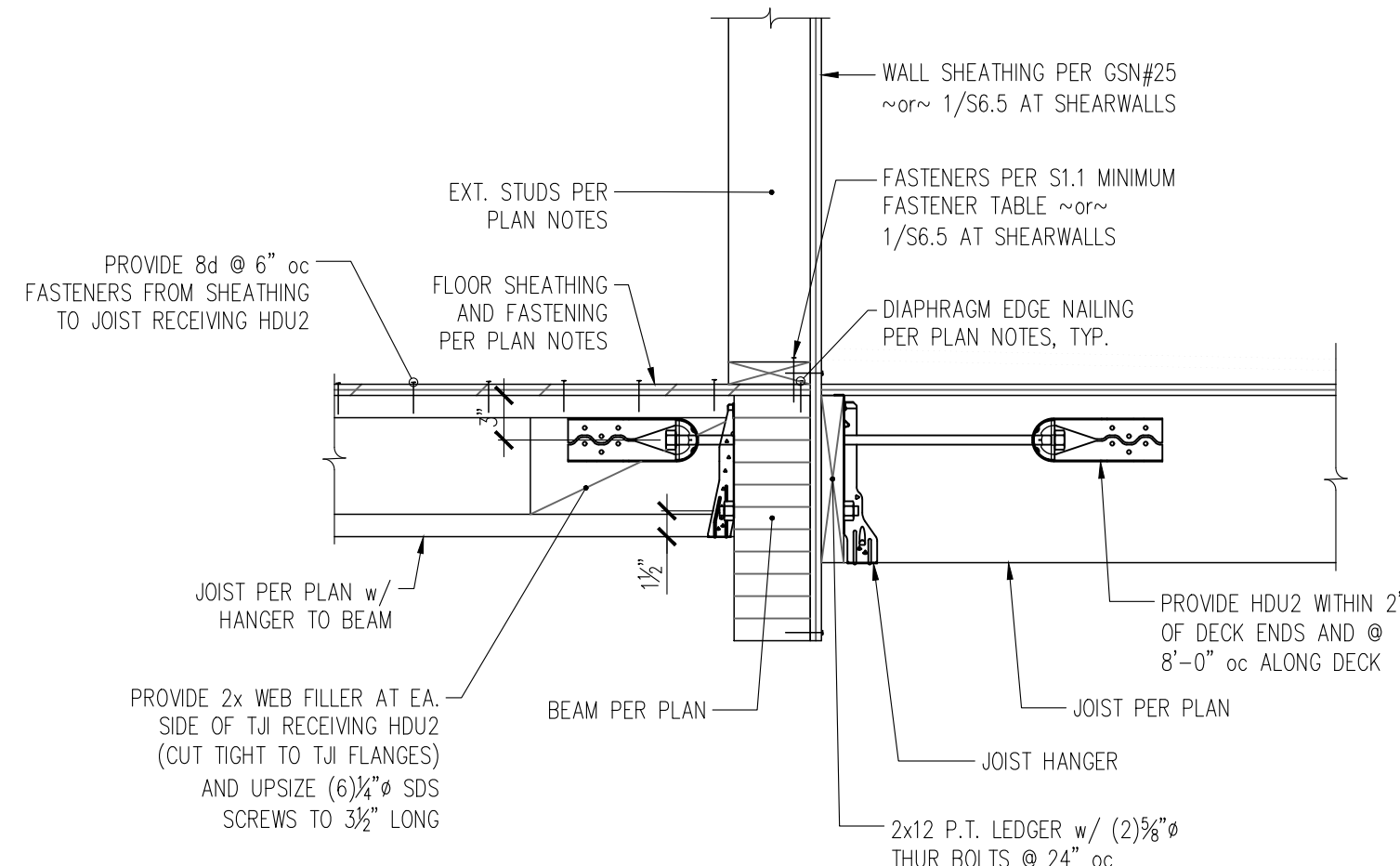
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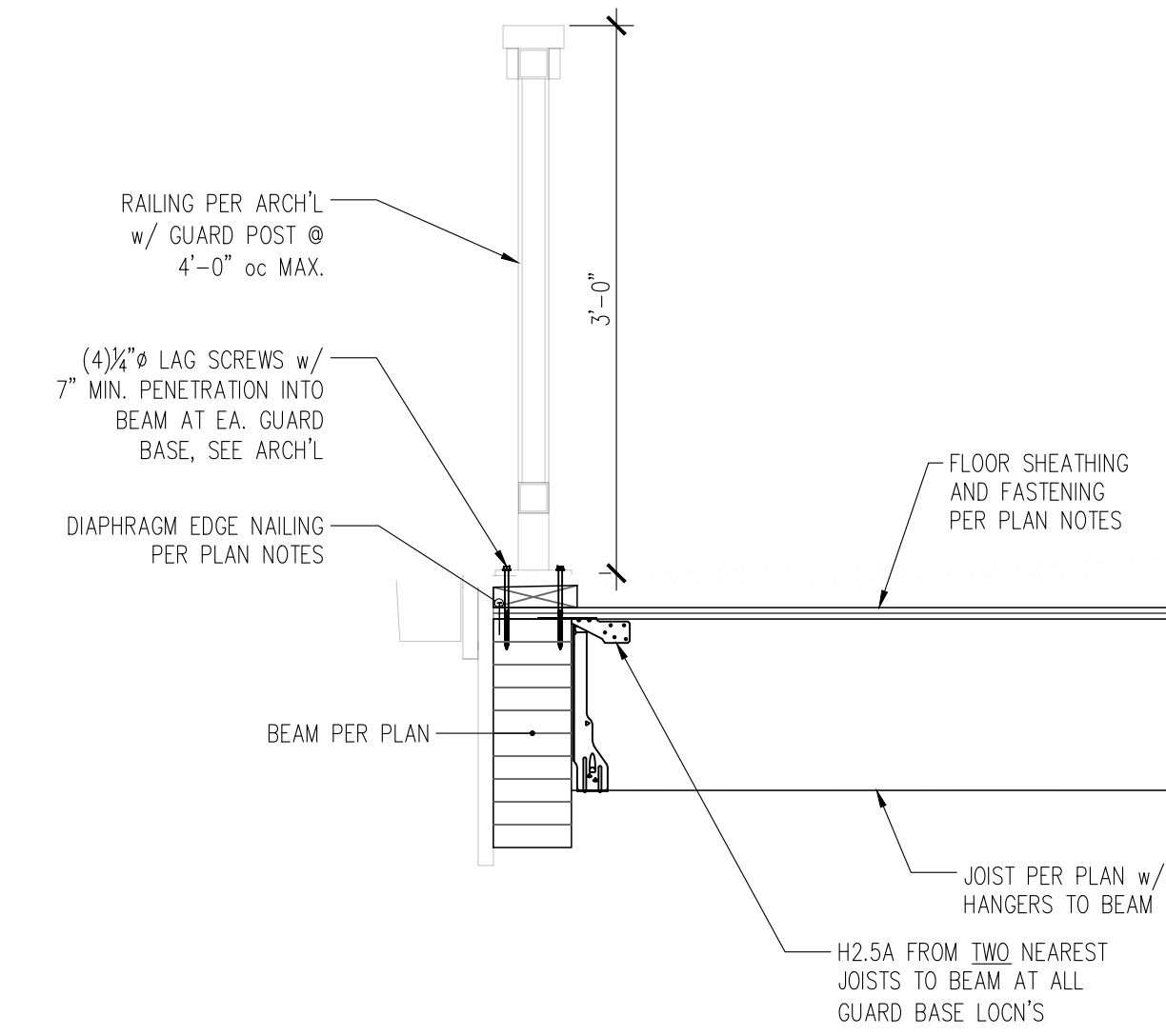
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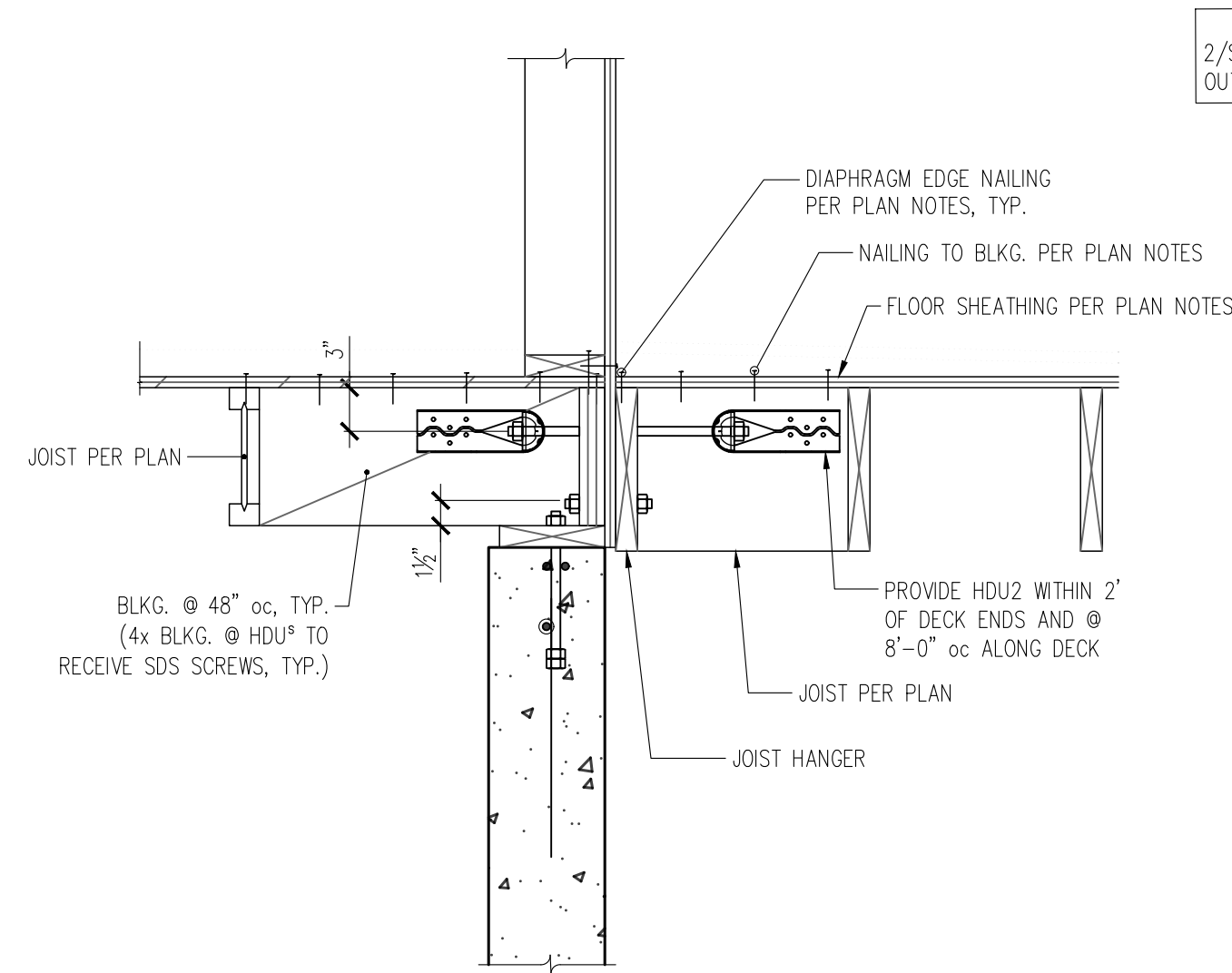
S6.2



