### **Project Information:**

Owner: Derek Loeser & Katie Van Kessel 7426 SE 71st Street Mercer Island WA. 98040

Site location: 7426 SE 71st Street Mercer Island WA. 98040

Tax parcel #: 536800-0290

Legal description: MČ LEAN ADDITION, PLAT LOT: 29 NW-25-24-4

### **Project Description:**

Replace exist. garage with new, reconfigure driveway, reducing concrete area

Zoning: R-15

# **Construction Type:**

### **Occupancy Group:**

**Code information:** - 2018 InternationalResidentialCode (IRC)

- 2018 International Building Code (IBC), as applicable - Washington State Energy Code - Minimum Design Loads for Buildings and Other Structures, ASCE 7-05 (ASCE) - 2008 Special Design Provisions for Wind and Seismic

(SDPWS) - Mercer Island City Code (MICC) Comply with all applicable codes and laws

**Energy Information** 

## Mechanical System:

# IRC Ventilation: N/A

T.O.S.

T.O.W.

V.I.F.

Wdw.

W/I

Top of Slab

Top of Wall

Vapor Barrier

Verify in Field

Window

Typical Unless Otherwise Noted

| A.F.F.        | Above Finish Floor     |
|---------------|------------------------|
| B.O.          | Bottom of              |
| C.L.          | Center Line            |
| Clr.          | Clear                  |
|               | crete Masonry Unit     |
| Col.          | Column                 |
| Conc.         | Concrete               |
| Cont.         | Continuous             |
| Dia.          | Diameter               |
| D.S.          | Downspout              |
| Ea.           | Each                   |
| El.           | Elevation              |
| Eq.           | Equivalent             |
| F.F.E.        | Finish Floor Elevation |
| Ftg.          | Footing                |
| Flr.          | Floor                  |
| F.O.C.        | Face of Concrete       |
| F.O.S.        | Face of Stud           |
| GWB           | Gypsum Wall Board      |
| Ht.           | Height                 |
| Jst.          | Joist                  |
| Mtl.          | Metal                  |
| O.C.          | On Center              |
| O/            | Over                   |
| PCC           | Pre-cast Concrete      |
| P.T.          | Pressure Treated       |
| Req.          | Required               |
| R.O.          | Rough Opening          |
| S.W.          | Shear Wall             |
| Sim.          | Similar                |
| S.O.G.        | Slab on Grade          |
| Sst.          | Stainless Steel        |
| Struct.       | Structural             |
| T&G           | Tongue and Groove      |
| T.O.          | Top of                 |
| T.O.Pl.       | Top of Plate           |
| $T \cap \cap$ | T f Ol-1-              |

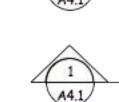
# Symbol Legend

Door number

Window Number

1 A1.4 Detail Reference

Exterior Elevation Reference Interior Elevation Reference





Wall Section

Section Reference





North Arrow



W - 1

Wall Type

S

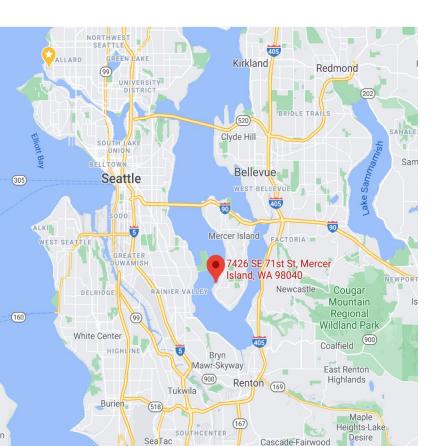
Smoke Detector

Existing Wall

New Wall

Demo

Exhaust Fan



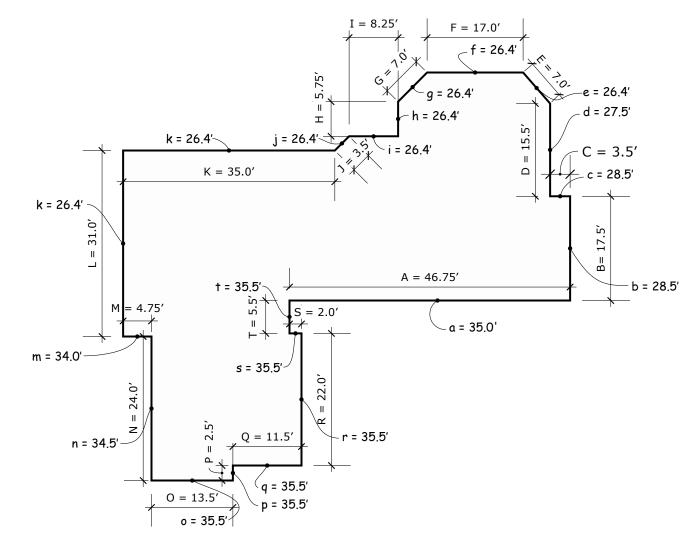
# 3 Vicinity Plan nts

### ABE Calculation:

46.75(35.3)+17.5(28.5)+3.5(28.5)+15.5(27.5)+7(26.4)+ 17(26.4) + 7(26.4) + 5.75(26.4) + 8.75(26.4) + 3.5(26.4) +35(26.4)+31(31)+4.75(34)+24(34.5)+13.5(35.5)+ 2.5(35.5)+11.5(35.5)+22(35.5)+2(35.5)+5.5(35.5)

46.75+17.5+3.5+15.5+7+17+7+5.75+8.75+3.5+35+ 31+4.75+24+13.5+2.5+11.5+22+2+5.5

8658.63 — = 30'.49 Average Building Elevation



| Ave  | rage Building Elevation Data | 05.06.2021                  |
|------|------------------------------|-----------------------------|
| Mark | Wall Segment Length - X (ft) | Midpoint Elevation - x (ft) |
| A/a  | 46.75                        | 35.3                        |
| B/b  | 17.50                        | 28.5                        |
| C/c  | 3.50                         | 28.5                        |
| D/d  | 15.50                        | 27.5                        |
| E/e  | 7.00                         | 26.4                        |
| F/f  | 17.00                        | 26.4                        |
| G/g  | 7.00                         | 26.4                        |
| H/h  | 5.75                         | 26.4                        |
| I/i  | 8.75                         | 26.4                        |
| J/j  | 3.50                         | 26.4                        |
| K/k  | 35.00                        | 26.4                        |
| L/I  | 31.00                        | 31                          |
| M/m  | 4.75                         | 34                          |
| N/n  | 24.00                        | 34.5                        |
| O/o  | 13.50                        | 35.5                        |
| P/p  | 2.50                         | 35.5                        |
| Q/q  | 11.50                        | 35.5                        |
| R/r  | 22.00                        | 35.5                        |
| S/s  | 2.00                         | 35.5                        |
| T/t  | 5.50                         | 35.5                        |

# Average Building Elevation

525 s.f.

4,224 s.f.

423 s.f.

1,800 s.f.

# Site Area Calculations

| Maximum Allowable Site Areas    |             |
|---------------------------------|-------------|
| Lot Area:                       | 13,482 s.f. |
| 40% of lot area = lot coverage: | 5,393 s.f.  |
| 60% of lot area = landscaping:  | 8,089 s.f.  |
| 9% of lot area = hardscape:     | 1,213 s.f.  |

|   | · · · · · · · · · · · · · · · · · · · | •                       |
|---|---------------------------------------|-------------------------|
|   | <u>Exist. Lot Coverage</u>            |                         |
|   | House (inc. roof overhangs):          | 2891 + 661 = 3,552 s.f. |
|   | Driveway:                             | 1,653 s.f.              |
| - | Total:                                | 5,206 s.f.              |

5,206/13,482 = <u>38.6% of lot area</u>

Proposed Lot Coverage House (inc. roof overhangs): 2891 + 808 = 3,699 s.f. Driveway: Total:

4,224/13,482 = <u>31.3% of lot area</u> 1169 s.f. under allowable maximum area Proposed addition reduces lot coverage by 982 s.f.

Existing Hardscape

Unused lot coverage may be used as hardscape per MICC 19.02.020.F.3.b.ii: 40% - 38.6% = 1.4%

Total allowable hardscape: (9% + 1.4% = 10.4% of 13,482 s.f. = 1402 s.f.

Patio, deck, stairs, retaining walls, rockeries, walkways: 440 + 519 + 40 + 360 + 18 = 1,377 s.f. 1,377/13,482 = 10.2% < 10.4%

# <u>Proposed Hardscape</u>

Unused lot coverage may be used as hardscape per MICC 19.02.020.F.3.b.ii: 40% - 31.3% = 8.7%

<u>Total allowable hardscape</u>: (9% + 8.7% = <u>17.7%</u> of 13,482 s.f. = <u>2386 s.f.</u> Existing hardscape: 1,377 s.f.

