

# Structural Engineer

All Japanese knotweed (Polygonum cuspidatum) and Regulated Class A, Regulated Class B, and Regulated Class C weeds identified on the King County Noxious Weed list, as amended, shall be removed from the property.

development proposals for a new single-family home shall remove japanese knotweed (polygonum cuspidatum) and regulated class a, regulated class b, and regulated class c weeds identified on the king county noxious weed list, as amended, from required landscaping areas established pursuant to subsection 19.02.020(f)(3)(a). new landscaping associated with new single-family home shall not incorporate any weeds identified on the king county noxious weed list, as amended. provided, that removal shall not be required if the removal will result in increased slope instability or risk of landslide or erosion.

	EL @ MIDPOINT	Segment	wia symm
Α	168.00	20.81	3496.08
A B	169.00	25.30	4275.70
С	169.00	21.40	3616.60
D	169.00	2.40	405.60
E F	169.00	24.70	4174.30
F	169.00	2.40	405.60
G	168.00	20.00	3360.00
Н	167.50	13.90	2328.25
	162.50	23.20	3770.00
J	159.00	12.25	1947.75
K	159.00	4.70	747.30
L	159.00	20.94	3329.46
M	159.00	4.70	747.30
N	159.60	15.06	2403.58
0	159.60	23.30	3718.68
Р	160.00	9.00	1440.00
Q	168.00	19.63	3297.84
R	168.00	12.81	2152.08
S	159.80	25.70	4106.86
T	167.50	12.81	2145.68
		315.01	51868.65

164.6572 AVG. EL =

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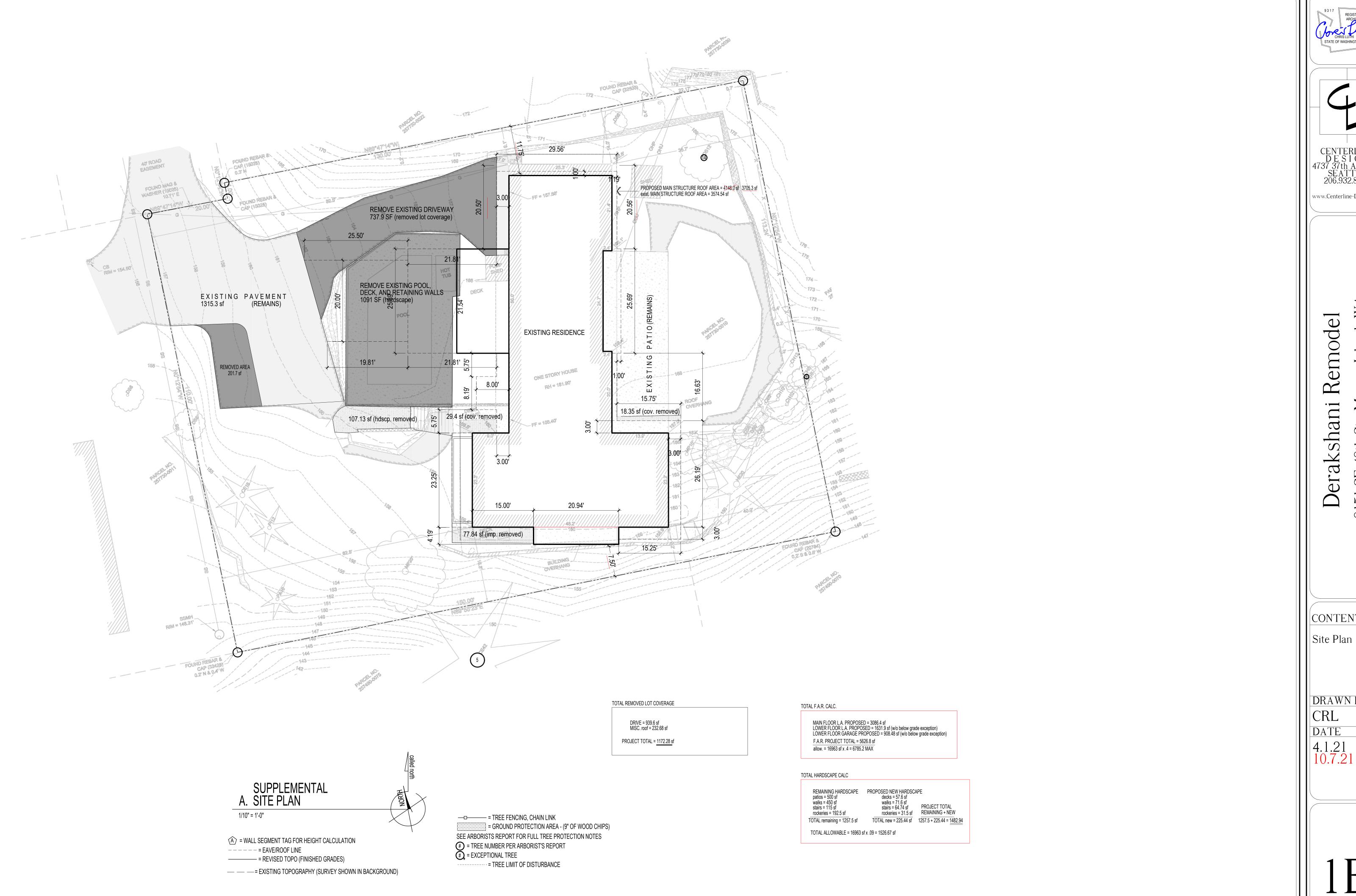
www.Centerline-Design.com

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CONTENTS Site Plan

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DATE 4.1.21 10.7.21



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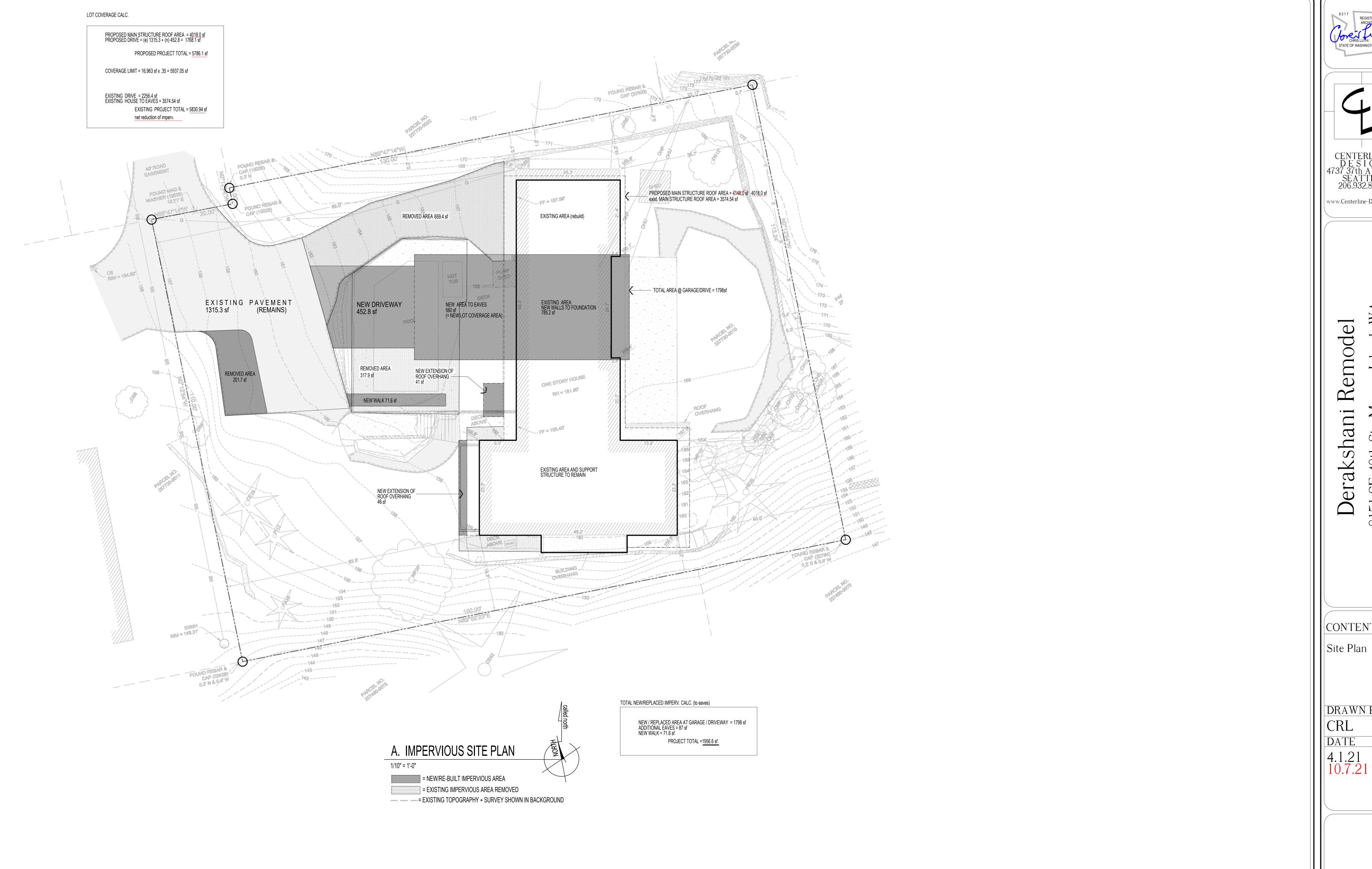
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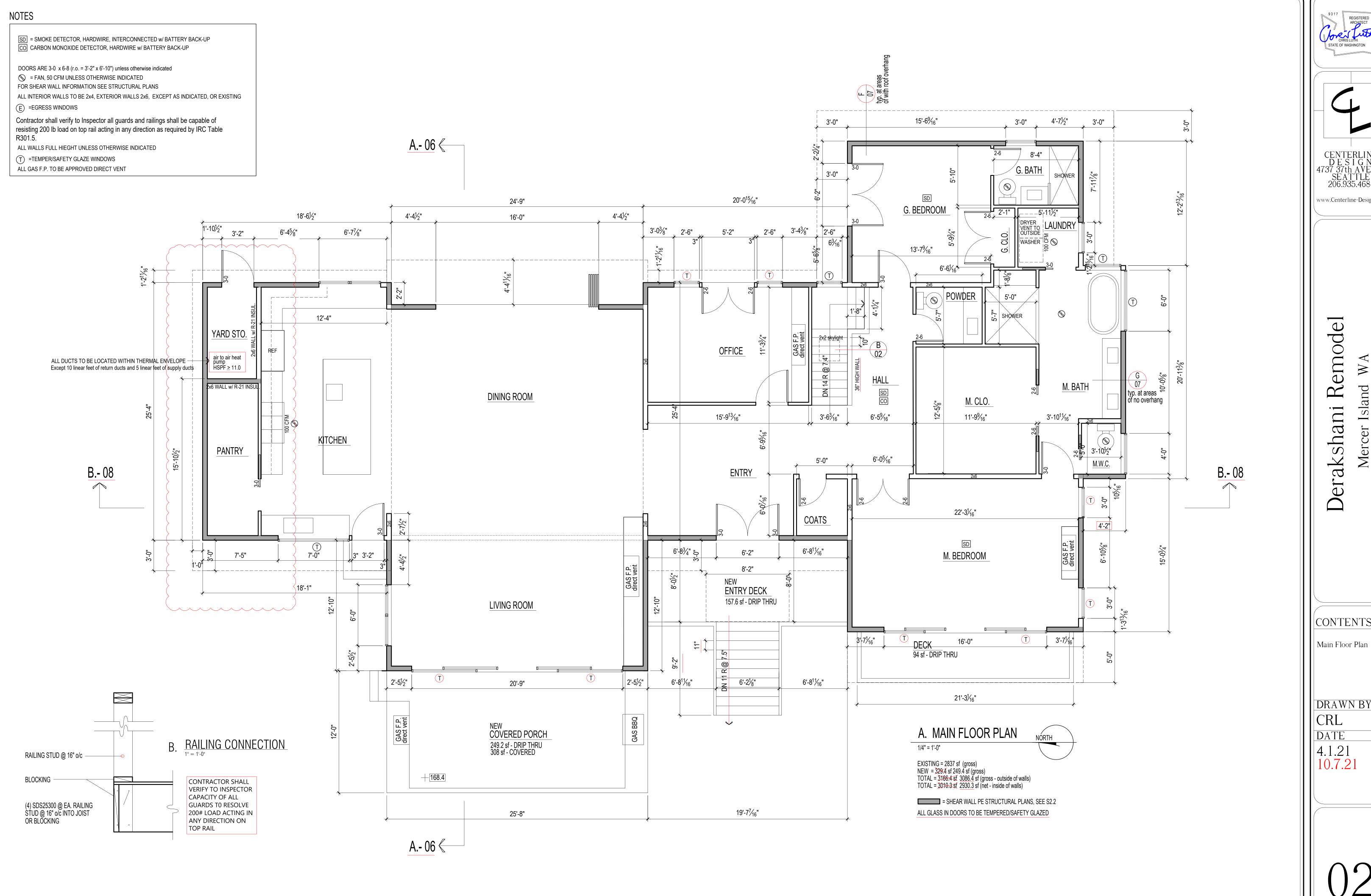
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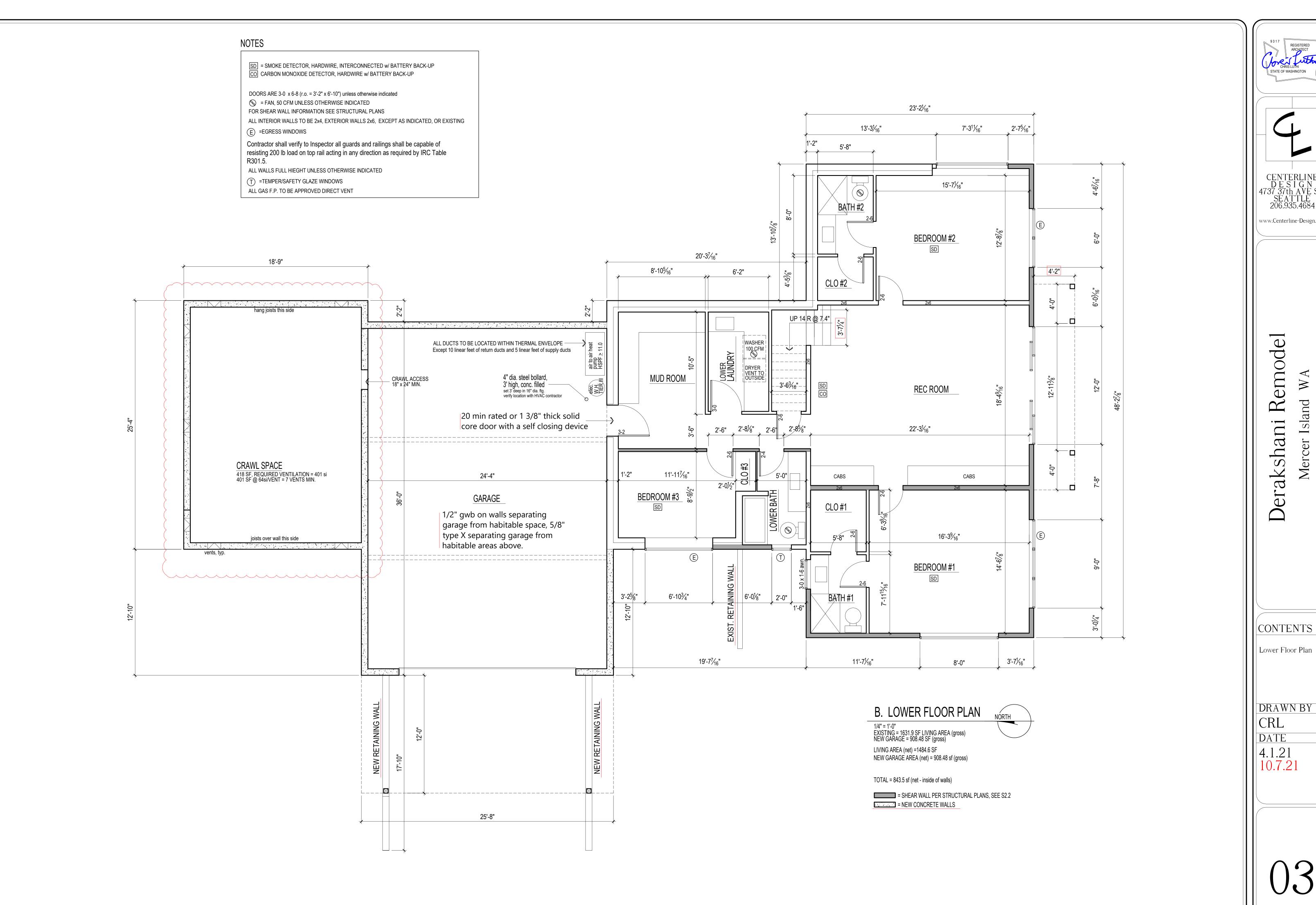
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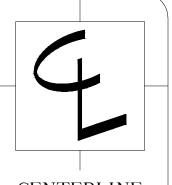
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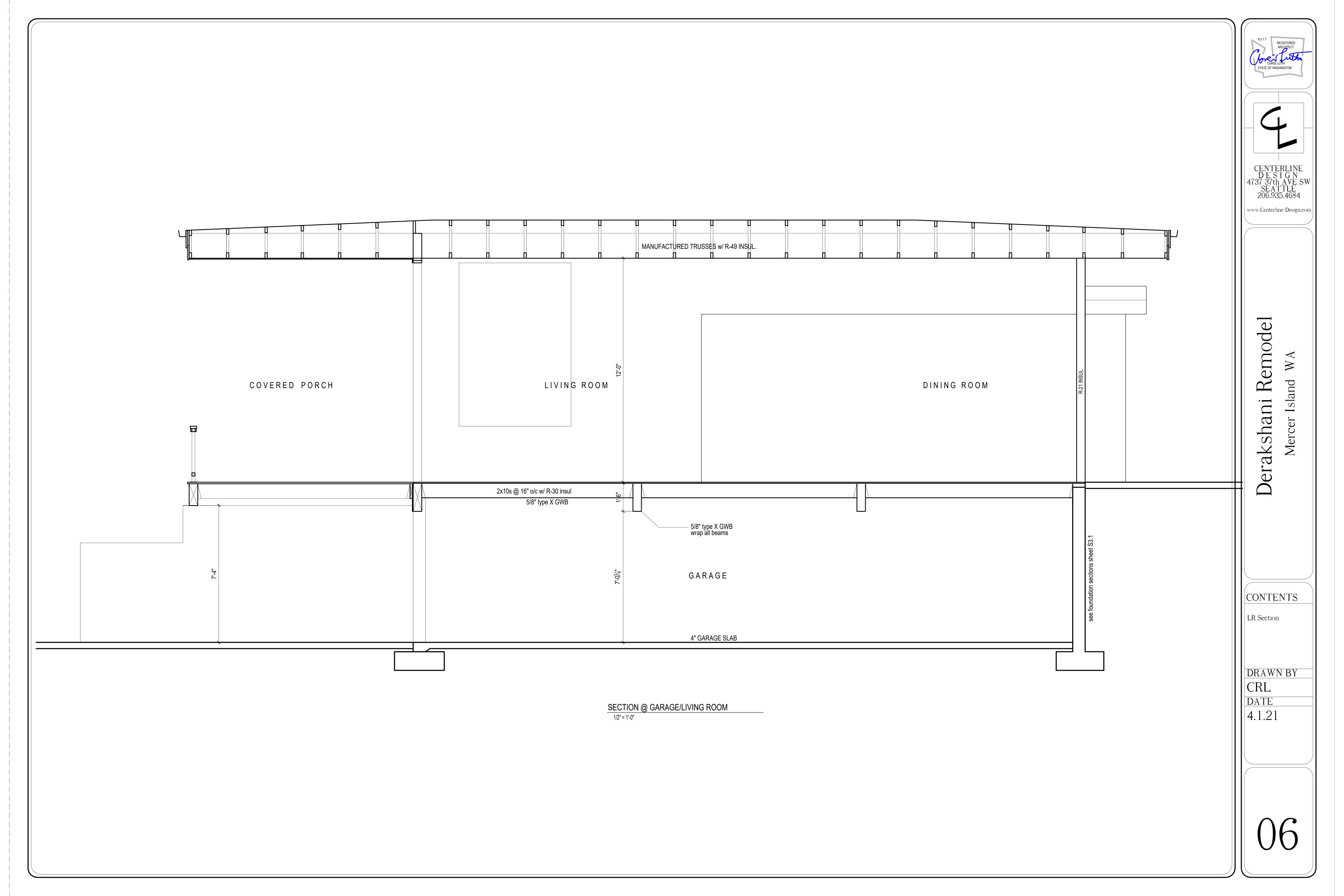
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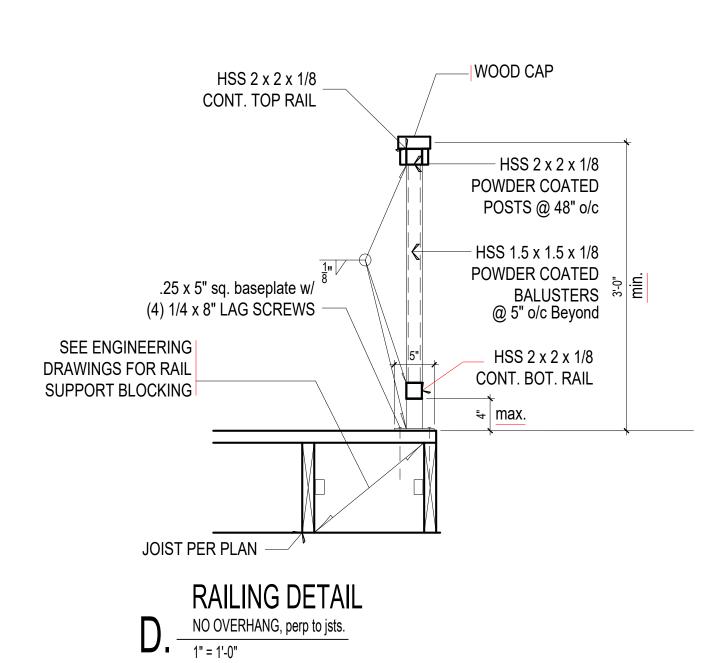
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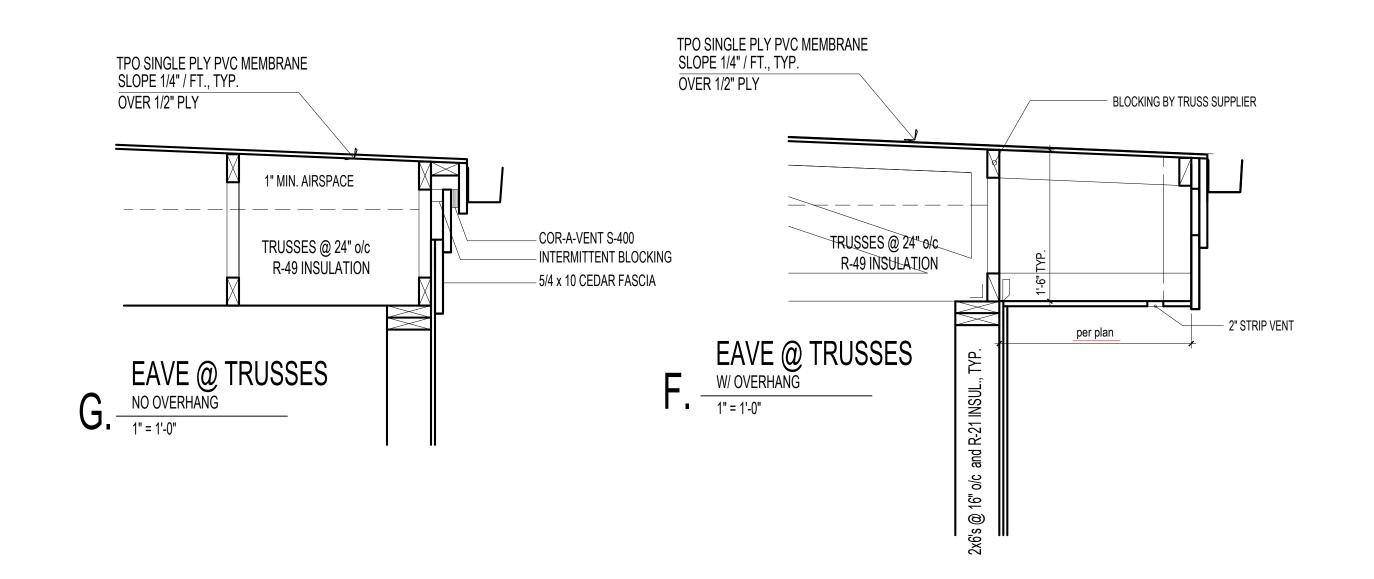
N and E Elevations

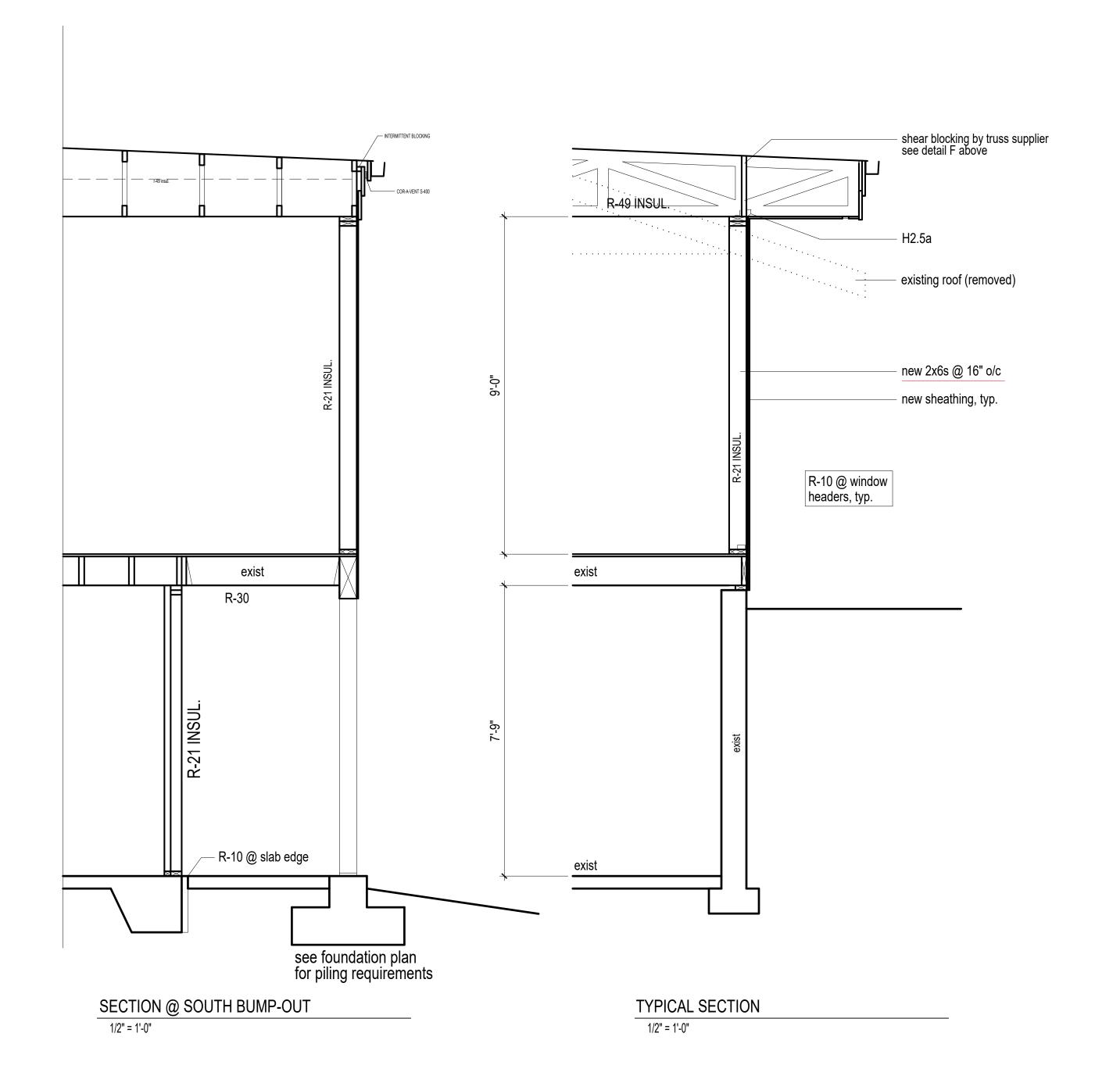
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DATE
4.1.21
10.7.21

4.1.21 10.7.21

07

2018 WA STATE PRESCRIPTIVE PATH FOR ALL CLIMATE ZONES
ENERGY CREDIT OPTIONS =
1.7(.5),2(1),2.1(.5),3.6(2),5.5(2) = 6 CREDITS
Vertical fenestration U = 0.30
Floor R-30

### PRIMARY RESIDENCE HVAC NOTES

DUCTED HEAT PUMP (HSPF>9.0) INT. AIR HANDLER INTEGRATED VENTILATION REQUIRED VENTILATION = CONTINUOUS 120CFM SET TO OPERATE AT 240 CFM FOR 2 HOURS IN EA. 4 HR PERIOD (50%) PROVIDED BY VARIABLE SPEED HIGH EFF. FAN (MAX .35 WATTS/CFM) CONTOLLED TO OPERATE AT LOW SPEED IN VENTILATION MODE ONLY.

design professional or builder shall complete and

post an "Insulation Certificate for Residential Construction" within 3' of the electrical panel prior to final inspection.

Maximum flow rates for shower heads and kitchen sink - 1.75 GPM or less. All other lavatory faucets - 1.0 GPM or less.

A minimum of 75 percent of permanently installed lamps in lighting fixtures shall be high-efficacy lamps.

Air leakage shall not exceed 3 air changes/ hour and shall be tested as such. A written report of the test results, shall be signed by the testing party and provided to the building inspector, prior to call for final inspection.

