

NFPA 13d Fire Sprinkler System and NFPA "Chapter 29" Monitored Fire Alarm Required. Monitored Alarm may substitute for typical "line voltage" smoke detectors as noted on the floor plans.

LOT SLOPE

HIGH POINT = 272.1
LOW POINT = 224.5
LOT SLOPE = 47.57
LOT COVERAGE = 3

F.A.R. CALCULATION

Main Floor FA= 2222.75 sf (inc. gar) Lower Floor FA = 2256 sf (with aadu) Upper Floor FA = 2120 sf 6598.75

excepted FA = (-1504.22 sf)stairs = $(74 \text{ sf } x^2 = 148 \text{ sf})$

TOTAL chargeable FA = 4946.5 sf allowable = 11,200 x .45 (w/adu) = 5040 sf 5020.5 / 11,200 = 44.17%

327 = SPOT ELEVATION, FINAL

----= EAVE/ROOF LINE ----- = EXTENT OF LIVING AREA SHADED AREA = BLDG EXTENTS TO EAVE EXISTING HOUSE, DRIVEWAY AND ALL HARDSCAPE ON PROPERTY TO BE REMOVED — — — = EXISTING TOPOGRAPHY

55' 7'/160' = 29.73% 35%

LOT COVERAGE (SHADED AREA)

House Roof to eaves (shaded x 2) = 2871 sf covered porches/decks = 295 sf driveway (shaded) = 615.3 sf TOTAL = 3781.3 sf allowable = 11,200 x .35 = 3,920 sf

amount available for hardscape = 138.7 sf

HARDSCAPE (DOTTED AREA) DECKS, PATIOS , WALKS ETC = 437.4 sf

allowable = 11,200 x .09 = 1008 sf extra lot cov. = 138.7 TOTAL allow. = 1146.7 sf

All Japanese knotweed (Polygonum cuspidatum) and Regulated Class A, Regulated Class B, and Regulated Class C weeds identified on the King County Noxious Weed list, as amended, shall be removed from the property.

development proposals for a new single-family home shall remove japanese knotweed (polygonum cuspidatum) and regulated class a, regulated class b, and regulated class c weeds identified on the king county noxious weed list, as amended, from required landscaping areas established pursuant to subsection 19.02.020(f)(3)(a). new landscaping associated with new single-family home shall not incorporate any weeds identified on the king county noxious weed list, as amended. provided, that removal shall not be required if the removal will result in increased slope instability or risk of landslide or erosion.

Civil Engineer

Nick Bossoff 191 NE Tari Lane Stevenson WA 98648 425.881.5904

Geotechnical Engineer

Sam Adettiwar, MS, PE, GE, P.Eng American Geoservices 24 Roy Street #727 Seattle, WA 98109 (206) 418-6634

Structural Engineer

Javid Abdi, PE, SE Atlas Consulting Structural Engineers 6810 NE 149th St Kenmore WA 98028 Phone: (206) 427-7233

Contractor

Mike Yeganeh Aspen Homes NW (206) 799-3016

Project Description

Demolish existing and build new single family residence with attached accessory dwelling unit.

Parcel Number/Legal

Parcel # = 502190-0490 Legal Description: MADRONA CREST ADD Plat Block: 4 Plat Lot: 5 ZONING = R-8.4lot size = 11,200 sf

Owner

ANANTA & SATYA GUDIPATY 3737 77TH AVE SE MERCER ISLAND WA 98040

REGISTERED ARCHITECT CHRIS LUTHI STATE OF WASHINGTON CENTERLINE DESIGN 4737 37th AVE SW SEATTLE 206.935.4684 www.Centerline-Design.com \triangleleft \geq sland Mithala SE Mercer Is Γ Ve \checkmark 90th 3632 CONTENTS Site Plan DRAWN BY CRL DATE 11.17.22 a



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td	
	4.97
	1.27
	4.61
	36.80
	24.00
	6.67
	16.00
	30.38
	1.69
	9.50
	1.36
	1.51

	EL @ MIDPOINT	segment	wtd sgmnt
		(ft)	
1	265.50	40	10620.00
2	257.70	62.04	15987.71
3	257.50	24	6180.00
4	256.00	9	2304.00
5	256.00	16	4096.00
6	261.80	2.33	609.99
7	262.00	2	524.00
8	263.00	12.46	3276.98
9	263.00	2	526.00
10	265.00	38.25	10136.25

1/10" = 1'-0"

 $\widehat{(A)}$ = WALL SEGMENT TAG FOR BASEMENT FAR EXCEPTION ① = WALL SEGMENT TAG FOR HEIGHT CALCULATION

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A. SUPPLEMENTAL SITE PLAN

-----= EAVE/ROOF LINE











ADU project narrative

An ADU attached to a new SFR as part of the new construction project (permit 2210-198) will include 770.5 sq. ft of living space, it will include a full kitchen with its own dishwasher, sink, oven, refrigerator, microwave and washer and dryer. There will be a separate entrance that connects by walkway to 90th ave SE. The ADU will include a living room and bedroom with an attached full bathroom. Heating control will be separate from the main house.