New walkways: Total proposed hardscape: 1,800/13,482 = 13.3% <17.7%

Landscape area:

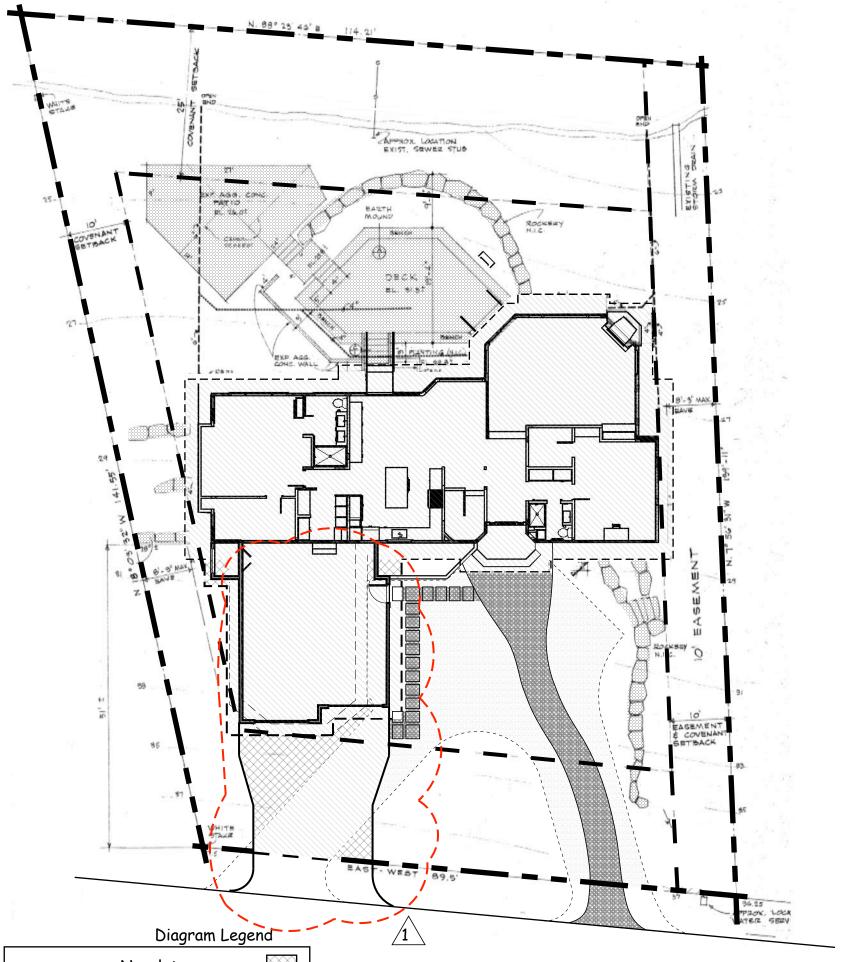
13,482 - 4322 - 1800 = 7360 s.f. = 54.6% 8089 - 7360 = 729 s.f.

729 s.f. of the landscape area is being used for hardscape (walkways, rockeries etc.)

Per MICC19.02.020.F.3.c up to 1,213 s.f. of the landscape area may be used for hardscape.

Lot Slope Calculations: Highest Pt. Elev. = 37.5 ft. Lowest Pt. Elev. = 23.0 ft. Elev. difference = 14.5 ft. Horizontal distance 128 ft. Lot Slope =  $14.5 \times 100 = 11.3\%$ 

LAKE WASHINGTON



New lot coverage New hardscape Lot Coverage/Hardscape Diagram Remove existing lot coverage 1/16" = 1'-0" Existing lot coverage to remain Existing hardscape to remain

13 October 2021 Permit Revisions

Hughes Studio

Seattle, WA 98109 Phone 206.264.1301

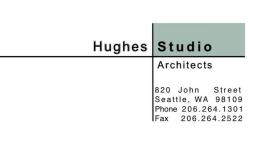
Building Permit Set 19 May 2021

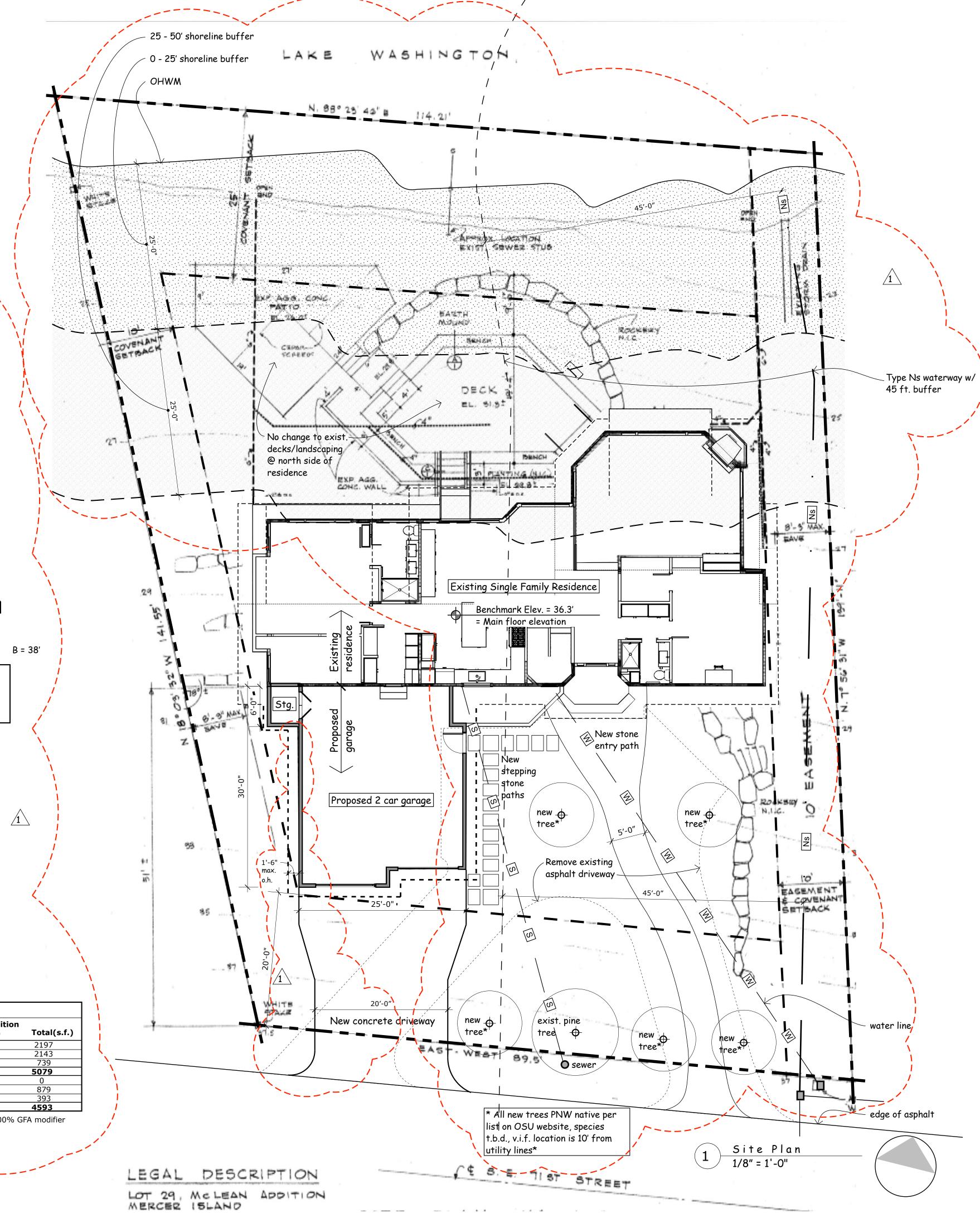
Loeser Van Kessel Residence

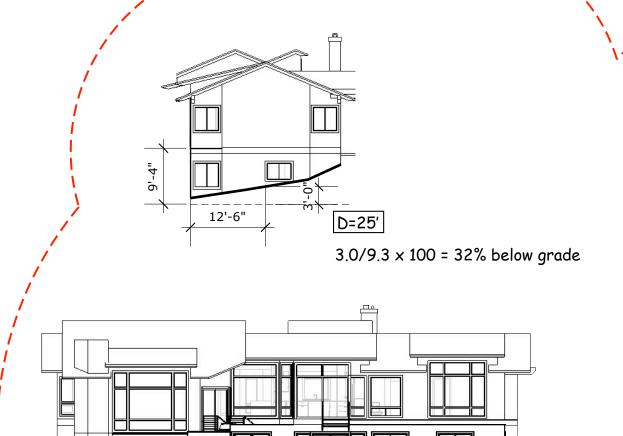
7426 S.E . 71st Street Mercer Island WA.98040

Site Information

A 1.0

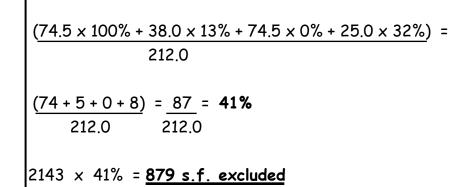






C=74.5'

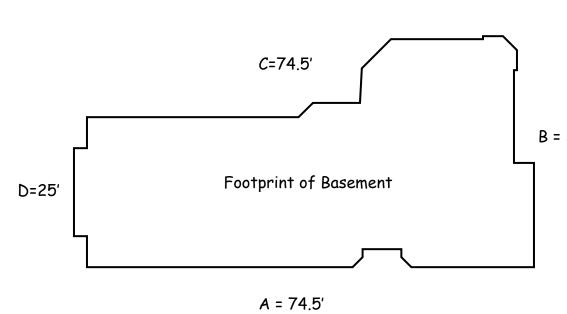
0% below grade

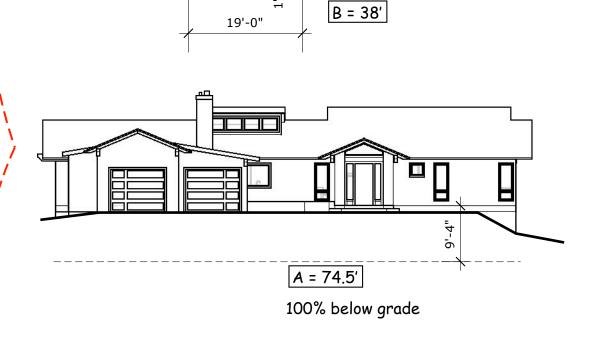


**Basement Floor Area Reduction Calculations:** 

Portion of excluded floor area:

| Basement Floor Area Reduction Chart |                 |                 |               |  |  |  |
|-------------------------------------|-----------------|-----------------|---------------|--|--|--|
| Wall<br>Segment                     | Length<br>(ft.) | Coverage<br>(%) | Result<br>(%) |  |  |  |
| Α                                   | 74.5            | 100             | 74%           |  |  |  |
| В                                   | 38.0            | 13              | 5%            |  |  |  |
| С                                   | 74.5            | 0               | 0%            |  |  |  |
| D                                   | 25.0            | 32              | 8%            |  |  |  |
| Totals                              | 212.0'          | NA              | 87%           |  |  |  |