Per WSEC R402.4, The building thermal Envelope shall be constructed to limit air leakage to 3.0 air changes per hour maximum. The results of the test shall be signed by the party conducting the test and provided to the code official (R402.4.1.2). Per WSEC R403.1.1, at least one thermostat per dwelling unit shall be capable of controlling the heating and cooling system on a daily schedule. Per WSEC R403.2.2, Ducts, air handlers, and filter boxes shall be sealed. Per WSEC R404.1, A minimum of 75 percent of the lamps in permanently installed lighting fixtures shall be high-efficacy lamps.

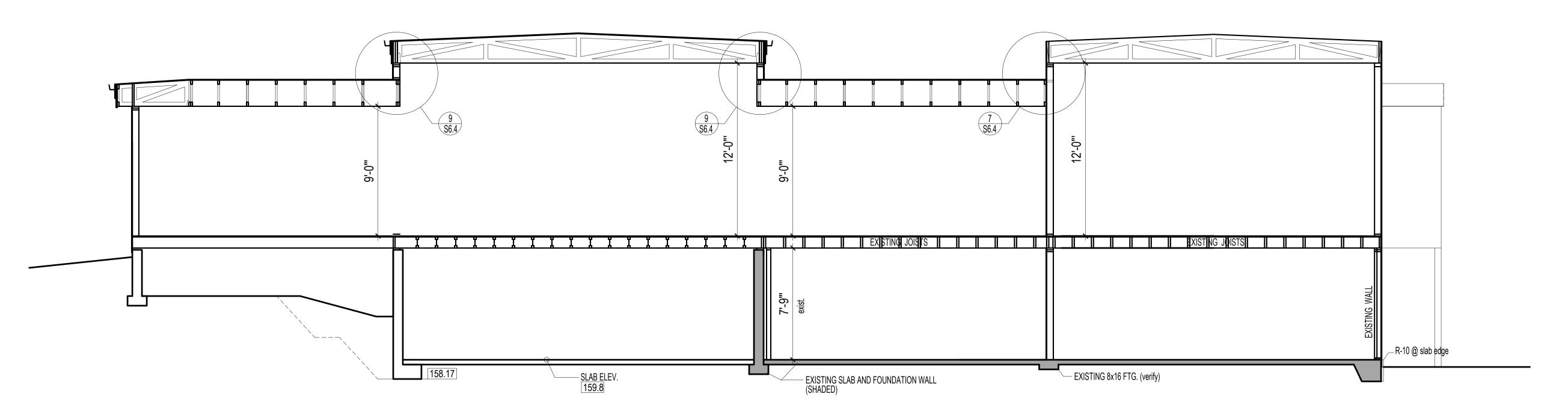
All Climate Zones (Table R402.1.1)				
	R-Value <sup>a</sup>	U-Factor <sup>a</sup>		
Fenestration U-Factor <sup>b</sup>	n/a	0.30		
Skylight U-Factor <sup>b</sup>	n/a	0.50		
Glazed Fenestration SHGC b,e	n/a	n/a		
Ceiling <sup>e</sup>	49	0.026		
Wood Frame Wall g,h	21 int	0.056		
Floor	30	0.029		
Below Grade Wall c,h	10/15/21 int + TB	0.042		
Slab <sup>d,f</sup> R-Value & Depth	10, 2 ft	n/a		

R-values are minimums. U-factors and SHGC are maximums. When insulation is installed in a cavity that is less than the label or design thickness of the insulation, the compressed R-value of the insulation from Appendix Table A101.4 shall not be less than the R-value specified in the table.

b The fenestration *U*-factor column excludes skylights.

"10/15/21 +5TB" means R-10 continuous insulation on the exterior of the wall, or R-15 continuous insulation on the interior of the wall, or R-21 cavity insulation plus a thermal break between the slab and the basement wall at

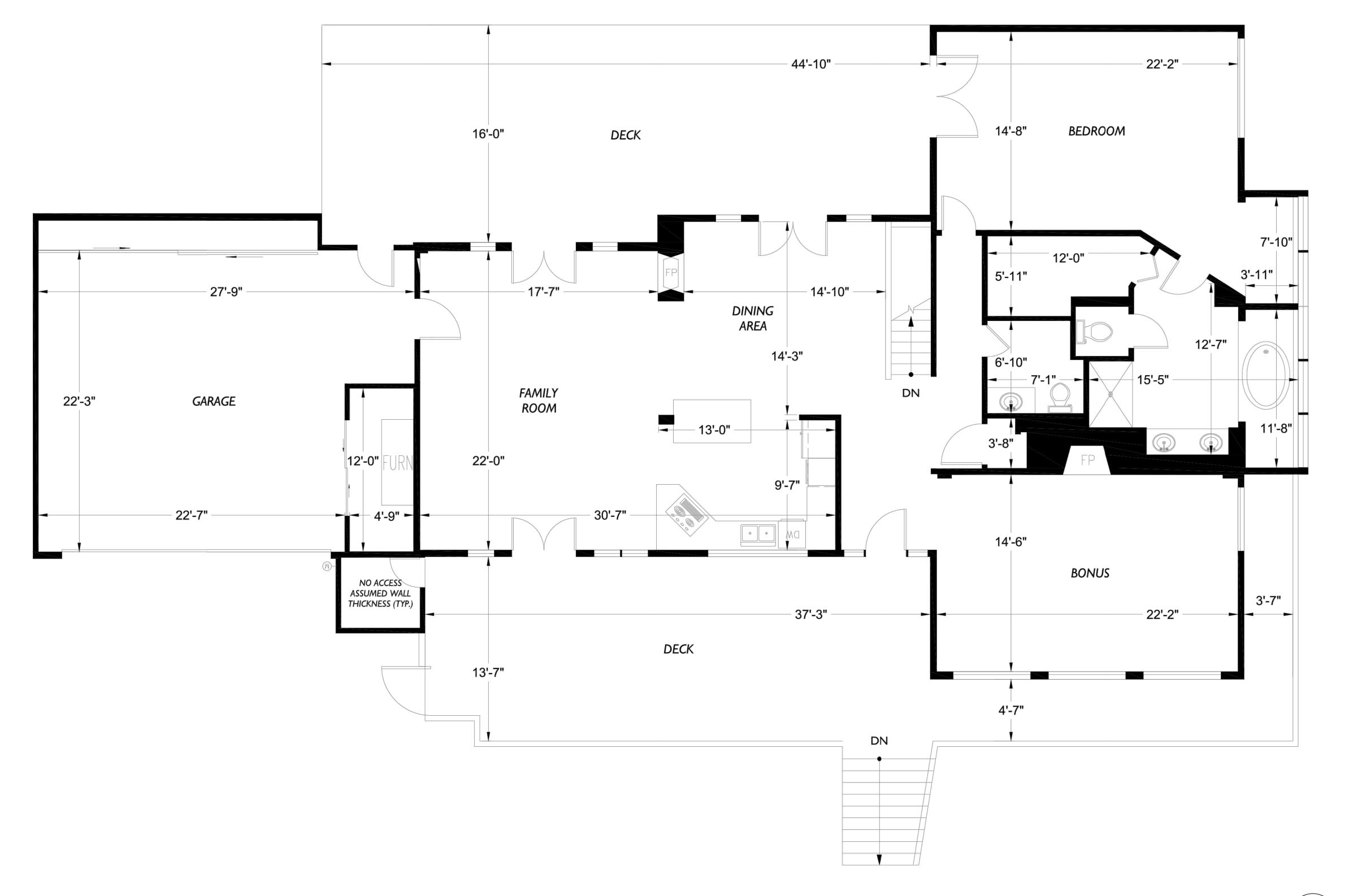
- the interior of the basement wall. "10/15/21 +5TB" shall be permitted to be met with R-13 cavity insulation on the interior of the basement wall plus R-5 continuous insulation on the interior or exterior of the wall. "5TB" means R-5 thermal break between floor slab and basement wall.
- d R-10 continuous insulation is required under heated slab on grade floors. See Section R402.2.9.1.
- e For single rafter- or joist-vaulted ceilings, the insulation may be reduced to R-38 if the full insulation depth extends over the top plate of the exterior wall.
- R-7.5 continuous insulation installed over an existing slab is deemed to be equivalent to the required perimeter slab insulation when applied to existing slabs complying with Section R503.1.1. If foam plastic is used, it shall meet the requirements for thermal barriers protecting foam plastics.
- For log structures developed in compliance with Standard ICC 400, log walls shall meet the requirements for climate zone 5 of ICC 400.
- Int. (intermediate framing) denotes framing and insulation as described in Section A103.2.2 including standard framing 16 inches on center, 78% of the wall cavity insulated and headers insulated with a minimum of R-10 insulation.



LONGITUDINAL SECTION B

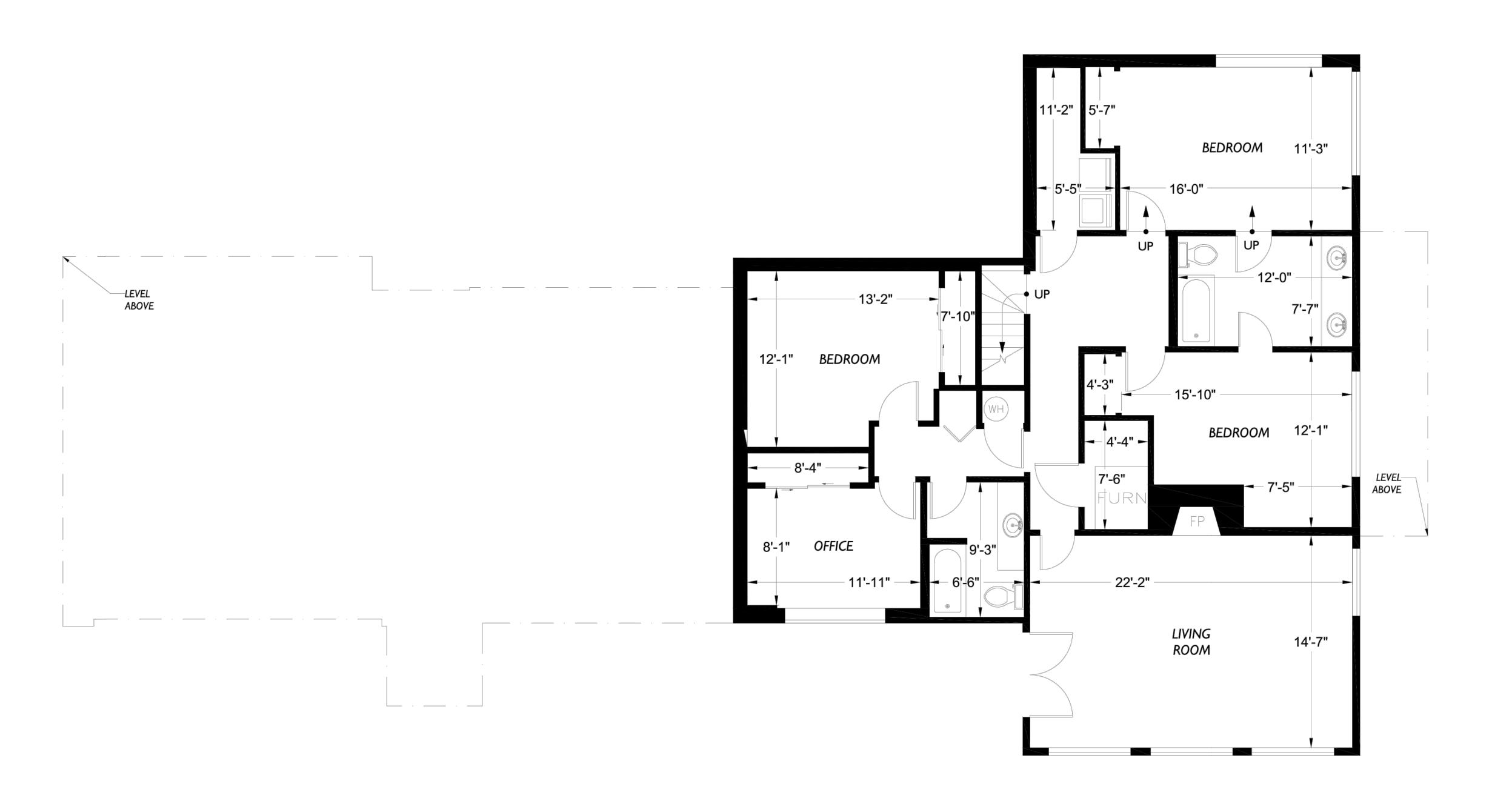
Shoring Layouts

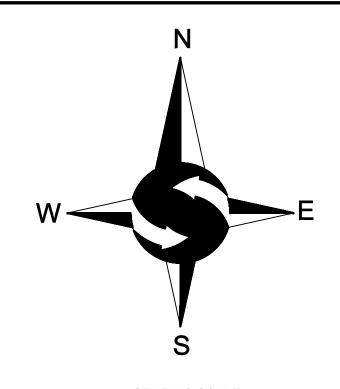
DATE 10.7.21



DATE 10.7.21







GRAPHIC SCALE
5 0 5 10 2

1INCH = 10 FT.

#### **LEGEND**

─ OHP─ OVERHEAD POWER FOUND MONUMENT AS DESCRIBED ─OHU─ OVERHEAD UTILITIES FOUND REBAR AS DESCRIBED —X— CHAINLINK FENCE FOUND MAG & WASHER —□— WOOD FENCE SET 5/8" X 24" IRON ROD W/1" YELLOW PLASTIC CAP CONCRETE WALL SET MAG NAIL AS DESCRIBED ROCKERY **GUY WIRE** POWER METER ASPHALT SURFACE UTILITY POLE CONCRETE SURFACE GAS METER SANITARY SEWER MANHOLE **GRAVEL SURFACE** CATCH BASIN APPROXIMATE LOCATION UNDERGROUND GAS LINE APPROXIMATE LOCATION SANITARY DS DECIDUOUS SEWER LINE APPROXIMATE LOCATION STORM HE HEMLOCK TIMBER WALL PI PINE

#### **LEGAL DESCRIPTION**

THAT PORTION OF GOVERNMENT LOT 7, SECTION 24, TOWNSHIP 24 NORTH, RANGE 4 EAST, WILLAMETTE MERIDIAN, IN KING COUNTY, WASHINGTON, DESCRIBED AS FOLLOWS: BEGINNING AT THE NORTHEAST CORNER OF GOVERMENT LOT 7 IN SAID SECTION 24, THENCE SOUTH 0°00'35" WEST ALONG THE EAST LINE THEREOF, 96 FEET; THENCE NORTH 89°33'45" WEST PARALLEL WITH THE NORTH LINE OF SAID SECTION, 208

FEET TO THE TRUE POINT OF BEGINNING; THENCE CONTINUING NORTH 89°33'45" WEST 130 FEET; THENCE SOUTH 0°00'35" WEST 4 FEET;

THENCE NORTH 89°33'45" WEST 20 FEET;

THENCE SOUTH 0°00'35" WEST 110.05 FEET TO A POINT BEARING NORTH 89°51'58" WEST FROM A POINT ON THE EAST LINE FO SAID SECTION 208.4 FEET SOUTH OF THE NORTHEAST CORNER THEREOF;
THENCE SOUTH 89°51'08" EAST TO A POINT BEARING SOUTH 0°00'35" WEST FROM THE

TRUE POINT OF BEGINNING;
THENCE NORTH 0°00'35" EAST 113.00 FEET, MORE OR LESS, TO THE TRUE POINT OF

BEGINNING;

TOGETHER WITH AN EASEMENT FOR ROAD PURPOSES OVER THE WEST 40 FEET OF THE EAST 378 FEET OF THE NORTH 100 FEET OF SAID SECTION 24.

## BASIS OF BEARINGS

RECORD OF SURVEY FOR HELEN SCHWEDENBERG BY NORTH POINTE SURVEYING AS RECORDED UNDER RECORDING NUMBER 20111108900002, RECORDS OF KING COUNTY, WASHINGTON.

#### PROJECT INFORMATION

SURVEYOR:

SITE SURVEYING, INC. 21923 NE 11TH ST SAMMAMISH, WA 98074 PHONE: 425.298.4412

\* INDICATES MULTI-TRUNK

PROPERTY OWNER: KAM DERAKSHANI 8151 SE 48TH STREET

MERCER ISLAND, WA 98040
TAX PARCEL NUMBER: 257730-0010

PROJECT ADDRESS: 8151 SE 48TH STREET
MERCER ISLAND, WA 98040

ZONING: R-

JURISDICTION: CITY OF MERCER ISLAND

PARCEL ACREAGE: 16,963 S.F. (0.389 ACRES) AS SURVEYED

#### **GENERAL NOTES**

MAY EXIST ON THIS SITE.

- THIS SURVEY WAS COMPLETED WITHOUT BENEFIT OF A CURRENT TITLE REPORT.
  EASEMENTS AND OTHER ENCUMBRANCES MAY EXIST ON THIS PROPERTY THAT
  ARE NOT SHOWN HEREON.
- INSTRUMENTATION FOR THIS SURVEY WAS A 3-SECOND SPECTRAPRECISION FOCUS 35 TOTAL STATION. PROCEDURES USED IN THIS SURVEY MEET OR EXCEED STANDARDS SET BY WAC 332-130-090.

THE INFORMATION ON THIS MAP REPRESENTS THE RESULTS OF A SURVEY MADE IN AUGUST 2020 AND CAN ONLY BE CONSIDERED AS INDICATING THE GENERAL

- CONDITIONS EXISTING AT THAT TIME.

  4. UTILITIES SHOWN ON THIS SURVEY ARE BASED UPON ABOVE GROUND OBSERVATIONS AND AS-BUILT PLANS WHERE AVAILABLE. ACTUAL LOCATIONS OF UNDERGROUND UTILITIES MAY VARY AND UTILITIES NOT SHOWN ON THIS SURVEY
- 5. ALL MONUMENTS WERE LOCATED DURING THIS SURVEY UNLESS OTHERWISE

#### **VERTICAL DATUM & CONTOUR INTERVAL**

DATUM

ELEVATIONS SHOWN ON THIS DRAWING WERE DERIVED FROM INFORMATION PROVIDED BY WCCS SURVEY CONTROL DATABASE.

THE MARK IS A MONUMENT IN CASE AT THE NE CORNER OF SECTION 24.

POINT ID NO. 8; ELEVATION: 202,49 FEET NAVD 88

1.0' CONTOUR INTERVAL - THE EXPECTED VERTICAL
ACCURACY IS EQUAL TO 1/2 THE CONTOUR INTERVAL OR PLUS
/ MINUS 0.5' FOR THIS PROJECT.



VICINITY MAP

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DATE REVISION DRN

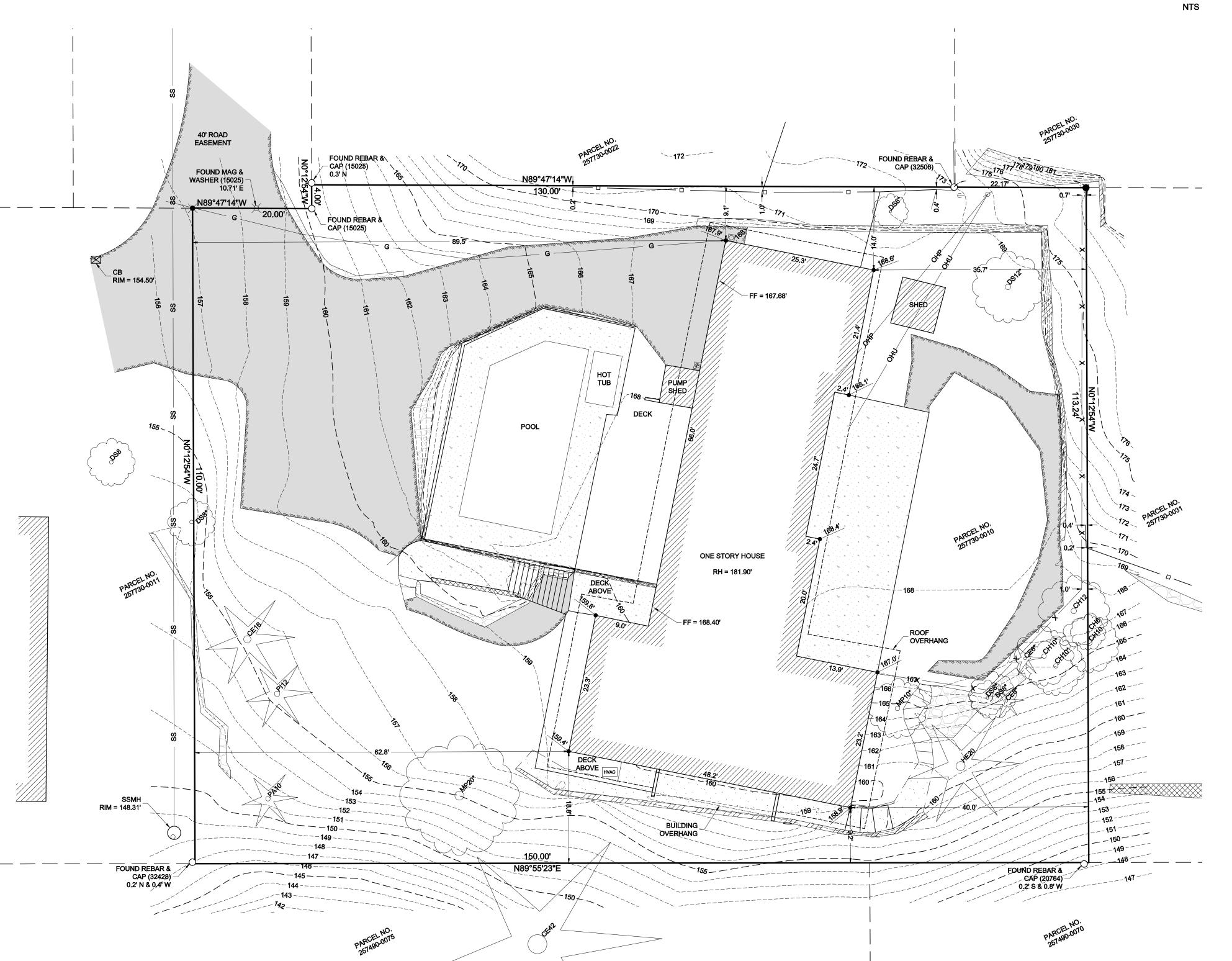
OGRAPHIC SURVEY
FRANK IMANI
8151 SE 48TH STREET

PROJECT NO. 20-346

DRAWN BY: MTS
CHECKED BY: TNW
DATE: 8/18/2

SHEET

8/18/2020 1 OF 1



## General Structural Notes (GSN's)

1. ALL MATERIALS, WORKMANSHIP, DESIGN, AND CONSTRUCTION SHALL CONFORM TO THE DRAWINGS,

SPECIFICATIONS, AND THE INTERNATIONAL BUILDING CODE (IBC) WITH WASHINGTON STATE ADMINISTRATIVE

CODE AMENDMENTS, 2018 EDITION.

 2. DESIGN LOADING CRITERIA

 RISK CATEGORY IBC TABLE 1604.5
 II

 ROOF SNOW LOAD
 25 PSF (I<sub>S</sub> = 1.0)

 ROOF RAIN ON SNOW SURCHARGE
 5 PSF 1

 ROOF DEAD LOAD
 20 PSF

 LIVE LOAD
 40 PSF

 DECK LIVE LOAD
 60 PSF

 FLOOR DEAD LOAD
 25 PSF

3. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ALL OTHER CONTRACT DOCUMENTS FOR BIDDING AND CONSTRUCTION. CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS FOR COMPATIBILITY AND SHALL NOTIFY ENGINEER OF ALL DISCREPANCIES PRIOR TO CONSTRUCTION. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE BUILDING LAYOUT DIMENSIONS (GRID LAYOUTS, SITE COORDINATES, ETC.) AMONGST ALL TRADES, INCLUDING SHOP FABRICATED ITEMS.

4. CONTRACTOR SHALL PROVIDE TEMPORARY BRACING, BOTH FOR VERTICAL LOADS AND LATERAL STABILITY, FOR THE STRUCTURE AND STRUCTURAL COMPONENTS UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE DRAWINGS.

5. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES REQUIRED TO PERFORM THE WORK.

6. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER.

7. ALL STRUCTURAL SYSTEMS COMPOSED OF COMPONENTS TO BE FIELD ERECTED SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH INSTRUCTIONS PREPARED BY THE SUPPLIER.

8. SEISMIC BRACING AND/OR GRAVITY SUPPORT AND ANCHORAGE OF ALL MECHANICAL OR ELECTRICAL EQUIPMENT SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF WASHINGTON, EXCEPT FOR ELEMENTS SPECIFICALLY SHOWN AND DETAILED ON THE STRUCTURAL DRAWINGS. THE MECHANICAL/ELECTRICAL CONTRACTOR MUST HIRE THE ENGINEER AND IS RESPONSIBLE FOR ALL COSTS RELATED TO THE PURCHASE AND INSTALLATION OF NECESSARY SUPPORTS, BRACING AND ANCHORAGE. SEISMIC BRACING AND ANCHORAGE DESIGN AND CONSTRUCTION SHALL COMPLY WITH CHAPTER 13 OF ASCE 7–10.

9. SHOP DRAWING REVIEW: SHOP DRAWINGS FOR TRUSSES SHALL BE SUBMITTED TO THE CONTRACTOR, ARCHITECT, AND ENGINEER OF RECORD FOR REVIEW PRIOR TO FABRICATION OF THESE ITEMS. DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE ENGINEER OF RECORD, AND THEREFORE MUST BE VERIFIED BY THE CONTRACTOR. THE CONTRACTOR SHALL REVIEW AND STAMP DRAWINGS PRIOR TO REVIEW BY ENGINEER OF RECORD. SUBMITTALS SHALL INCLUDE A REPRODUCIBLE AND ONE COPY. THE REPRODUCIBLE SHALL BE MARKED AND RETURNED. SHOP DRAWING SUBMITTALS PROCESSED BY THE ENGINEER ARE NOT CHANGE ORDERS. THE PURPOSE OF SHOP DRAWING SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE ENGINEER THAT THE CONTRACTOR UNDERSTANDS THE DESIGN CONCEPT BY INDICATING WHICH MATERIAL IS INTENDED TO BE FURNISHED AND INSTALLED AND BY DETAILING THE INTENDED FABRICATION AND INSTALLATION METHODS. IF DEVIATIONS, DISCREPANCIES, OR CONFLICTS BETWEEN SHOP DRAWING SUBMITTALS AND THE CONTRACT DOCUMENTS ARE DISCOVERED EITHER PRIOR TO OR AFTER SHOP DRAWING SUBMITTALS ARE PROCESSED BY THE ENGINEER, THE DESIGN DRAWINGS AND SPECIFICATIONS SHALL CONTROL AND SHALL BE FOLLOWED.

10. DEFERRED SUBMITTALS SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE OF WASHINGTON. THE COMPONENT DESIGNER SHALL BE A REGISTERED STRUCTURAL ENGINEER IF REQUIRED BY THE BUILDING OFFICIAL OF THE LOCAL JURISDICTION. BUILDING COMPONENT SUBMITTALS SHALL INCLUDE THE DESIGNING PROFESSIONAL ENGINEER'S STAMP AND SHALL BE APPROVED BY THE COMPONENT DESIGNER PRIOR TO CURSORY REVIEW BY THE ENGINEER OF RECORD FOR LOADS IMPOSED ON THE BASIC STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE INCLUDING ACCOMMODATION FOR STRUCTURAL DISPLACEMENT PER ASCE 7–10 SECTION 13.3.2. AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. DEFERRED SUBMITTALS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON BASIC STRUCTURE. DESIGN CALCULATIONS SHALL BE INCLUDED IN THE SUBMITTAL. THE CONTRACTOR SHALL FORWARD DEFERRED SUBMITTALS TO THE BUILDING OFFICIAL AND HAVE THE DEFERRED SUBMITTALS ON SITE FOR THE GOVERNING JURISDICTIONS INSPECTORS USE AND REFERENCE. THE FOLLOWING BUILDING COMPONENTS SHALL BE DEFERRED SUBMITTALS FOR THIS PROJECT:

PREFABRICATED CONNECTOR PLATE WOOD ROOF TRUSSES (SEE NOTE 23)

GEOTECHNICAL:

11. FOUNDATION NOTES: SUBGRADE PREPARATION INCLUDING DRAINAGE, EXCAVATION, COMPACTION, AND FILLING REQUIREMENTS, SHALL CONFORM STRICTLY WITH THE RECOMMENDATIONS GIVEN IN THE SPECIFICATIONS OR AS DIRECTED BY THE OWNER APPOINTED GEOTECHNICAL ENGINEER. FOOTINGS SHALL BEAR ON FIRM, UNDISTURBED EARTH OR CONTROLLED, COMPACTED STRUCTURAL FILL AT LEAST 18"

BELOW LOWEST ADJACENT FINISHED GRADE. THE OWNER APPOINTED GEOTECHNICAL ENGINEER SHALL APPROVE FOOTING EXCAVATION/PREPARATION PRIOR TO PLACEMENT OF ALL FOOTINGS. BACKFILL BEHIND ALL RETAINING WALLS WITH FREE DRAINING, GRANULAR FILL AND PROVIDE FOR SUBSURFACE DRAINAGE AS NOTED IN THE SPECIFICATIONS OR AS DIRECTED BY THE OWNER APPOINTED GEOTECHNICAL ENGINEER

Critical Area Study...Project JN21062...March 29, 2021"

11b. PIPE PILES SHALL BE 2"Ø EXTRA-STRONG STEEL WITH AN ALLOWABLE COMPRESSIVE LOAD OF 3-TONS. INSTALLATION, FINAL PENETRATION RATE, FINISH, CONNECTION, ETC. SHALL CONFORM STRICTLY WITH THE RECOMMENDATIONS GIVEN IN THE ABOVE GEOTECHNICAL REPORT REFERENCE. PIPES SHALL BE DRIVEN TO REFUSAL USING A METHOD APPROVED BY THE PROJECT GEOTECHNICAL ENGINEER. ACTUAL LENGTH OF PILES TO ACHIEVE RECOMMENDED REFUSAL RATE SHALL NOT BE LESS THAN 7 FEEL BELOW THE EXISTING GRADE PER GEOTECHNICAL REPORT. PIPE PILE DEPTHS ARE SUBJECT TO ON—SITE VERIFICATION AND APPROVAL BY THE PROJECT GEOTECHNICAL ENGINEER. BATTERED PILES SHALL BE BATTERED DOWN TOWARD THE SOUTH AT A 1:5 (H:V) INCLINATION. DUE TO THE GROUND SURFACE SLOPING AWAY FROM THE SOUTHERN EDGE OF THE RESIDENCE, NO PASSIVE PRESSURE WAS ACCOUNTED FOR AGAINST THE PILE CAPS/GRADE BEAMS FOR THE SOUTHERN BUMP OUT ADDITION. THE LATERAL CAPACITY OF A BATTERED PILE IS EQUAL TO ONE—HALF OF THE LATERAL COMPONENT OF THE ALLOWABLE COMPRESSIVE LOAD, WITH A MAXIMUM ALLOWABLE LATERAL CAPACITY OF 500 POUNDS. THE ALLOWABLE VERTICAL

CAPACITY OF BATTERED PILES DOES NOT NEED TO BE REDUCED IF THE PILES ARE BATTERED STEEPER

12. DRIVE PINS AND OTHER POWDER-ACTUATED FASTENERS SHALL BE ONE OF THE FOLLOWING INSTALLED IN STRICT ACCORDANCE WITH THE ICC-ES REPORTS INDICATED AND MANUFACTURER'S INSTRUCTIONS INCLUDING MINIMUM EMBED REQUIREMENTS: "TE SERIES" (0.157" DIAMETER) AS MANUFACTURED BY ITW RAMSET (ICC-ES NO. 1799); OR "X-U" (0.157" DIAMETER) AS MANUFACTURED BY HILTI, INC. (ICC-ES NO. 2269); OR "STRONG-TIE PDPA" (0.157" DIAMETER) AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC. (ICC-ES NO. 2138); OR "CSI PIN" (0.157" DIAMETER) AS MANUFACTURED BY DEWALT/POWERS (ICC-ES NO. 2024); OR AN APPROVED EQUIVALENT IN STRENGTH AND EMBEDMENT. MINIMUM EMBEDMENT IN CONCRETE SHALL BE 1" UNLESS OTHERWISE NOTED. MAINTAIN AT LEAST

THAN 1:5 (HORIZONTAL: VERTICAL).