The ADU is within the size limits of 19.02.030 B4. The location meets 19.02.030 B5. The entrance of the ADU meets 19.02.030 B6 Parking for the ADU meets 19.02.030 B9

The ADU will be recorded as such with the King County Department of records and elections which runs with the land and identifies the address of the property, states the owner resides in either principle dwelling unit or the accessory dwelling unit, includes a statement that the owners will notify any prospective purchasers of the limitations of this section, and provides for the removal of the accessory dwelling unit if any of the requirements of this chapter are violated.



NOTES

SD] = SMOKE DETECTOR, HARDWIRE, INTERCONNECTED w/ BATTERY BACK-UP

CO = CARBON MONOXIDE DETECTOR, HARDWIRE w/ BATTERY BACK-UP HD = HEAT DETECTOR, HARDWIRE w/ BATTERY BACK-UP

DOORS ARE 3-0 x 6-8 (r.o. = 3'-2" x 6'-10") unless otherwise indicated

S = FAN, 50 CFM UNLESS OTHERWISE INDICATED

FOR SHEAR WALL INFORMATION SEE STRUCTURAL PLANS

ALL INTERIOR WALLS TO BE 2x4, EXTERIOR WALLS 2x6, EXCEPT AS INDICATED, OR EXISTING E =EGRESS WINDOWS

Contractor shall verify to Inspector all guards and railings shall be capable of resisting 200 lb load on top rail acting in any direction as required by IRC Table R301.5.

ALL WALLS FULL HIEGHT UNLESS OTHERWISE INDICATED

T) =TEMPER/SAFETY GLAZE WINDOWS

ALL GAS F.P. TO BE APPROVED DIRECT VENT





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R301.5. ALL WALLS FULL HIEGHT UNLESS OTHERWISE INDICATED

(T) =TEMPER/SAFETY GLAZE WINDOWS

ALL GAS F.P. TO BE APPROVED DIRECT VENT

FOAM INSULATION NOTES

Closed cell spray foam directly applied to underside of sheathing (min R-10) + batts to = r-49 (R-38 min. @ vaulted areas) Spray foam product to be "Spraytite 178" as manufactured by BASF (ESR-2642), or equal. Spray foam insulation shall be installed per IRC 806.5.1.3. A copy of the ICC ESR report for the product used must be provided on the job site for field inspector verification The applied spray foam must be installed by a certified installer.

ADU CLG. SOUND/FIRE REQUIREMENTS

Provide sound insulation (STC rating of at least 45 & ICC rating of at least 50) and 1 hr fire resistance in the entire ADU ceiling (including under stairs). See ESR-1153 Assembly B. Requirements:

1. 48/24 tongue-and-groove span rated sheathing (Exposure 1).

1. Two layers of 1/2 inch thick Type X gypsum board. 2. TJI Joist.

3. Optional minimum 3-1/2 inch thick glass fiber insulation or non-combustible insulation that is rated R-30 or less, with resilient channels



















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Energy Code Info - Primary

2018 WA STATE PRESCRIPTIVE PATH OVER 5000 SF HEATED SPACE - 7 CREDITS REQ.

energy credit option credit value summary

1.7	0.5	ins. over wall, .28 windows
2	1	heat pump
2.2	1	2.0 ACH + HRV
3.5	1.5	central HP, HSPF>=11
4.1	0.5	AH in heated space
5.5	2	elec. HP WH
7.1	0.5	appliance package

PRIMARY RESIDENCE HVAC NOTES

total credits

DUCTED HEAT PUMP (HSPF>11.0) INT. AIR HANDLER HEAT RECOVERY VENTILATION REQUIRED VENTING = CONTINUOUS 120CFM SET TO OPERATE AT 240 CFM FOR 2 HOURS IN EA. 4 HR PERIOD (50%) PROVIDED BY VARIABLE SPEED HIGH EFF. FAN (MAX .35 WATTS/CFM) CONTOLLED TO OPERATE AT LOW SPEED IN VENTILATION MODE ONLY.

design professional or builder shall complete and post an "Insulation Certificate for **Residential Construction**" within 3' of the electrical panel prior to final inspection.

Maximum flow rates for shower heads and kitchen sink - 1.75 GPM or less. All other lavatory faucets - 1.0 GPM or less.

Per WSEC R402.4, The building thermal Envelope shall be constructed to limit air leakage to 2.0 air changes per hour maximum. The results of the test shall be signed by the party conducting the test and provided to the code official (R402.4.1.2). Per WSEC R403.1.1, at least one thermostat per dwelling unit shall be capable of controlling the heating and cooling system on a daily schedule. Per WSEC R403.2.2, Ducts, air handlers, and filter boxes shall be sealed. Per WSEC R404.1, A minimum of 75 percent of the lamps in permanently installed lighting fixtures shall be high-efficacy lamps.