**Gross Floor Area Calculations:** R-15 Zone District allows max. 40% of lot area = .4 × 13,482 = <u>5393 s.f.</u>

Proposed *G*ross Floor Area = 4593 s.f. = **<u>34%</u> < 40%** 

|                       | 208.7 s.f.<br>exist, area > 12'                          |
|-----------------------|--|
| 147.5 s.f. 118.9 s.f. | ceiling ht.  exist. area > 12' ceiling ht. to be removed |
|                       | new area > 12'<br>ceiling ht.                            |

| Building<br>Area       | Existing<br>Area(s.f.) | Removed Area(s.f.) | New/Addition Area(s.f.) | Total(s.f.) |
|------------------------|------------------------|--------------------|-------------------------|-------------|
| Main Floor             | 2197                   | 0                  | 0                       | 2197        |
| Basement               | 2143                   | 0                  | 0                       | 2143        |
| Garage/Carport         | 444                    | 444                | 739                     | 739         |
| Total Floor Area       | 4784                   | 444                | 739                     | 5079        |
| Accessory Buildings    | 0                      | 0                  | 0                       | 0           |
| Basement Area Excluded | 879                    | 0                  | 0                       | 879         |
| 150% GFA Modifier      | 378                    | 59                 | 74                      | 393         |
| Total Building Area    | 4283                   | 503                | 813                     | 4593        |