3-1/2" TO NEAREST CONCRETE EDGE.

<u>ANCHORAGE:</u>

CONCRETE:

13. CONCRETE SHALL BE MIXED, PROPORTIONED, CONVEYED AND PLACED IN ACCORDANCE WITH ACI 318–14 CHAPTER 26 AND ACI 301. CONCRETE SHALL ATTAIN A 28-DAY STRENGTH OF f'c = 4,000 PSI (4,500 PSI AT ALL CONCRETE EXPOSED TO WEATHER). MAXIMUM WATER-CEMENTITIOUS MATERIAL RATIO FOR INTERIOR SLABS SHALL BE BETWEEN 0.40 AND 0.44. ALL CONCRETE SHALL BE EXPOSURE CLASSES FO, SO, WO, AND CO PER ACI 318–14 TABLES 19.3.1.1 AND 19.3.2.1 EXCEPT AS NOTED BELOW.

ALL CONCRETE EXPOSED TO EARTH (FOUNDATIONS, ETC.): (FO, SO, WO, C1)

ALL CONCRETE EXPOSED TO WEATHER: (F1, S0, W0, C1)

SEE SPECIFICATIONS FOR SHRINKAGE REDUCING CONCRETE MIX CRITERIA WHERE INDICATED ON DRAWINGS.

CONCRETE MIXES SHALL MEET OR EXCEED THE REQUIREMENTS SPECIFIED ABOVE. MIXES SHALL BE

SUBMITTED TO THE ENGINEER AND BUILDING OFFICIAL FOR APPROVAL TWO WEEKS PRIOR TO PLACING

ANY CONCRETE AND SHALL INCLUDE THE AMOUNTS OF CEMENT, CEMENTITOUS MATERIAL, FINE AND

COARSE AGGREGATE, WATER AND ADMIXTURES, AS WELL AS THE WATER—CEMENT RATIO, SLUMP,

CONCRETE YIELD AND SUBSTANTIATING STRENGTH DATA IN ACCORDANCE WITH ACI 318—14, CHAPTER 26

AND 27. REVIEW OF MIX SUBMITTALS BY THE ENGINEER OF RECORD INDICATES ONLY THAT INFORMATION

PRESENTED CONFORMS GENERALLY WITH CONTRACT DOCUMENTS. CONTRACTOR OR SUPPLIER MAINTAINS

FULL RESPONSIBILITY FOR SPECIFIED PERFORMANCE.

14. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, fy = 60,000 PSI. GRADE 60 REINFORCING BARS WHICH ARE TO BE WELDED SHALL CONFORM TO ASTM A706. REINFORCEMENT COMPLYING WITH ASTM A615(S1) MAY BE WELDED ONLY IF MATERIAL PROPERTY REPORTS INDICATING CONFORMANCE WITH WELDING PROCEDURES SPECIFIED IN A.W.S. D1.4 ARE SUBMITTED. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064.

15. REINFORCING STEEL SHALL BE DETAILED (INCLUDING HOOKS AND BENDS) IN ACCORDANCE WITH ACI 315-99 AND 318-14. LAP ALL CONTINUOUS REINFORCEMENT IN ACCORDANCE WITH "REINFORCEMENT SPLICE AND DEVELOPMENT LENGTH SCHEDULE" OF 10/S3.1. PROVIDE CORNER BARS AT ALL WALL AND FOOTING INTERSECTIONS. LAP ADJACENT MATS OF WELDED WIRE FABRIC A MINIMUM OF 12" AT SIDES AND ENDS. NO BARS PARTIALLY EMBEDDED IN HARDENED CONCRETE SHALL BE FIELD BENT UNLESS OTHERWISE NOTED ON THE DRAWINGS OR APPROVED BY THE STRUCTURAL ENGINEER.

17. BONDING AGENT SHALL BE "MASTEREMACO ADH 326" BY BASF CORPORATION. OR EQUIVALENT, AND SHALL BE USED WHERE NEW CONCRETE IS PLACED AGAINST HARDENED CONCRETE. PLACE IN STRICT ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS, INCLUDING PREPARATION OF EXISTING SURFACES. CONCRETE SHALL BE CONSIDERED HARDENED AFTER 56 DAYS.

18. NON-SHRINK GROUT SHALL BE FURNISHED BY AN APPROVED MANUFACTURER AND SHALL BE MIXED AND PLACED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. GROUT STRENGTH SHALL BE AT LEAST EQUAL TO THE MATERIAL ON WHICH IT IS PLACED (6,000 PSI MINIMUM).

19. FRAMING LUMBER SHALL BE KILN DRIED OR MC-19, AND GRADED AND MARKED IN CONFORMANCE WITH W.C.L.I.B. STANDARD GRADING RULES FOR WEST COAST LUMBER NO. 17 OR W.W.P.A. WESTERN LUMBER GRADING RULES. FURNISH TO THE FOLLOWING MINIMUM STANDARDS:

PLATES, LEDGERS & MISC.

DOUGLAS FIR NO. 3 OR STUD GRADE

LIGHT FRAMING:

MIN. BASIC DESIGN STRESS, F<sub>b</sub> = 525 PSI, E = 1400 KSI

20. MANUFACTURED LUMBER SHALL BE AS MANUFACTURED BY TRUS JOIST OR APPROVED EQUAL. REQUESTS FOR APPROVAL AS EQUAL WILL REQUIRE SUBMITTAL OF ICC REPORT EQUIVALENT TO ESR-1387 FOR LAMINATED VENNER LUMBER (LVL, LAMINATED STRAND LUMBER (LSL), OR PARALLEL STRAND LUMBER (PSL). THE MINIMUM ALLOWABLE DESIGN VALUES ARE AS FOLLOWS:

 $LVL - F_b = 2,600$   $F_v = 290 \text{ PSI}$  E = 2,000,000 PSI  $LSL - F_b = 1,900$   $F_v = 150 \text{ PSI}$  E = 1,300,000 PSI

21. ENGINEERED WOOD I—JOISTS SHALL BE FURNISHED AND INSTALLED IN CONFORMANCE WITH THE MANUFACTURER'S INSTRUCTIONS. ALL NECESSARY BRIDGING, BLOCKING, BLOCKING PANELS, STIFFENERS, ETC., SHALL BE DETAILED AND FURNISHED BY THE MANUFACTURER. PERMANENT AND TEMPORARY BRIDGING SHALL BE INSTALLED IN CONFORMANCE WITH MANUFACTURER'S INSTRUCTIONS. ALL JOIST HANGERS AND OTHER HARDWARE SHALL BE COMPATIBLE IN SIZE WITH ENGINEERED WOOD I—JOISTS PROVIDED. DESIGN SHOWN ON THE DRAWINGS IS BASED ON RESIDENTIAL JOISTS MANUFACTURED BY WEYERHAUSER IN ACCORDANCE WITH ICC—ES REPORT NO. ESR—1153. ALTERNATE ENGINEERED WOOD I—JOISTS MAY BE USED SUBJECT TO REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER OF RECORD

22. GLUED LAMINATED MEMBERS SHALL BE FABRICATED IN CONFORMANCE WITH ASTM AND A.I.T.C. STANDARDS IN ACCORDANCE WITH IBC SECTION 2303.1.3. EACH MEMBER SHALL BEAR AN A.I.T.C. IDENTIFICATION MARK AND SHALL BE ACCOMPANIED BY AN A.I.T.C. CERTIFICATE OF CONFORMANCE. HORIZONTAL MEMBERS AND INCLINED MEMBERS OF LESS THAN 1:1 SLOPE SHALL HAVE A RADIUSED CAMBER OF 3,500 FT. UNLESS OTHERWISE NOTED.

SIMPLE SPAN BEAMS DOUGLAS FIR COMBINATION 24F-VR

 $F_b = 2400$  PSI;  $F_v = 265$  PSI; E = 1,800,000 PSI GLUED LAMINATED MEMBERS EXPOSED TO WEATHER OR MOISTURE SHALL BE TREATED WITH A NON-CORROSIVE, APPROVED PRESERVATIVE.

WIND UPLIFT (TOP CHORD)

PRESSURE-TREATED MEMBERS.

23. PREFABRICATED CONNECTOR PLATE WOOD ROOF TRUSSES SHALL BE DESIGNED BY THE MANUFACTURER IN ACCORDANCE WITH ANSI/TPI I-2007 AND IBC SECTION 2303.4 FOR THE SPANS AND CONDITIONS SHOWN ON THE DRAWINGS.

DESIGN LOADS SHALL BE AS FOLLOWS:

TOP CHORD LIVE LOAD

BOTTOM CHORD LIVE LOAD

TOP CHORD DEAD LOAD

TOP CHORD DEAD LOAD

15 PSF

BOTTOM CHORD DEAD LOAD

5 PSF

THE TRUSS MANUFACTURER SHALL COORDINATE LOCATIONS AND SUPPORT CONFIGURATIONS OF PLUMBING, MECHANICAL UNITS, DUCTS, AND/OR OTHER MISCELLANEOUS ITEMS WITH THE CONTRACTOR PRIOR TO TRUSS FABRICATION. THE TRUSS MANUFACTURER SHALL DESIGN TRUSSES TO SUPPORT ALL LOADS ASSOCIATED WITH SUCH ITEMS. THE TRUSS SHOP DRAWINGS SHALL INCLUDE ALL DESIGN LOADS AND APPROVED HANGER CONNECTION DETAILS TO TRUSS CHORDS FOR SUPPORT OF HUNG MECHANICAL SYSTEM COMPONENTS AS APPLICABLE.

SEE NOTE#2 COMPONENTS & CLADDING ROOF LOADS

WOOD TRUSSES SHALL UTILIZE APPROVED CONNECTOR PLATES (GANGNAIL OR EQUAL). SHOP DRAWINGS AND CALCULATIONS SHALL BE PROVIDED AS A DEFERRED SUBMITTAL TO THE CONTRACTOR AND STRUCTURAL ENGINEER OF RECORD PER GENERAL STRUCTURAL NOTE 13. SHOP DRAWINGS SHALL INDICATE SHAPES, BEARING POINTS, INTERSECTIONS, HIPS, VALLEYS, ETC. EXACT COMPOSITION OF SPECIAL HIP, VALLEY, AND INTERSECTION AREAS (USE OF GIRDER TRUSSES, JACK TRUSSES, STEP—DOWN TRUSSES, ETC.) SHALL BE DETERMINED BY THE MANUFACTURER UNLESS OTHERWISE NOTED ON THE DRAWINGS. THE TRUSS MANUFACTURER SHALL PROVIDE ALL TRUSS—TO—TRUSS BEAM/JOIST CONNECTION DETAILS AND REQUIRED CONNECTION MATERIALS. THE TRUSS MANUFACTURER SHALL DESIGN AND PROVIDE DETAILS FOR ALL TEMPORARY AND PERMANENT TRUSS BRACING AND BRIDGING.

24. ROOF & WALL SHEATHING SHALL BE APA RATED, EXTERIOR OR EXPOSURE 1 PLYWOOD OR ORIENTED STRAND BOARD (OSB) IN CONFORMANCE WITH IBC SECTION 2303.1.5. SHEATHING SHALL BE MANUFACTURED UNDER THE PROVISIONS OF VOLUNTARY PRODUCT STANDARDS DOC PS 1-09, PS 2-10, OR APA PRP-108 PERFORMANCE STANDARDS AND POLICIES FOR STRUCTURAL USE PANELS. SEE DRAWINGS FOR THICKNESS, SPAN RATING, AND NAILING REQUIREMENTS.

25. AT NON-SHEAR WALL EXTERIOR WALLS, UNLESS OTHERWISE NOTED, WALL SHEATHING SHALL BE  $\frac{1}{2}$ " (NOMINAL) WITH SPAN RATING OF  $\frac{24}{6}$ ; WITH 8d @ 6" oc PANEL NAILING (APPLIES TO ALL SHEATHING PANEL EDGES); AND 8d @ 12" oc TO INTERMEDIATE FRAMING.

26. ALL PRESSURE-TREATED (P.T.) WOOD MEMBERS SPECIFIED ON THE DRAWINGS THAT OCCUR ABOVE

GROUND AND CONTINUOUSLY PROTECTED FROM MOISTURE (INTERIOR LOCATIONS) SHALL BE PRESSURE—TREATED WITH DOT SODIUM BORATE (SBX) WITHOUT NaSIO2. AT LOCATIONS PERMANENTLY EXPOSED TO WEATHER OR IN CONTACT WITH THE GROUND, WOOD MEMBERS SHALL BE PRESSURE—TREATED WITH ALKALINE COPPER QUAT (ACQ—C FOR DOUGLAS—FIR) PRESERVATIVE UNLESS OTHERWISE NOTED. AMMONIACAL COPPER ZINC ARSENATE (ACZA) PRESERVATIVE OR OTHER PRESERVATIVES WITH AMMONIA CARRIERS, SHALL NOT BE USED. GLUED LAMINATED MEMBERS EXPOSED TO WEATHER OR MOISTURE SHALL BE TREATED WITH A NON—CORROSIVE, APPROVED PRESERVATIVE.

SEE NOTE #27 FOR MATERIAL REQUIREMENTS OF CONNECTORS AND FASTENERS IN CONTACT WITH

27. TIMBER CONNECTORS CALLED OUT BY LETTERS AND NUMBERS SHALL BE "STRONG-TIE" BY SIMPSON COMPANY, AS SPECIFIED N THEIR WOOD CONSTRUCTION CONNECTORS CATALOG NO. C-C-2017-18. INSTALL NUMBER AND SIZE OF FASTENERS AS SPECIFIED BY MANUFACTURER. CONNECTORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. WHERE CONNECTOR STRAPS CONNECT TWO MEMBERS, CENTER STRAP ON JOINT AND INSTALL NUMBER AND SIZE OF FASTENERS AS SPECIFIED BY MANUFACTURER, WITH EQUAL NUMBER AND SIZE OF FASTENERS IN EACH MEMBER. ALL BOLTS IN WOOD MEMBERS SHALL CONFORM TO ASTM A307. INSTALL WASHERS UNDER THE HEADS AND NUTS OF ALL BOLTS AND LAG SCREWS BEARING ON WOOD. ALL SHIMS SHALL BE SEASONED AND DRIED AND THE SAME GRADE (MINIMUM) AS MEMBERS CONNECTED.

ALL TIMBER CONNECTORS IN CONTACT WITH PRESSURE-TREATED WOOD THAT USED PRESERVATIVE CHEMICALS OTHER THAN DOT SODIUM BORATE (SBX) WITHOUT  $N_0SIO_2$  SHALL BE MANUFACTURED FROM  $Z_{MAX}$  STEEL BY SIMPSON (G185 STEEL PER ASTM A653), OR TYPE 304 OR 316 STAINLESS STEEL. ALTERNATIVELY, CONNECTORS CAN BE POST HOT DIP GALVANIZED PER ASTM A123 OR MECHANICALLY GALVANIZED PER ASTM B695, CLASS 55 OR GREATER. STAINLESS STEEL FASTENERS SHALL BE USED WITH STAINLESS STEEL CONNECTORS, AND HOT DIP GALVANIZED FASTENERS PER ASTM A153 SHALL BE USED WITH GALVANIZED CONNECTORS.

28. WOOD FRAMING NOTES: THE FOLLOWING SHALL APPLY UNLESS OTHERWISE NOTED ON THE DRAWINGS:

A. ALL WOOD FRAMING DETAILS SHALL BE CONSTRUCTED TO THE MINIMUM STANDARDS OF THE IBC. MINIMUM NAILING SHALL CONFORM TO IBC TABLE 2304.9.1 OR CURRENT ICC—ES REPORT NER—272. COORDINATE THE SIZE AND LOCATION OF ALL OPENINGS WITH MECHANICAL AND ARCHITECTURAL DRAWINGS. INSTALL WASHERS UNDER THE HEADS AND NUTS OF ALL BOLTS AND LAG SCREWS BEARING ON WOOD. INSTALLATION OF LAG SCREWS SHALL CONFORM TO 2012 NDS SECTION 11.1.4, AND INSTALLATION OF BOLTS SHALL CONFORM TO 2012 NDS SECTION 11.1.3.

B. WALL FRAMING: TWO STUDS MINIMUM SHALL BE INSTALLED AT THE ENDS OF ALL WALLS, UNLESS NOTED OTHERWISE NOTED. INSTALL SOLID BLOCKING FOR WOOD COLUMN THROUGH FLOOR SPACES TO SUPPORTS BELOW.

ALL STUD WALLS SHALL HAVE THEIR LOWER WOOD PLATES ATTACHED TO WOOD FRAMING BELOW WITH 16d NAILS @ 12" oc STAGGERED OR BOLTED TO CONCRETE WITH ½ ANCHOR BOLTS @ 4'-0" oc PER IBC SECTION 2308.6 (EMBED 7"), UNLESS OTHERWISE NOTED. 3" x 3" x 0.229" PLATE WASHERS SHALL BE USED WITH ALL SILL PLATE ANCHOR BOLTS AND INSTALLED PER AF&PA SDPWS-2008 SECTION 4.3.6.4.3. INDIVIDUAL MEMBERS OF BUILT-UP STUD POSTS SHALL BE NAILED TO EACH OTHER WITH 16d @ 12" oc STAGGERED.

C. FLOOR AND ROOF FRAMING: INSTALL SOLID BLOCKING AT ALL BEARING POINTS. TOENAIL JOISTS TO SUPPORTS WITH (2)16d NAILS. ATTACH TIMBER JOISTS TO FLUSH HEADERS OR BEAMS WITH SIMPSON METAL JOIST HANGERS IN ACCORDANCE WITH NOTES ABOVE. NAIL ALL MULTI-JOIST BEAMS TOGETHER WITH 16d@12"oc STAGGERED.

ROOF AND FLOOR SHEATHING SHALL BE LAID UP WITH GRAIN PERPENDICULAR TO SUPPORTS AND NAILED AS SHOWN ON THE DRAWINGS. INSTALL APPROVED PANEL EDGE CLIPS CENTERED BETWEEN JOISTS/TRUSSES AT UNBLOCKED ROOF SHEATHING EDGES. ALL FLOOR SHEATHING EDGES SHALL HAVE APPROVED TONGUE—AND—GROOVE JOINTS OR SHALL BE SUPPORTED WITH SOLID BLOCKING ALLOW 1/8" SPACING AT ALL PANEL EDGES AND ENDS OF LOOR AND ROOF SHEATHING. TOENAIL BLOCKING TO SUPPORTS WITH 16d@12"oc. IN ACCORDANCE WITH IBC SECTION 1604.8.3, DECKS SHALL BE POSITIVELY ANCHORED TO THE STRUCTURE BY MEANS OTHER THAN NAILS SUBJECT TO WITHDRAWAL. ANCHOR WITH MINIMUM (1) CS16 STRAP AT EACH END ATTACHED TO DECK JOISTS AND TO A SOLID BLOCKING MEMBER WITHIN THE BUILDING.

POST-INSTALLED ANCHORS AND EPOXY ADHESIVE

29. EPOXY-GROUTED RODS OR REBAR TO CONCRETE SPECIFIED ON THE DRAWINGS SHALL BE ONE OF THE FOLLOWING INSTALLED IN STRICT ACCORDANCE WITH THE ICC-ES REPORTS INDICATED AND MANUFACTURER'S INSTRUCTIONS INCLUDING MINIMUM EMBED REQUIREMENTS: "SET-XP" AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC. (ICC-ES NO. 2508); OR "HIT-HY 200" AS MANUFACTURED BY HILTI, INC. (ICC-ES NO. 3187), "SAFE-SET" INSTALLATION WITH HOLLOW CARBIDE DRILL BIT IS PERMITTED; OR "PURE110+" AS MANUFACTURED BY DEWALT/POWERS (ICC-ES NO. 3298). SUBSTITUTES PROPOSED BY CONTRACTOR SHALL BE SUBMITTED FOR REVIEW WITH ICC-ES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. IN ADDITION, SUBSTITUTIONS SHALL MEET ICC-ES ACCEPTANCE CRITERIA AC308. SPECIAL INSPECTION OF EPOXY-GROUTED ANCHOR INSTALLATION IS REQUIRED. EPOXY GROUTED RODS OR REBAR SHALL NOT BE USED AS SUBSTITUTES FOR CAST-IN-PLACE ANCHOR BOLTS OR REINFORCING STEEL UNLESS SPECIFICALLY APPROVED BY THE STRUCTURAL ENGINEER. NOTIFY ENGINEER IF ANCHOR LOCATIONS CONFLICT WITH REINFORCING STEEL -DO NOT CUT REINFORCING OR REDUCE EMBEDMENT DEPTHS WITHOUT PRIOR APPROVAL. INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCLINED TO SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED BY CERTIFIED PERSONNEL IN CONFORMANCE TO ACI 318-14 SECTION 17.8.2.2. HOLES SHALL BE HAMMER DRILLED AND DRY.

30.EXPANSION ANCHORS SHALL BE ONE OF THE APPROVED PRODUCTS BELOW:

- KWIK BOLT TZ ANCHORS AS MANUFACTURED BY HILTI, INC. AND INSTALLED IN STRICT ACCORDANCE WITH ICC-ES REPORT NO. 1917, OR

AND INSTALLED IN STRICT ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS.

- STRONG-BOLT 2 AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC. AND INSTALLED IN STRICT ACCORDANCE WITH ICC-ES REPORT NO. 3037

	IBC TABLE 1705.3 REQUIRE AND TESTS OF CONCR				
REQUIRED?	VERIFICATION & INSPECTION	CONTINUOUS	PERIODIC	REF STD.	IBC REF.
N*	INSPECT REINFORCEMENT, INCLUDING     PRESTRESSING     TENDONS AND VERIFY PLACEMENT.		х	ACI 318 CH. 20, 25.2, 25.3, 26.5.1-26.5.3	1908.4
N	2. REINFORCING BAR WELDING: A. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A 706. B. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"; AND		Х	AWSD1.4 ACI 318 26.5.4	
	C. INSPECT ALL OTHER WELDS	X	Х		
YES	3. INSPECT ANCHORS CAST IN CONCRETE.		X	ACI 318: 17.8.2	
YES	4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS.  A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST  SUSTAINED TENSION LOADS  B. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.A	х	X	ACI 318: 17.8.2.4 ACI 318:17.8.2	
N*	5. VERIFY USE OF REQUIRED DESIGN MIX.		Х	ACI 318: CH. 19, 26.4.3, 26.4.4	1904.1, 1904.2 1908.2, 1908.3
N*	6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP  AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X		ASTM C 172 ASTM C 31 ACI 318: 26.4.5, 26.12	1908.10
N*	7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	Х		ACI 318: 26.4.5	1908.6, 1908.7 1908.8
N*	8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.		Х	ACI 318: 26.4.7-26.4.9	1908.9
N	9. INSPECT PRESTRESSED CONCRETE FOR: A. APPLICATION OF PRESTRESSING FORCES; AND B. GROUTING OF BONDED PRESTRESSING TENDONS	X X		ACI 318: 26.9.2.1 ACI 218: 26.9.2.3	
N	10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.		Х	ACI 318: CH. 26.8	
N*	11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.		Х	ACI 318: 26.10.2	
N*	12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.		х	ACI318: 26.10.1(b)	

## Minimum Connectors and Fasteners for Wood Members per IBC 2018

	DESCRIPTION OF BUILDING ELEMENT	NUMBER AND TYPE OF FASTENERS	SPACING & LOCATION		DESCRIPTION OF BLDG. ELEMENT
1.	BLOCKING BETWEEN CEILING JOISTS, RAFTERS, OR TRUSSES TO TOP PLATE OR OTHER FRAMING BELOW	3-8d COMMON ( $2\frac{1}{2}$ " x 0.131"); or 3-10d BOX (3" x 0.128"); or 3-3" x 0.131" NAILS; or 3-3" x 14 GAGE STAPLES, $\frac{7}{16}$ " CROWN	EACH END, TOENAIL	22.	JOIST TO SILL, TOP PLATE, OR GIRDER
	BLOCKING BETWEEN RAFTERS OR TRUSS NOT AT THE WALL TOP PLATE, TO RAFTER OR TRUSS	2-8d COMMON ( $2\frac{1}{2}$ " x 0.131") 2-3" x 0.131" NAILS 2-3" x 14 GAGE STAPLES 2-16d COMMON ( $3\frac{1}{2}$ " x 0.162")	EACH END, TOENAIL	23.	RIM JOIST, BAND JOIS OR BLOCKING TO TOP PLATE, SILL, OR OTHE FRAMING BELOW
		3-3" x 0.131" NAILS 3-3" x 14 GAGE STAPLES		24.	1" x 6" SUBFLOOR OR LESS TO EACH JOIST
	FLAT BLOCKING TO TRUSS AND WEB FILLER	16d COMMON (3½" x 0.162") @ 6" oc 3" x 0.131" NAILS @ 6" oc 3" x 14 GAGE STAPLES @ 6" oc	FACE NAIL	25.	2" SUBFLOOR TO
2.	CEILING JOISTS TO TOP PLATE	3-8d COMMON (2½" x 0.131"); or 3-10d BOX (3" x 0.128"); or 3-3" x 0.131" NAILS; or 3-3" x 14 GAGE STAPLES, ½6" CROWN	EACH JOIST, TOENAIL	26.	JOIST OR GIRDER  2" PLANKS (PLANK & BEAM — FLOOR & RO
3.	CEILING JOIST NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITION (NO THRUST) (SEE 2308.7.3.1, TABLE 2308.7.3.1)	3-16d COMMON (3½" x 0.162"); or 4-10d BOX (3" x 0.128"); or 4-3" x 0.131" NAILS; or 4-3" x 14 GAGE STAPLES, 7/6" CROWN	FACE NAIL	27.	BUILT-UP GIRDERS AN BEAMS, 2" LUMBER LAYERS
4.	CEILING JOIST ATTACHED TO	PER TABLE 2308.7.3.1	FACE NAIL		
5.	PARALLEL RAFTER (HEEL JOINT)  COLLAR TIE TO RAFTER	3-10d COMMON (3" x 0.148"); or 4-10d BOX (3" x 0.128"); or 4-3" x 0.131" NAILS; or 4-3" x 14 GAGE STAPLES, 76" CROWN	FACE NAIL		
6.	RAFTER OR ROOF TRUSS TO TOP PLATE (SEE 2308.7.5, TABLE 2308.7.5)	3-10d COMMON (3" x 0.148"); or 3-16d BOX (3½" x 0.135"); or 4-10d BOX (3" x 0.128"); or 4-3" x 0.131" NAILS; or 4-3" x 14 GAGE STAPLES, ½6" CROWN	TOENAIL	28.	LEDGER STRIP SUPPORTING JOISTS OR RAFTERS
7.	ROOF RAFTERS TO RIDGE VALLEY OR HIP RAFTERS; OR ROOF RAFTER TO 2" RIDGE BEAM	2-16d COMMON (3½" x 0.162"); or 3-10d BOX (3" x 0.128"); or 3-3" x 0.131 NAILS; or	END NAIL	29.	JOIST TO BAND JOIST OR RIM JOIST
		3-3" x 14 GAGE STAPES, 7/6" CROWN  3-10d COMMON (3½" x 0.148"); or  3-16d BOX (3½" x 0.135"); or  4-10d BOX (3" x 0.128"); or  4-3" x 0.131 NAILS; or  4-3" x 14 GAGE STAPES, 7/6" CROWN	TOENAIL	30.	BRIDGING OR BLOCKIN TO JOIST, RAFTER, OR TRUSS
8.	STUD TO STUD (NOT AT SHEARWALL	WALL  16d COMMON (3½" × 0.162")"	24" oc FACE NAIL		
	CHORDS)	10d BOX (3" x 0.128"); or 3" x 0.131" NAILS; or 3−3" x 14 GAGE STAPLES, ½6" CROWN	16" oc FACE NAIL		
9.	STUD TO STUD AND ABUTTING STUDS AT INTERSECTION WALL CORNERS	16d COMMON (3½" x 0.162")"; or 16d BOX (3½" x 0.135")"; or 3" x 0.131" NAILS; or	16" oc FACE NAIL 12" oc FACE NAIL 12" oc FACE NAIL		
1.0	DUILT UD HEADED	3-3" x 14 GAGE STAPLES, 7/6" CROWN	40" 54 5005		
	BUILT-UP HEADER (2" TO 2" HDR.)	16d COMMON (3½" x 0.162")"; or 16d BOX (3½" x 0.135")	16" oc EA. EDGE, FACE NAIL 12" oc EA. EDGE, FACE NAIL		
11.	CONTINUOUS HEADER TO STUD	4-8d COMMON (2½" x 0.131"); or 4-10d BOX (3" x 0.128")	TOENAIL		
12.	TOP PLATE TO TOP PLATE	16d COMMON ( $3\frac{1}{2}$ " x 0.162"); or 10d BOX ( $3$ " x 0.128"); or 3" x 0.131" NAILS; or $3$ " x 14 GAGE STAPLES, $\frac{1}{16}$ " CROWN	16" oc FACE NAIL  12" oc FACE NAIL		
13.	TOP PLATE TO TOP PLATE, AT END JOINTS	8-16d COMMON (3½" x 0.162"); or 12-10d BOX (3" x 0.128"); or 12-3" x 0.131" NAILS; or 12-3" x 14 GAGE STAPLES, ½6" CROWN	EACH SIDE OF END JOINT, FACE NAIL (MINIMUM 24" LAP SPLICE LENGTH EA. SIDE OF END JOINT		
14.	BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING NOT AT SHEARWALL	16d COMMON (3½" x 0.162")"; or 16d BOX (3½" x 0.135")"; or 3" x 0.131" NAILS; or	16" oc FACE NAIL  12" oc FACE NAIL		
15.	BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING AT SHEARWALL	3" x 14 GAGE STAPLES, ½6" CROWN  2-16d COMMON (3½" x 0.162"); or  3-16d BOX (3½" x 0.135"); or  4-3" x 0.131" NAILS; or  4-3" x 14 GAGE STAPLES, ½6" CROWN	16" oc FACE NAIL		
16.	STUD TO TOP OR BOTTOM PLATE	4-8d COMMON (2½" x 0.131"); or 4-10d BOX (3" x 0.128"); or 4-3" x 0.131" NAILS; or 4-3" x 14 GAGE STAPLES, ½" CROWN	TOENAIL		
		2-16d COMMON ( $3\frac{1}{2}$ " x 0.162"); or 3-10d BOX ( $3$ " x 0.128"); or 3-3" x 0.131" NAILS; or 3-3" x 14 GAGE STAPLES, $\frac{1}{16}$ " CROWN	END NAIL		
17.	TOP OR BOTTOM PLATE TO STUD	2-16d COMMON ( $3\frac{1}{2}$ " x 0.162"); or 3-10d BOX ( $3$ " x 0.128"); or 3-3" x 0.131" NAILS; or 3-3" x 14 GAGE STAPLES, $\frac{7}{16}$ " CROWN	END NAIL		
18.	TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS	2-16d COMMON ( $3\frac{1}{2}$ " x 0.162"); or 3-10d BOX ( $3$ " x 0.128"); or 3-3" x 0.131" NAILS; or 3-3" x 14 GAGE STAPLES, $\frac{1}{16}$ " CROWN	FACE NAIL		
19.	1" BRACE TO EACH STUD AND PLATE	2-8d COMMON ( $2\frac{1}{2}$ " x 0.131"); or 2-10d BOX (3" x 0.128"); or 2-3" x 0.131" NAILS; or 2-3" x 14 GAGE STAPLES, $\frac{7}{16}$ " CROWN	FACE NAIL		
	1" x 6" SHEATHING TO EACH BEARING	2-8d COMMON (2½" x 0.131"); or 2-10d BOX (3" x 0.128"); or	FACE NAIL		
21	1" x 8" AND WIDER SHEATHING TO	3-8d COMMON (2½" x 0.131"): or	Exos		