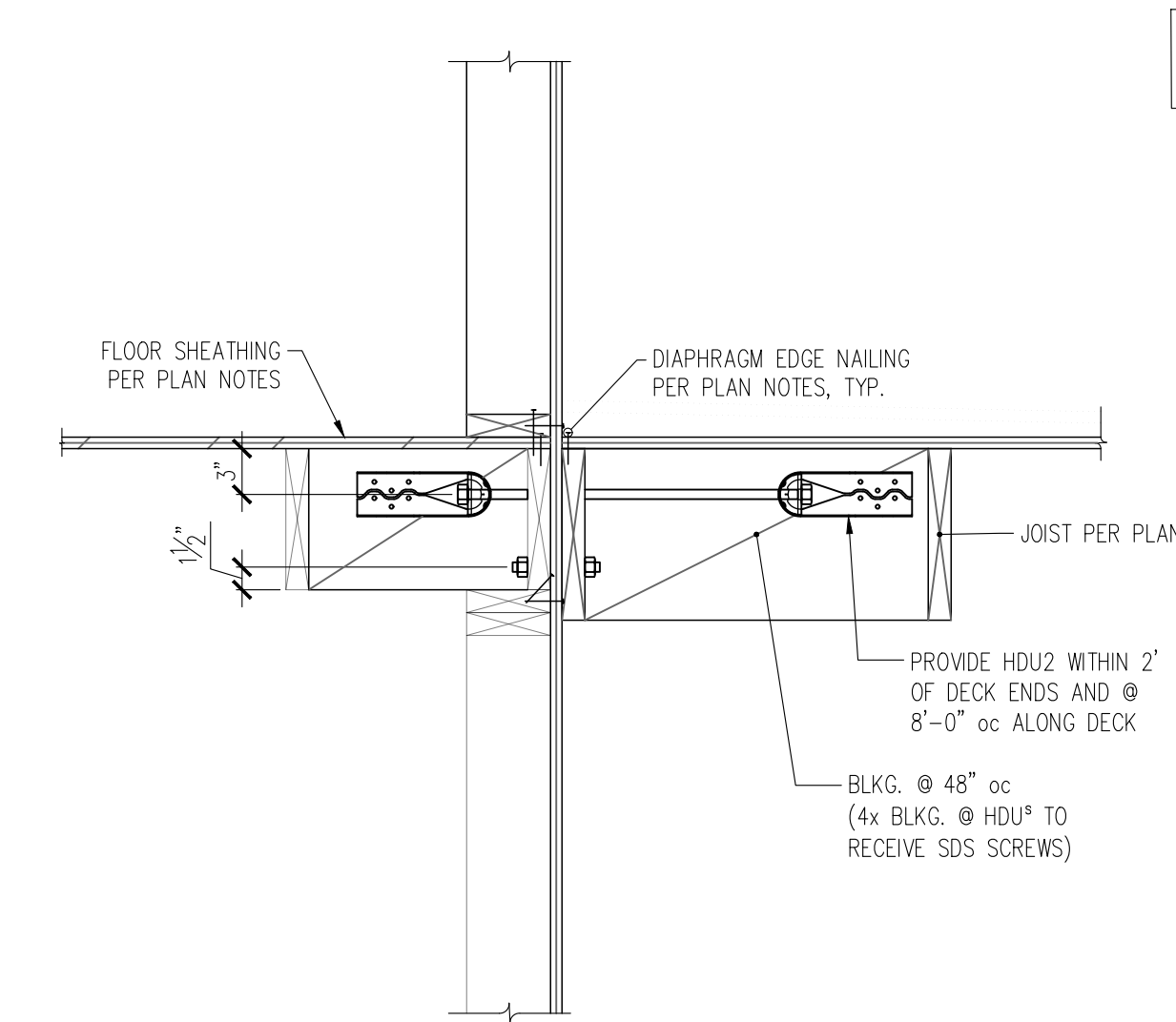
6 SECTION THROUGH EXTERIOR WALL AT PERPENDICULAR JOISTS AND PERPENDICULAR DECK JOISTS  
S6.3 1" = 1'-0"



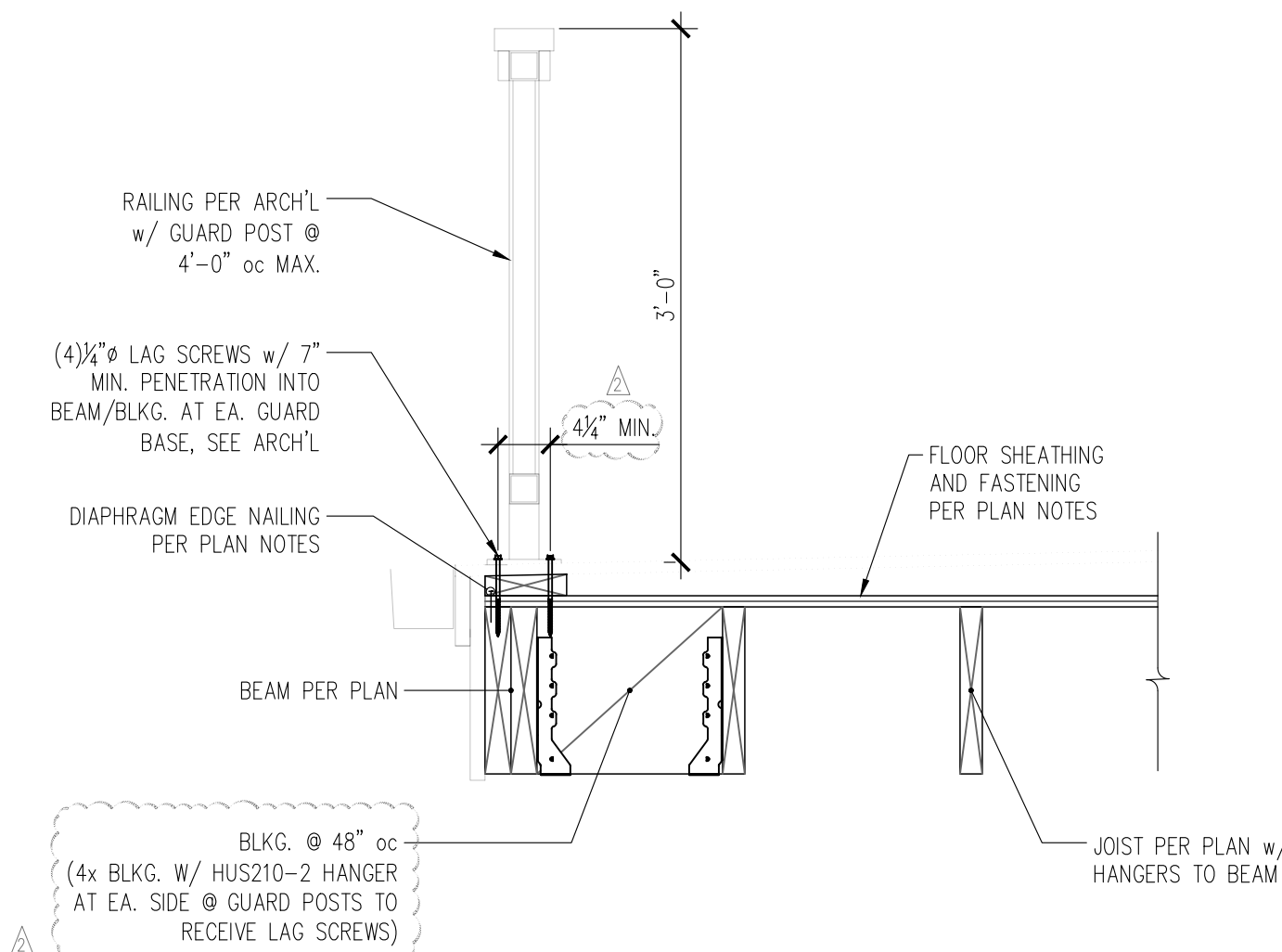
3 SECTION THROUGH DECK EDGE AT PERPENDICULAR JOISTS  
S6.3 1" = 1'-0"



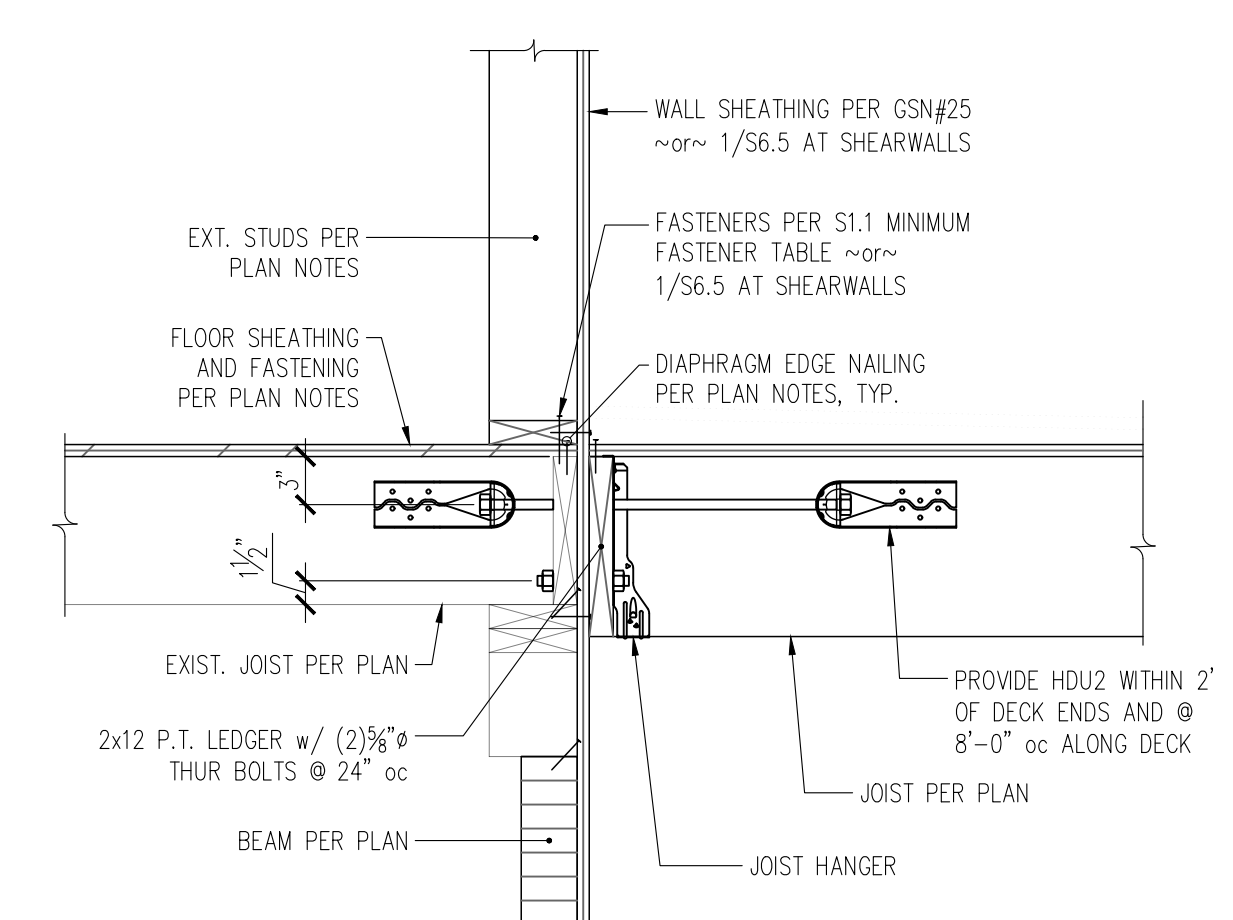
5 SECTION THROUGH EXTERIOR FOUNDATION WALL AT PARALLEL JOISTS AND PARALLEL DECK JOISTS  
S6.3 1" = 1'-0"



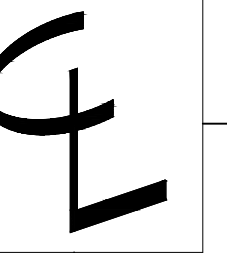
2 SECTION THROUGH EXTERIOR WALL AT PARALLEL JOISTS AND PARALLEL DECK JOISTS  
S6.3 1" = 1'-0"



4 SECTION THROUGH DECK EDGE AT PARALLEL JOISTS  
S6.3 1" = 1'-0"

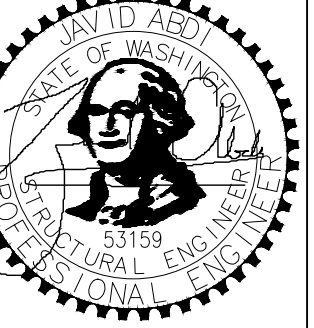
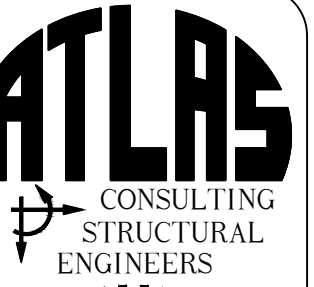