13 October 2021 Permit Revisions

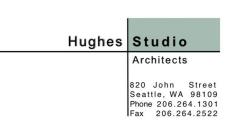
Building Permit Set 19 May 2021

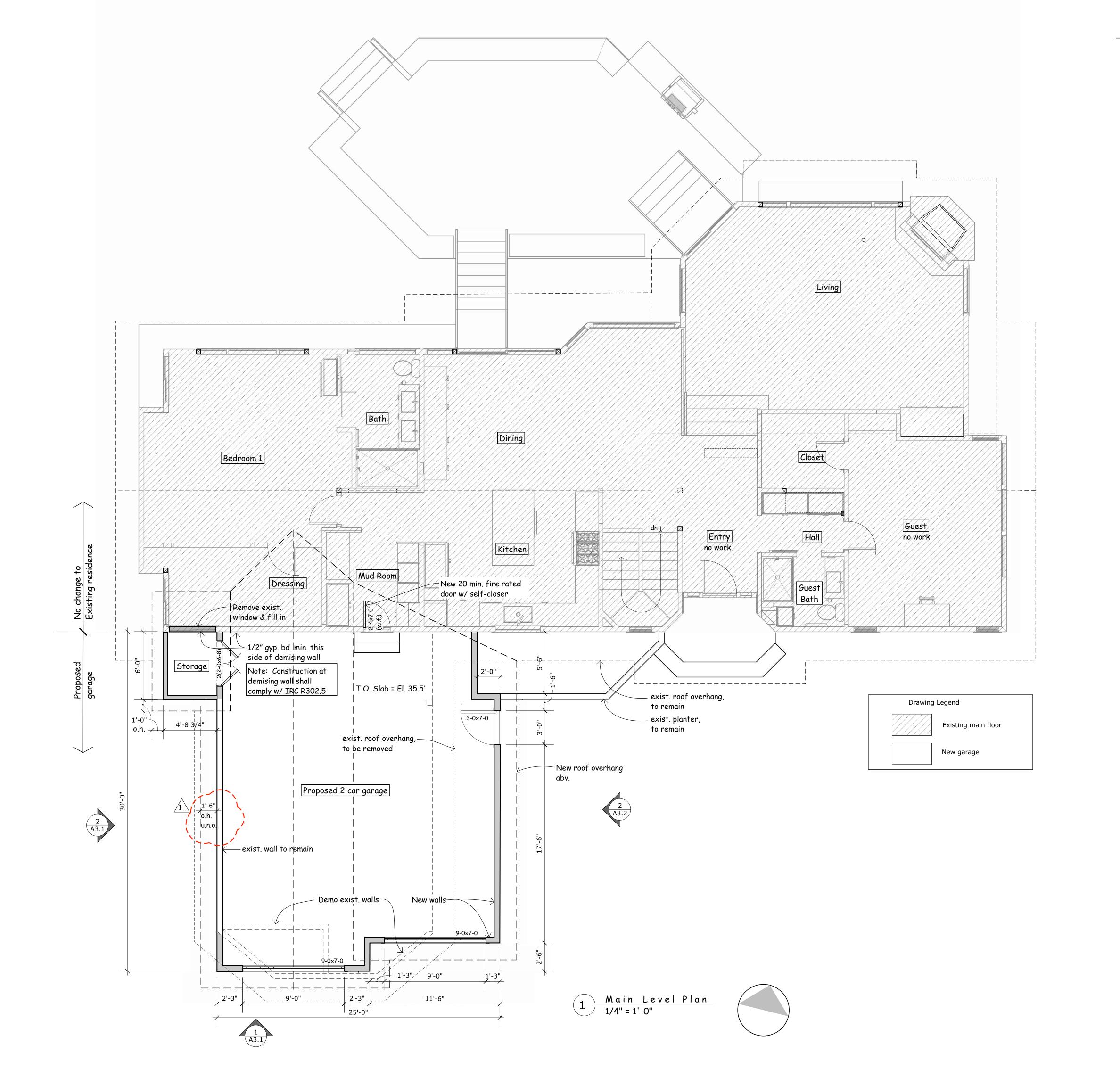
Loeser Van Kessel Residence

7426 S.E . 71st Street Mercer Island WA.98040

Site Plan

A 1.1







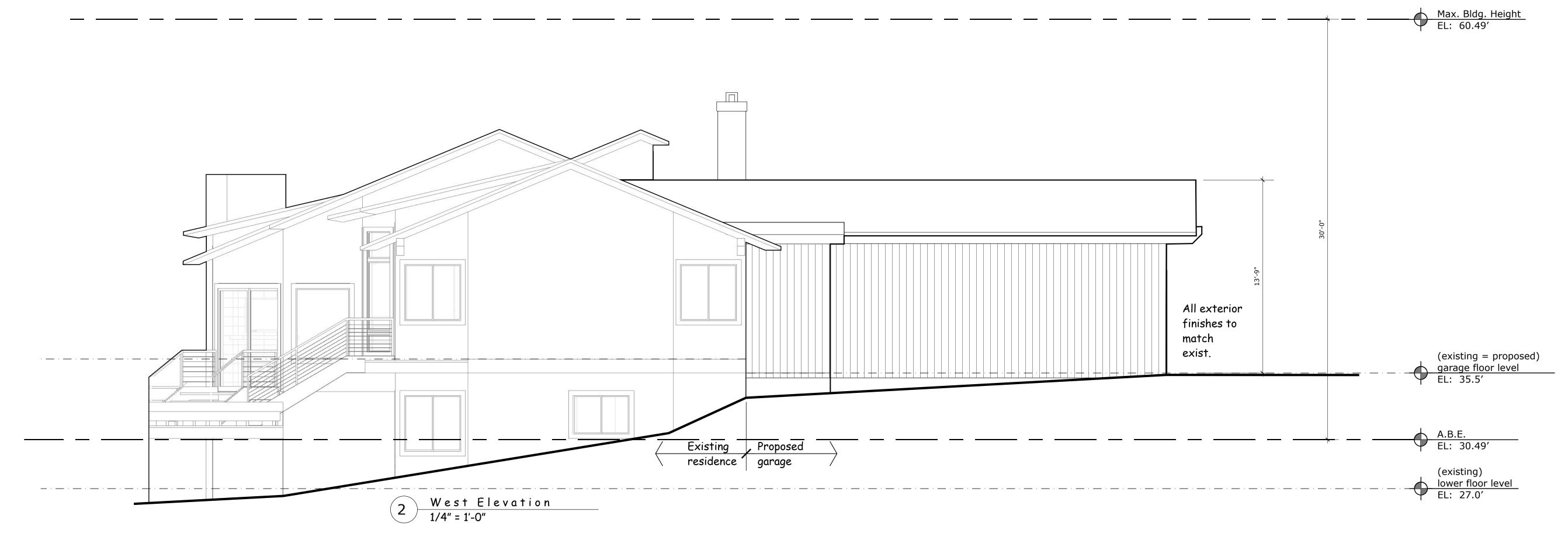
13 October 2021 Permit Revisions

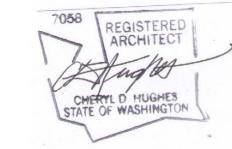
> Building Permit Set 19 May 2021

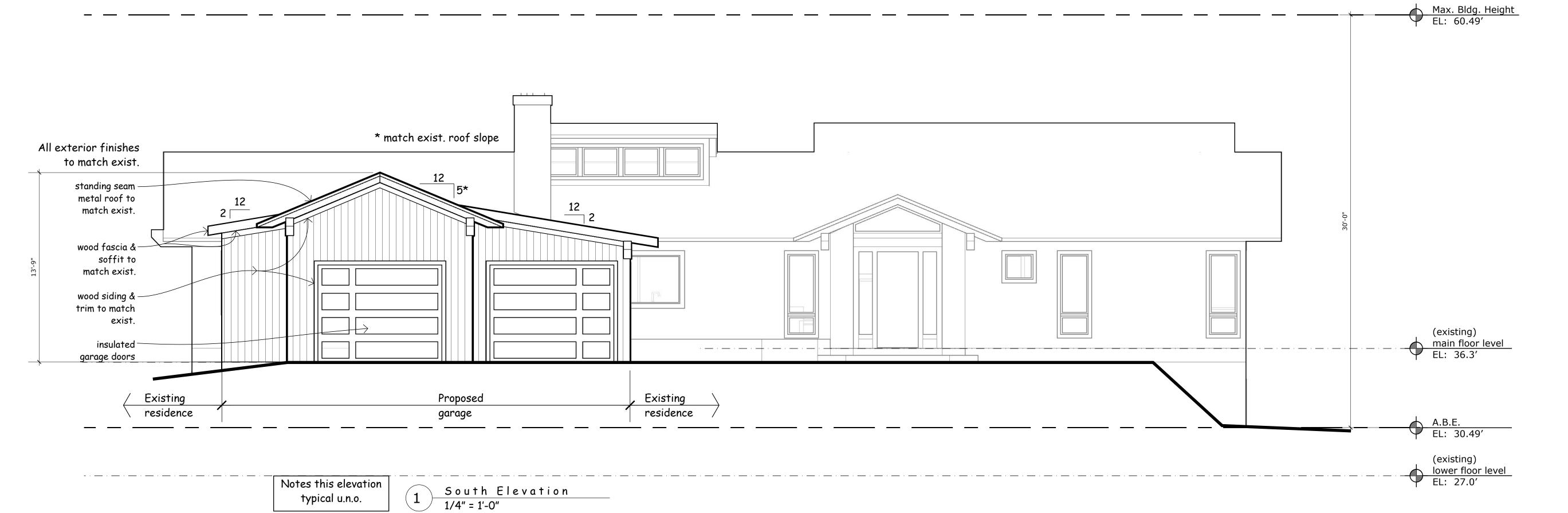
Loeser Van Kessel Residence

7426 S.E . 71st Street Mercer Island WA.98040

Main Level Plan







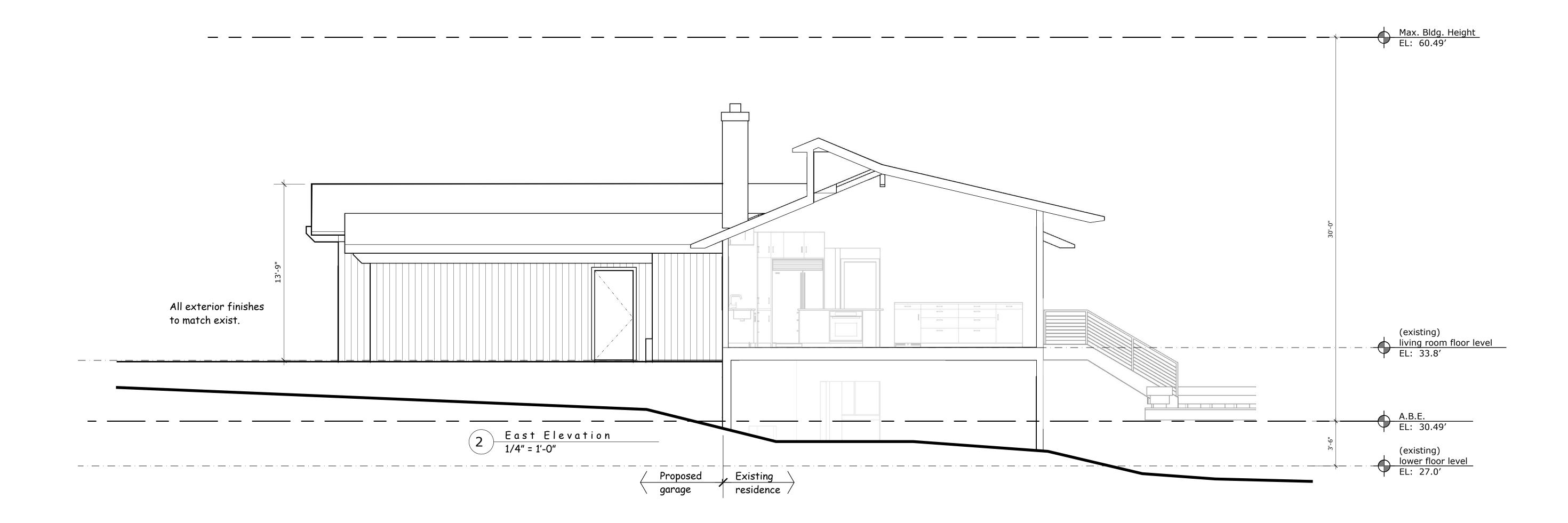
13 October 2021
Permit Revisions

Building Permit Set 19 May 2021

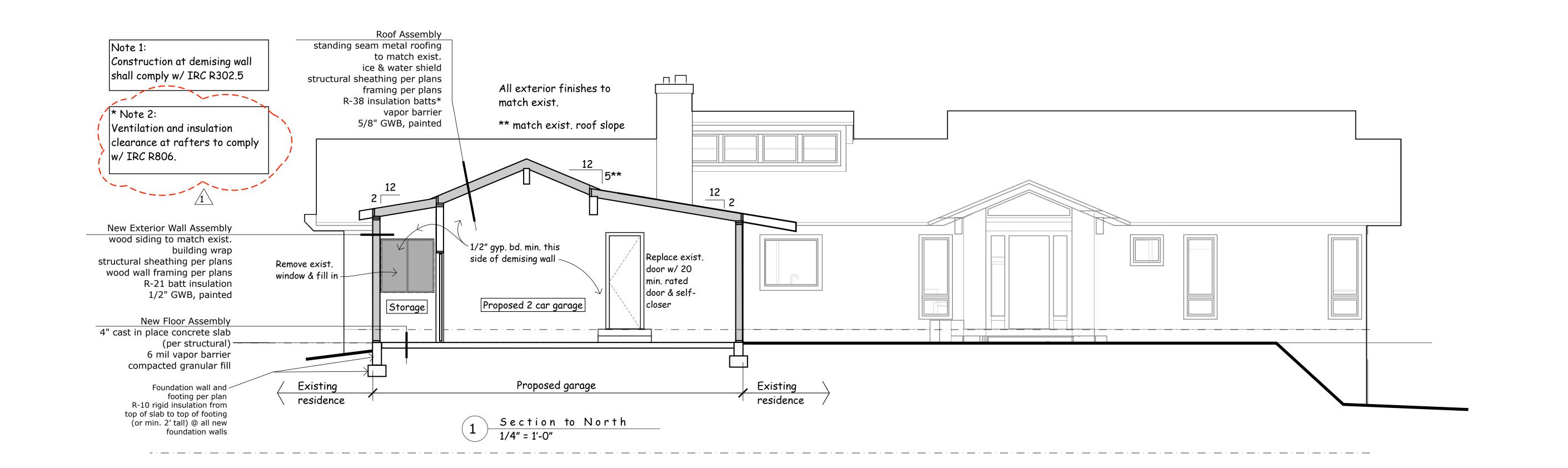
Loeser Van Kessel Residence

7426 S.E . 71st Street Mercer Island WA.98040

Elevations







13 October 2021
Permit Revisions

Building Permit Set 19 May 2021

Loeser Van Kessel Residence

7426 S.E . 71st Street Mercer Island WA.98040

> Section Details

#### GENERAL RESIDENTIAL STRUCTURAL NOTES

#### (The following apply unless shown otherwise on the plans)

### <u>CRITERIA</u>

ALL MATERIALS WORKMANSHIP, DESIGN, AND CONSTRUCTION SHALL CONFORM TO THE DRAWINGS, SPECIFICATIONS, AND THE 2015 INTERNATIONAL BUILDING CODE (IBC).

DESIGN LOADING CRITERIA

SNOW LOAD FLOOR LIVE LOAD (RESIDENTIAL) WIND (MAIN WIND FORCE RESISTING SYSTEM)

Vasd = 85 MPHRISK CATEGORY = II EXPOSURE C, GCpi = 0.18

EARTHQUAKE (EQUIVALENT LATERAL FORCE PROCEDURE) (BASED ON 2008 USGS "HAZARD DATA)

Ss=1.468, Sds =0.979 S1=0.562, Sd1 =0.562 le=1.0, SITE CLASS = D SEISMIC DESIGN CATEGORY= D RISK CATEGORY = II

25 PSF

40 PSF

Vult = 110 MPH

R = 6.5 FOR WOOD FRAMED SHEARWALL LATERAL SYSTEM OVER STRENGTH FACTOR,  $\Omega_0 = 3.0$ REDUNDANCY FACTOR =1.0 Cs = 0.151, GARAGE BASE SHEAR = 2.1 KIPS

SEE PLANS FOR ADDITIONAL LOADING CRITERIA

- CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS, MEMBER SIZES, AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE STRUCTURAL DRAWINGS ARE INTENDED AS GUIDELINES ONLY AND MUST BE
- CONTRACTOR SHALL PROVIDE TEMPORARY BRACING FOR THE STRUCTURE AND STRUCTURAL COMPONENTS UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE PLANS.
- CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES REQUIRED TO PERFORM HIS WORK. THE STRUCTURAL ENGINEER HAS NO OVERALL SUPERVISORY AUTHORITY OR ACTUAL AND/OR DIRECT RESPONSIBILITY FOR THE SPECIFIC WORKING CONDITIONS AT THE SITE AND/OR FOR ANY HAZARDS RESULTING FROM THE ACTIONS OF ANY TRADE CONTRACTOR. THE STRUCTURAL ENGINEER HAS NO DUTY TO INSPECT, SUPERVISE, NOTE, CORRECT, OR REPORT ANY HEALTH OR SAFETY DEFICIENCIES OF THE OWNER, CONTRACTORS, OR OTHER ENTITIES OR PERSONS AT THE PROJECT SITE.
- CONTRACTOR-INITIATED CHANGES SHALL BE SUBMITTED IN WRITING TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO FABRICATION OR CONSTRUCTION. CHANGES SHOWN ON SHOP DRAWINGS ONLY WILL NOT SATISFY THIS REQUIREMENT.
- DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED, BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER.
- ALL STRUCTURAL SYSTEMS WHICH ARE TO BE COMPOSED OF COMPONENTS TO BE FIELD ERECTED SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE, AND ERECTION IN ACCORDANCE WITH INSTRUCTIONS PREPARED BY THE SUPPLIER.