21. 1" x 8" AND WIDER SHEATHING TO 3-8d COMMON ( $2\frac{1}{2}$ " x 0.131"); or

3-10d BOX (3" x 0.128"); or

EACH BEARING

FACE NAIL

ION	DESCRIPTION OF BLDG. ELEMENT	NUMBER AND TYPE OF FASTENERS	SPACING & LOCATION
		FLOOR	
2	22. JOIST TO SILL, TOP PLATE, OR GIRDER	3-8d COMMON ( $2\frac{1}{2}$ " x 0.131"); or 3-10d BOX ( $3$ " x 0.128"); or 3-3" x 0.131" NAILS; or 3-3" x 14 GAGE STAPLES, $\frac{1}{16}$ " CROWN	TOENAIL
	23. RIM JOIST, BAND JOIST, OR BLOCKING TO TOP PLATE, SILL, OR OTHER FRAMING BELOW	8d COMMON (2½" x 0.131"); or 10d BOX (3" x 0.128"); or 3" x .131" NAILS; r 3" x 14 GAGE STAPLES, ½6" CROWN	6" o.c., TOENAIL
	24. 1" x 6" SUBFLOOR OR LESS TO EACH JOIST	2-8d COMMON (2½" x 0.131"); or 2-10d BOX (3" x 0.128")	FACE NAIL
	25. 2" SUBFLOOR TO JOIST OR GIRDER	2-16d COMMON (3½" x 0.162")	FACE NAIL
	26. 2" PLANKS (PLANK & BEAM – FLOOR & ROOF)	2-16d COMMON (3½" x 0.162")"	EA. BEARING, FACE NAIL
	27. BUILT-UP GIRDERS AND BEAMS, 2" LUMBER LAYERS	20d COMMON (4" x 0.192")	32" o.c., FACE NAIL TOP & BOT. STAGGERED ON OPPOSITE SIDES
		10d BOX (3" x 0.128"); or 3" x 0.131" NAILS; or 3" x 14 GAGE STAPLES, ½6" CROWN	24" o.c., FACE NAIL AT TOP & BOT. STAGGERED ON OPP. SIDES
		AND: $2-20d$ COMMON (4" x 0.192"); or $3-10d$ BOX (3" x 0.128"); or $3-3$ " x 0.131" NAILS; or $3-3$ " x 14 GAGE STAPLES, $\frac{7}{16}$ " CROWN	ENDS AND AT EACH SPLICE, FACE NAIL
2	28. LEDGER STRIP SUPPORTING JOISTS OR RAFTERS	$3-16d$ COMMON ( $3\frac{1}{2}$ " x 0.162"); or $4-10d$ BOX ( $3$ " x 0.128"); or $4-3$ " x 0.131" NAILS; or $4-3$ " x 14 GAGE STAPLES, $\frac{1}{16}$ " CROWN	EACH JOIST OR RAFTER, FACE NAIL
	29. JOIST TO BAND JOIST OR RIM JOIST	$3-16d$ COMMON ( $3\frac{1}{2}$ " x 0.162"); or $4-10d$ BOX ( $3$ " x 0.128"); or $4-3$ " x 0.131" NAILS; or $4-3$ " x 14 GAGE STAPLES, $\frac{7}{16}$ " CROWN	END NAIL
	30. BRIDGING OR BLOCKING TO JOIST, RAFTER, OR TRUSS	2-8d COMMON ( $2\frac{1}{2}$ " x 0.131"); or 2-10d BOX ( $3$ " x 0.128"); or 2-3" x 0.131" NAILS; or 2-3" x 14 GAGE STAPLES, $\frac{7}{16}$ " CROWN	EACH END, TOENAIL

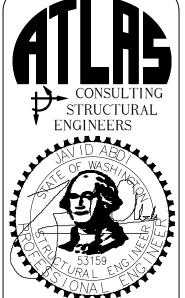
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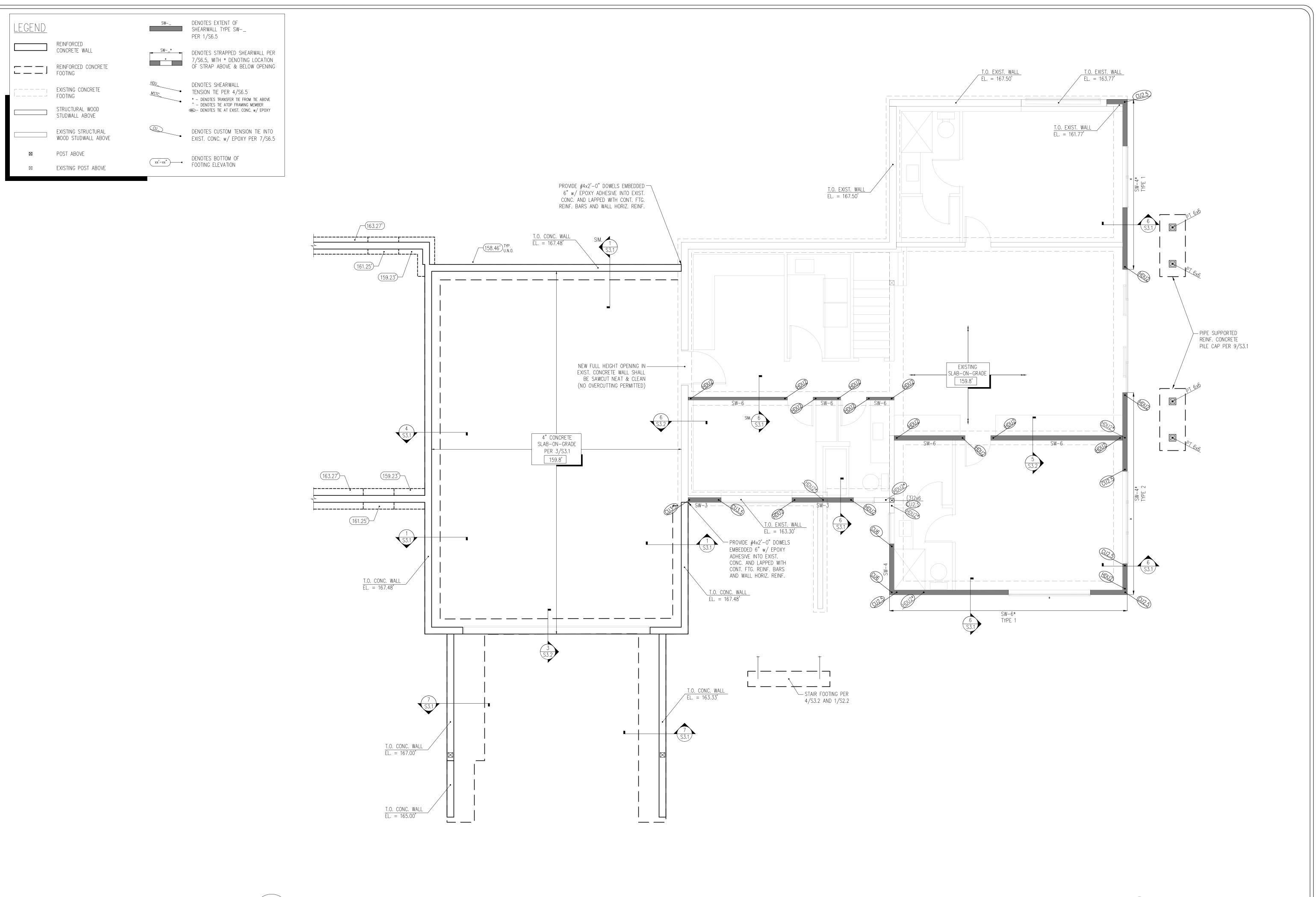
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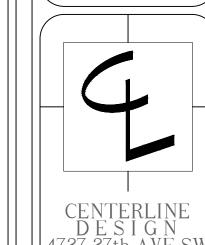
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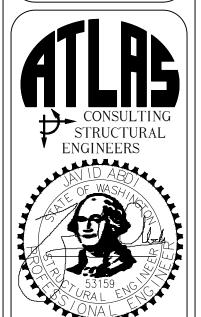
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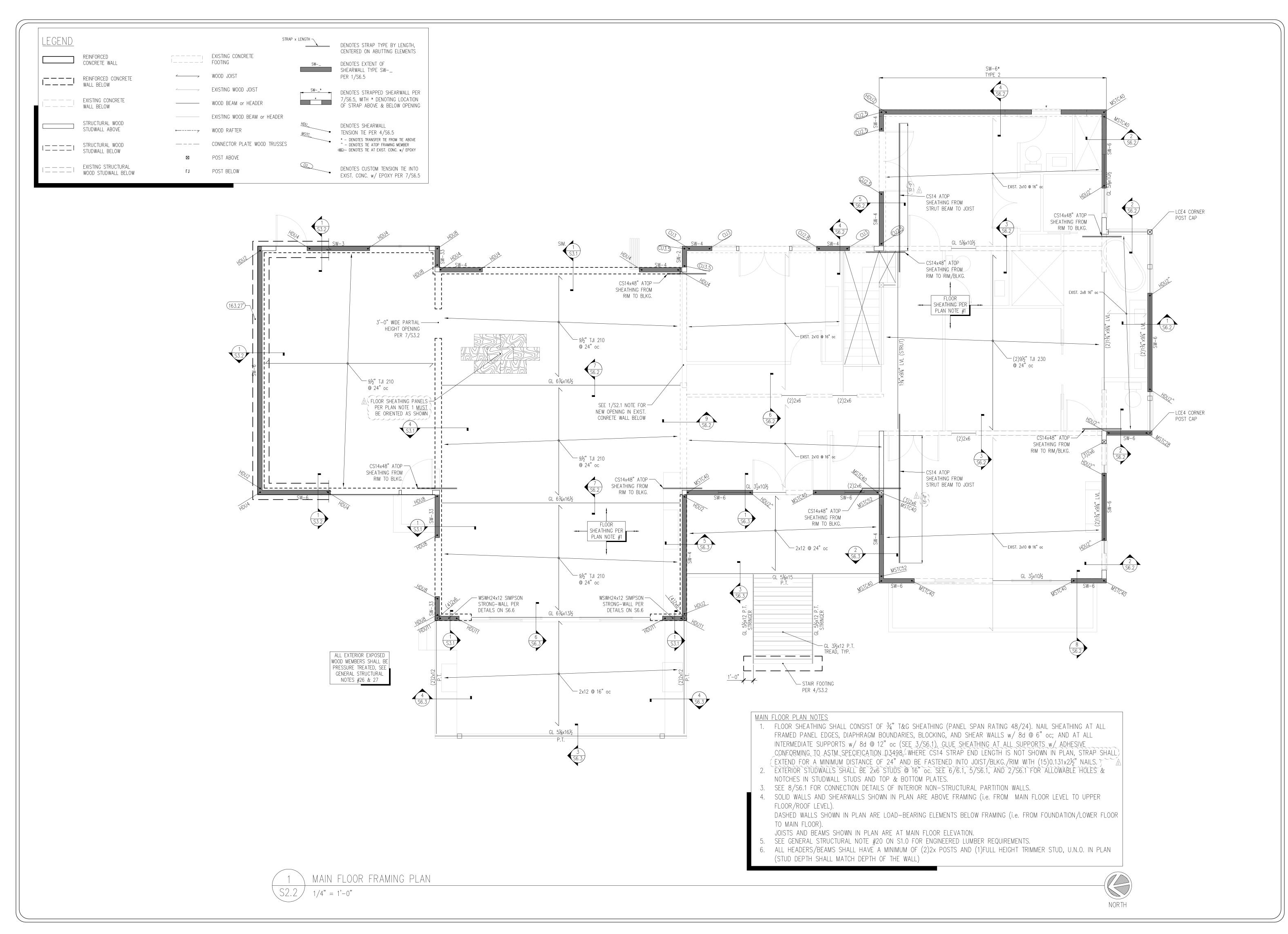
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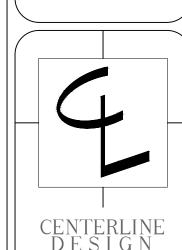
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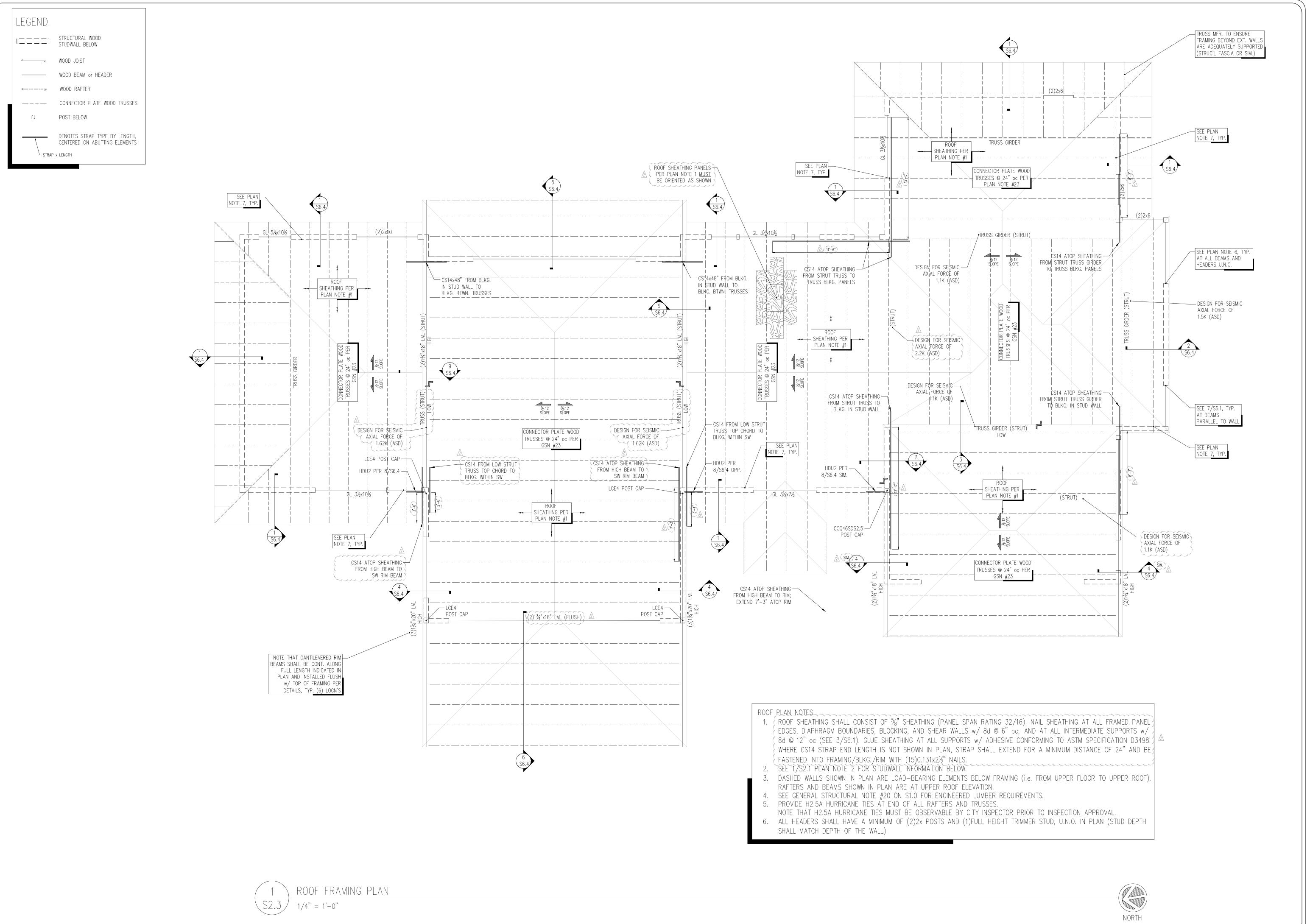
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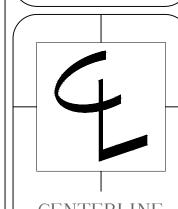
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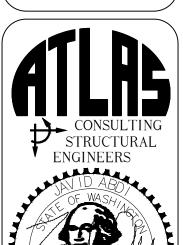
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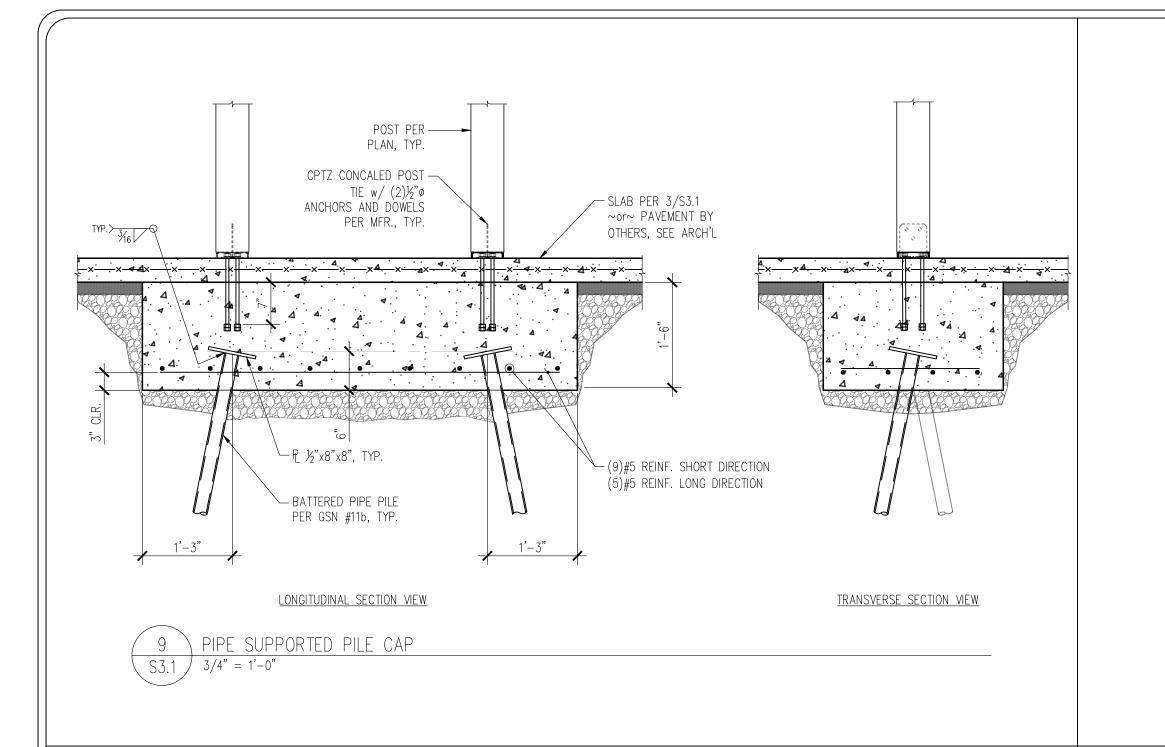
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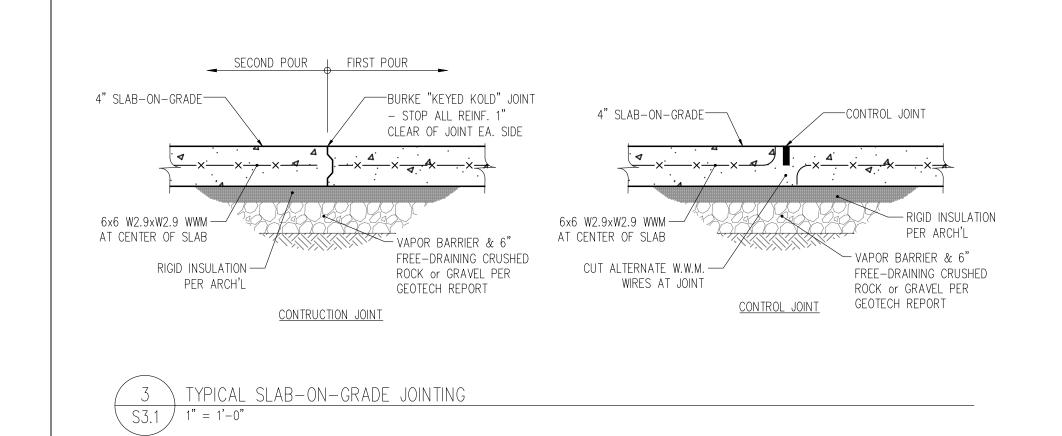
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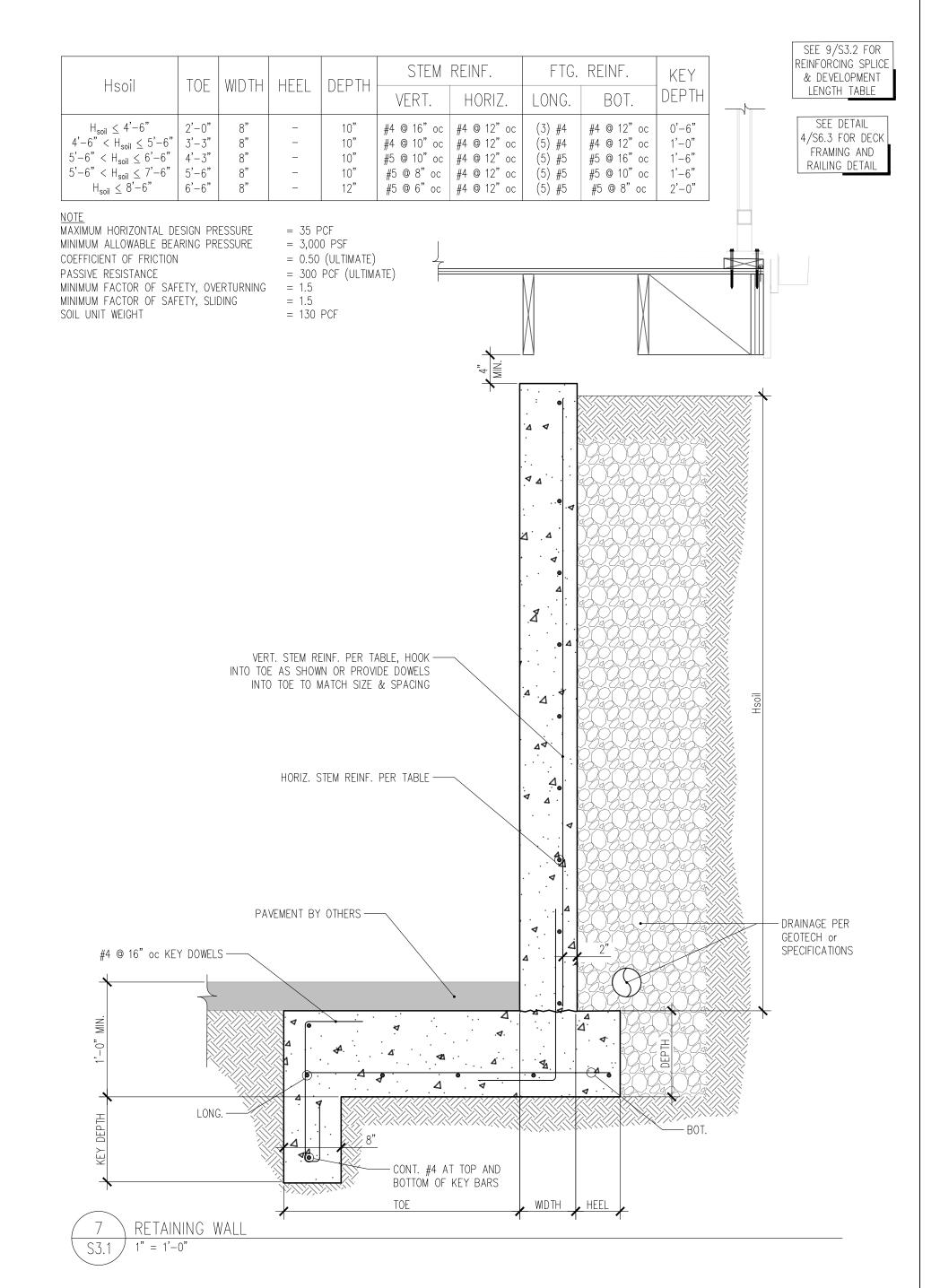
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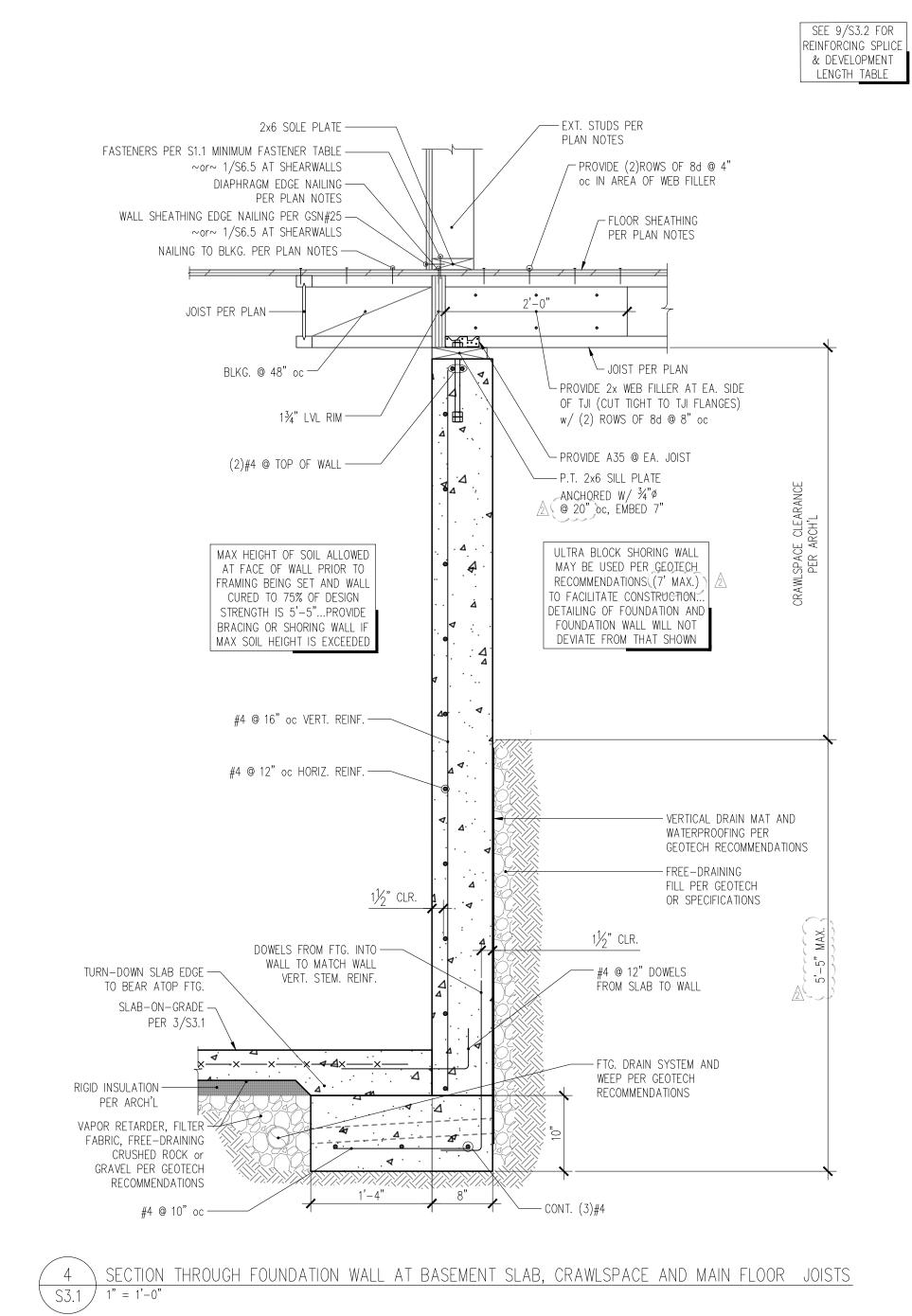
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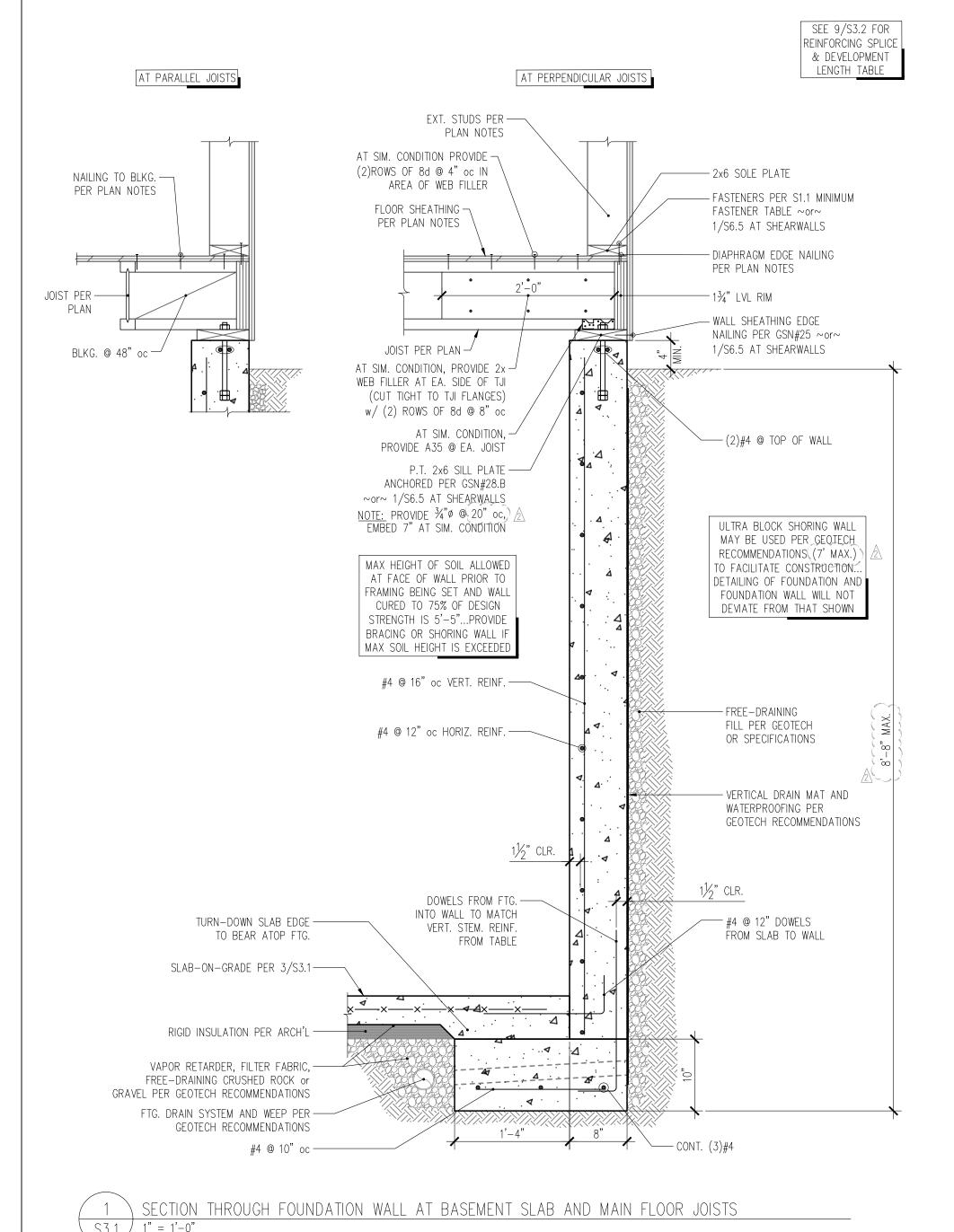
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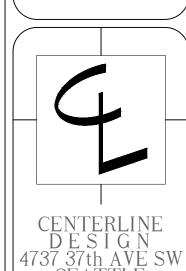