1 SECTION THROUGH EXTERIOR WALL AT PERP. EXIST. JOISTS AND PERP. DECK JOISTS  
S6.3 1" = 1'-0"



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Wood Floor Framing Details

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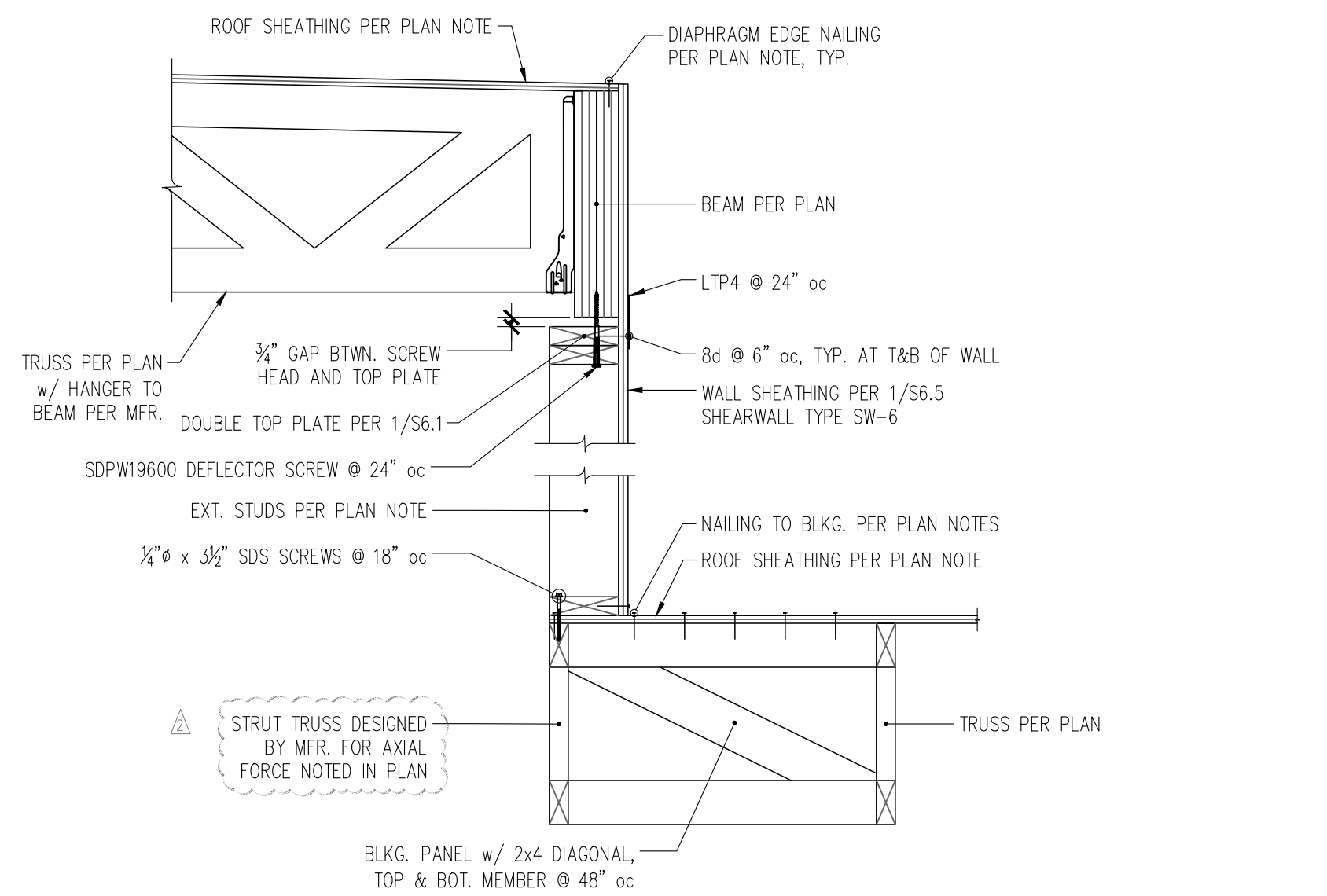
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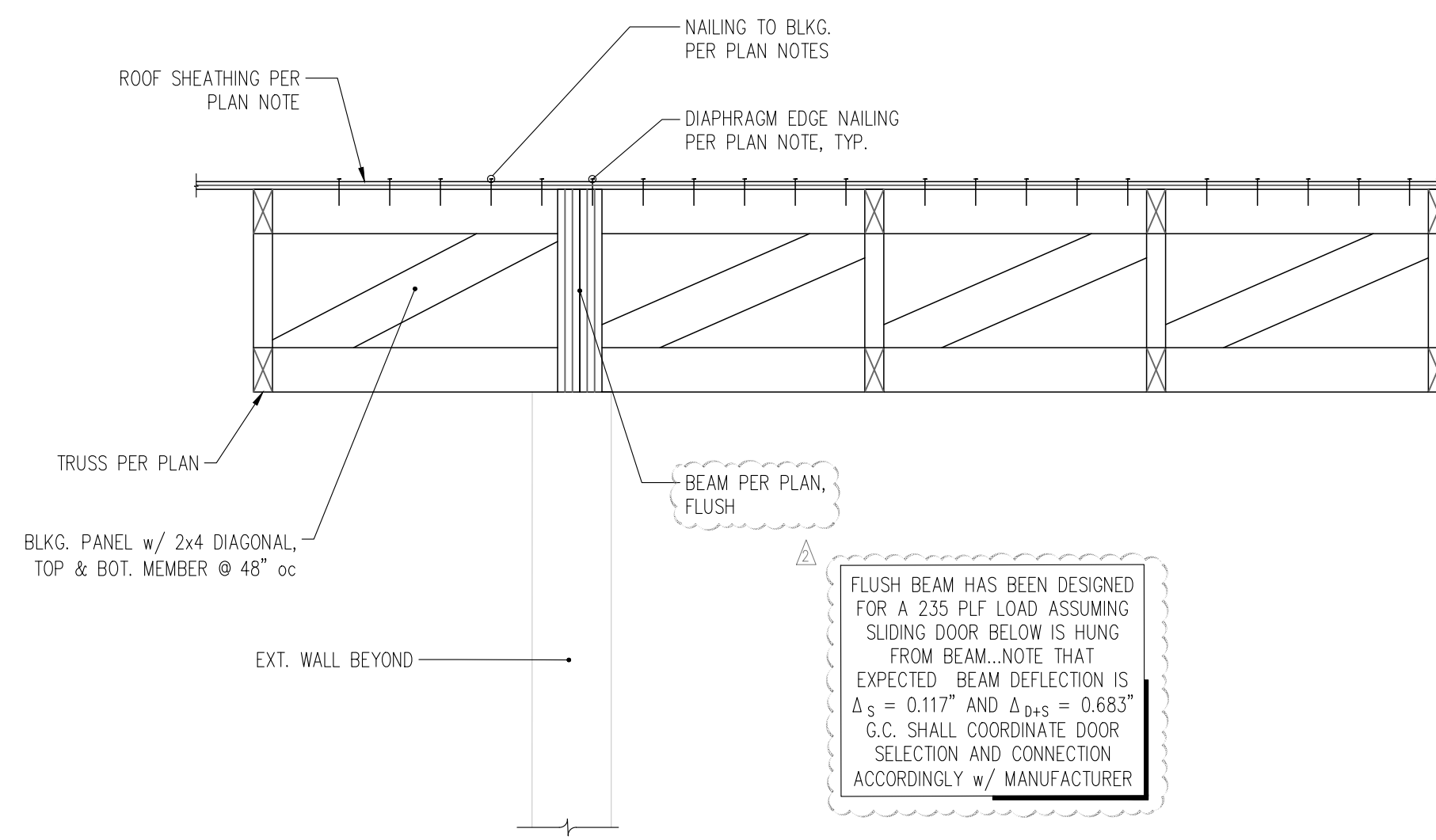
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12.03.21