### GEOTECHNICAL

10. FOUNDATION NOTES: FOUNDATION NOTES: ALLOWABLE SOIL PRESSURE AND LATERAL EARTH PRESSURE ARE ASSUMED AND, THEREFORE, MUST BE VERIFIED BY A QUALIFIED SOILS ENGINEER. IF SOILS ARE FOUND TO BE OTHER THAN ASSUMED, NOTIFY THE STRUCTURAL ENGINEER FOR POSSIBLE FOUNDATION REDESIGN.

FOOTINGS SHALL BEAR ON FIRM, UNDISTURBED EARTH AT LEAST 18" BELOW ADJACENT FINISHED GRADE, UNLESS OTHERWISE NOTED,

7H (ULTIMATE LOAD) LATERAL EARTH PRESSURE (SEISMIC) PASSIVE EARTH PRESSURE (INCLUDES FACTOR OF SAFETY = 1.5) 350 PCF COEFFICIENT OF FRICTION (INCLUDES FACTOR OF SAFETY = 1.5) 0.35

### **CONCRETE**

CONCRETE SHALL BE MIXED, PROPORTIONED, CONVEYED, AND PLACED IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE SECTION 1905 AND ACI 301-11. CONCRETE SHALL ATTAIN A 28-DAY STRENGTH OF f'c = 3,000 PSI, SHALL CONTAIN NO LESS THAN 5½ SACKS OF CEMENT, HAVE A MAXIMUM WATER / CEMENT RATIO OF 0.45, AND A SLUMP OF 5" OR LESS. CONCRETE HAS BEEN DESIGNED USING 2,500 PSI PER INTERNATIONAL BUILDING CODE SECTION 1705.3 EXCEPTION 2.3 TO AVOID SPECIAL INSPECTIONS.

MIX DESIGNS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER AND BUILDING DEPARTMENT FOR APPROVAL TWO WEEKS PRIOR TO PLACING ANY CONCRETE. THE PERFORMANCE MIX SHALL INCLUDE THE AMOUNTS OF CEMENT, FINE AND COARSE AGGREGATE, WATER AND ADMIXTURES AS WELL AS THE WATER CEMENT RATIO, SLUMP, CONCRETE YIELD, AND SUBSTANTIATING STRENGTH DATA IN ACCORDANCE WITH CHAPTER 5 OF ACI 318-11. THE USE OF A PERFORMANCE MIX REQUIRES BATCH PLANT INSPECTION, THE COST OF WHICH SHALL BE PAID BY THE GENERAL CONTRACTOR. REVIEW OF MIX SUBMITTALS BY THE ENGINEER OF RECORD INDICATES ONLY THAT INFORMATION PRESENTED CONFORMS TO THE CONTRACT DOCUMENTS. CONTRACTOR OR SUPPLIER MAINTAINS FULL RESPONSIBILITY FOR SPECIFIED PERFORMANCE.

ALL CONCRETE WITH SURFACES EXPOSED TO STANDING WATER SHALL BE AIR-ENTRAINED WITH AN AIR-ENTRAINING AGENT CONFORMING TO ASTM C260, C494M, AND C618. UNLESS OTHERWISE NOTED THE TOTAL AIR CONTENT FOR FROST-RESISTANT CONCRETE SHALL BE 5% IN ACCORDANCE EXPOSURE CLASS F1 PER ACI 318-11 TABLE 4.3.1 AND TABLE 4.4.1.

REINFORCING STEEL SHALL CONFORM TO ASTM A615 (INCLUDING SUPPLEMENTS S1), GRADE 60, fy = 60,000 PSI. EXCEPTIONS: ANY BARS SPECIFICALLY SO NOTED ON THE DRAWINGS SHALL BE GRADE 40, fy = 40,000 PSI.

WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185

DETAILING OF REINFORCING STEEL (INCLUDING HOOKS AND BENDS) SHALL BE IN ACCORDANCE WITH ACI SP-66-04 AND 318-11. LAP ALL CONTINUOUS REINFORCEMENT #5 AND SMALLER 48 BAR DIAMETERS OR 2'-0" MINIMUM. A MAXIMUM OF ONE HALF OF THE TOTAL REINFORCEMENT SHALL BE LAPPED WITH THE REQUIRED LAP LENGTH. PROVIDE CORNER BARS AT ALL WALL AND FOOTING INTERSECTIONS. LAP ADJACENT MATS OF WELDED WIRE FABRIC A MINIMUM OF 8" AT SIDES AND ENDS.

NO BARS PARTIALLY EMBEDDED IN HARDENED CONCRETE SHALL BE FIELD BENT UNLESS SPECIFICALLY SO DETAILED OR APPROVED BY THE STRUCTURAL ENGINEER. FIELD BENDING OF GRADE 60 REINFORCEMENT SHALL NOT BE ALLOWED.

CONCRETE PROTECTION (COVER) FOR REINFORCING STEEL SHALL BE AS FOLLOWS:

FOOTINGS AND OTHER UNFORMED SURFACES CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH

SLABS-ON-GRADE: UNLESS NOTED OTHERWISE SHALL BE 4" CONCRETE, REINFORCED WITH 6X6 W1.4XW1.4 WELDED WIRE FABRIC CENTERED IN SLAB. UNLESS OTHERWISE DIRECTED BY SOILS ENGINEER PROVIDE MINIMUM 6 MIL VAPOR BARRIER OVER 4" OF COMPACTED SAND OR GRAVEL.

- CAST-IN-PLACE CONCRETE: SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS AND DIMENSIONS OF DOOR AND WINDOW OPENINGS IN ALL CONCRETE WALLS. SEE ARCHITECTURAL DRAWINGS FOR ALL GROOVES, NOTCHES, CHAMFERS, FEATURE STRIPS, COLOR, TEXTURE, AND OTHER FINISH DETAILS AT ALL EXPOSED CONCRETE SURFACES.
- NON-SHRINK GROUT SHALL BE FURNISHED BY AN APPROVED MANUFACTURER AND SHALL BE MIXED AND PLACED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED RECOMMENDATIONS. GROUT STRENGTH SHALL BE AT LEAST EQUAL TO THE MATERIAL ON WHICH IT IS PLACED (3,000 PSI MINIMUM).

### POST INSTALLED ANCHORS

- POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE ENGINEER-OF-RECORD PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REINFORCEMENT. HOLES SHALL BE DRILLED AND CLEANED IN ACCORDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS AND ICC-ES REPORT. SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE SPECIFIED BELOW, SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER-OF-RECORD FOR APPROVAL.
  - ADHESIVE ANCHORS FOR USE IN CRACKED AND UNCRACKED CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOI
  - USE IN ACCORDANCE WITH ICC-ES AC308. PRE-APPROVED ADHESIVE ANCHORS INCLUDE:

HEM-FIR NO. 2

DOUGLAS FIR NO. 1

- SIMPSON STRONG-TIE "SET-XP" (ICC-ES ESR-2508)
- SIMPSON STRONG-TIE "AT-XP" (IAPMO UES ER-263)
- HILTI "HIT-RE 500-SD" (ICC-ES ESR-2322)

#### <u>WOOD</u>

(2X, 3X, AND 4X MEMBERS)

CONCRETE ANCHORS

**BEAM AND STRINGERS:** (6 X AND LARGER MEMBERS)

(6 X AND LARGER MEMBERS)

STUDS PLATES & MISCELLANEOUS LIGHT FRAMING

(FINGER JOINTED STUDS MAY NOT BE USED WITH APPROVAL FROM STRUCTURAL ENGINEER)

2X AND 3X TONGUE AND GROOVE DECKING

MINIMUM BASE VALUE,  $F_b = 850 \text{ PSI}$ 

MINIMUM BASIC DESIGN STRESS, F<sub>b</sub> = 1,200 PSI, F<sub>C</sub> = 1,000 PSI DOUGLAS FIR/ HEM-FIR NO. 2,  $F_b = 850 \text{ PSI}$ ,  $F_C = 1,300 \text{ PSI}$ 

MINIMUM BASIC DESIGN STRESS,  $F_b = 1,350 \text{ PSI}$ 

HEM-FIR COMMERCIAL DEX, F<sub>b</sub> = 1,350 PSI

PLYWOOD SHEATHING SHALL BE GRADE C-D, EXTERIOR GLUE OR STRUCTURAL II, EXTERIOR GLUE IN CONFORMANCE WITH DOC PS OR PS 2-10 AND AMERICAN PLYWOOD ASSOCIATION PERFORMANCE STANDARD PRP-108. ORIENTED STRAND BOARD OF EQUIVALEI THICKNESS, EXPOSURE RATING AND PANEL INDEX MAY BE USED IN LIEU OF PLYWOOD. SEE PLANS FOR THICKNESS, PANEL IDENTIFICATION INDEX AND NAILING REQUIREMENTS. EACH PANEL SHALL BE IDENTIFIED FOR GRADE AND GLUE TYPE BY THE TRADEMARKS OF AN APPROVED TESTING AND GRADING AGENCY.