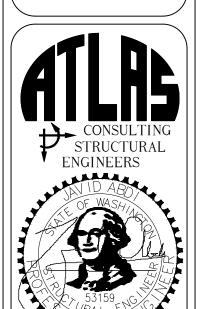








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MIN. STR	MIN. STRAIGHT DEVELOPMENT LENGTH				P SPLICE LENGTI	H (CLASS B)
BAR SIZE	TOP BARS	OTHER BARS		BAR SIZE	TOP BARS	OTHER BARS
#4	25"	19"		#4	33"	25"
#5	31"	24"		#5	41"	31"

'TOP BARS' ARE HORIZONTAL BARS WITH MORE THAN 12" DEPTH OF CONCRETE CAST BELOW THEM

IF CLEAR CONCRETE COVER IS LESS THAN 1x THE DIAMETER OF THE BAR OR THE CENTER—TO—CENTER
SPACING IS LESS THAN (3) BAR DIAMETERS, THEN VALUES SHALL BE INCREASED BY 50%

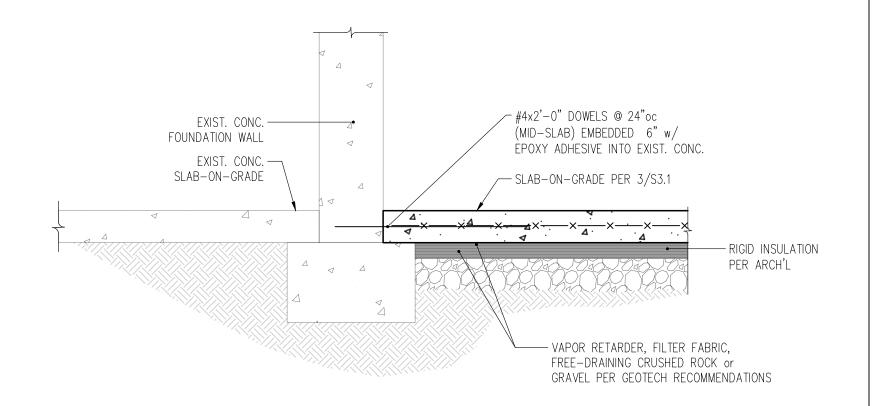
MIN. EM	BEDMENT LENGTH
FOR STAN	IDARD END HOOKS
BAR SIZE	LENGTH
#4	7"

#5

1. SIDE COVER MUST BE EQUAL TO OR GREATER THAN 2½"
2. END COVER FOR 90° HOOKS MUST BE EQUAL TO OR GREATER THAN 2"

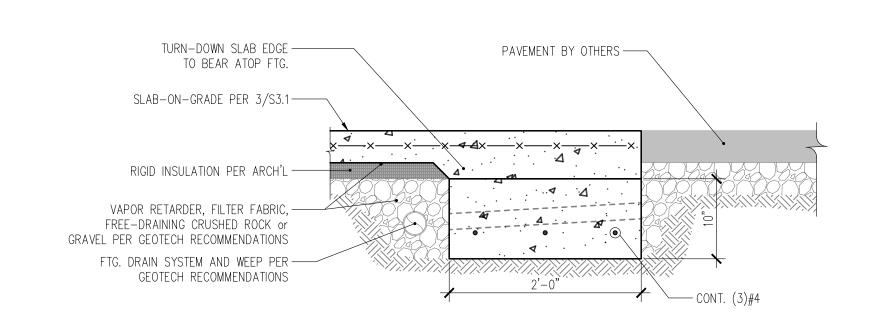
9 CONCRETE REINFORCING DEVELOPMENT AND SPLICE LENGTH TABLES

\$3.2 N/A



6 SECTION AT NEW SLAB AND EXISTING FOUNDATION WALL

S3.2 1" = 1'-0"

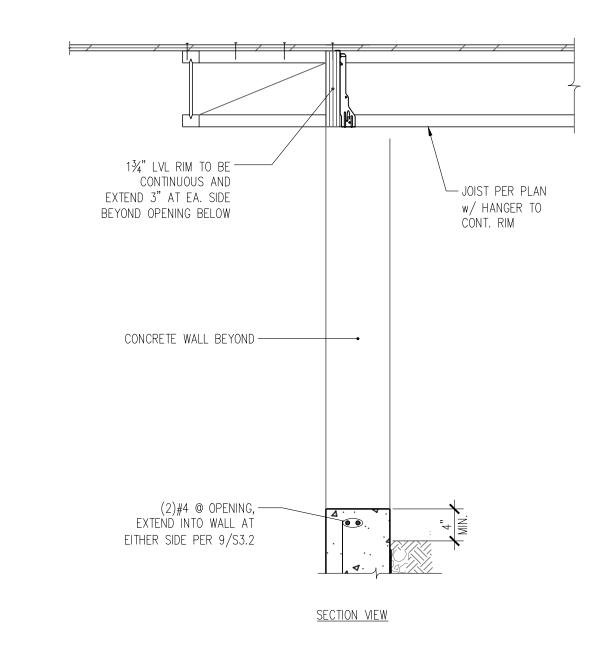


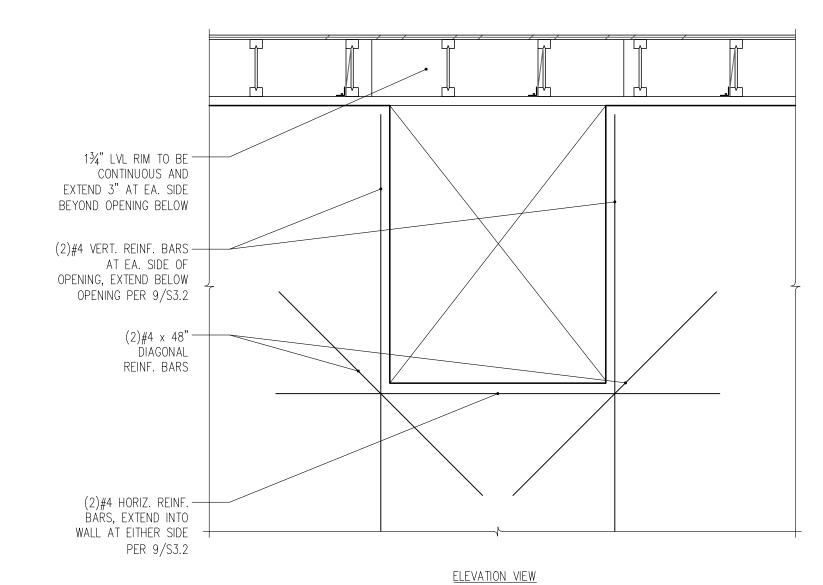
AT PARALLEL JOISTS

8d @ 4" oc TO BLKG.

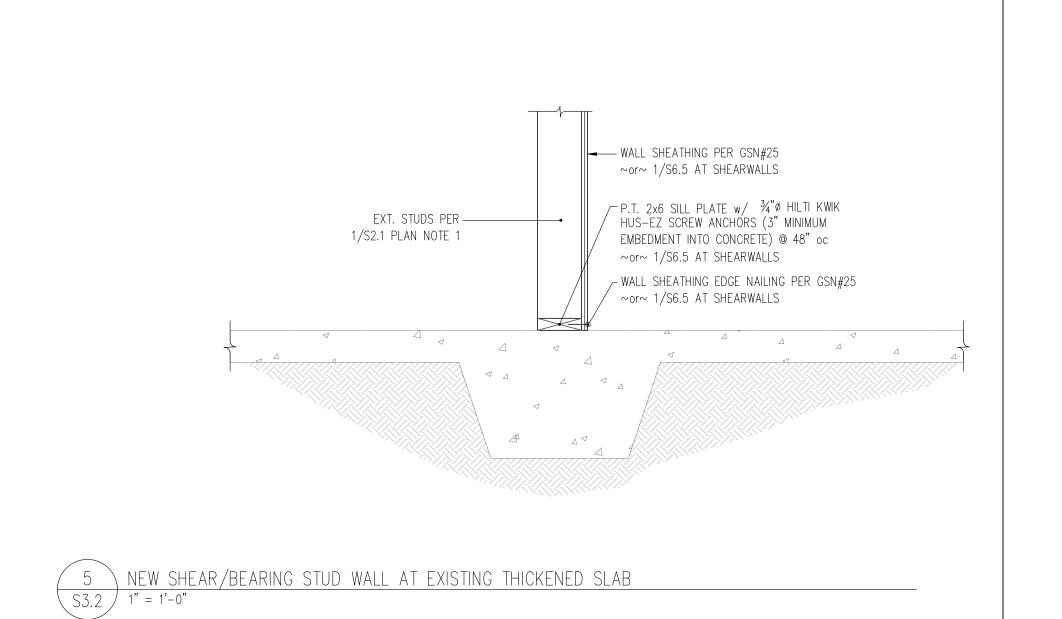
3 SECTION AT GARAGE ENTRANCE S3.2 1" = 1'-0"

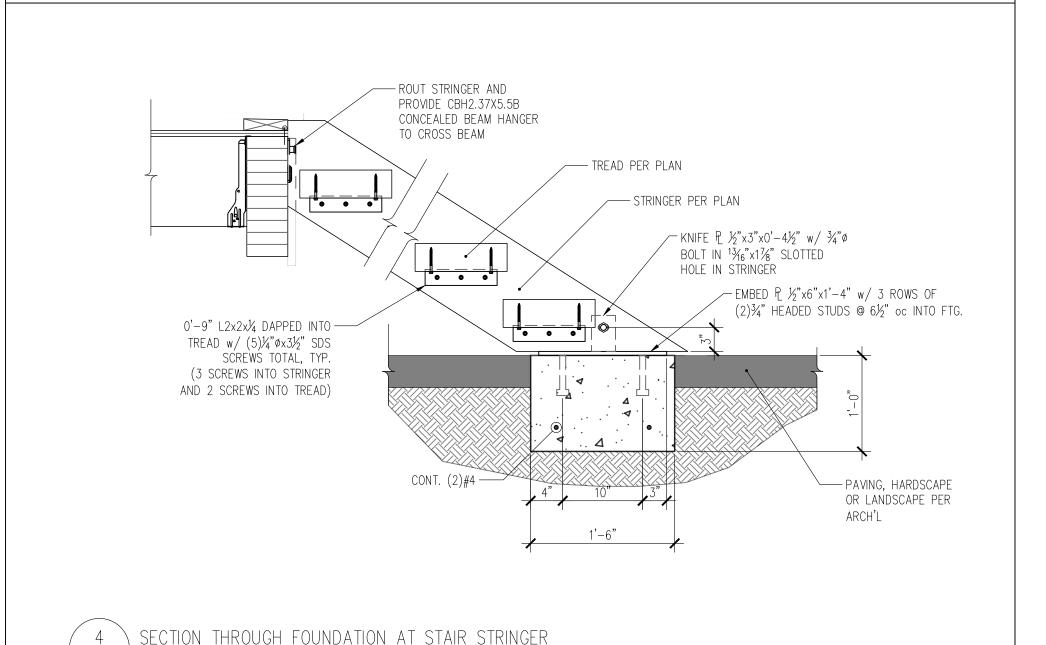
JOIST PER —

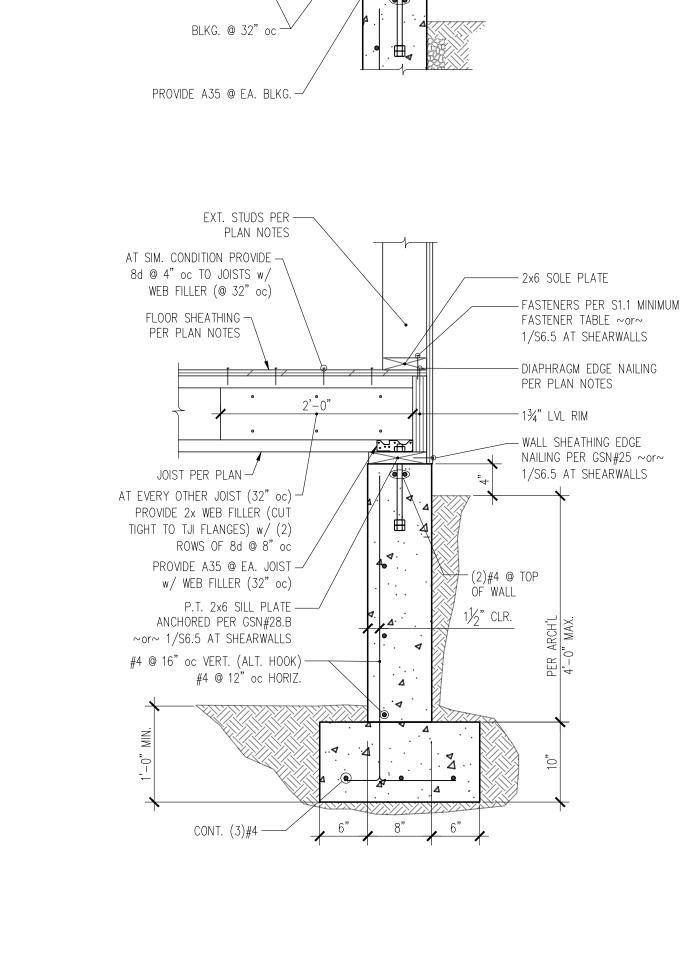




7 PARTIAL HEIGHT OPENING IN NEW FOUNDATION WALL

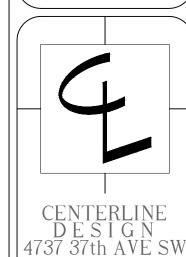




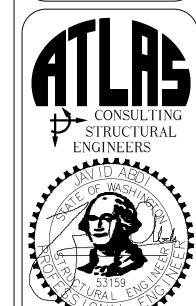








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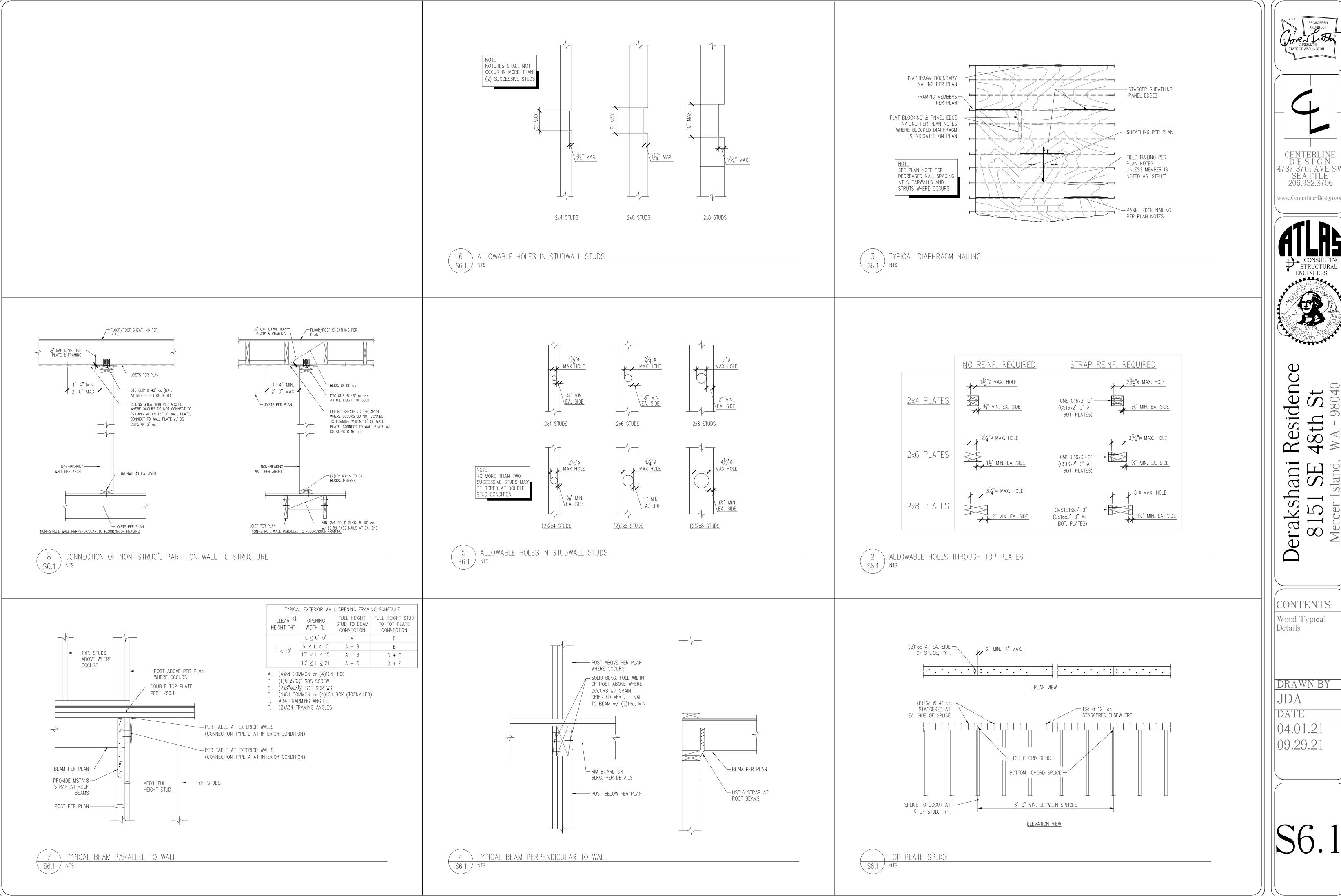
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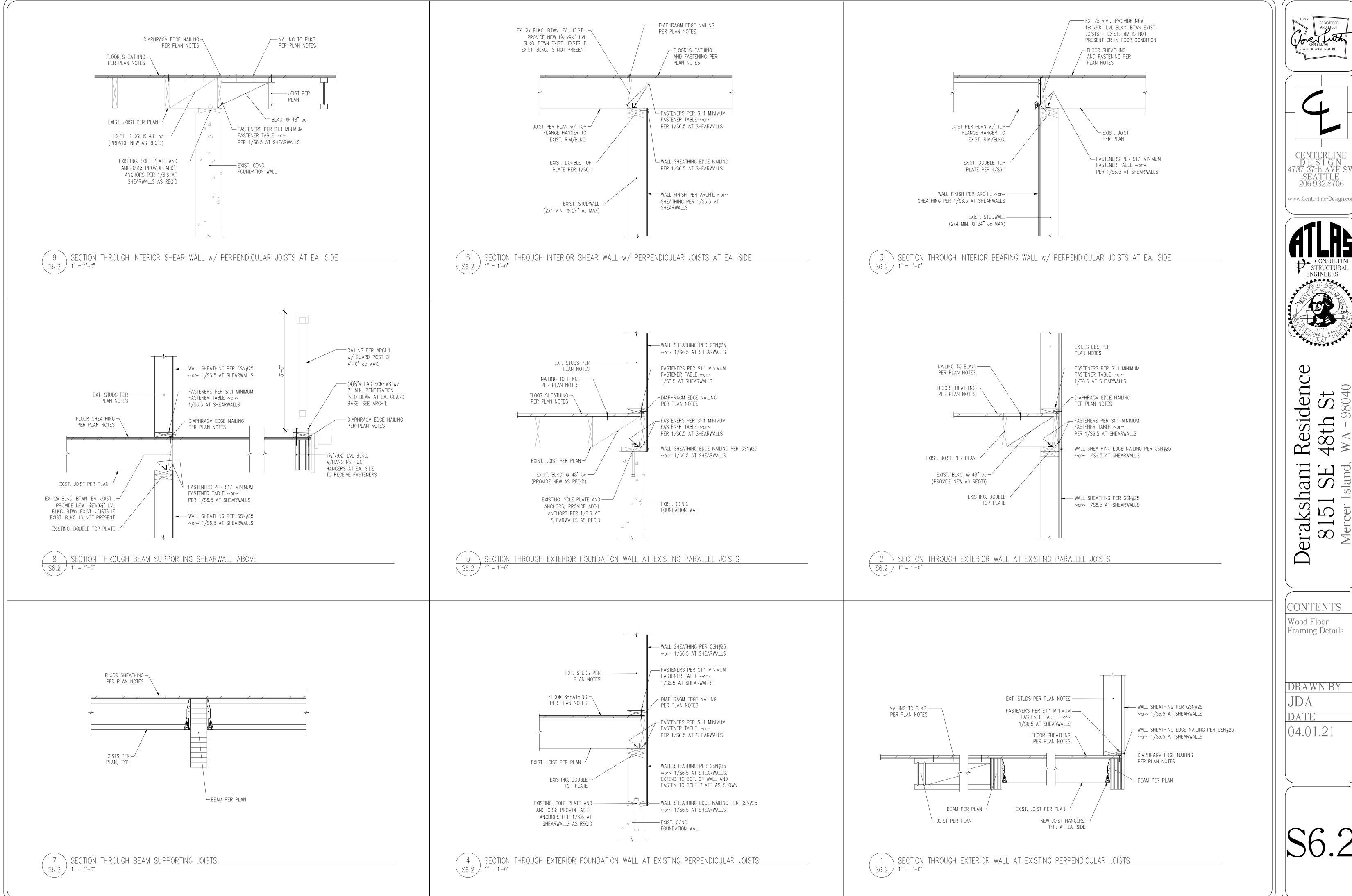
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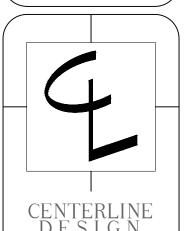
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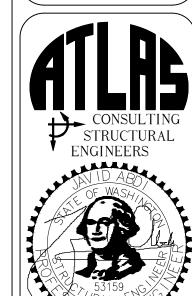
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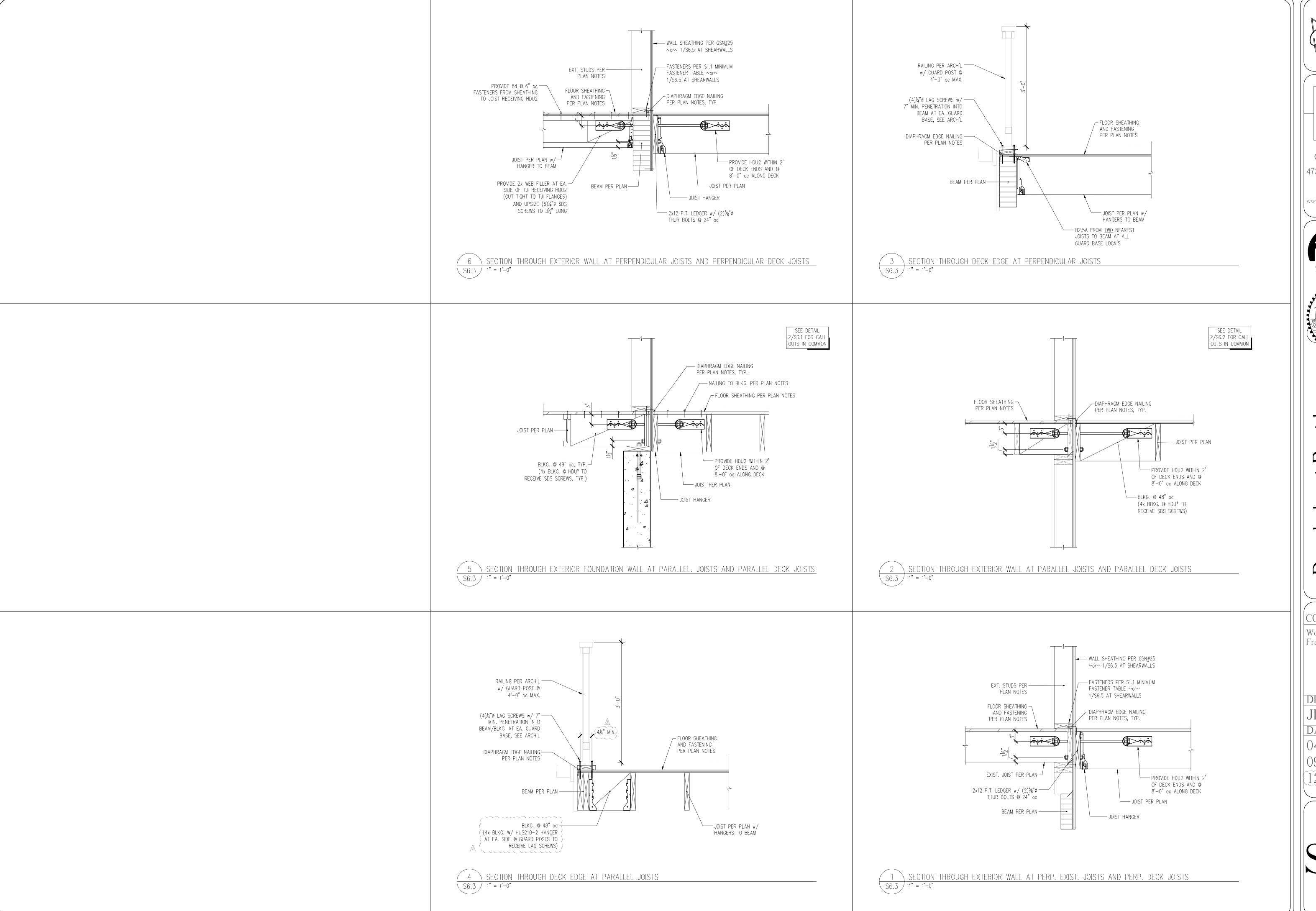
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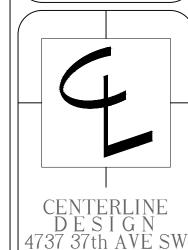
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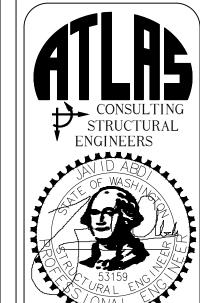
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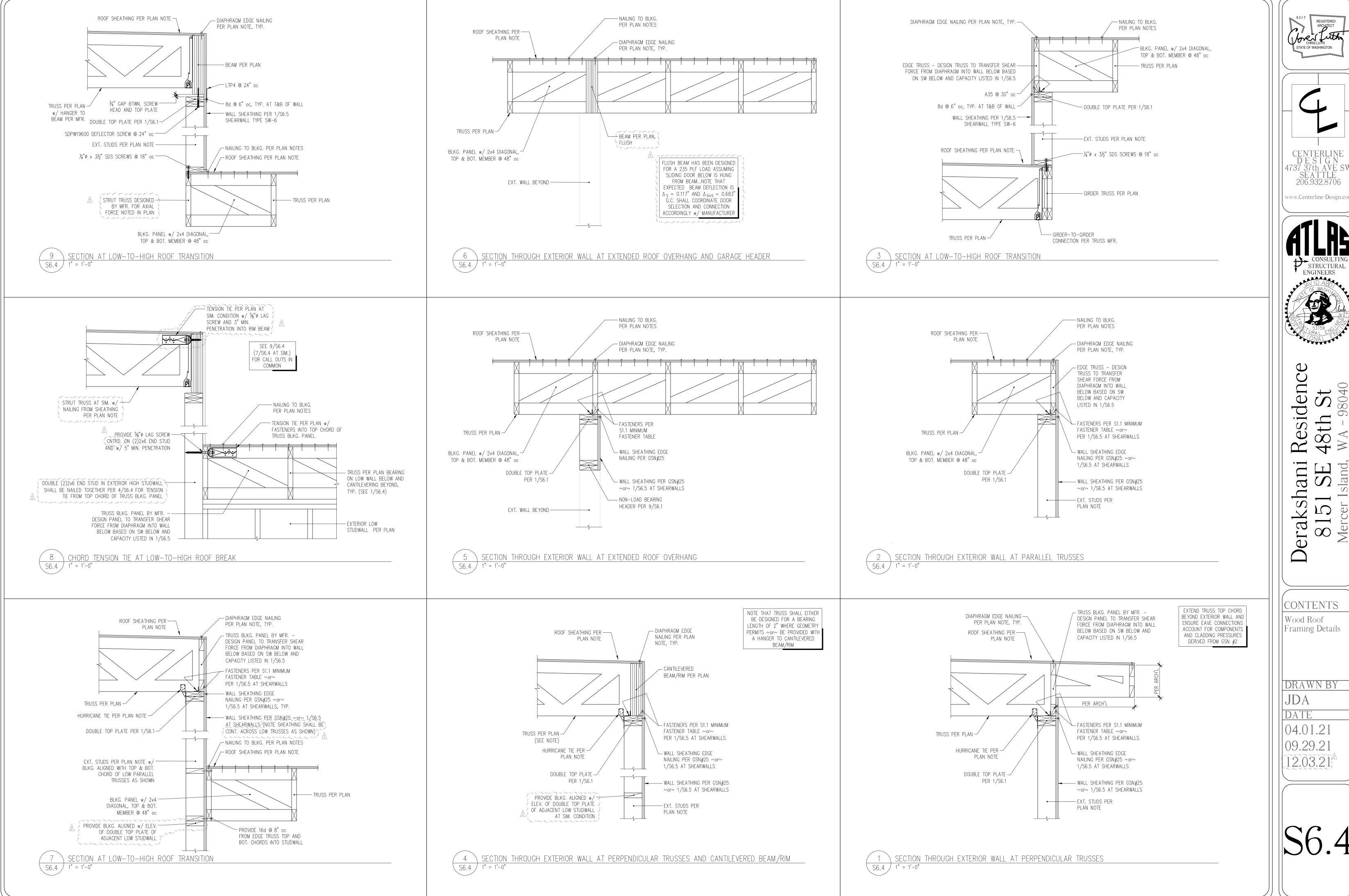
Wood Floor Framing Details