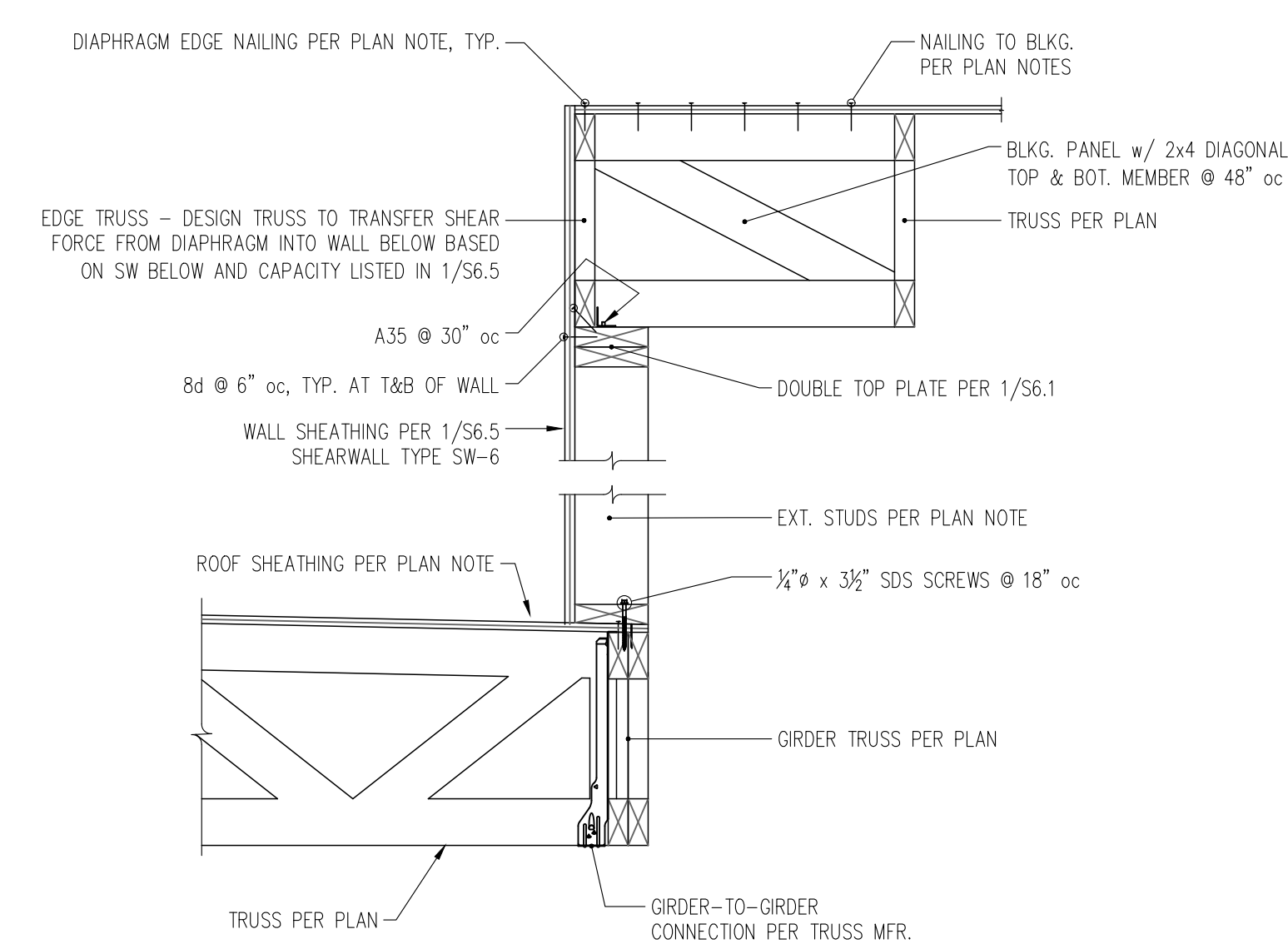
S6.3



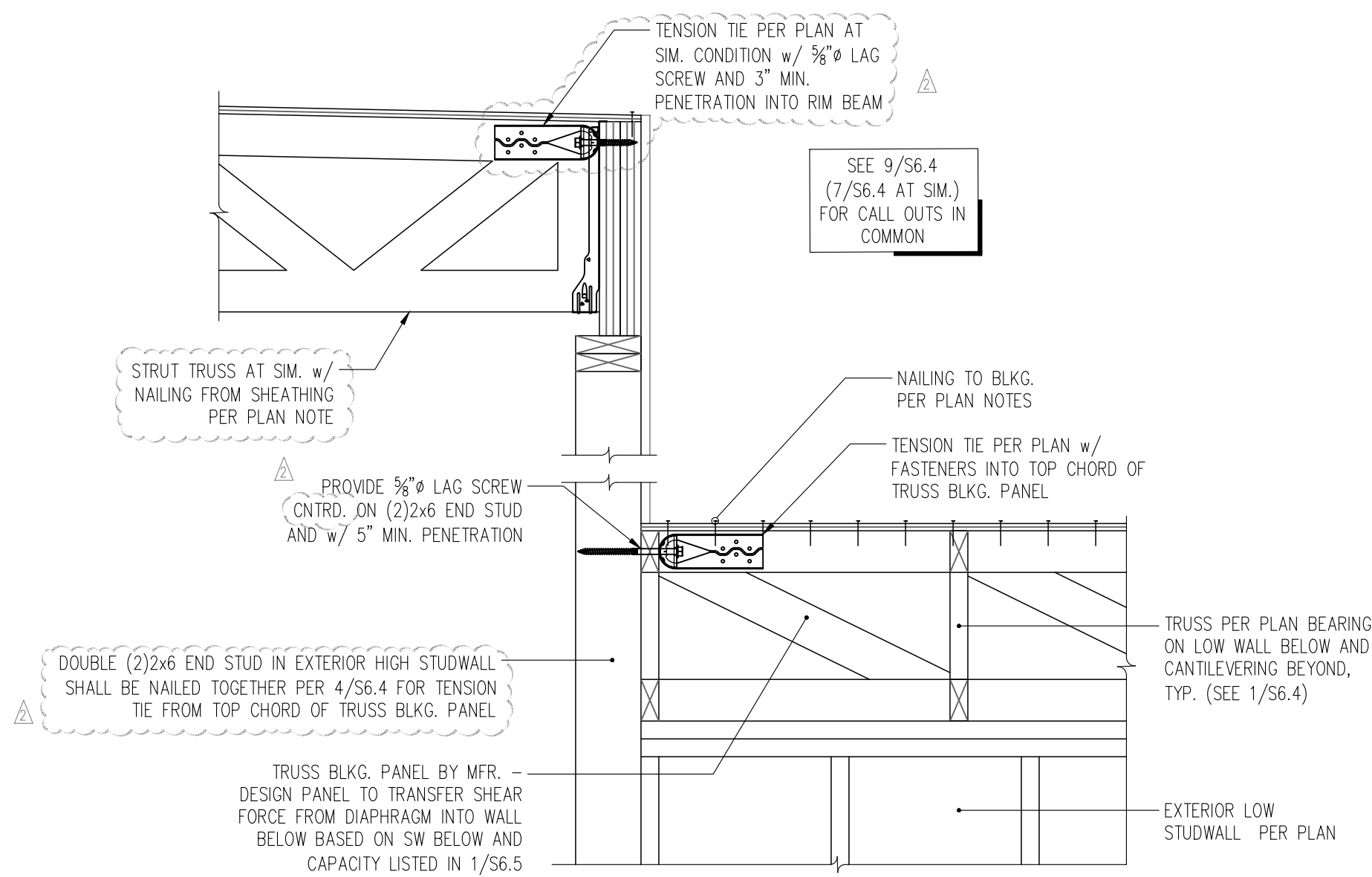
9 SECTION AT LOW-TO-HIGH ROOF TRANSITION  
S6.4 1" = 1'-0"



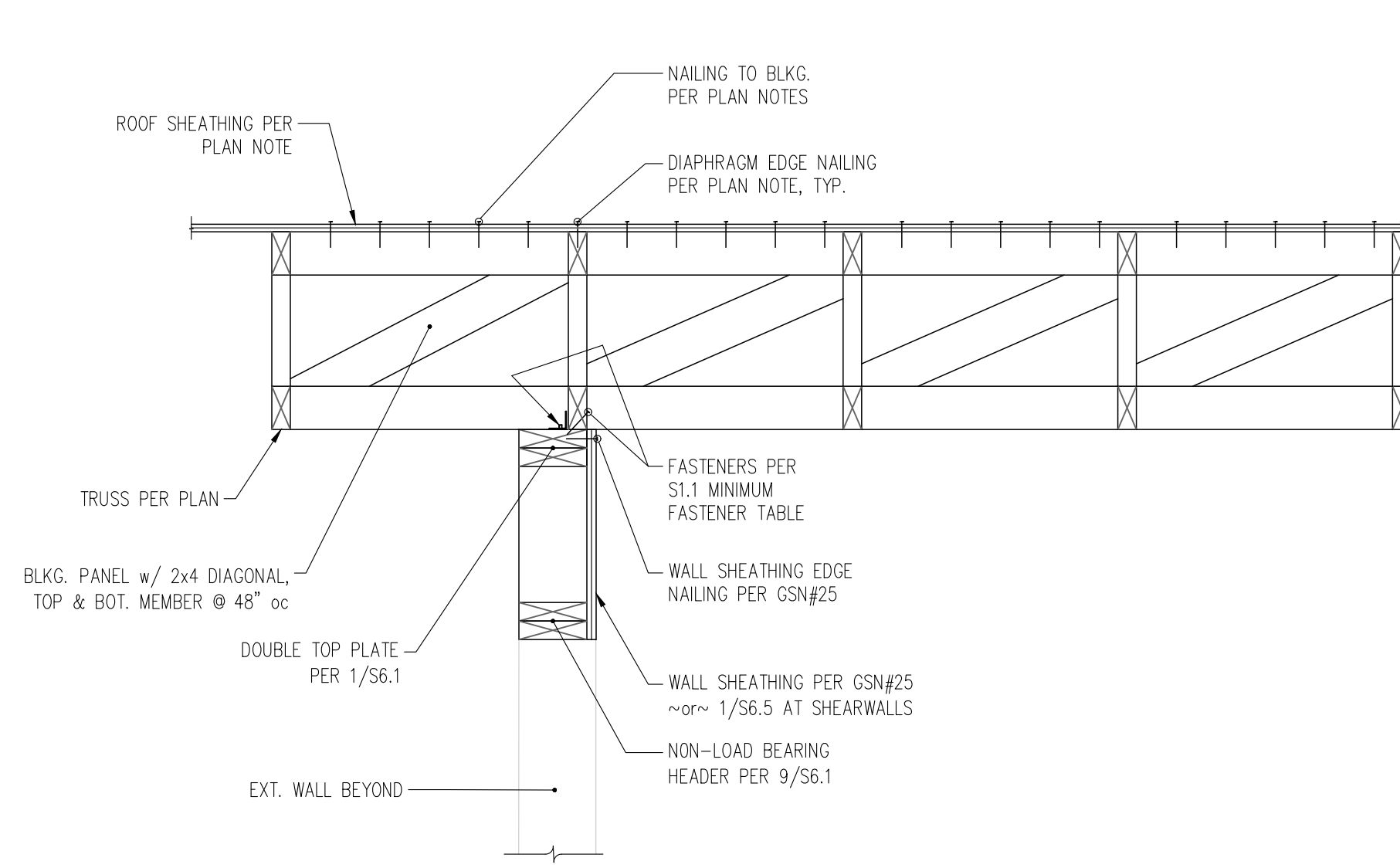
6 SECTION THROUGH EXTERIOR WALL AT EXTENDED ROOF OVERHANG AND GARAGE HEADER  
S6.4 1" = 1'-0"



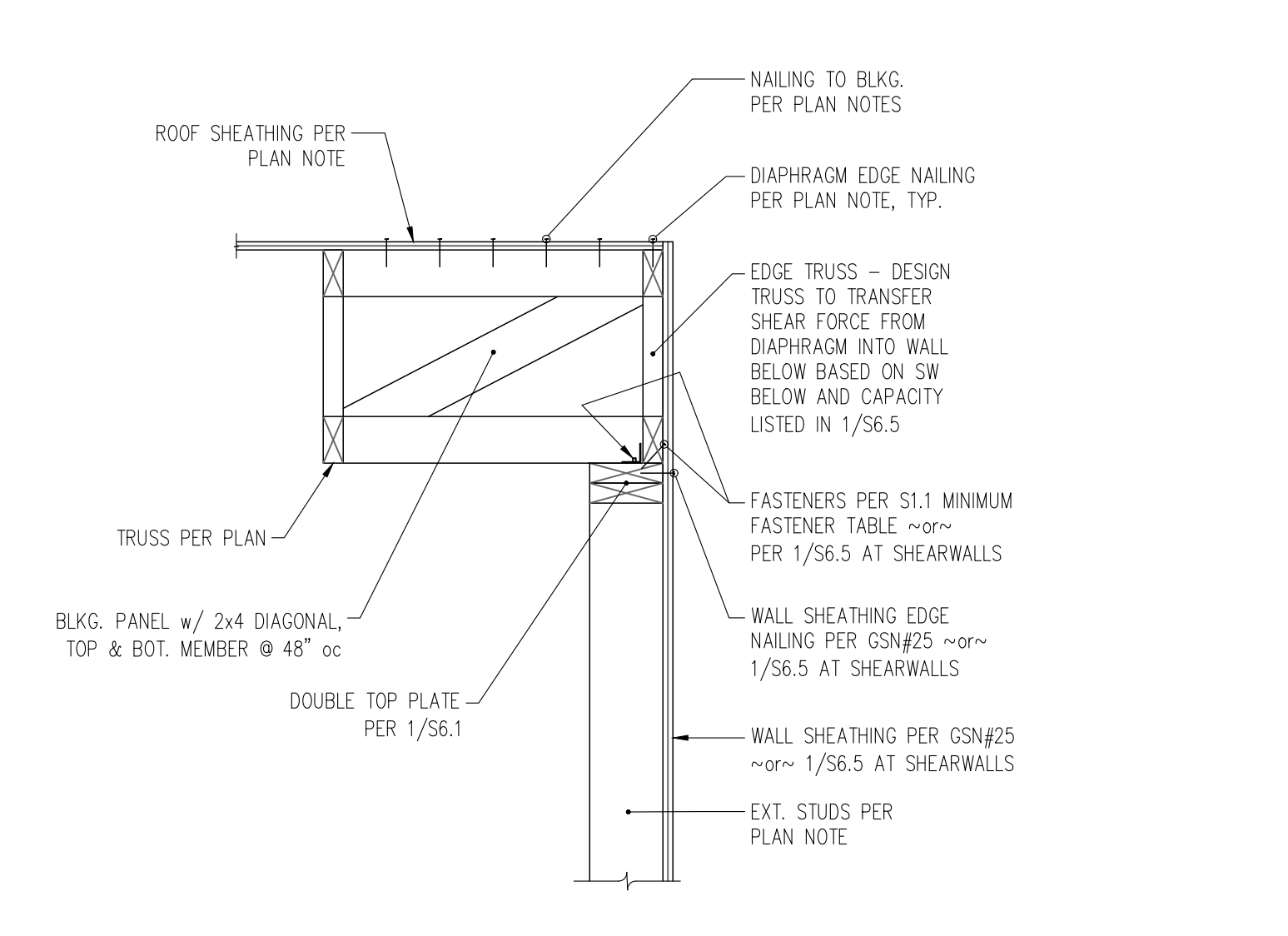
3 SECTION AT LOW-TO-HIGH ROOF TRANSITION  
S6.4 1" = 1'-0"