ALL WOOD PLATES IN DIRECT CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE-TREATED WITH AN APPROVED PRESERVATIVE, PROVIDE 2 LAYERS OF ASPHALT IMPREGNATED BUILDING PAPER BETWEEN UNTREATED LEDGERS, BLOCKING, ETC CONCRETE OR MASONRY.

PRESSURE TREATED LUMBER SHALL COMPLY WITH THE AMERICAN WOOD PROTECTION ASSOCIATION (AWPA) STANDARD U1, COMMODITY SPECIFICATION A AS INDICATED BELOW OR HAVE EQUIVALENT ICC-ES APPROVAL.

| PROPOSED USE | AWPA USE |
|--------------|----------|
|              | CATEGORY |

A. IDENTIFICATION OF TREATING MANUFACTURER

B. TYPE OF PRESERVATIVE USED

C. MINIMUM PRESERVATIVE RETENTION (PCF) D. END USE FOR WHICH THE PRODUCT IS TREATED

E. IDENTITY OF THE ACCREDITED INSPECTION AGENCY

F. STANDARD TO WHICH THE PRODUCT IS TREATED

TIMBER CONNECTORS CALLED OUT BY LETTERS AND NUMBERS SHALL BE "STRONG-TIE" BY SIMPSON COMPANY, AS SPECIFIED IN T CATALOG NUMBER C-2013. EQUIVALENT DEVICES BY OTHER MANUFACTURERS MAY BE SUBSTITUTED, PROVIDED THEY HAVE ICC-E APPROVAL FOR EQUAL OR GREATER LOAD CAPACITIES. PROVIDE NUMBER AND SIZE OF FASTENERS AS SPECIFIED BY MANUFACTU CONNECTORS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. WHERE CONNECTOR STR CONNECT TWO MEMBERS, PLACE ONE-HALF OF THE NAILS OR BOLTS IN EACH MEMBER. ALL BOLTS IN WOOD MEMBERS SHALL CONFORM TO ASTM A307. PROVIDE WASHERS UNDER THE HEADS AND NUTS OF ALL BOLTS AND LAG SCREWS BEARING ON WOOD. UNLESS NOTED OTHERWISE, ALL NAILS SHALL BE COMMON AND MAXIMUM NUMBER OF NAILS AS SPECIFIED BY THE MANUFACTURE SHALL BE PROVIDED. ALL SHIMS SHALL BE SEASONED AND DRIED AND THE SAME GRADE (MINIMUM) AS MEMBERS CONNECTED. AI SAWN LUMBER JOISTS SHALL BE CONNECTED TO FLUSH BEAMS WITH "LUS" SERIES JOIST HANGERS AND ALL PREFABRICATED PLYWOOD WEB JOISTS SHALL BE CONNECTED TO FLUSH BEAMS WITH "IUS" SERIES JOIST HANGERS UNLESS NOTED OTHERWISE

ALL CONNECTIONS IN CONTACT WITH PRESERVATIVE-TREATED OR FIRE-RETARDANT-TREATED WOOD, SHALL BE OF HOT DIPPED Z COATED GALVANIZED STEEL OR STAINLESS STEEL. HOT DIPPED GALVANIZED FASTENERS SHOULD CONFORM TO ASTM STANDARD AND HOT DIPPED GALVANIZED CONNECTORS SHOULD CONFORM TO ASTM STANDARD A653 (CLASS G-185). STAINLESS STEEL FASTENERS AND CONNECTORS SHOULD BE TYPE 304 OR 316. NOTE: ELECTROPLATED GALVANIZED FASTENERS AND CONNECTOR: NOT TO BE USED WITH PRESSURE TREATED WOOD. SIMPSON PRODUCT FINISHES CORRESPONDING TO THE ABOVE REQUIREMENT ARE ZMAX (HOT DIPPED GALVANIZED) AND SST300 (STAINLESS STEEL). STAINLESS STEEL HARDWARE AND FASTENERS SHALL NOT COMBINED WITH UNTREATED OR GALVANIZED MATERIAL.

23. WOOD FASTENERS:

NAIL SIZES SPECIFIED ON DRAWINGS ARE BASED ON THE FOLLOWING SPECIFICATIONS:

| <u>SIZE</u> | <u>LENGTH</u> | <u>DIAMETER</u> |
|-------------|---------------|-----------------|
| 6d          | 2"            | 0.113"          |
| 8d          | 2-1/2"        | 0.131"          |
| 10d         | 3"            | 0.148"          |
| 12d         | 3-1/4"        | 0.148"          |
| 16d         | 3-1/2"        | 0.162"          |
|             |               |                 |

DESIGN IS BASED ON COMMON STEEL WIRE NAILS MEETING THE REQUIREMENTS OF ASTM F1667. USE OF ALTERNATE FASTENERS MUST BE SUBMITTED FOR REVIEW AND APPROVAL BY THE STRUCTURAL ENGINEER PRIOR TO THE START OF CONSTRUCTION.

- B. NAILS PLYWOOD (APA RATED SHEATHING) FASTENERS TO FRAMING SHALL BE DRIVEN FLUSH TO FACE OF SHEATHING WITH NO COUNTERSINKING PERMITTED.
- 24. WOOD FRAMING NOTES THE FOLLOWING APPLY UNLESS OTHERWISE SHOWN ON THE PLANS:

A. ALL WOOD FRAMING DETAILS NOT SHOWN OTHERWISE SHALL BE CONSTRUCTED TO THE MINIMUM STANDARDS OF THE INTERNATIONAL BUILDING CODE. MINIMUM NAILING, UNLESS OTHERWISE NOTED, SHALL CONFORM TO TABLE 2304.9.1 OF THE INTERNATIONAL BUILDING CODE. UNLESS NOTED OTHERWISE, ALL NAILS SHALL BE AS SPECIFIED ABOVE. COORDINATE THE SIZE AND LOCATION OF ALL OPENINGS WITH MECHANICAL AND ARCHITECTURAL DRAWINGS. PROVIDE WASHERS UNDER THE HEADS AND NUTS OF ALL BOLTS AND LAG SCREWS BEARING ON WOOD. INSTALLATION OF BOLTS AND LAG SCREWS SHALL CONFORM TO SECTIONS 11.1.2 AND 11.1.3 OF THE 2015 NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION. NATURALLY DURABLE OR PRESSURE TREATED WOOD SHALL BE PROVIDED WHERE REQUIRED BY SECTION 2304.11 OF THE INTERNATIONAL BUILDING CODE.

B. WALL FRAMING: ALL STUD WALLS SHOWN AND NOT OTHERWISE NOTED SHALL BE 2X6 AT 16" O.C. TWO STUDS MINIMUM SHALL BE PROVIDED AT THE END OF ALL WALLS AND AT EACH SIDE OF ALL OPENINGS. TWO 2 x 8 HEADERS SHALL BE PROVIDED OVER ALL OPENINGS NOT OTHERWISE NOTED AND SHALL BEAR FULLY ON A MINIMUM OF TWO STUDS. SOLID BLOCKING FOR WOOD COLUMNS SHALL BE PROVIDED THROUGH FLOORS TO SUPPORTS BELOW. PROVIDE

BORED HOLES SHALL NOT BE LOCATED WITH 5/8" FROM THE EDGE OF THE STUD OR AT THE SAME LOCATION AS A NOTCH

WALLS SHALL HAVE A SINGLE BOTTOM PLATE AND A DOUBLE TOP PLATE. END NAIL TOP PLATE TO EACH STUD WITH TWO 16d NAILS, AND TOENAIL OR END NAIL EACH STUD TO BOTTOM PLATE WITH TWO 16d NAILS. FACE NAIL DOUBLE TOP PLATE WITH 16d AT 12" O.C. AND LAP MINIMUM 4'-0" AT JOINTS AND PROVIDE EIGHT 16d NAILS AT 4" O.C. EACH SIDE OF JOINT.

ALL STUD WALLS SHALL HAVE THEIR LOWER WOOD PLATES ATTACHED TO WOOD FRAMING BELOW WITH 16d NAILS AT 12" O.C. STAGGERED OR BOLTED TO CONCRETE WITH 5/8" DIAMETER ANCHOR BOLTS (WITH 7" MINIMUM EMBEDMENT) @ 4'-0" O.C. UNLESS INDICATED OTHERWISE. PROVIDE 3"x3" x1/4" HOT-DIPPED GALVANIZED PLATE WASHERS AT ALL ANCHOR BOLTS. INDIVIDUAL MEMBERS OF BUILT-UP POSTS SHALL BE NAILED TO EACH OTHER WITH 16d NAILS @ 12" O.C. STAGGERED. REFER TO THE PLANS AND SHEAR WALL SCHEDULE FOR REQUIRED SHEATHING AND NAILING. WHEN NOT OTHERWISE NOTED, PROVIDE GYPSUM WALLBOARD ON INTERIOR SURFACES NAILED TO ALL STUDS, TOP AND BOTTOM PLATES AND BLOCKING WITH NAILS AT 7" O.C. USE 5d COOLER NAILS FOR 1/2" GWB AND 6d COOLER NAILS FOR 5/8" GWB. PROVIDE 15/32" APA RATED SHEATHING (SPAN RATING 24/0) ON EXTERIOR SURFACES NAILED AT ALL PANEL EDGES (BLOCK UNSUPPORTED EDGES), TOP AND BOTTOM PLATES WITH 8d NAILS @ 6" O.C. AND TO ALL INTERMEDIATE STUDS AND BLOCKING WITH NAILS @ 12" O.C. ALLOW 1/8" SPACING AT ALL PANEL EDGES AND ENDS.