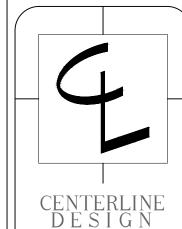
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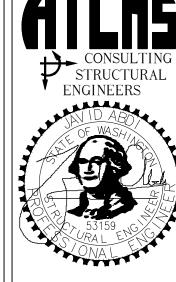
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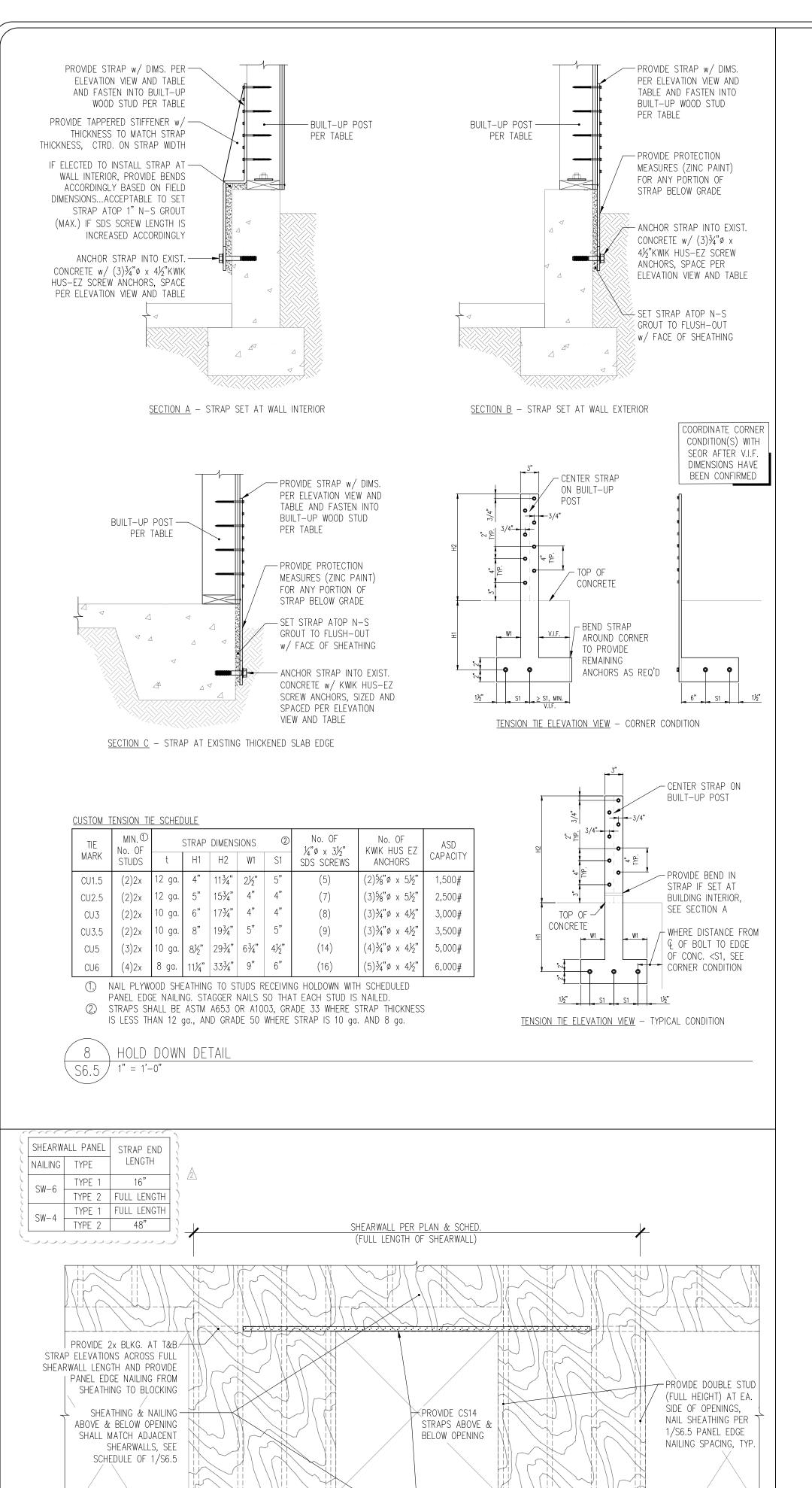


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EXTEND STRAPS AT –

T&B BEYOND THE

OPENING PER TABLE

ABOVE AND NAIL

EVERY HOLE

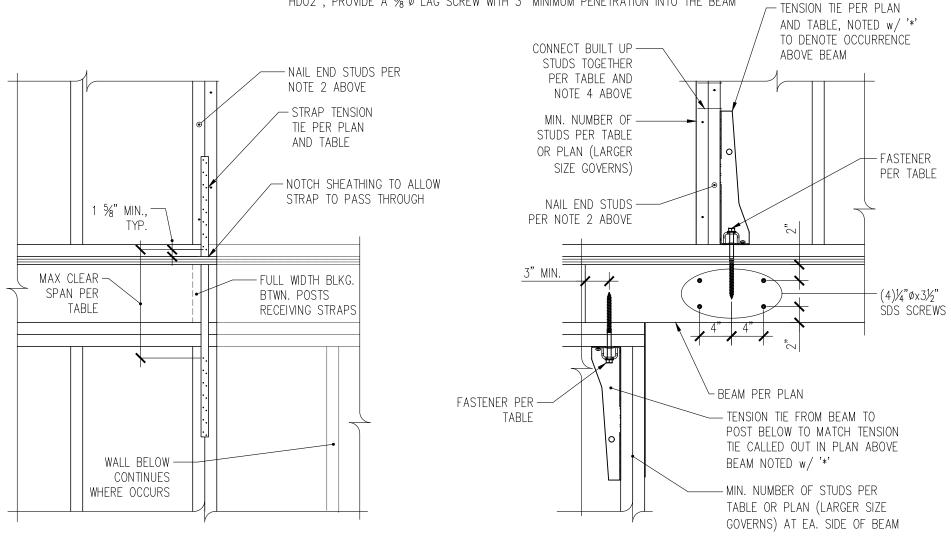
STRAPPED SHEARWALL DETAIL

	TIE <sup>①</sup> MARK	MIN. NUMBER <sup>②</sup> OF STUDS	© CLEAR SPAN - TOTAL FASTENERS	ASD CAPACITY	BUILT-UP STUD FACE NAILS or SCREWS ④
	HDU2^	(2)2x	(6)¼"ø x 2½" SDS SCREWS	1,500#	10d @ 6" oc
	MSTC28	(2)2x	16" - (16)0.148"ø x 3¼"	1,330#	10d @ 6"oc
	MSTC40	(3)2x	16" - (32)0.148"ø x 3½"	2,655#	(5)¼"øx4½" SDS
	MSTC52	(3)2x	16" - (48)0.148"ø x 3¼"	3,985#	(8)¼"øx4½" SDS
	MSTC66	(4)2x	16" - (68)0.148"ø x 3¼"	5,850#	(11)¼"øx6" SDS
	_				

- TENSION TIE TYPES REFER TO SIMPSON STRONG-TIE CATALOG CALLOUTS.
   NAIL PLYWOOD SHEATHING TO STUDS RECEIVING HOLDOWN WITH SCHEDULED PANEL
- EDGE NAILING. STAGGER NAILS SO THAT EACH STUD IS NAILED.

  (3) FASTENERS NOTED IN TABLE ABOVE REPRESENT THE TOTAL AMOUNT. FOR STRAPS,
- HALF OF THE FASTENERS SHALL BE PROVIDED INTO EACH STUD.

  (4) SCREWS SHALL BE SPACED EQUALLY ALONG FULL HEIGHT OF STUD ABOVE TENSION TIE.
- PROVIDE SCREWS AS NOTED IN TABLE AT ONE FACE OF BUILT-UP STUD, AND 10d @ 6" oc NAILS AT OPPOSITE FACE OF BUILT UP STUD.
- ^ DENOTES TENSION TIE THAT OCCURS ATOP OF A FRAMING MEMBER BELOW. FOR HDU2^, PROVIDE A 5/8" Ø LAG SCREW WITH 3" MINIMUM PENETRATION INTO THE BEAM



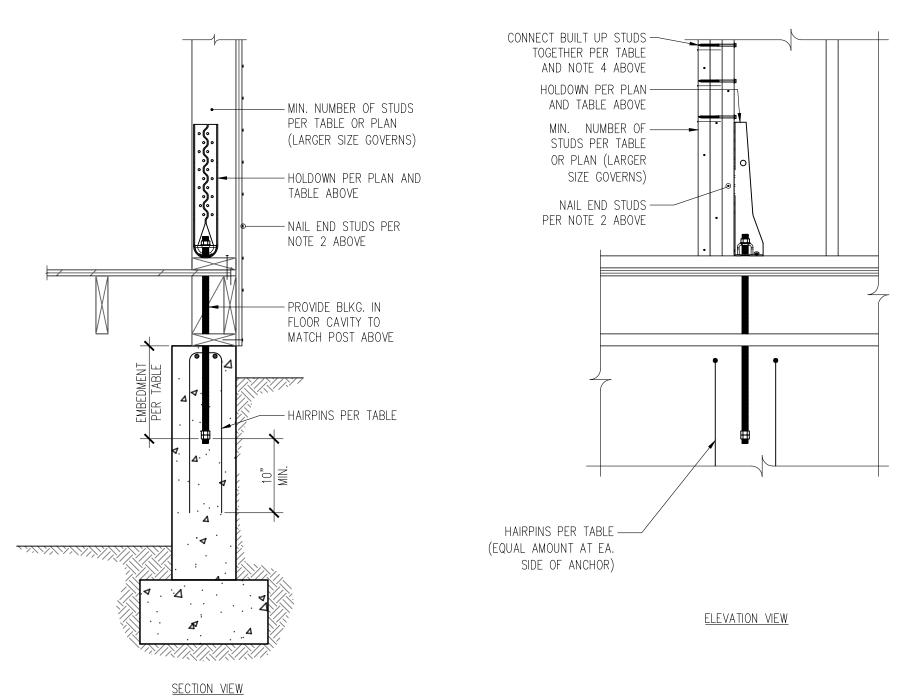
<u>ELEVATION VIEW</u> - TYPICAL CONDITION

<u>ELEVATION VIEW</u> — TENSION TIE ABOVE BEAM

	HOLDOWN T	ENSION TIE SCHE				
TIE $^{\textcircled{1}}$ MIN. NUMBER $^{\textcircled{2}}$ ANCHOR ( $\emptyset$ x EMBEDMI MARK OF STUDS and No. OF HAIRPIN DC		ANCHOR (Ø x EMBEDMENT) $^{rac{\Im}{}}$ and No. OF HAIRPIN DOWELS	FASTENERS FROM TIE TO STUD	ASD CAPACITY	BUILT-UP STUD FACE NAILS or SCREWS ④	
	HDU2	(2)2x	%"ø x 20" − (2)#4 HAIRPIN	(6)1/4"ø x 21/2" SDS SCREWS	3,075#	10d @ 6" oc
	HDU4	(3)2x	%"ø x 20" − (2)#4 HAIRPIN	(10)1/4"ø x 21/2" SDS SCREWS	4,565#	(9)¼"øx4½" SDS
	HDU8	(4)2x	½"ø x 20" − (4)#4 HAIRPIN	(20)¼"ø x 2½" SDS SCREWS	7,870#	(15)¼"øx6" SDS
	HDU11	(5)2x	1"ø x 20" - (4)#4 HAIRPIN	(30)1/4"ø x 21/2" SDS SCREWS	11,175#	(21)¼"øx8" SDS

- TENSION TIE TYPES REFER TO SIMPSON STRONG-TIE CATALOG CALLOUTS.
   NAIL PLYWOOD SHEATHING TO STUDS RECEIVING HOLDOWN WITH SCHEDULED PANEL EDGE NAILING. STAGGER NAILS SO
- ) NAIL PLYWOOD SHEATHING TO STUDS RECEIVING HOLDOWN WITH SCHEDULED PANEL EDGE NAILING. STAGGER NAILS SO THAT EACH STUD IS NAILED.
- ANCHORS SHALL BE HEAVY HEX HEAD WITH DOUBLE NUT CAST INTO CONCRETE.

  ASTM F 1554 Gr. 36 FOR 5% Ø ANCHOR
- ASTM F 1554 Gr. 105 FOR 7/8 ANCHOR ASTM F 1554 Gr. 55 FOR 1"Ø ANCHOR
- (4) SCREWS SHALL BE SPACED EQUALLY ALONG FULL HEIGHT OF STUD ABOVE TENSION TIE. PROVIDE SCREWS AS NOTED IN TABLE AT ONE FACE OF BUILT-UP STUD, AND 10d @ 6" oc NAILS AT OPPOSITE FACE OF BUILT UP STUD.

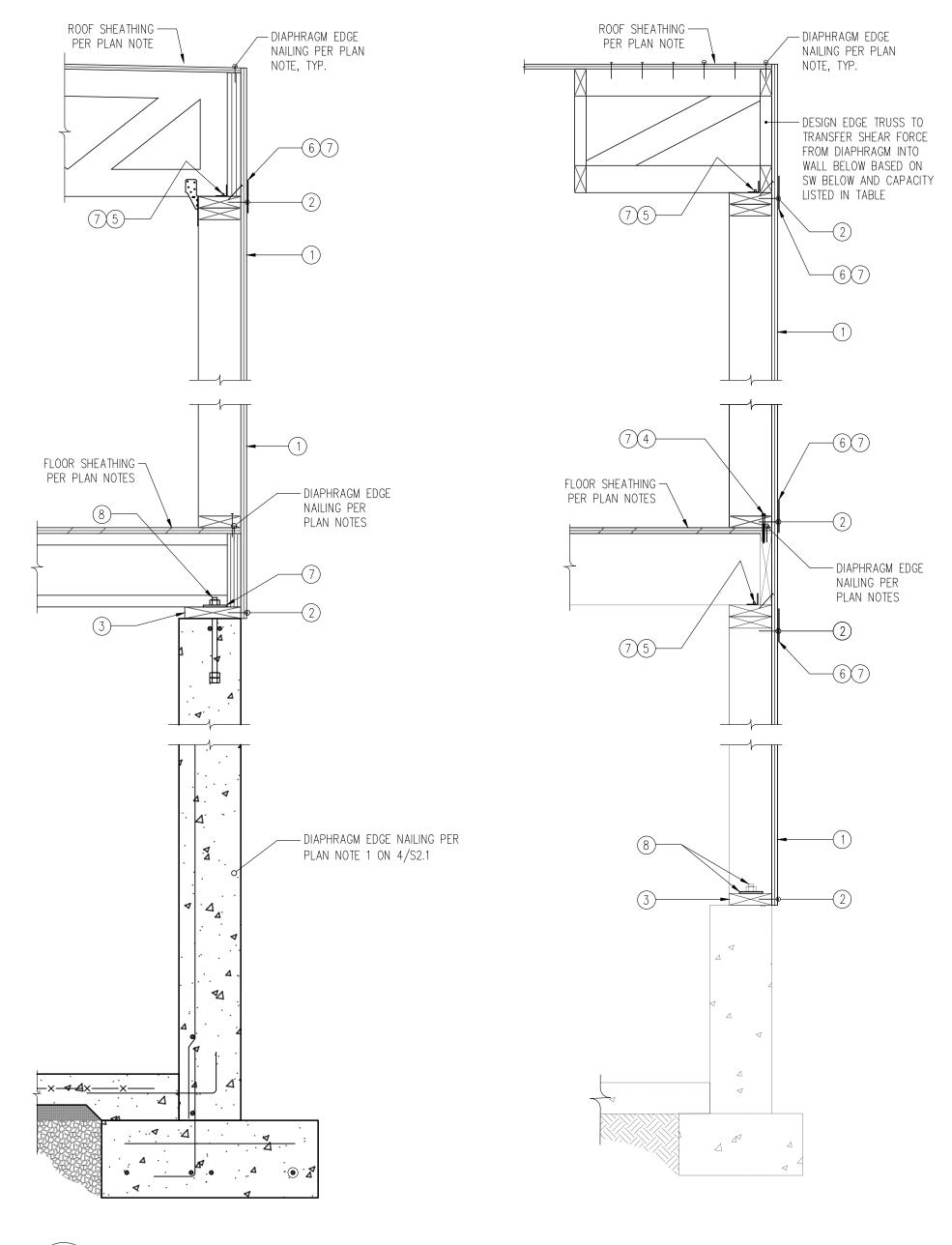




	①	2	③ STUD/BLKG. AT ABUTTING PANEL	⑦ CONN. OF BLKG. OR FRAMING to TOP PLATE; AND SOLE PLATE TO SILL PLATE			ANC BOLT	S TO	ASD CAPACITY,
SHEARWALL PANEL TYPE	SHEATHING THICKNESS	0.131" x 21/4" PANEL NAILING	EDGES & SILL PLATE THICKNESS	④ ¼"ø x 3½" SDS SCREWS	⑤ A35 CLIPS	6 LTP4 PLATES	00l %"ø	VC. 3/4"ø	PLF
SW-6	1/2"	6" oc	2x	18" oc	30" oc	28" oc	48" oc	48" oc	260
SW-4	1/2"	4" oc	3x	12" oc	20" oc	19" oc	46" oc	48" oc	380
SW-3	½"	3" oc	3x	9" oc	15" oc	14" oc	36" oc	48" oc	490
SW-2	1/2"	2" oc	3x	7" oc	12" oc	11" oc	27" oc	38" oc	640
SW-33	1/2"	3" oc EA. SIDE	3x	4" oc	7" oc	7" oc	18" oc	24" oc	980

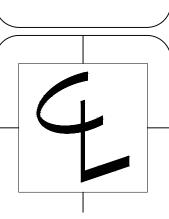
- SHEATHING SHALL CONSIST OF ½" PLYWOOD AND HAVE A MINIMUM SPAN RATING OF 2%; PERMISSIBLE TO RE-USE EXISTING SHEATHING AT EXISTING STUDWALLS IF THICKNESS & SPAN RATING CAN BE VERIFIED AND STUDS & SHEATHING ARE IN SUITABLE CONDITION.
- PANEL NAILING APPLIES TO ALL SHEATHING PANEL EDGES. IF RE-USING EXISTING SHEATHING PER NOTE 1 ABOVE, PROVIDE ADDITIONAL FASTENERS AS REQUIRED TO MEET SPACING REQUIREMENTS. INSTALL BLOCKING AT ALL UNFRAMED PANEL EDGES. ENSURE SHEATHING IS NAILED TO EXISTING INTERMEDIATE FRAMING WITH PANEL NAILS AT 12" oc.
- (3) DOUBLE 2x MEMBERS MAY BE SUBSTITUTED FOR 3x MEMBERS AT WALLS WITH ONLY ONE LAYER OF SHEATHING. 2x MEMBERS SHALL BE NAILED TOGETHER WITH 8d FACE: @ 5" oc FOR SW-6, @ 3½" oc FOR SW-4, @ 2½" oc FOR SW-3, AND @ 2" oc FOR SW-2 (116#/NAIL)
- ROWS OF NAILS AND SDS SCREWS SHALL BE OFFSET AT LEAST ½" AND STAGGERED. MINIMUM EDGE DISTANCE FOR NAILS AND SDS SCREWS INTO EDGE OF MEMBERS SHALL BE 5%" (400#/SCREW)
- (5) A35 CLIPS SHALL BE INSTALLED w/ (12)0.131 x 1½ " NAILS (650#/CLIP)
- (6) LTP4 LATERAL TIE PLATES MAY BE INSTALLED OVER SHEATHING w/ (12)0.131 x 2½" NAILS (625#/CLIP)
- (7) CONTRACTOR SHALL USE A35 or LT4P CLIPS TO CONNECT ROOF TO DOUBLE TOP PLATE
  AND SDS SCRWS or LTP4 CLIPS TO CONNECT SOLE PLATE TO RIM BOARD AT MAIN FLOOR.
- EXTEND SHEATHING TO BOTTOM OF SOLE PLATE AT MAIN FLOOR FOUNDATION WALL AND PROVIDE EDGE FASTENING AS NOTED IN TABLE.
- 8) PLATE WASHERS IN 2x4 STUD WALLS AND <u>ALL</u> SINGLE SIDED SHEAR WALLS SHALL BE 3"x3"x0.229". DOUBLE SIDED 2x6 SHEAR WALLS SHALL HAVE 4½"x3"0.229" PLATE WASHERS. THE EDGE OF PLATE WASHERS SHALL BE LOCATED WITHIN ½" OF THE EDGE OF BOTTOM PLATE ON THE SIDE WITH SHEATHING.
- 9 CAST ANCHORS A MINIMUM OF 7" INTO CONCRETE. INSTALL ADDITIONAL ANCHOR BOLTS AT EACH SIDE OF PLATE BREAKS AND PENETRATIONS EXCEEDING THE "NO REINFORCING" HOLE SIZE PER 2/S6.1.

  AT EXISTING STUD WALLS, A COMBINATION OF EXISTING AND NEW ANCHOR BOLTS CAN BE COUNTED TOWARDS THE SPACING REQUIREMENTS NOTE IN THE TABLE PROVIDED THEY ADHERE TO NOTE #8 ABOVE. NEW ANCHOR BOLTS SHALL BE ¾"Ø HILTI KWIK HUS-EZ SCREW ANCHORS WITH 3" MINIMUM EMBEDMENT INTO CONCRETE. AS AN ALTERNATVIE TO NEW ANCHOR BOLTS, SIMPSON FREP RETROFIT FOUNDATION PLATES WITH (5)¼"Ø SDS SCREWS THAT PENETRATE THE SILL PLATE 2½" MAY BE USED (#1810/PLATE) IF SPACED ACCORDINGLY: @ 72" oc FOR SW-6, @ 56" oc FOR SW-4, @ 42" oc FOR SW-3,@ 32" oc FOR SW-2, AND @ 20" oc FOR SW-33



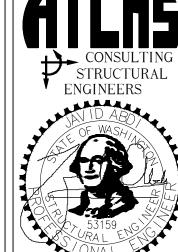






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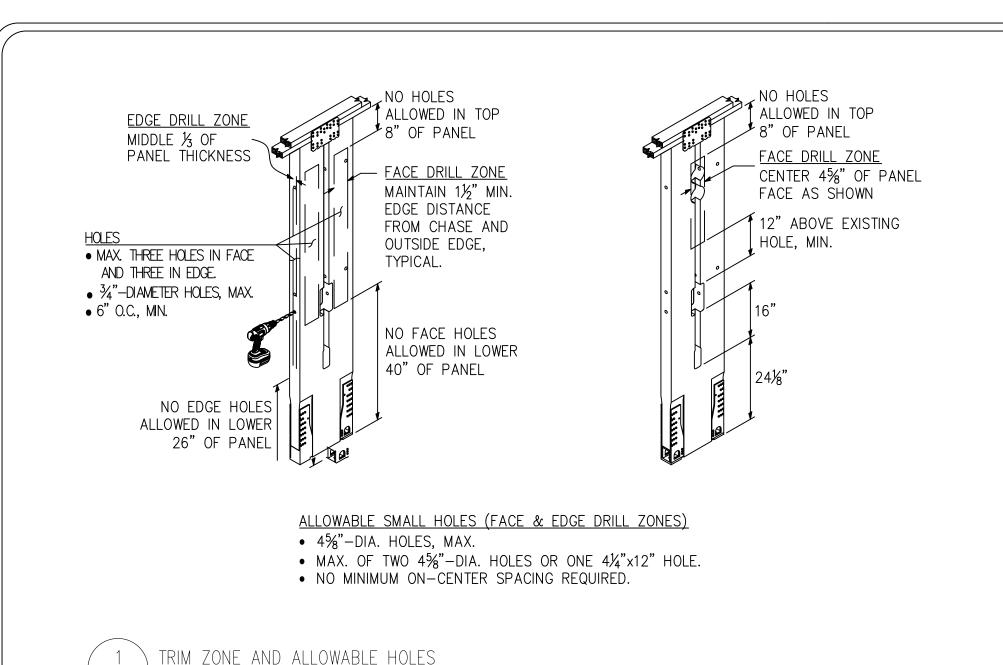
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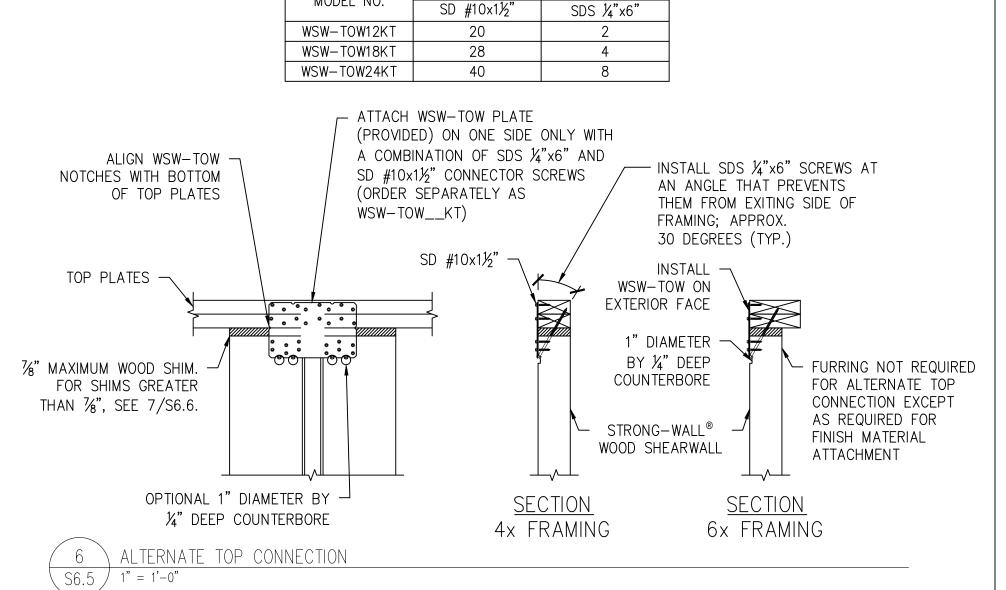
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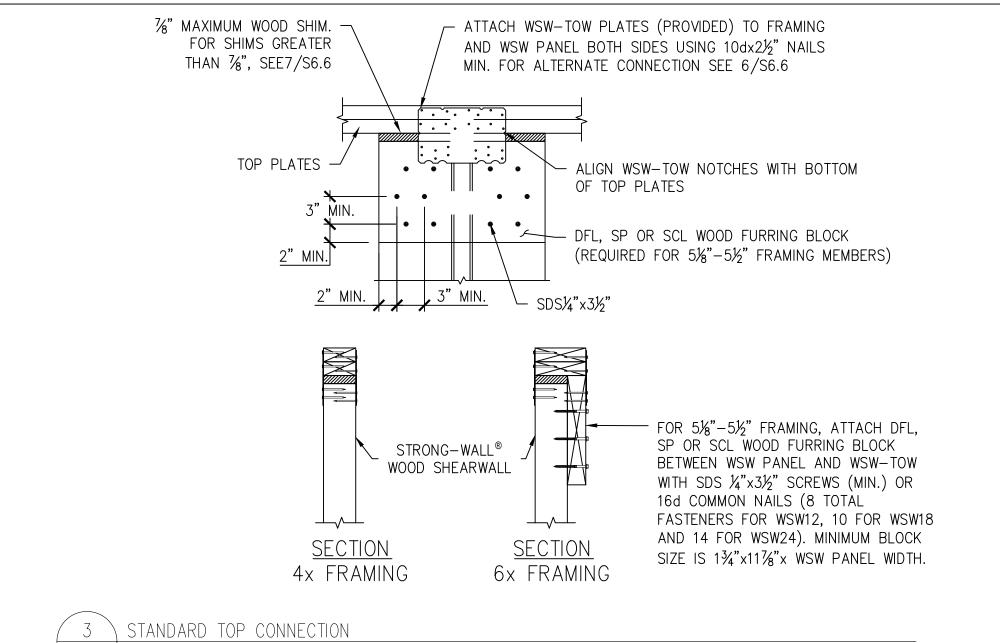
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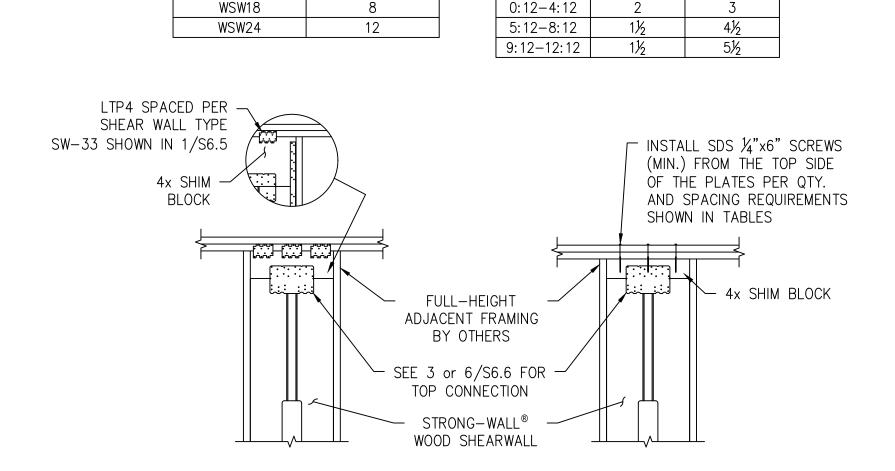




WSW-TOW ALTERNATE CONNECTION KIT

FASTENER QUANTITY







TOP OF WALL HEIGHT ADJUSTMENTS

QTY. OF SDS1/4"x6" SCREWS REQ'D.