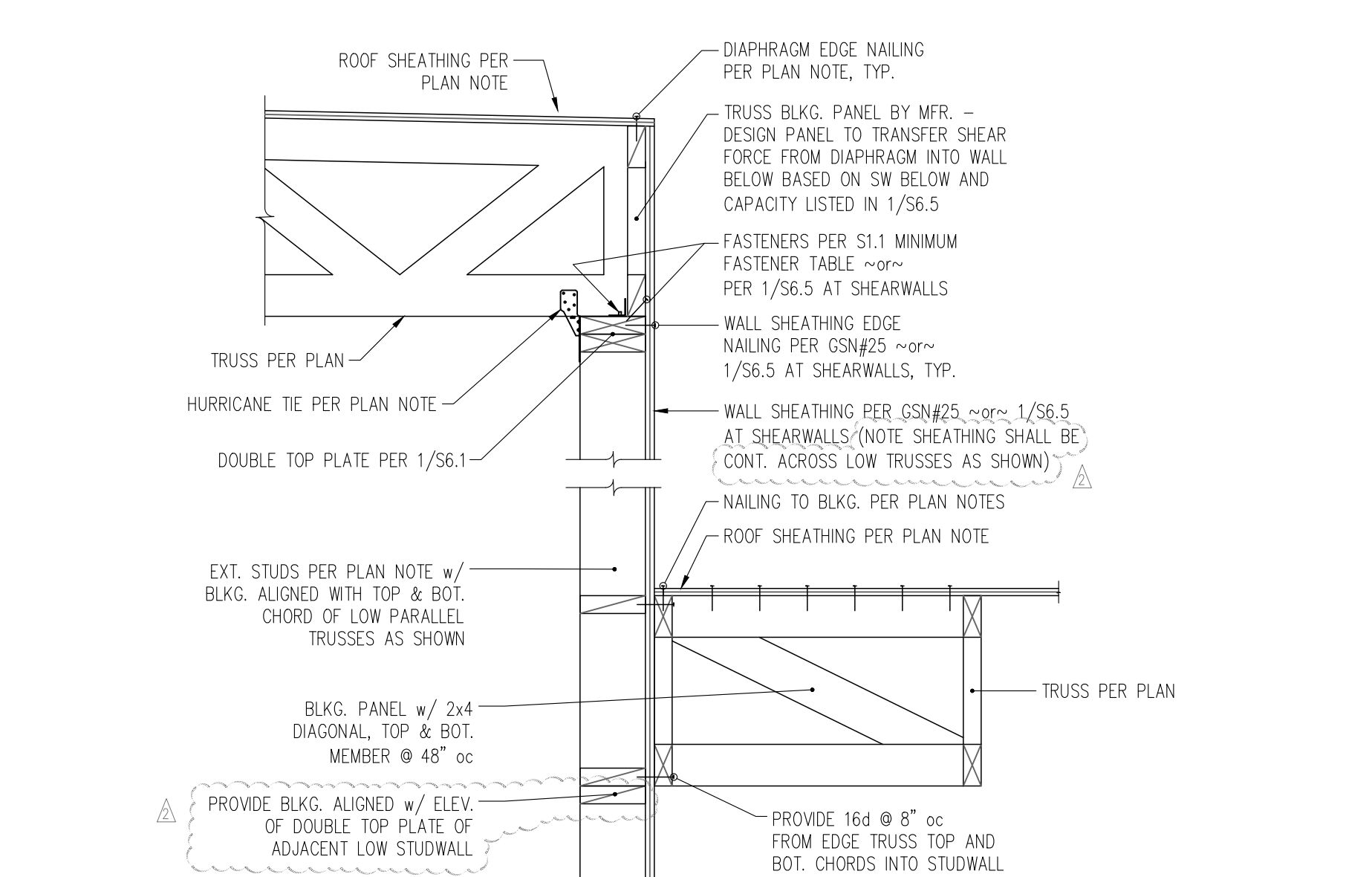
8 CHORD TENSION TIE AT LOW-TO-HIGH ROOF BREAK  
S6.4 1" = 1'-0"



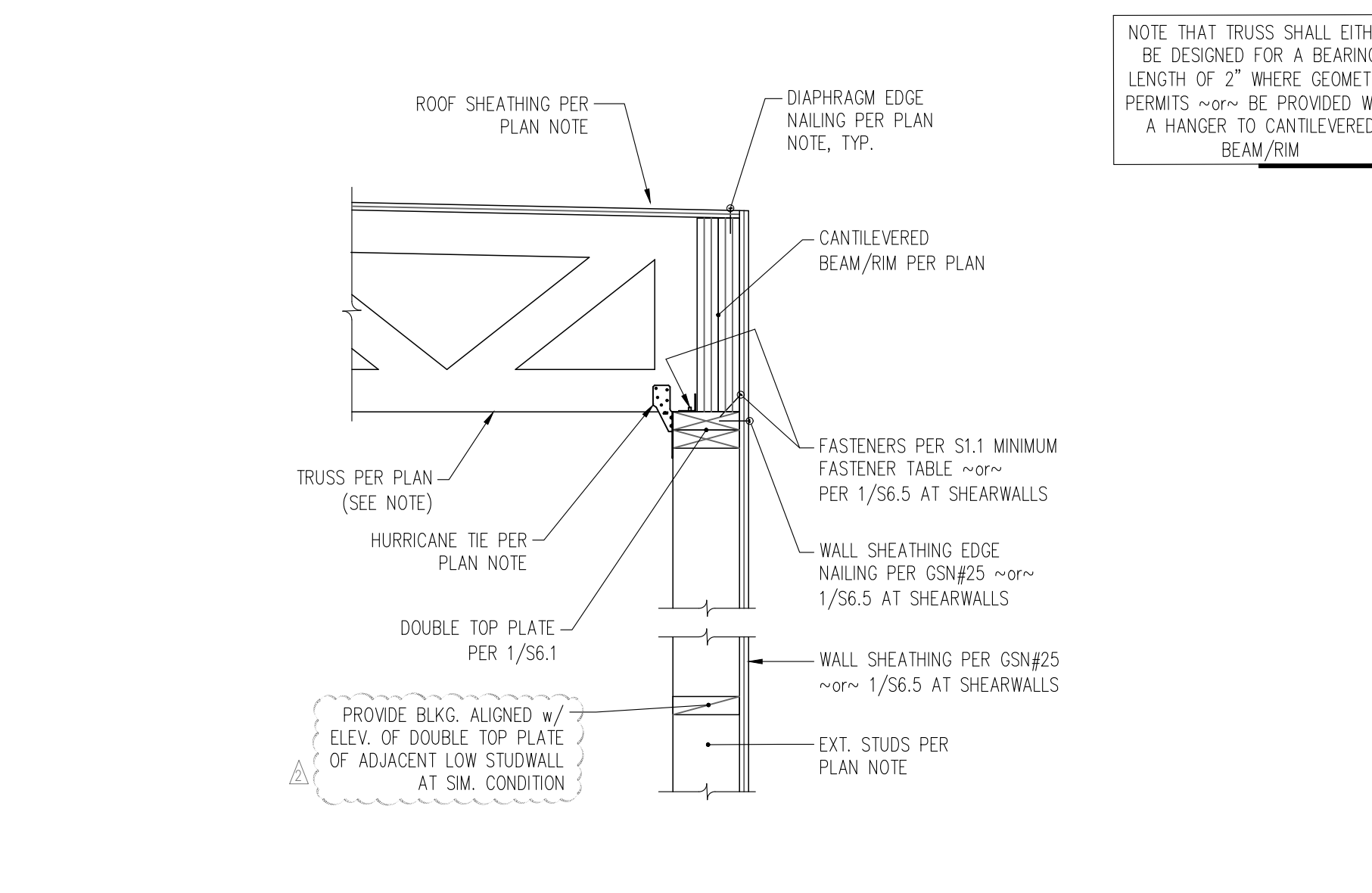
5 SECTION THROUGH EXTERIOR WALL AT EXTENDED ROOF OVERHANG  
S6.4 1" = 1'-0"



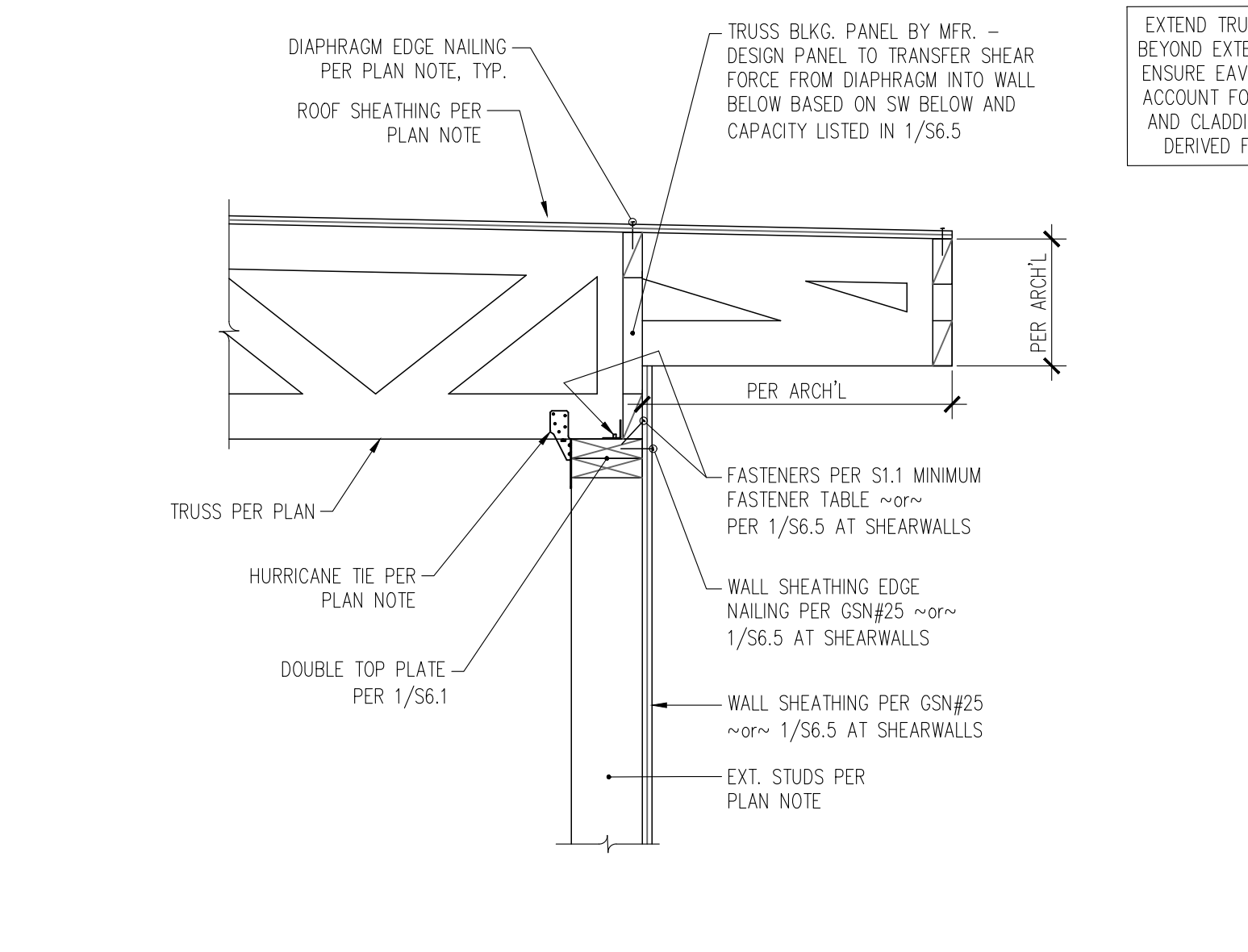
2 SECTION THROUGH EXTERIOR WALL AT PARALLEL TRUSSES  
S6.4 1" = 1'-0"



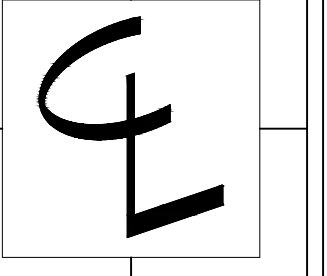
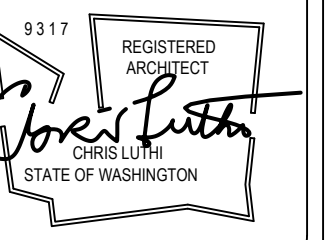
7 SECTION AT LOW-TO-HIGH ROOF TRANSITION  
S6.4 1" = 1'-0"



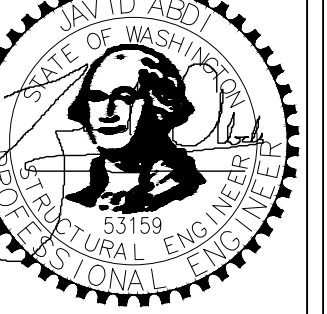
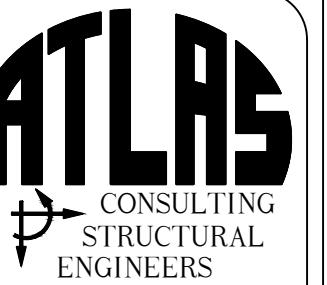
4 SECTION THROUGH EXTERIOR WALL AT PERPENDICULAR TRUSSES AND CANTILEVERED BEAM/RIM  
S6.4 1" = 1'-0"