C. FLOOR AND ROOF FRAMING: PROVIDE DOUBLE JOISTS UNDER ALL PARALLEL PARTITIONS THAT EXTEND OVER MORE THAN HALF THE JOIST LENGTH AND AROUND ALL OPENINGS IN FLOORS OR ROOFS UNLESS OTHERWISE NOTED. PROVIDE SOLID BLOCKING AT ALL BEARING POINTS.

NOTCHES AT THE END OF JOISTS AND RAFTERS SHALL NOT EXCEED 1/4 THE DEPTH OF THE MEMBER. NOTCHES IN THE TOP OR BOTTOM SHALL NOT EXCEED 1/6 THE DEPTH OF THE MEMBER AND SHALL NOT BE LOCATED WITHIN THE MIDDLE 1/3 OF THE SPAN. THE DIAMETER OF ROUND HOLES BORED IN JOISTS AND RAFTERS SHALL NOT EXCEED 1/3 OF THE DEPTH OF THE MEMBER AND SHALL NOT BE LOCATED WITHIN 2" FROM THE TOP OR BOTTOM EDGE.

TOENAIL JOISTS TO SUPPORTS WITH TWO 16d NAILS. ATTACH TIMBER JOISTS TO FLUSH HEADERS OR BEAMS WITH SIMPSON METAL JOIST HANGERS IN ACCORDANCE WITH NOTES ABOVE. NAIL ALL MULTI-JOIST BEAMS TOGETHER WITH TWO ROWS OF 16d @ 12" O.C. ATTACH RAFTERS AT BEARING LINES WITH H2.5 @ 24" O.C. UNLESS OTHER METAL CONNECTIONS ARE PROVIDED.

UNLESS OTHERWISE NOTED ON THE PLANS. APA RATED ROOF AND FLOOR SHEATHING SHALL BE LAID UP WITH STRENGTH AXIS PERPENDICULAR TO SUPPORTS AND NAILED WITH NAILS @ 6" O.C. TO FRAMED PANEL EDGES AND OVER STUD WALLS AS SHOWN ON PLANS AND @ 12" O.C. TO INTERMEDIATE SUPPORTS. PROVIDE APPROVED PLYWOOD EDGE CLIPS CENTERED BETWEEN JOISTS/TRUSSES AT UNBLOCKED ROOF SHEATHING EDGES. ALL FLOOR SHEATHING EDGES SHALL

2X DECKING SHALL BE TOENAILED THROUGH THE TONGUE AND FACE NAILED WITH ONE 16d NAIL PER PIECE PER SUPPORT.

3X AND 4X DECKING SHALL BE TOENAILED WITH ONE 40d NAIL AND FACE NAILED WITH ONE 60d NAIL PER SUPPORT. COURSES SHALL BE SPIKED TOGETHER WITH 8" SPIKES AT 30" O.C. (MAXIMUM) AND AT 10" (MAXIMUM) FROM EACH END OF EACH PIECE. SPIKES SHALL BE INSTALLED IN PREDRILLED EDGE HOLES.

> 13 October 2021 Permit Revisions

**Building Permit Set** 19 May 2021

Loeser Van Kessel

Mercer Island WA.98040

Residence 7426 S.E . 71st Street

Structural

Notes

### **ROOF FRAMING PLAN NOTES**

- 1. ROOF SHEATHING SHALL BE 15/32" APA RATED SHEATHING WITH A PANEL INDEX OF 24/0. NAIL TO FRAMING WITH 8D COMMON NAILS AT 6" OC AT PANEL EDGES AND 12" OC IN FIELD UNLESS NOTED OTHERWISE ON PLANS. WHERE NOTED ON THE PLANS ALL PANEL EDGES SHALL BE BLOCK WITH MINIMUM 2X MATERIAL.
- 2. ALL HEADERS AND BEAMS SHALL BE (2) 2X8 MINIMUM, U.N.O. REFER TO NOTE 3 FOR SUPPORT REQUIREMENTS.
- 3. ALL COLUMNS SHALL BE DOUBLE STUD MINIMUM, U.N.O., WITH THE BEAM OR HEADER BEARING FULLY ON THE COLUMN. INDIVIDUAL STUDS SHALL BE NAILED TOGETHER PER THE GENERAL STRUCTURAL NOTES.
- 4. EXTERIOR WALL SHEATHING SHALL BE 15/32" APA RATED SHEATHING WITH A PANEL INDEX OF 24/0 (ORIENTED STRAND BOARD OF EQUIVALENT THICKNESS, EXPOSURE RATING, AND PANEL INDEX MAY BE USED IN LIEU OF PLYWOOD AT CONTRACTORS OPTION).

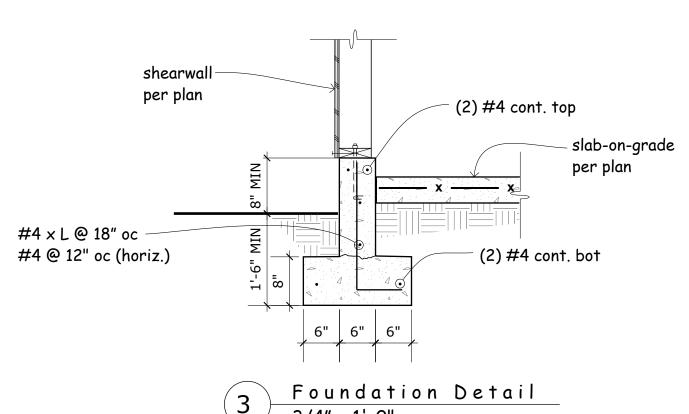
#### FOUNDATION PLAN NOTES

- 1. ALL SLABS-ON-ON-GRADE SHALL BE 4" REINFORCED WITH WWF6X6 W1.4XW1.4 U.N.O. PROVIDE MINIMUM 6-MIL VISQUEEN VAPOR BARRIER UNDER ALL SLABS. SLABS SHALL BE SUPPORTED ON A MINIMUM 4 INCHES OF FREE DRAINING
- 2. AT HOLDOWNS PROVIDE THE FOLLOWING ANCHOR BOLTS:

HOLDOWN ANCHOR

HDU2 SSTB16 (OR %"Ø BOLT, SET EPOXY EMBED 10")

3. ALL ANCHORS TO BE INSTALLED AS REQUIRED BY MANUFACTURER. MINIMUM (2) 2X STUDS UNLESS OTHERWISE NOTED ON PLANS.



|      | <u> </u>             |        |                               |   | Bottom Plate Attachment |                                       |  | Capacity           |
|------|----------------------|--------|-------------------------------|---|-------------------------|---------------------------------------|--|--------------------|
| Mark | Sheathing            | Blck'g | Panel<br>Nailing <sup>1</sup> | Attachment<br>to top plate <sup>3</sup> | Rim<br>Joist Req'd      | Nailing to <sup>4</sup><br>wood below | A. Bolts to <sup>5</sup><br>concrete below | (plf)<br>(Seismic) |
| SW 1 | 15/32" APA Sheathing | Yes    | 8d @ 6"oc                     | CLIP @ 24"oc                            | 2x                      | 16d @ 6"oc                            | 5/8" @ 48"oc                               | 240                |

<sup>1</sup> Nails shall be 8d box. Nailing applies to all panel edges (block all unsupported panel edges), top & bottom plates and blocking. Nail to intermediate framing members w/ 8d @ 12"oc. (Note: where stud spacing is 24"oc, nail to intermediate framing members with 8d@6"oc.)

<sup>2</sup> Not used. <sup>3</sup> Clip shall be either A35, LTP4.

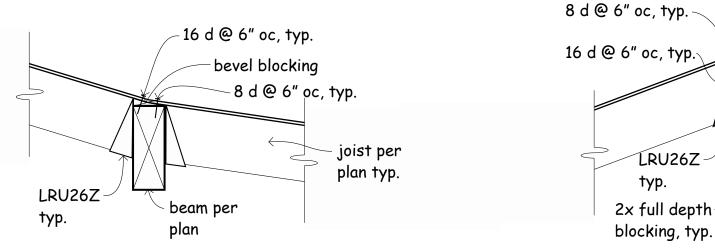
<sup>4</sup> Nails shall be 16d box (0.135Øx3½") or 10d common (0.148Øx3½")

Screws shall be Simpson SDS25412 (1/4"Øx4½"min).

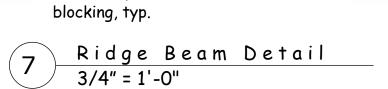
<sup>5</sup> Provide 3"x3"x0.229" plate washer at all anchor bolts. Anchor bolts shall be positioned such

that plate edge of plate washer is with 1/2" of the edge of the bottom plate.