WSW12

1" TO 4" SHIM BLOCK

EDGE DISTANCE FOR SCREWS

A (in.)

B (in.)

SLOPE

FOR 8" TO 12" BLOCK DEPTHS: ATTACH SIMPSON STRONG-TIE® CS16 STRAPS AT EDGE OF WSW PANEL (EACH SIDE) USING 10dx11/2" NAILS

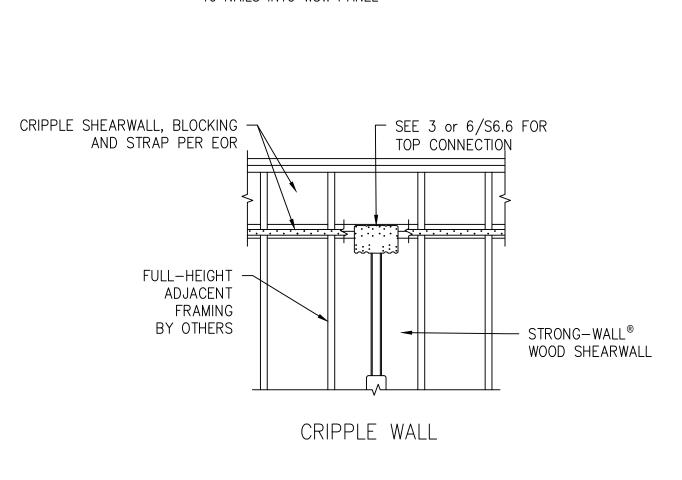
SHIM BLOCK HEIGHTS GREATER THAN 8" AND UP TO 10": 8 NAILS INTO BLOCK

8 NAILS INTO WSW PANEL

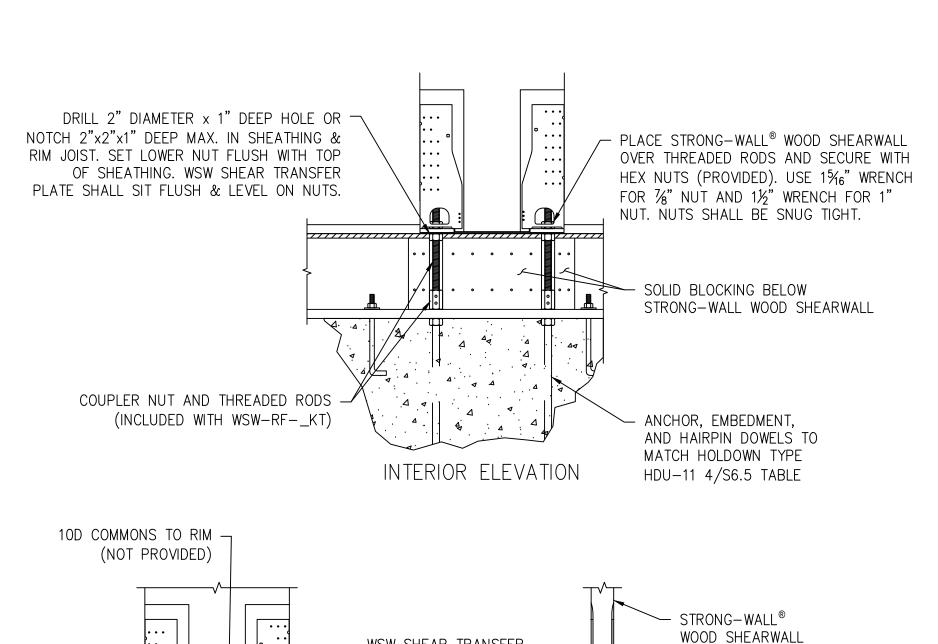
SHIM BLOCK HEIGHTS GREATER THAN 10" AND UP TO 12":

10 NAILS INTO WSW PANEL

10 NAILS INTO BLOCK







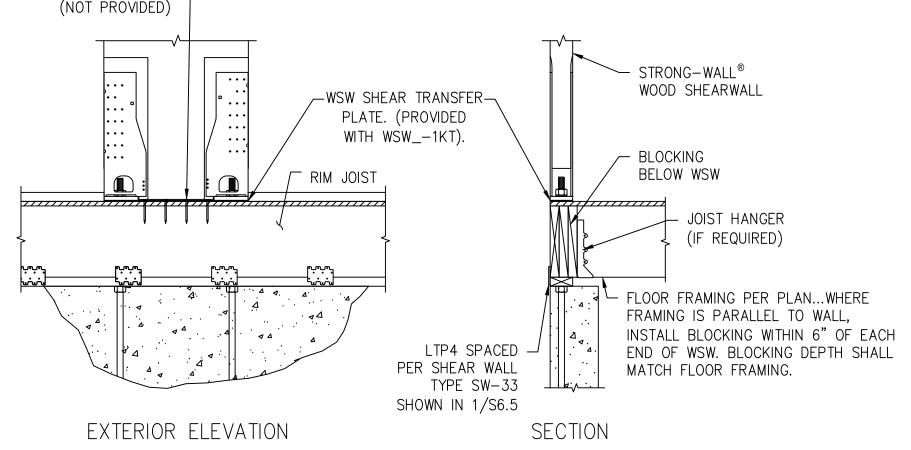
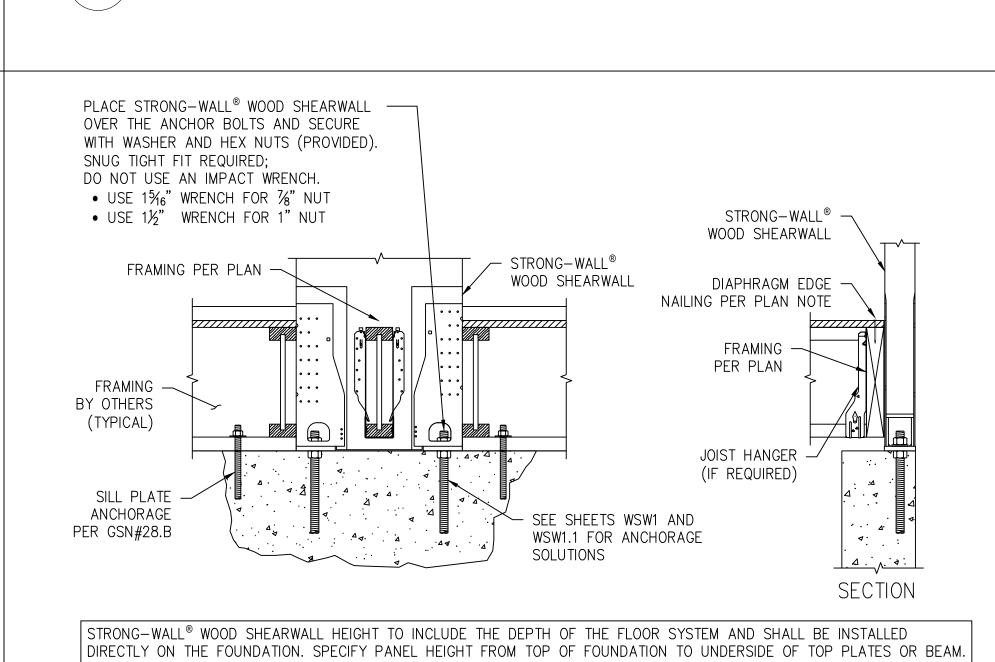


PLATE
18" 1 A36)
TIONS
1

ORDER FIRST FLOOR CONNECTION KIT SEPARATELY. MODEL WSW-RF-\_KT. EXAMPLE WSW-RF-18KT

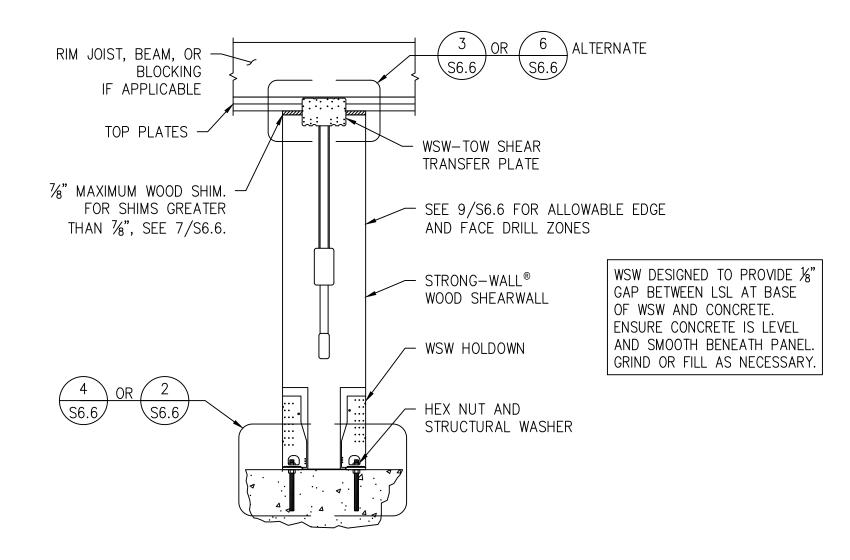


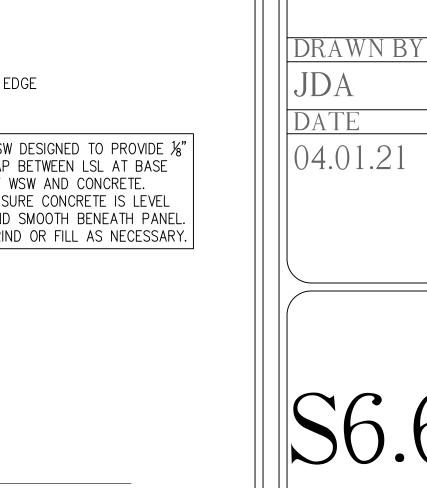
ALTERNATE INSTALLATION AT BASE OF STRONG-WALL

SINGLE STORY WSW ON CONCRETE

S6.6 1" = 1'-0"

S6.6 1" = 1'-0"





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STRUCTURAL **ENGINEERS** ALID AND

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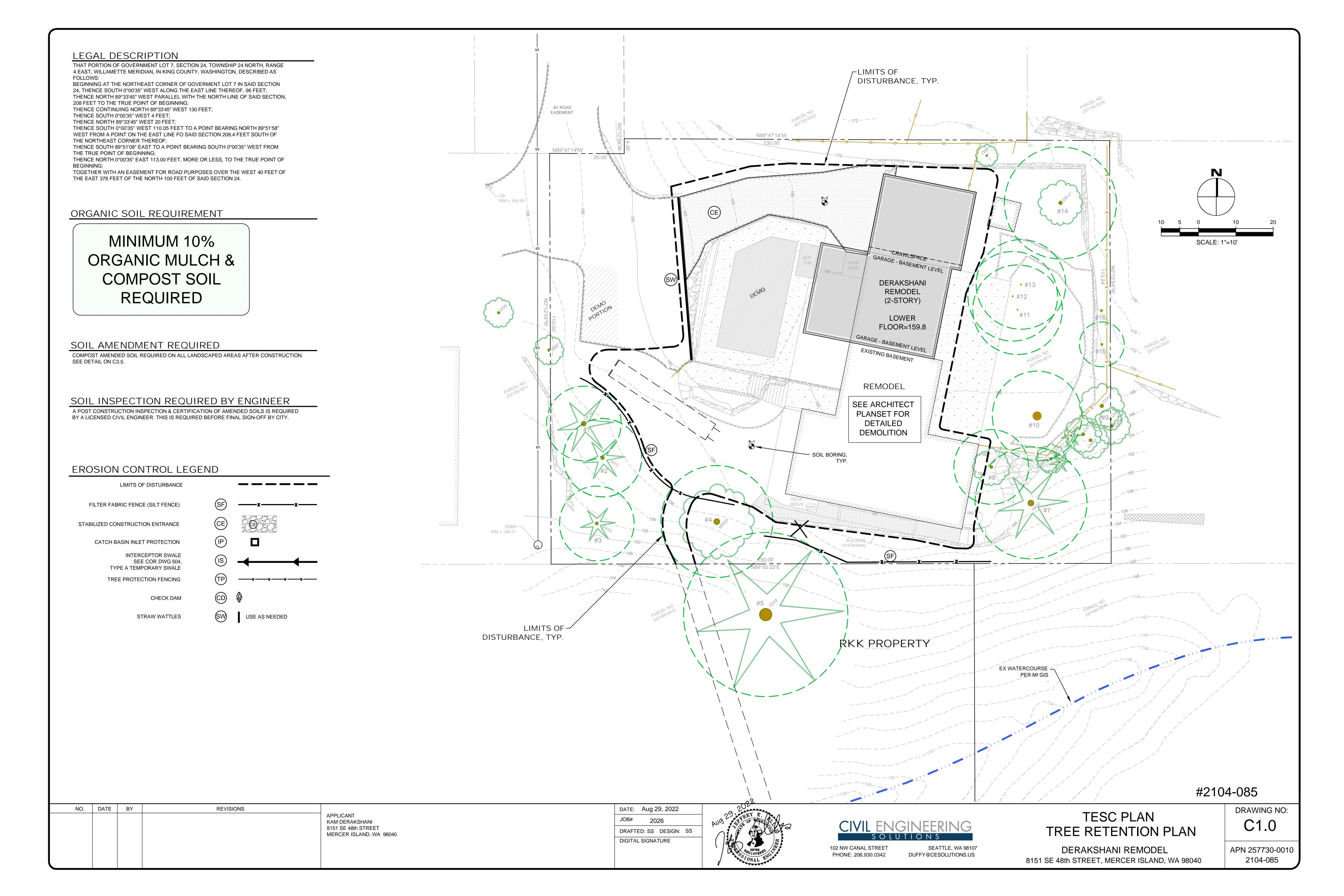
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CONTENTS

Simpson Strong-Wall

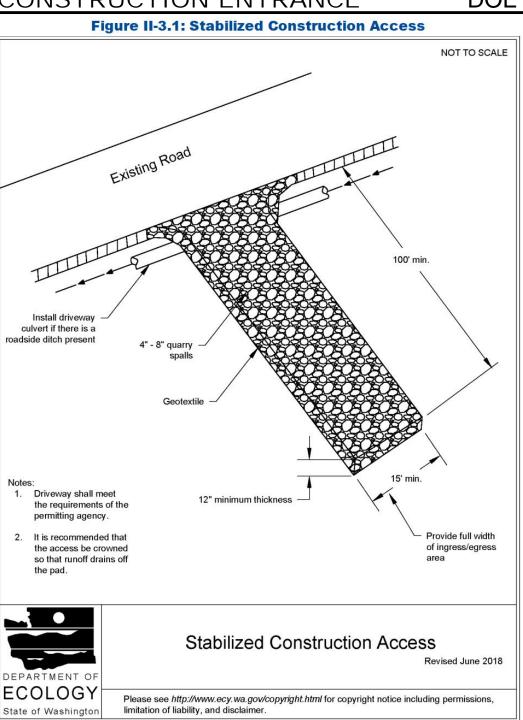
Details



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### CONSTRUCTION ENTRANCE DO



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#### RECOMMENDED CONSTRUCTION SEQUENCE

A DETAILED CONSTRUCTION SEQUENCE IS NEEDED TO ENSURE THAT EROSION AND SEDIMENT CONTROL MEASURES ARE APPLIED AT THE APPROPRIATE TIMES. A RECOMMENDED CONSTRUCTION SEQUENCE IS PROVIDED BELOW:

1. HOLD AN ONSITE PRE-CONSTRUCTION MEETING.

2. POST SIGN WITH NAME AND PHONE NUMBER OF ESC SUPERVISOR (MAY BE CONSOLIDATED WITH THE REQUIRED NOTICE OF CONSTRUCTION SIGN).

3. FLAG OR FENCE CLEARING LIMITS.

4. INSTALL CATCH BASIN PROTECTION, IF REQUIRED.

5. GRADE AND INSTALL CONSTRUCTION ENTRANCE(S).

6. INSTALL PERIMETER PROTECTION (SILT FENCE, BRUSH BARRIER, ETC.).

7. CONSTRUCT SEDIMENT PONDS AND TRAPS.

8. GRADE AND STABILIZE CONSTRUCTION ROADS.

9. CONSTRUCT SURFACE WATER CONTROLS (INTERCEPTOR DIKES, PIPE SLOPE DRAINS, ETC.) SIMULTANEOUSLY WITH CLEARING AND GRADING FOR PROJECT DEVELOPMENT.

10. MAINTAIN EROSION CONTROL MEASURES IN ACCORDANCE WITH CITY OF MERCER ISLAND STANDARDS AND MANUFACTURER'S RECOMMENDATIONS.

11. RELOCATE SURFACE SURFACE WATER CONTROLS OR TESC MEASURES, OR INSTALL NEW MEASURES SO THAT AS SITE CONDITIONS CHANGE, THE TESC IS ALWAYS IN ACCORDANCE WITH CITY OF MERCER ISLAND TESC REQUIREMENTS.

12. COVER ALL AREAS THAT WILL BE UN-WORKED FOR MORE THAN SEVEN DAYS DURING THE DRY SEASON (MAY 1 TO SEPT 30) OR TWO DAYS DURING THE WET SEASON (OCT 1 TO APRIL 30) WITH STRAW, WOOD FIBER MULCH, COMPOST, PLASTIC SHEETING, OR EQUIVALENT.

13. STABILIZE ALL AREAS WITHIN SEVEN DAYS OF REACHING FINAL GRADE.

14. SEED, SOD, STABILIZE, OR COVER ANY AREAS TO REMAIN UNWORKED FOR MORE THAN 30 DAYS.

15. UPON COMPLETION OF THE PROJECT, STABILIZE ALL DISTURBED AREAS AND REMOVE BMPS IF APPROPRIATE.

## DENUDED AREAS REQUIREMENTS

APRIL 1 TO SEPT 30

ALL DENUDED AREAS MUST BE STABILIZED WITHIN 7 DAYS OF CONSTRUCTION. PLEASE READ ALL CITY TESC NOTES ON SHEET C1.2.

OCT 1 TO MARCH 31

ALL DENUDED AREAS MUST BE STABILIZED WITHIN 2 DAYS OF GRADING. IF AN EROSION PROBLEM ALREADY EXISTS ON THE SITE, OTHER COVER PROTECTION AND EROSION CONTROL WILL BE REQUIRED.

#### **EROSION CONTROL NOTES**

D.8.2 STANDARD ESC PLAN NOTES

THE STANDARD ESC PLAN NOTES MUST BE INCLUDED ON ALL ESC PLANS. AT THE APPLICANT'S DISCRETION, NOTES THAT IN NO WAY APPLY TO THE PROJECT MAY BE OMITTED; HOWEVER, THE REMAINING NOTES MUST NOT BE RENUMBERED. FOR EXAMPLE, IF ESC NOTE #3 WERE OMITTED, THE REMAINING NOTES SHOULD BE NUMBERED 1, 2, 4, 5, 6, ETC.

1. APPROVAL OF THIS EROSION AND SEDIMENTATION CONTROL (ESC) PLAN DOES NOT CONSTITUTE AN APPROVAL OF PERMANENT ROAD OR DRAINAGE DESIGN (E.G., SIZE AND LOCATION OF ROADS, PIPES, RESTRICTORS, CHANNELS, RETENTION FACILITIES, UTILITIES, ETC.).

2. THE IMPLEMENTATION OF THESE ESC PLANS AND THE CONSTRUCTION, MAINTENANCE, REPLACEMENT, AND

UPGRADING OF THESE ESC FACILITIES IS THE RESPONSIBILITY OF THE APPLICANT/ESC SUPERVISOR UNTIL ALL CONSTRUCTION IS APPROVED.

3. THE BOUNDARIES OF THE CLEARING LIMITS SHOWN ON THIS PLAN SHALL BE CLEARLY FLAGGED BY SURVEY TAPE OR FENCING, IF REQUIRED, PRIOR TO CONSTRUCTION (SWDM APPENDIX D). DURING THE CONSTRUCTION PERIOD, NO DISTURBANCE BEYOND THE CLEARING LIMITS SHALL BE PERMITTED. THE CLEARING LIMITS SHALL BE MAINTAINED BY THE APPLICANT/ESC SUPERVISOR FOR THE DURATION OF CONSTRUCTION.

4. STABILIZED CONSTRUCTION ENTRANCES SHALL BE INSTALLED AT THE BEGINNING OF CONSTRUCTION AND MAINTAINED FOR THE DURATION OF THE PROJECT. ADDITIONAL MEASURES, SUCH AS CONSTRUCTED WHEEL WASH SYSTEMS OR WASH PADS, MAY BE REQUIRED TO ENSURE THAT ALL PAVED AREAS ARE KEPT CLEAN AND TRACK OUT TO ROAD RIGHT OF WAY DOES NOT OCCUR FOR THE DURATION OF THE PROJECT.

5. THE ESC FACILITIES SHOWN ON THIS PLAN MUST BE CONSTRUCTED PRIOR TO OR IN CONJUNCTION WITH ALL CLEARING AND GRADING SO AS TO ENSURE THAT THE TRANSPORT OF SEDIMENT TO SURFACE WATERS, DRAINAGE SYSTEMS, AND ADJACENT PROPERTIES IS MINIMIZED.

6. THE ESC FACILITIES SHOWN ON THIS PLAN ARE THE MINIMUM REQUIREMENTS FOR ANTICIPATED SITE CONDITIONS. DURING THE CONSTRUCTION PERIOD, THESE ESC FACILITIES SHALL BE UPGRADED AS NEEDED FOR UNEXPECTED STORM EVENTS AND MODIFIED TO ACCOUNT FOR CHANGING SITE CONDITIONS (E.G. ADDITIONAL COVER MEASURES, ADDITIONAL SUMP PUMPS, RELOCATION OF DITCHES AND SILT FENCES, PERIMETER PROTECTION ETC.) AS DIRECTED BY CITY OF MERCER ISLAND.

7. THE ESC FACILITIES SHALL BE INSPECTED DAILY BY THE APPLICANT/ESC SUPERVISOR AND MAINTAINED TO ENSURE CONTINUED PROPER FUNCTIONING. WRITTEN RECORDS SHALL BE KEPT OF WEEKLY REVIEWS OF THE ESC FACILITIES.

8. ANY AREAS OF EXPOSED SOILS, INCLUDING ROADWAY EMBANKMENTS, THAT WILL NOT BE DISTURBED FOR TWO CONSECUTIVE DAYS DURING THE WET SEASON OR SEVEN DAYS DURING THE DRY SEASON SHALL BE IMMEDIATELY STABILIZED WITH THE APPROVED ESC METHODS (E.G., SEEDING, MULCHING, PLASTIC COVERING, ETC.).

9. ANY AREA NEEDING ESC MEASURES THAT DO NOT REQUIRE IMMEDIATE ATTENTION SHALL BE ADDRESSED WITHIN SEVEN (7) DAYS.

10. THE ESC FACILITIES ON INACTIVE SITES SHALL BE INSPECTED AND MAINTAINED A MINIMUM OF ONCE A MONTH DURING THE DRY SEASON, BI-MONTHLY DURING THE WET SEASON, OR WITHIN TWENTY FOUR (24) HOURS FOLLOWING A STORM EVENT.

11. AT NO TIME SHALL MORE THAN ONE (1) FOOT OF SEDIMENT BE ALLOWED TO ACCUMULATE WITHIN A CATCH BASIN. ALL CATCH BASINS AND CONVEYANCE LINES SHALL BE CLEANED PRIOR TO PAVING. THE CLEANING OPERATION SHALL NOT FLUSH SEDIMENT-LADEN WATER INTO THE DOWNSTREAM SYSTEM.

12. ANY PERMANENT RETENTION/DETENTION FACILITY USED AS A TEMPORARY SETTLING BASIN SHALL BE MODIFIED WITH THE NECESSARY EROSION CONTROL MEASURES AND SHALL PROVIDE ADEQUATE STORAGE CAPACITY. IF THE FACILITY IS TO FUNCTION ULTIMATELY AS AN INFILTRATION SYSTEM, THE TEMPORARY FACILITY MUST BE ROUGH GRADED SO THAT THE BOTTOM AND SIDES ARE AT LEAST THREE FEET ABOVE THE FINAL GRADE OF THE PERMANENT FACILITY.

13. COVER MEASURES WILL BE APPLIED IN CONFORMANCE WITH APPENDIX D OF THE SURFACE WATER DESIGN MANUAL

14. PRIOR TO THE BEGINNING OF THE WET SEASON (OCT. 1), ALL DISTURBED AREAS SHALL BE REVIEWED TO IDENTIFY WHICH ONES CAN BE SEEDED IN PREPARATION FOR THE WINTER RAINS. DISTURBED AREAS SHALL BE SEEDED WITHIN ONE WEEK OF THE BEGINNING OF THE WET SEASON.

#### CITY NOTES

 ANY CHANGES TO THE APPROVED PLANS REQUIRES CITY APPROVAL THROUGH A REVISION.

2. APPLICANT IS RESPONSIBLE FOR ANY DAMAGES TO UNDERGROUND UTILITIES CAUSED FROM THIS CONSTRUCTION.

3. CATCH BASIN FILTERS SHOULD BE PROVIDED FOR ALL STORM DRAIN CATCH BASINS/INLETS DOWNSLOPE AND WITHIN 500 FEET OF THE CONSTRUCTION AREA. CATCH BASIN FILTERS SHOULD BE DESIGNED BY THE MANUFACTURER FOR USE AT CONSTRUCTION SITES AND APPROVED BY THE CITY INSPECTOR. CATCH BASIN FILTERS SHOULD BE INSPECTED FREQUENTLY, ESPECIALLY AFTER STORM EVENTS. IF THE FILTER BECOMES CLOGGED, IT SHOULD BE CLEANED OR REPLACED.