1 SECTION THROUGH EXTERIOR WALL AT PERPENDICULAR TRUSSES  
S6.4 1" = 1'-0"



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Wood Roof Framing Details

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DATE

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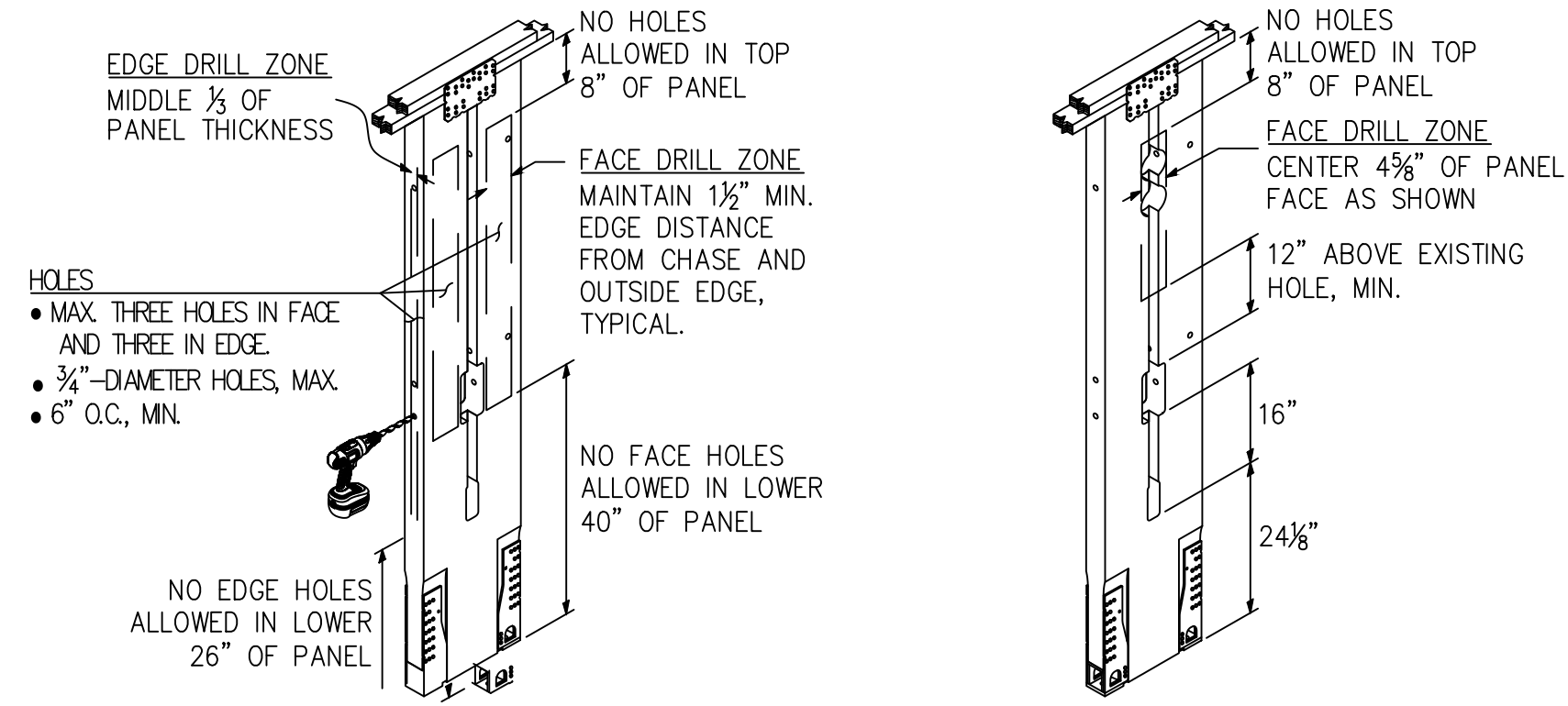
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12.03.21

S6.4

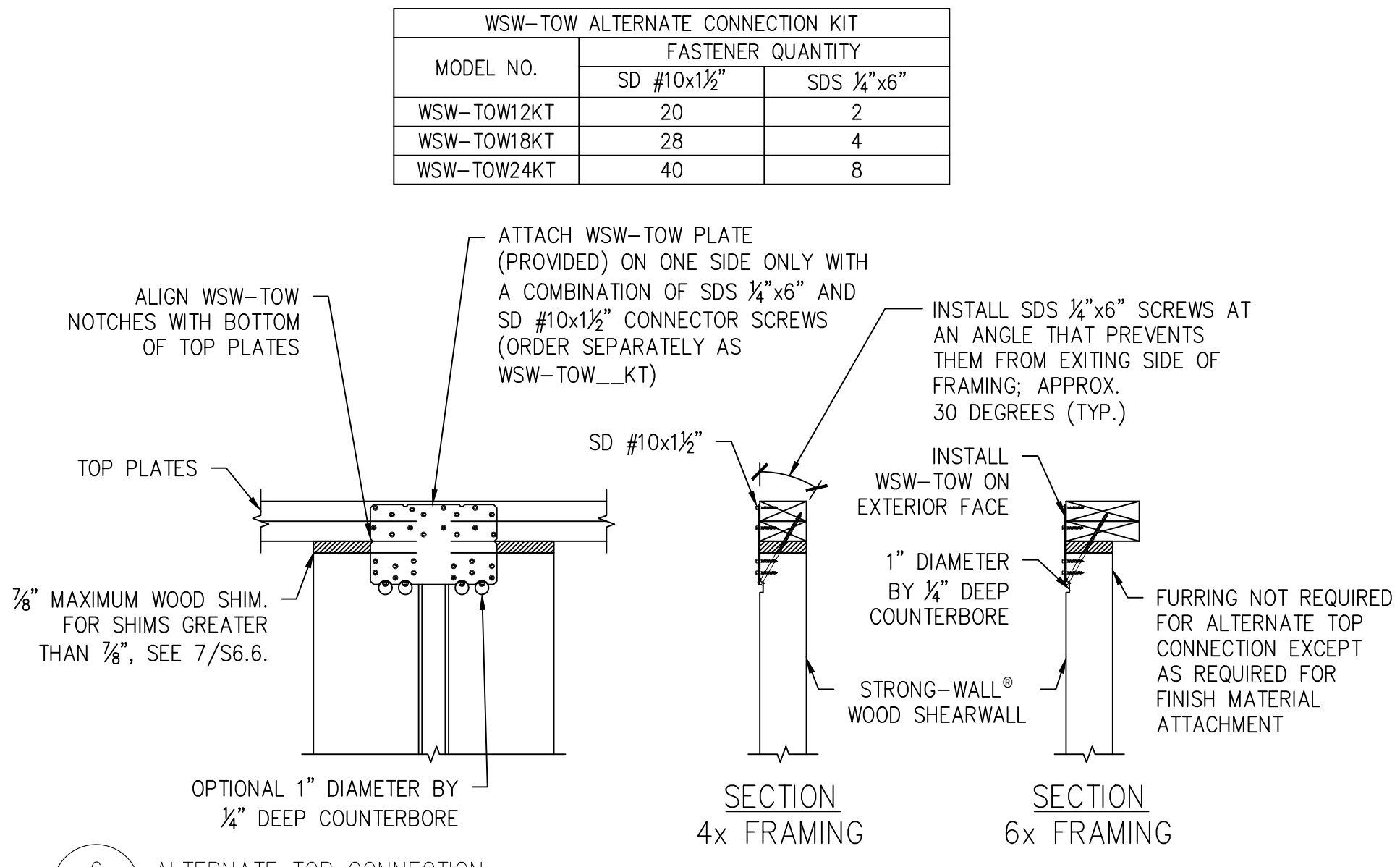




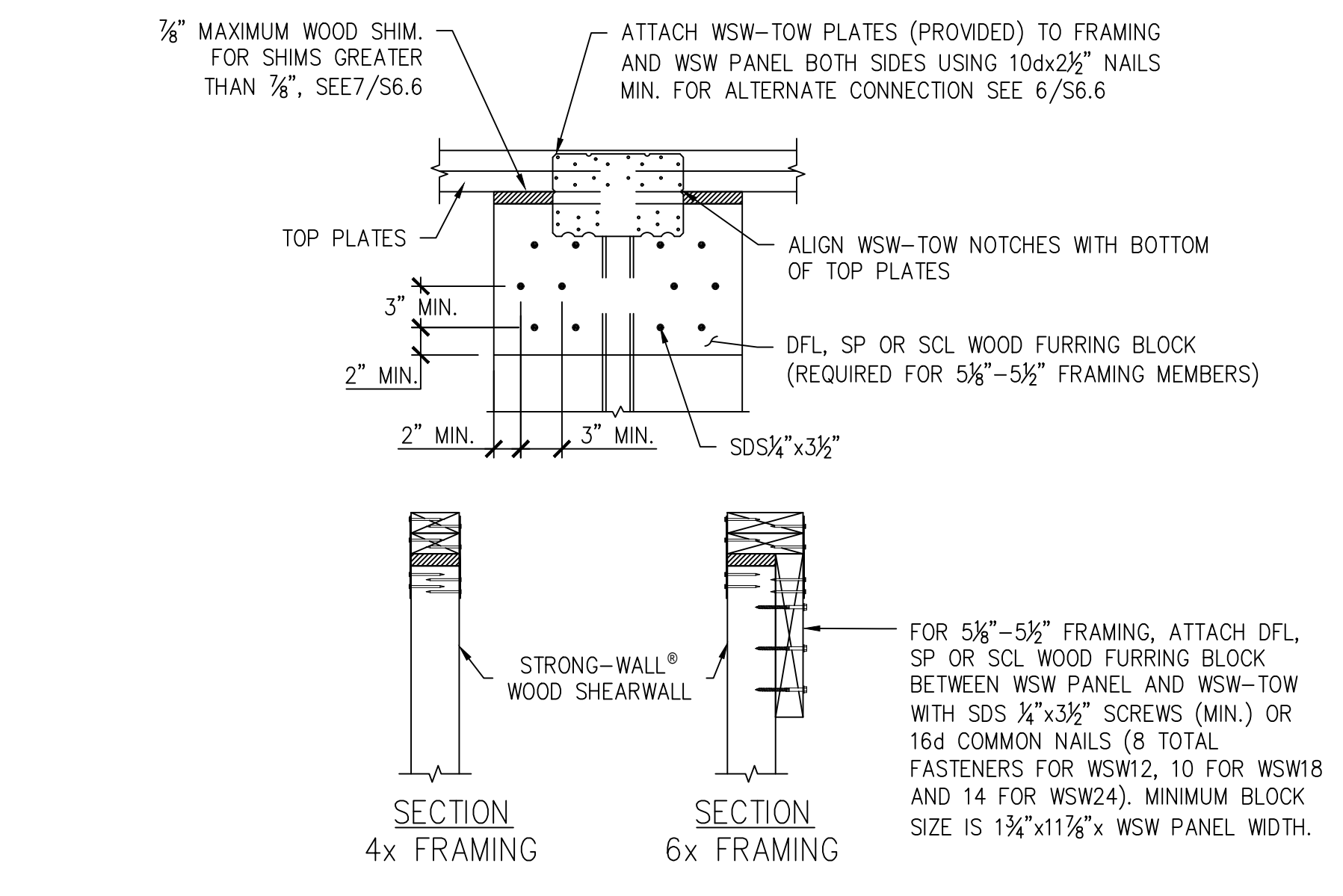


- ALLOWABLE SMALL HOLES (FACE & EDGE DRILL ZONES)**
- 4 5/8" -DIA. HOLES, MAX.
  - MAX. OF TWO 4 5/8" -DIA. HOLES OR ONE 4 1/4" x 12" HOLE.
  - NO MINIMUM ON-CENTER SPACING REQUIRED.