(Plate washers may be diagonally slotted with a width of up to 13/16" and a length not to exceed 1¾")



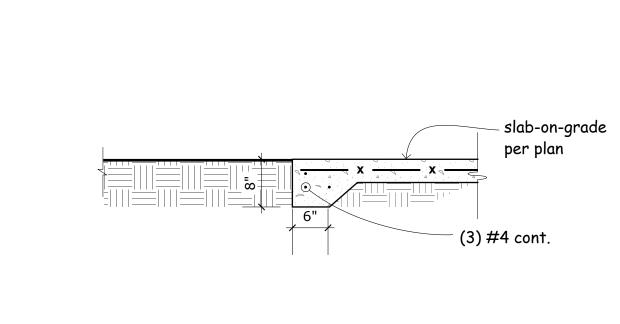
Edge Beam Detail 3/4" = 1'-0"

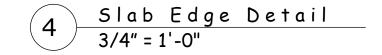


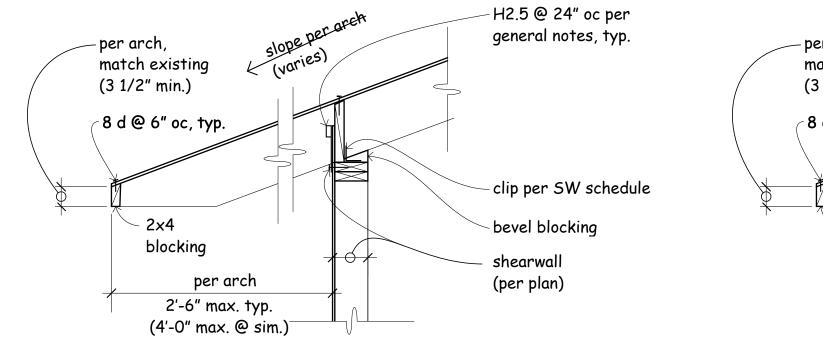
beam per

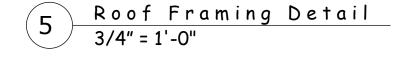
joist per

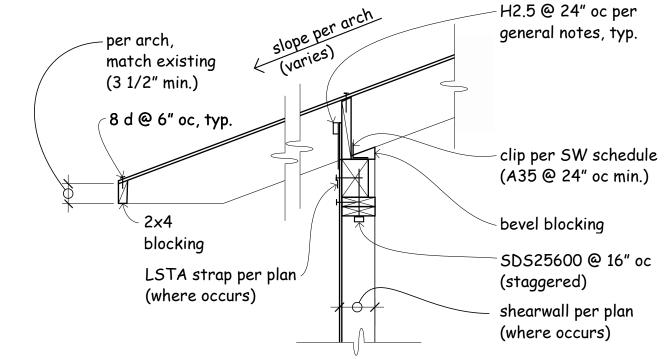
plan typ.

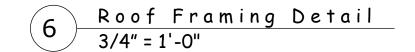






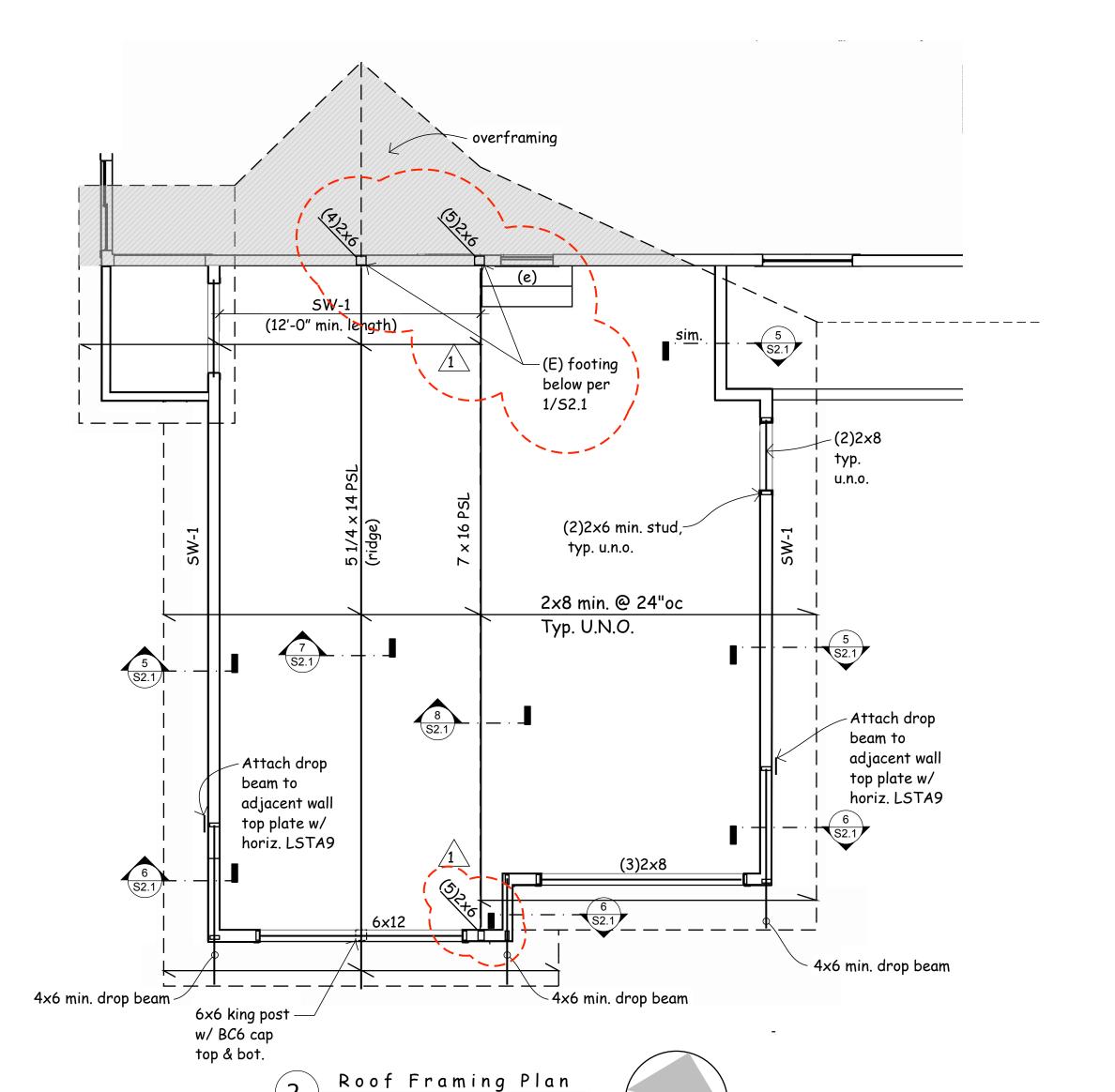




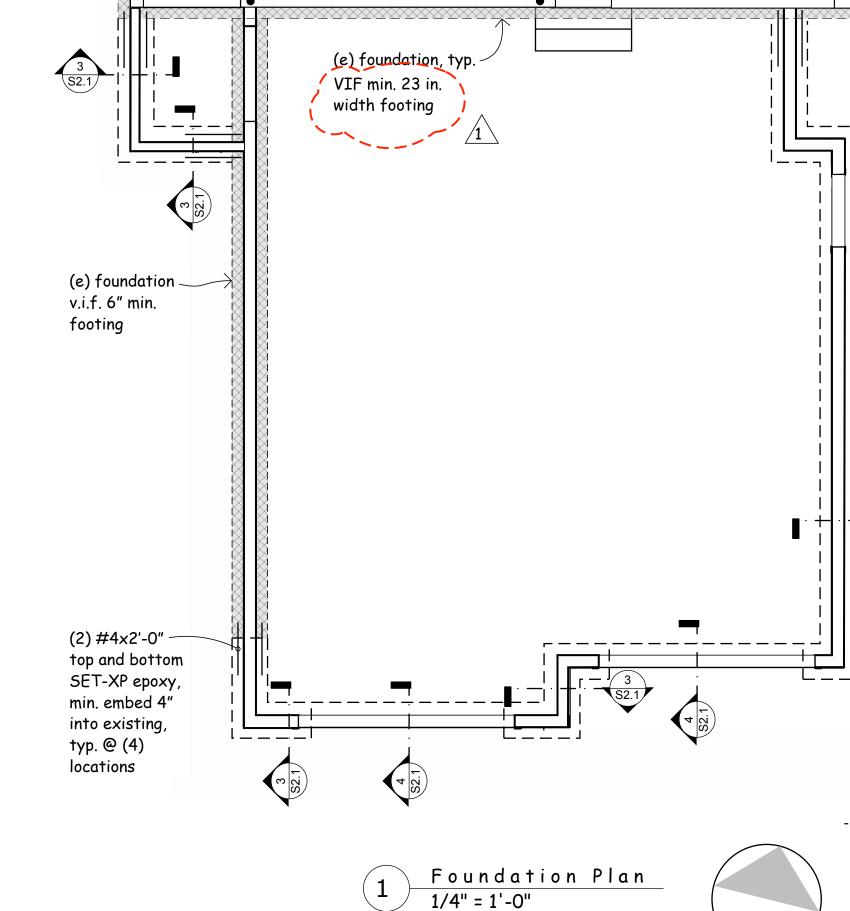


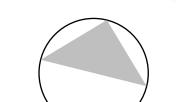
new foundation, typ.

S2.1



1/4" = 1'-0"





13 October 2021 Permit Revisions

Hughes Studio

820 John Street Seattle, WA 98109 Phone 206.264.1301 Fax 206.264.2522

Building Permit Set 19 May 2021

Loeser Van Kessel Residence

7426 S.E . 71st Street Mercer Island WA.98040

Framing Plans & Details