4. CONTRACTORS SHALL VERIFY LOCATIONS AND DEPTHS OF UTILITES.

5. AT LEAST 48 HOURS PRIOR TO CONSTRUCTION, CALL "ONE CALL" AT 1.800.424.5555

6. DO NOT BACKFILL WITH NATIVE MATERIAL ON PUBLIC RIGHT-OF-WAY. ALL MATERIAL MUST BE IMPORTED

7. EROSION CONTROL: ALL "LAND DISTURBING ACTIVITY" IS SUBJECT TO PROVISIONS OF MERCER ISLAND ORDINANCE 95C-118 "STORM WATER MANAGEMENT." SPECIFIC ITEMS TO BE FOLLOWED AT YOUR SITE:

8. PROTECT ADJACENT PROPERTIES FROM ANY INCREASED RUNOFF OR SEDIMENTATION DUE TO THE CONSTRUCTION PROJECT THROUGH THE USE OF APPROPRIATE "BEST MANAGEMENT PRACTICES" (BMP) EXAMPLES INCLUDE, BUT ARE NOT LIMITED TO, SEDIMENT TRAPS, SEDIMENT PONDS, FILTER FABRIC FENCES, VEGETATIVE BUFFER STRIPS OR BIOENGINEERED SWALES.

9. CONSTRUCTION ACCESS TO THE SITE SHOULD BE LIMITED TO ONE ROUTE. STABILIZE ENTRANCE WITH QUARRY SPALLS TO PREVENT SEDIMENT FROM LEAVING THE SITE OR ENTERING THE STORM DRAINS.

10. PREVENT SEDIMENT, CONSTRUCTION DEBRIS, PAINTS, SOLVENTS, ETC., OR OTHER TYPES OF POLLUTION FROM ENTERING PUBLIC STORM DRAINS. KEEP ALL POLLUTION ON YOUR SITE.

11. ALL EXPOSED SOILS SHALL REMAIN DENUDED FOR NO LONGER THAN SEVEN (7) DAYS AND SHALL BE STABILIZED WITH MULCH, HAY, OR THE APPROPRIATE GROUND COVER. ALL EXPOSED SOILS SHALL BE COVERED IMMEDIATELY DURING ANY RAIN EVENT.

12. INSTALLATION OF CONCRETE DRIVEWAYS, TREES, SHRUBS, IRRIGATION, BOULDERS, BERMS, WALLS, GATES, AND OTHER IMPROVEMENTS ARE NOT ALLOWED IN THE PUBLIC RIGHT-OF-WAY WITHOUT PRIOR APPROVAL, AND AN ENCROACHMENT AGREEMENT AND RIGHT OF WAY PERMIT FROM THE SENIOR DEVELOPMENT ENGINEER.

13. OWNER SHALL CONTROL DISCHARGE OF SURFACE DRAINAGE RUNOFF FROM EXISTING AND NEW IMPERVIOUS AREAS IN A RESPONSIBLE MANNER. CONSTRUCTION OF NEW GUTTERS AND DOWNSPOUTS, DRY WELLS, LEVEL SPREADERS OR DOWNSTREAM CONVEYANCE PIPE MAY BE NECESSARY TO MINIMIZE DRAINAGE IMPACT TO YOUR NEIGHBORS. CONSTRUCTION OF MINIMUM DRAINAGE IMPROVEMENTS SHOWN OR CALLED OUT ON THIS PLAN DOES NOT IMPLY RELIEF FROM CIVIL LIABILITY FOR YOUR DOWNSTREAM DRAINAGE.

14. POT HOLING THE PUBLIC UTILITIES IS REQUIRED PRIOR TO ANY GRADING ACTIVITIES LESS THAN 6" OVER THE PUBLIC MAINS (WATER, SEWER AND STORM SYSTEMS). IF THERE IS A CONFLICT, THE APPLICANT IS REQUIRED TO SUBMIT A REVISION FOR APPROVAL PRIOR TO ANY GRADING ACTIVITIES OVER THE PUBLIC MAINS

15. REMEMBER: EROSION CONTROL IS YOUR FIRST INSPECTION.

16. ROOF DRAINS MUST BE CONNECTED TO THE STORM DRAIN SYSTEM AND INSPECTED BY THE PUBLIC WORKS DEPARTMENT PRIOR TO ANY BACKFILLING OF PIPE.

17. SILENT FENCE: CLEAN AND PROVIDE REGULAR MAINTENANCE OF THE SILT FENCE. THE FENCE IS TO REMAIN VERTICAL AND IS TO FUNCTION PROPERLY THROUGHOUT THE TERM OF THE PROJECT.

18. WORK IN PUBLIC RIGHT OF WAY REQUIRES A RIGHT-OF-WAY USE PERMIT.

19. REFER TO WATER SERVICE PERMIT FOR ACTUAL LOCATION OF NEW WATER METER AND SERVICE LINE DETERMINED BY MERCER ISLAND WATER DEPARTMENT.

16. THE TV INSPECTION OF THE EXISTING SIDE SEWER TO THE CITY SEWER MAIN IS REQUIRED. IF THE RESULT OF THE TV INSPECTION IS NOT IN SATISFACTORY CONDITION, AS DETERMINED BY THE CITY OF MERCER ISLAND INSPECTOR, THE REPLACEMENT OF THE EXISTING SIDE SEWER IS REQUIRED. ALTERNATELY, A PRESSURE TEST OF THE SIDE SEWER, FROM SEWER MAIN TO POINT OF CONNECTION, MAY BE SUBSTITUTED FOR THE VIDEO INSPECTION.

20. NEWLY INSTALLED SIDE SEWER REQUIRES A 4 P.S.I. AIR TEST OR PROVIDE 10' OF HYDROSTATIC HEAD TEST.

21. POT HOLING THE PUBLIC UTILITIES IS REQUIRED PRIOR TO ANY GRADING ACTIVITIES LESS THAN 6" OVER THE PUBLIC MAINS (WATER, SEWER AND STORM SYSTEMS). IF THERE IS A CONFLICT, THE APPLICANT IS REQUIRED TO SUBMIT A REVISION FOR APPROVAL PRIOR TO ANY GRADING ACTIVITIES OVER THE PUBLIC MAINS.

22. THE LIMITS AND EXTENDS OF THE PAVEMENT IN THE PUBLIC RIGHT OF WAY SHALL BE DETERMINED BY THE CITY ENGINEER PRIOR TO FINALIZE THE PROJECT.

#2104-085

NO. DATE BY REVISIONS

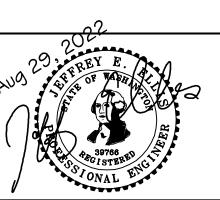
APPLICANT
KAM DERAKSHANI
8151 SE 48th STREET
MERCER ISLAND, WA 98040

DATE: Aug 29, 2022

JOB# 2026

DRAFTED: SS DESIGN: DE

DIGITAL SIGNATURE



CIVIL ENGINEERING SOLUTIONS

SEATTLE, WA 98107

DUFFY@CESOLUTIONS.US

**102 NW CANAL STREET** 

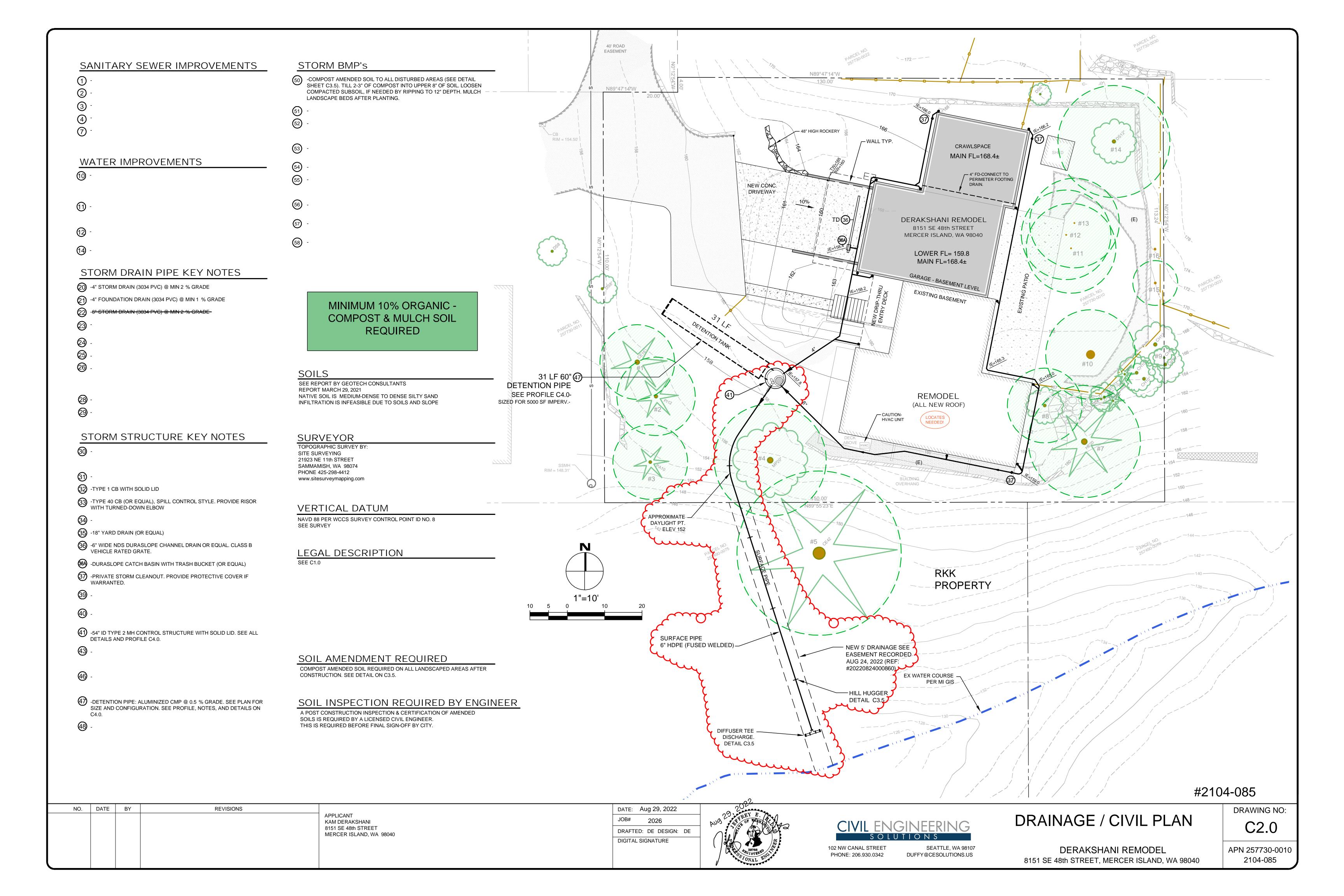
PHONE: 206.930.0342

TESC & CITY NOTES
TESC DETAILS
DERAKSHANI REMODEL

8151 SE 48th STREET, MERCER ISLAND, WA 98040

DRAWING NO:

APN 257730-0010 2104-085



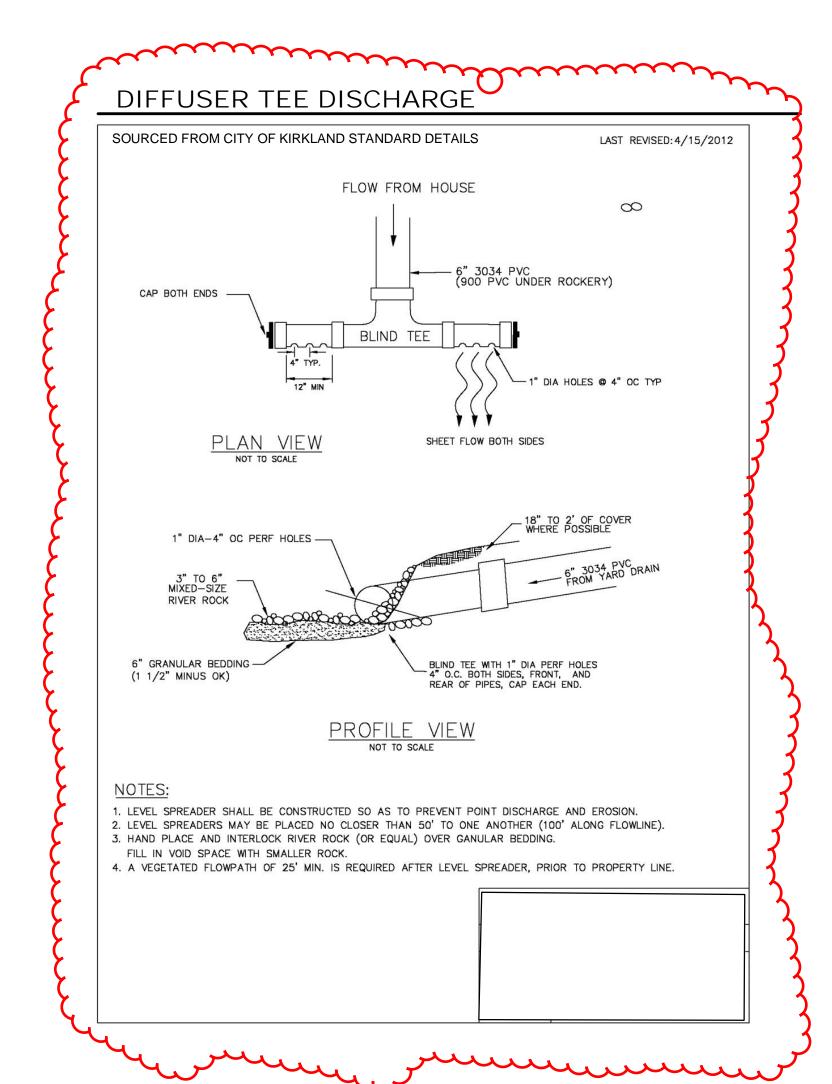
# MINIMUM 10% ORGANIC -COMPOST SOIL REQUIRED

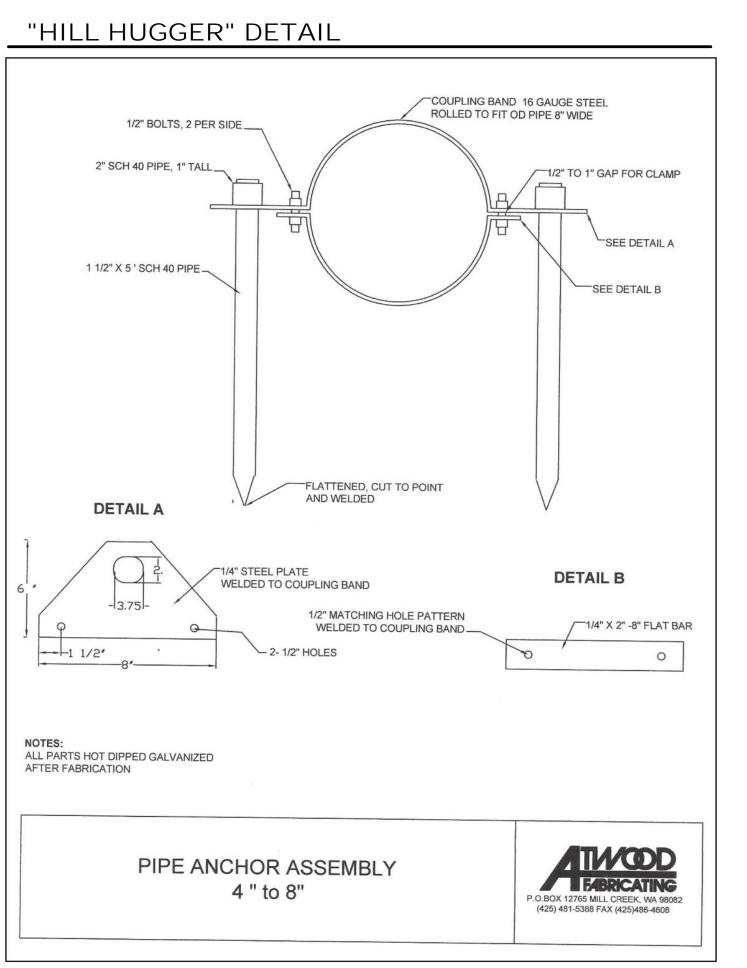
### SOIL AMENDMENT REQUIRED

COMPOST AMENDED SOIL REQUIRED ON ALL LANDSCAPED AREAS AFTER CONSTRUCTION. SEE DETAIL BELOW.

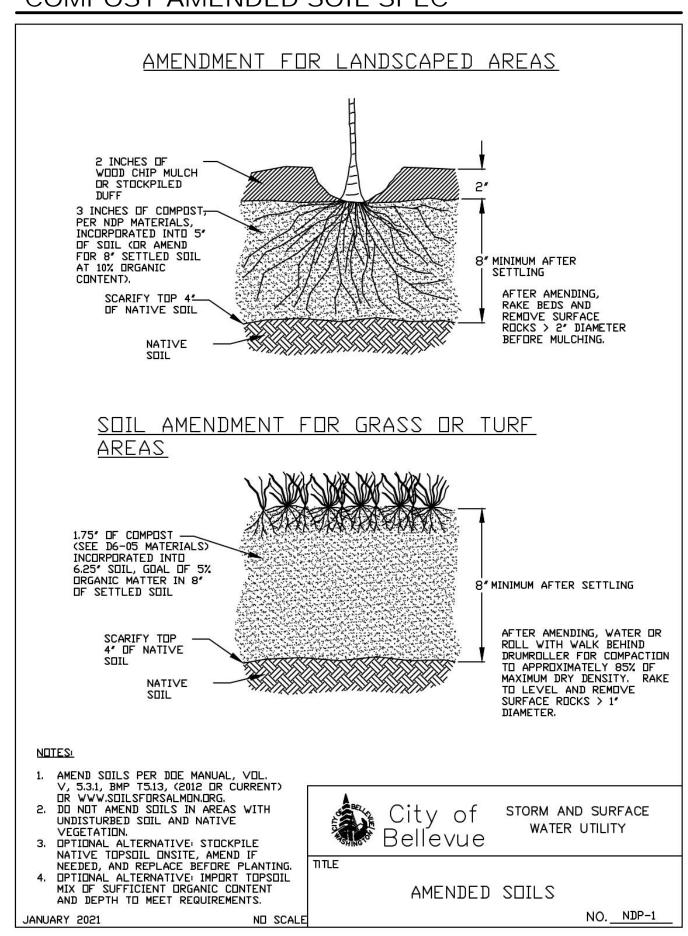
#### SOIL INSPECTION REQUIRED BY ENGINEER

A POST CONSTRUCTION INSPECTION & CERTIFICATION OF AMENDED SOILS IS REQUIRED BY A LICENSED CIVIL ENGINEER. THIS IS REQUIRED BEFORE FINAL SIGN-OFF BY CITY.





## COMPOST AMENDED SOIL SPEC



## #2104-085

NO.	DATE	BY	REVISIONS	
				APPLICANT KAM DERAKSHANI 8151 SE 48th STREET MERCER ISLAND, WA 98040

DATE: Aug 29, 2022 2026 DRAFTED: SS DESIGN: SS DIGITAL SIGNATURE





102 NW CANAL STREET SEATTLE, WA 98107 PHONE: 206.930.0342 DUFFY@CESOLUTIONS.US

STORM, BMP DETAILS

DERAKSHANI REMODEL

DRAWING NO: C3.5

APN 257730-0010 2104-085 8151 SE 48th STREET, MERCER ISLAND, WA 98040

MERCER ISLAND DETENTION "TABLE 1" **DETENTION PROFILE** SCALE: HORIZONTAL 1"=10', VERTICAL 1"=5' ON-SITE DETENTION DESIGN FOR PROJECTS BETWEEN 500 SF AND 9,500 SF NEW PLUS REPLACED IMPERVIOUS SURFACE AREA ⊢60" DIA TANK Distance from Outlet Invert Second Orifice New and Replaced mpervious Surface Area 500 to 1,000 sf 3.3 INCOMING 4" SD FOR REFERENCE TOP OF RISER 1,001 to 2,000 sf 0.5 3.2 =5.10' ABOVE IE 2" VENT-- INSERTA-TEE - DAYLIGHT PIPE, USE 2,001 to 3,000 sf 0.5 3.1 2.8 0.9 RESTRAINED BEND OR (OR EQUAL, CONTINUOUS HDPE 3,001 to 4,000 sf 42 0.5 2.8 2.9 0.8 0.5 1.3 31 LF @ - ELBOW RESTRICTOR FUSE WELDED PIPE) 156 156 3.8 0.9 0.50% T. ON FIVE & J.5 FT ABOVE IE OUT 60" SD 4,001 to 5,000 sf 27 LF- 6" SD @ 1.0% -FUSE WELDED HDPE SURFACE PIPE SEE C2.0 FOR ALIGNMENT 152 5,001 to 6,000 sf 0.5 3.5 2.9 1.7 & RECORDED EASEMENT# THRU RKK PROPERTY. 60"->36" REDUCER -3.7 2.9 6,001 to 7,000 sf 0.5 RESTRICTOR PLATE 0.5" Ø ORIFICE 0.5 7,001 to 8,000 sf 3.8 2.9 144 8,001 to 8,500 sf<sup>(1)</sup> 3.7 2.9 0.5 NA (1) 2.9 0.5 NA (1) 8,501 to 9,000 sf 140 NA (1) 2.2 NA (1) 0.5 2.1 94 0.5 0.5 NA (1) 2.9 NA (1) 2.0 9,001 to 9,500 sf<sup>(2)</sup> 136 • Minimum Requirement #7 (Flow Control) is required when the 100-year flow frequency causes a 0.15 cubic feet per second increase (when modeled in WWHM with a 15-minute timestep). Breakpoints shown in this table are based on a flat slope (0-5%). The 100-year flow frequency will need to be evaluated on a site-specific basis for projects on moderate (5-15%) or steep (> 15%) slopes. Soil type to be determined by geotechnical analysis or soil map. **Basis of Sizing Assumptions:**  Sizing includes a Volume Correction Factor of 120%. Sized per MR#5 in the Stormwater Management Manual for 132 Upper bound contributing area used for sizing. Puget Sound Basin (1992 Ecology Manual) <sup>(1)</sup>On Type B soils, new plus replaced impervious surface areas DATUM ELEV SBUH, Type 1A, 24-hour hydrograph exceeding 8,500 sf trigger Minimum Requirement #7 (Flow Control) 2-year, 24-hour storm = 2 in; 10-year, 24-hour <sup>(2)</sup>On Type C soils, new plus replaced impervious surface areas storm = 3 in; 100-year, 24-hour storm = 4 in exceeding 9,500 sf trigger Minimum Requirement #7 (Flow Control) Predeveloped = second growth forest (CN = 72 for Type B (3) Minimum orifice diameter = 0.5 inches soils, CN = 81 for Type C soils) in = inch Developed = impervious (CN = 98) ft = feet 0.5 foot of sediment storage in detention pipe sf = square feet Overland slope = 5% 11+00 10+00 IMPERVIOUS TABLE - STORMWATER MERCER ISLAND DETENTION DETAIL Impervious Area Spreadsheet - Stormwater Version ATTACHMENT 1 CITY OF MERCER ISLAND Derekshani Remodel - 8151 SE 48th Street, Mercer Island, WA 98040 ON-SITE DETENTION SYSTEM WORKSHEET (FOR NEW PLUS REPLACED IMPERVIOUS 16,963 Gross Site area AREA OF 9,500 SF OR LESS) 0.389 acres 6" 6" PLATE WELDED TO ELBOW WITH ORIFICE AS SPECIFIED OWNER: DERAKANI ADDRESS: 8151 SE 48th Street PREPARED BY: DUFFY ELLIS, P.E. Existing roof to remain 2,117 Mercer Island, WA 98040 PHONE: **ELBOW RESTRICTOR DETAIL** NEW PLUS REPLACED IMPERVIOUS 4800 SF DETENTION PIPE DIA (INCH): 60" DIA DETENTION PIPE LENGTH (FT): 31 LF ORIFICE #1 DIA \* INCH, ELEV \* Proposed Impervious Area SOIL TYPE: Type C (TILL) PIPE MATERIAL: CMP OR HDPE ORIFICE #2 DIA \_\* INCH, ELEV \_\_ \* PLAN VIEW Roof over new foundation (north side) 2,152 sf concrete Driveway \*SEE TABLE 1, THIS SHEET concrete walkway sf total on-site new + replaced = sf sf existing impervious to remain = total impervious = 4,801 elbow Restrictor See Detail — MIN. 12" RISER TO CITY APPROVED DISCHARGE POINT DETENTION PIPE TIGHTLINE INV. ELEV\_\_\_ 6" TYP ON SINGLE -FAMILY. SEE C2.0 - INVERT ELEV \_\_\_\_\_ SEE NOTES

② & ⑤ -8" SHEAR GATE WITH CONTROL ROD FOR UPPER CATCH BASIN (SEE CONTROL STRUCTURE CLEANOUT/DRAIN (ROD BENT AS REQUIRED FOR VERTICAL ALIGNMENT WITH COVER) INVERT & ELEVATION PER PLANS CONTROL STRUCTURE 1' SECTION OF PIPE ATTACHED BY GASKETED BAND TO ALLOW REMOVAL (SEE DETAIL THIS SHEET) ON-SITE DETENTION SYSTEM NOT TO SCALE (ENGINEER TO FILL IN BLANKS) RESTRICTOR PLATE WITH ORIFICE DIAM. AS SPECIFIED OF CONTROL STRUCTURE DETAIL NOT TO SCALE ON-SITE DETENTION SYSTEM NOTES: CALL DEVELOPMENT SERVICES (206-275-7605) 24 HOURS IN ADVANCE FOR A DETENTION SYSTEM INSPECTION BEFORE BACKFILLING AND FOR FINAL INSPECTIONS. ① USE A MINIMUM OF A 54 IN. DIAM. TYPE 2 CATCH BASIN. THE ACTUAL SIZE IS DEPENDENT ON CONNECTING PIPE MATERIAL AND DIAMETER. (6) PROVIDE AT LEAST ONE 3 X 0.090 GAUGE SUPPORT BRACKET ANCHORED TO CONCRETE WALL WITH 5/8 IN. STANLESS STEEL EXPANSION BOLTS OR EMBEDDED SUPPORTS 2 IN. INTO CATCH BASIN WALL (MAXIMUM 3'-0" RESPONSIBILITY FOR OPERATION AND MAINTANANCE OF DRAINAGE SYSTEMS ON PRIVATE PROPERTY IS RESPONSIBILITY OF THE PROPERTY OWNER. MATERIAL ACCUMULATED IN THE STORAGE PIPE MUST BE REMOVED FROM CATCH BASINS TO ALLOW PROPER OPERATION. THE OUTLET CONTROL ORIFICE MUST BE KEPT OPEN AT ALL TIMES. 2 OUTLET PIPE: MIN. 6 INCH. 2 OUTLET PIPE: MIN. 6 INCH.

3 METAL PARTS: CORROSION RESISTANT. NON-GALVANIZED PARTS PREFERRED. GALVANIZED PIPE PARTS TO HAVE

4 FRAME AND LADDER OR STEPS OFFSET SO:

A. CLEANOUT GATE IS VISIBLE FROM TOP;
B. CLIMB-DOWN SPACE IS CLEAR OF RISER AND CLEANOUT GATE;
C. FRAME IS CLEAR OF CURB.

4 THE SHEAR GATE SHALL BE MADE OF ALUMINUM ALLOY IN ACCORDANCE WITH ASTM B 26M AND ASTM B 275,
DESIGNATION ZG3ZA; OR CAST IRON IN ACCORDANCE WITH ASTM A 48, CLASS 30B.
THE LIFT HANDLE SHALL BE MADE OF A SIMILAR METAL TO THE GATE (TO PREVENT GALVANIC CORROSION),
IT MAY BE OF SOLID ROD OR HOLLOW TUBING, WITH ADJUSTABLE HOOK AS REQUIRED.
A NEOPRENE RUBBER GASEALL BE MADE OF A SIMILAR METAL TO THE GATE (TO PREVENT GALVANIC CORROSION),
IT MAY BE OF SOLID ROD OR HOLLOW TUBING, WITH ADJUSTABLE HOOK AS REQUIRED.
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IT MAY BE OF SOLID ROD OR HOLLOW TUBING, WITH ADJUSTABLE HOOK AS REQUIRED.
A NEOPRENE RUBBER GASEAT IN MARK IS LEVEL WHEN THE GATE IS CLOSED.
THE MATING SURFACES OF THE LID AND THE BODY SHALL BE MACHINED FOR PROPER FIT.
ALL SHEAR GATE BOLTS FAIL BE MADE OF ALUMINUM ALLOY IN ACCORDANCE WITH ASTM B 26M AND ASTM B 275,
DESIGNATION ZG3ZA; OR CAST IRON IN ACCORDANCE WITH ASTM B 26M AND ASTM B 275,
DESIGNATION ZG3ZA; OR CAST IRON IN ACCORDANCE WITH ASTM B 2 PIPE MATERIAL, JOINT, AND PROTECTIVE TREATMENT SHALL BE IN ACCORDANCE WITH SECTION 7.04 AND 9.05 OF THE WSDOT STANDARD SPECIFICATION FOR ROAD, BRIDGE, AND MUNICIPAL CONSTRUCTION, LATEST VERSION. SUCH MATERIALS INCLUDE THE FOLLOWING, LINED A. CLEANOUT GATE IS VISIBLE FROM TOP;
B. CLIMB-DOWN SPACE IS CLEAR OF RISER AND CLEANOUT GATE;
C. FRAME IS CLEAR OF CURB. CORRUGATED POLYETHYLENE PIPE (LCPE), ALUMINIZED TYPE 2 CORRUGATED STEEL PIPE AND PIPE ARCH (MEETS AASHTO DESIGNATIONS M274 AND M36), CORRUGATED OR SPIRAL RIB ALUMINUM PIPE, OR REINFORCED CONCRETE PIPE. CORRUGATED STEEL PIPE IS NOT ALLOWED. (5) IF METAL OUTLET PIPE CONNECTS TO CEMENT CONCRETE PIPE, OUTLET PIPE TO HAVE SMOOTH O.D. EQUAL TO (8) THE UPPER CATCH BASIN IS REQUIRED IF THE LENGTH OF THE DETENTION PIPE IS GREATER THAN 50 FT. CONCRETE PIPE I.D. LESS 1/4 IN. 4. FOOTING DRAINS SHALL NOT BE CONNECTED TO THE DETENTION SYSTEM. NO. DATE BY REVISIONS DATE: Aug 29, 2022 DRAWING NO: **APPLICANT** 2026 KAM DERAKSHANI **DETENTION TANK DETAILS** 8151 SE 48th STREET DRAFTED: SS DESIGN: SS MERCER ISLAND, WA 98040 DIGITAL SIGNATURE

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