1 TRIM\_ZONE AND ALLOWABLE HOLES  
S6.6 1" = 1'-0"



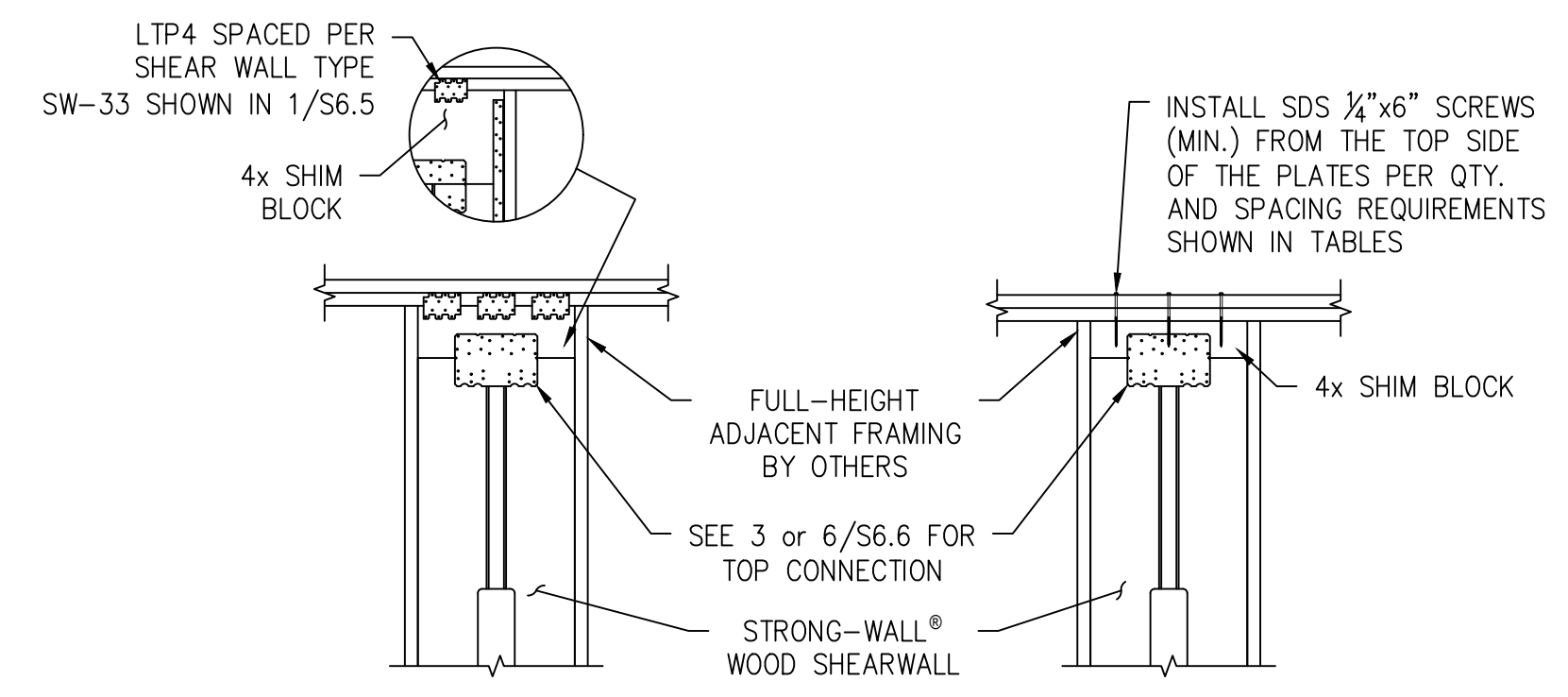
6 ALTERNATE TOP CONNECTION  
S6.5 1" = 1'-0"



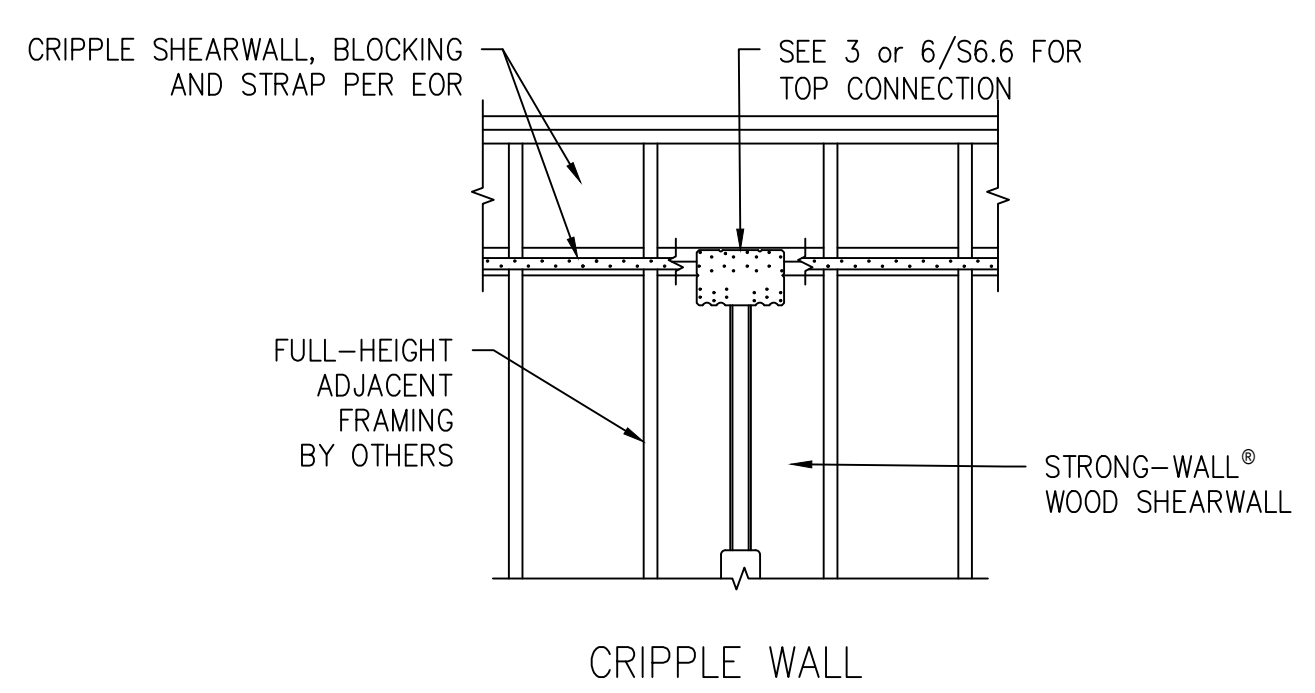
3 STANDARD TOP CONNECTION  
S6.5 1" = 1'-0"

QTY. OF SDS 1/4"x6" SCREWS REQ'D.	EDGE DISTANCE FOR SCREWS
WSW12	4
WSW18	8
WSW24	12

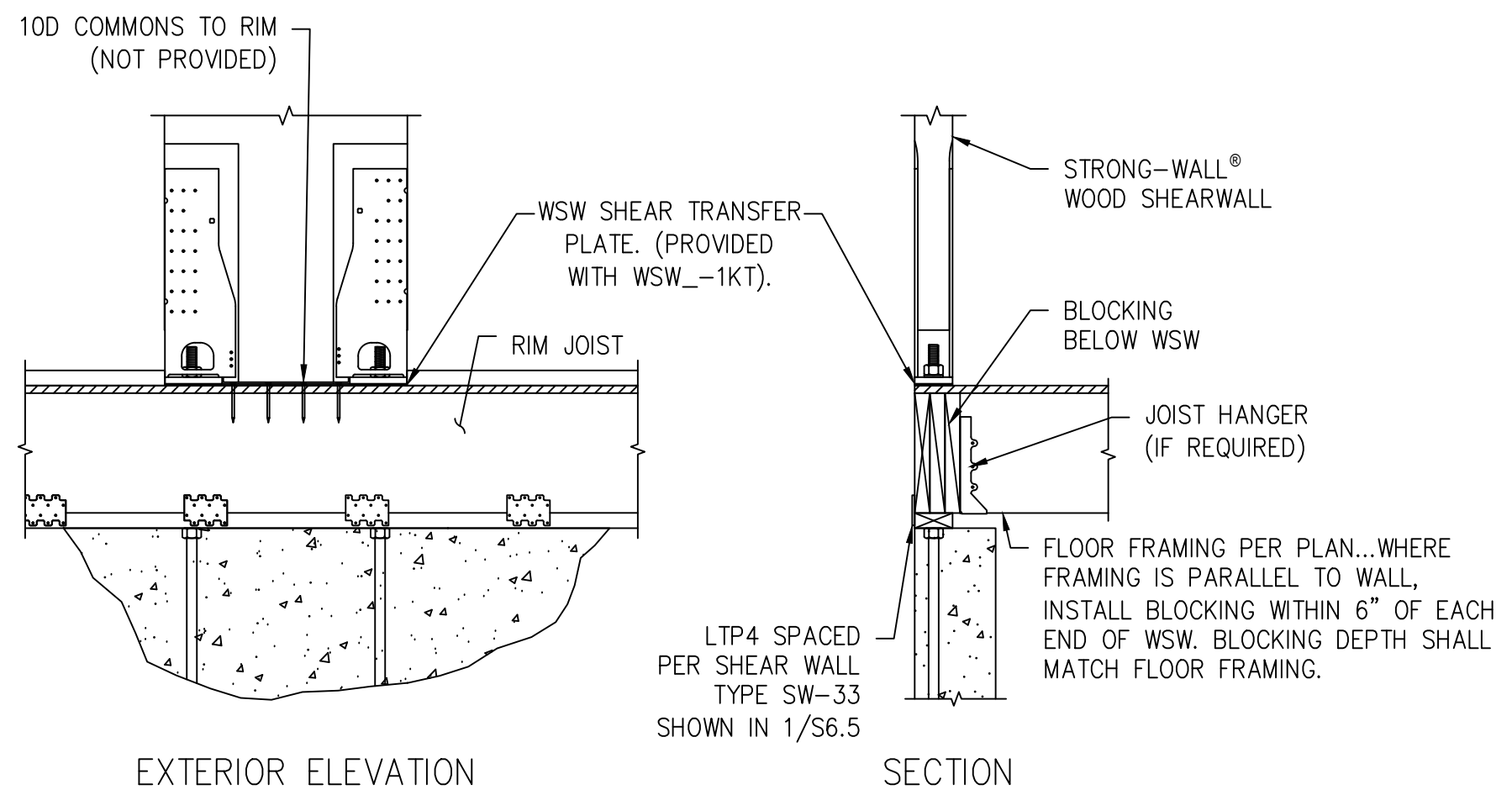
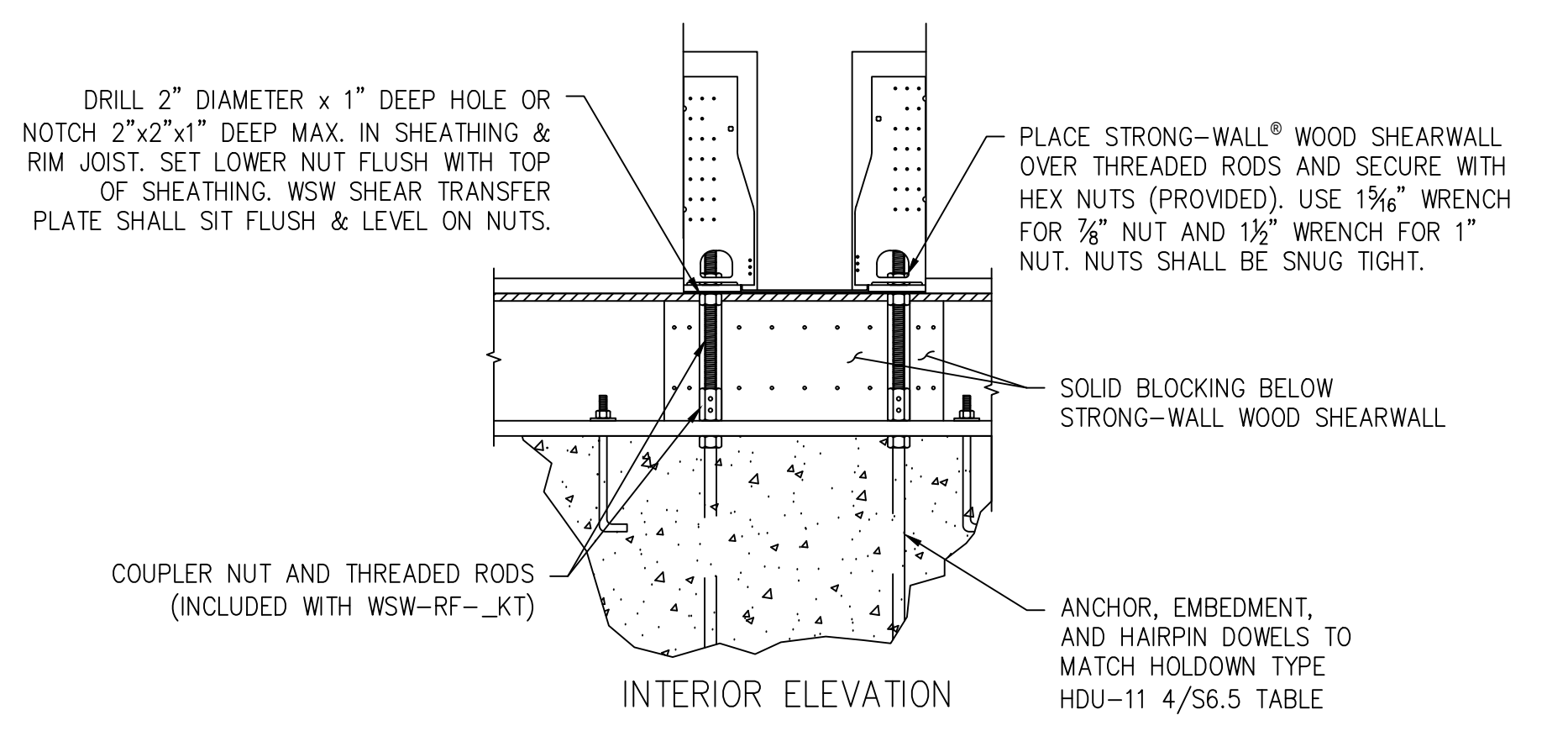
SLOPE	A (in.)	B (in.)
0:12-4:12	2	3
5:12-8:12	1 1/2	4 1/2
9:12-12:12	1 1/2	5 1/2



- 4 1/8" TO 12" SHIM BLOCK**
- FOR 8" TO 12" BLOCK DEPTHS:**  
ATTACH SIMPSON STRONG-TIE® CS16 STRAPS AT EDGE OF WSW PANEL (EACH SIDE) USING 10d x 1 1/2" NAILS
- SHIM BLOCK HEIGHTS GREATER THAN 8" AND UP TO 10":**
- 8 NAILS INTO BLOCK
  - 8 NAILS INTO WSW PANEL
- SHIM BLOCK HEIGHTS GREATER THAN 10" AND UP TO 12":**
- 10 NAILS INTO BLOCK
  - 10 NAILS INTO WSW PANEL



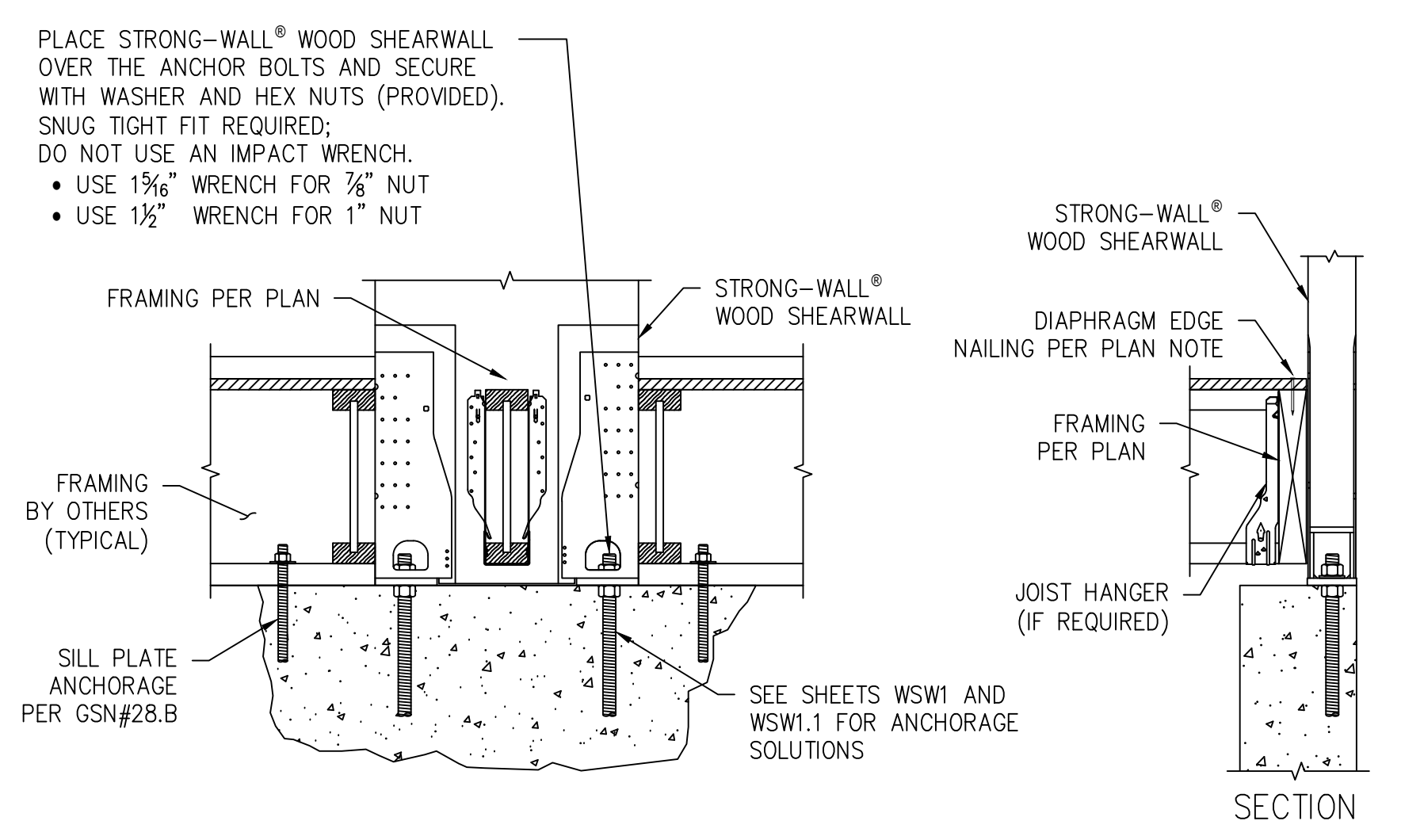
7 TOP OF WALL HEIGHT ADJUSTMENTS  
S6.6 1" = 1'-0"



WALL WIDTH (in.)	MODEL NO.	CONTENTS
12	WSW-RF-12KT	EACH KIT CONTAINS (1) SHEAR TRANSFER PLATE (2) 7/8" x 18" OR 1" x 18" THREADED RODS (ASTM A36) (2) COUPLER NUTS
18	WSW-RF-18KT	(2) HEAVY HEX NUTS
24	WSW-RF-24KT	INSTALLATION INSTRUCTIONS

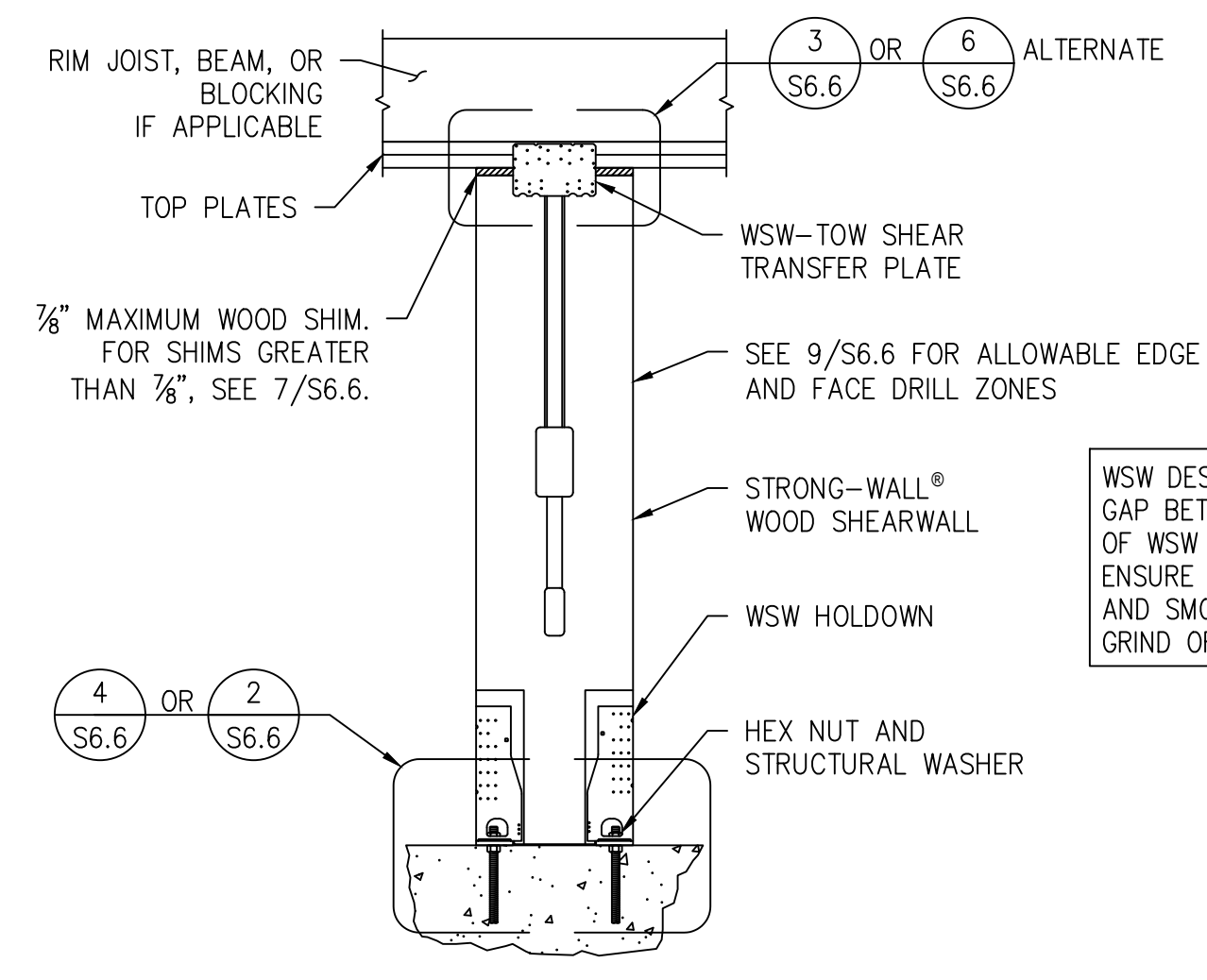
ORDER FIRST FLOOR CONNECTION KIT SEPARATELY. MODEL WSW-RF-\_\_KT. EXAMPLE WSW-RF-18KT

4 STANDARD INSTALLATION AT BASE OF STRONG-WALL  
S6.6 1" = 1'-0"

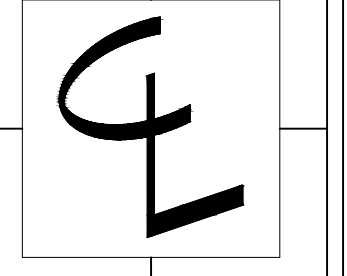
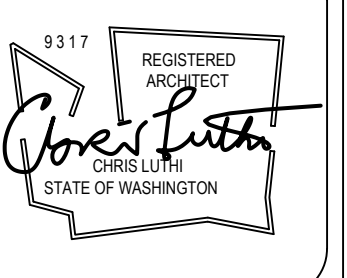


STRONG-WALL® WOOD SHEARWALL HEIGHT TO INCLUDE THE DEPTH OF THE FLOOR SYSTEM AND SHALL BE INSTALLED DIRECTLY ON THE FOUNDATION. SPECIFY PANEL HEIGHT FROM TOP OF FOUNDATION TO UNDERSIDE OF TOP PLATES OR BEAM.

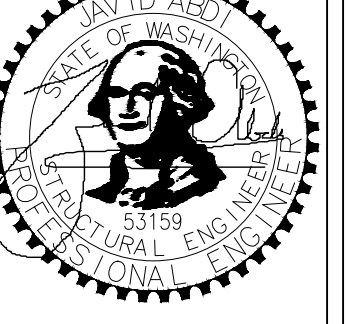
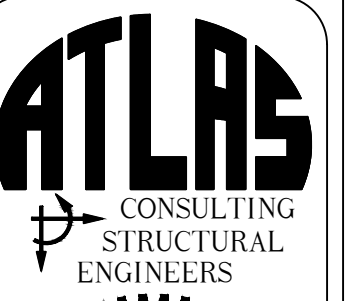
2 ALTERNATE INSTALLATION AT BASE OF STRONG-WALL  
S6.6 1" = 1'-0"



1 SINGLE STORY WSW ON CONCRETE  
S6.6 1" = 1'-0"



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Simpson Strong-Wall Details

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S6.6