	All Climate Zones (Table R402.1.1)				
	R-Value ^a	U-Factor ^a			
Fenestration U-Factor ^b	n/a	>₀.30 <.28 (primary only)			
Skylight U-Factor ^b	n/a	0.50			
Glazed Fenestration SHGC ^{b,e}	n/a	n/a			
Ceiling ^e	49	0.026			
Wood Frame Wall ^{g,h}	21 int	0.056			
Floor	30	0.029			
Below Grade Wall ^{c,h}	10/15/21 int + TB	0.042			
Slab ^{d,f} R-Value & Depth	10, 2 ft	n/a			
R-values are minimums. U-fac	tors and SHGC are maximums. When insu	lation is installed in a cavity that is less			
a than the label or design thickn	ess of the insulation, the compressed <i>R</i> -v	alue of the insulation from Appendix			
Table A101.4 shall not be less	than the <i>R</i> -value specified in the table.				
b The fenestration <i>U</i> -factor colu	mn excludes skylights.				
"10/15/21 +5TB" means R-10	continuous insulation on the exterior of th	ne wall, or R-15 continuous insulation on			
the interior of the wall, or R-2	or of the wall, or R-21 cavity insulation plus a thermal break between the slab and the basement wall at				
c the interior of the basement w	e interior of the basement wall. "10/15/21 +51B" shall be permitted to be met with R-13 cavity insulation on				
the interior of the basement w	vall plus R-5 continuous insulation on the	Interior or exterior of the Wall. "51B"			
means R-5 thermal break betv	veen floor slap and basement wall.	- Cas Castier D402.2.0.1			
a R-10 continuous insulation is r	equired under neated slab on grade hoor	S. See Section R402.2.9.1.			
e evente aver the ter plate of t	ed cellings, the insulation may be reduced	t to R-38 if the full insulation depth			
B.7.5 continuous insulation in	extends over the top plate of the exterior wall.				
f clab insulation when applied t	statied over an existing slab is deemed to	be equivalent to the required perimeter			
stab insulation when applied t	503.1.1. Il Toam plastic is used, it shall				
For log structures developed	ermai barners protecting toam plastics.				
For log structures developed I	n compliance with Standard ICC 400, log v	valls shall meet the requirements for			
climate zone 5 of ICC 400.	notoo fuqueing and inculation of described	in Continue A102 2 2 in all diversities at an devel			
Int. (Intermediate framing) de	notes framing and insulation as described	in Section A103.2.2 including standard			
in Traming 16 inches on center, /	8% of the wall cavity insulated and heade	ers insulated with a minimum of R-10			
insulation.					

Energy Code Info - AADU

2018 WA STATE PRESCRIPTIVE PATH LESS THAN 1500 SF HEATED SPACE - 3 CREDITS REQ.

energy credit option credit value summary

2	1	heat pui
3.6	2	mini-spl

total credits

AADU RESIDENCE HVAC NOTES

MINI-SPLIT HEAT PUMP (HSPF>10.0) HEAT RECOVERY VENTILATION REQUIRED VENTING = CONTINUOUS 120CFM SET TO OPERATE AT 240 CFM FOR 2 HOURS IN EA. 4 HR PERIOD (50%) PROVIDED BY VARIABLE SPEED HIGH EFF. FAN (MAX .35 WATTS/CFM) CONTOLLED TO OPERATE AT LOW SPEED IN VENTILATION MODE ONLY.

1.7

ENERGY CREDIT DESCRIPTIONS

Advanced framing and raised heel trusses or rafters Vertical Glazing U-0.28

R-49 Advanced (U-0.020) as listed in Section A102.2.1, Ceilings below a vented attic and R-49 vaulted ceilings with full height of uncompressed insulation extending over the wall top plate at the eaves.

2.2

Compliance based on Section R402.4.1.2: Reduce the tested air leakage to 2.0 air changes per hour at maximum 50 Pascals or

For R-2 Occupancies, optional compliance based on Section R402.4.1.2: Reduce the tested air leakage to 0.25 cfm/sf maximum at 50 Pascals and

All whole house ventilation requirements as determined by Section M1507.3 of the International Residential Code or Section 403.8 of the International Mechanical Code shall be met with a heat recovery ventilation system with minimum sensible heat recovery efficiency of 0.65.

3.5

Air-source, centrally ducted heat pump with minimum HSPF of 11.0.

4.1

All supply and return ducts located in an unconditioned attic shall be deeply buried in ceiling insulation in accordance with Section R403.3.7.

For mechanical equipment located outside the conditioned space, a maximum of 10 linear feet of return duct and 5 linear feet of supply duct connections to the equipment may be outside the deeply buried insulation. All metallic ducts located outside the conditioned space must have both transverse and longitudinal joints sealed with mastic. If flex ducts are used, they cannot contain splices.

Duct leakage shall be limited to 3 cfm per 100 square feet of conditioned floor area. Air handler(s) shall be located within the conditioned space.

5.5

Water heating system shall include one of the following: Electric heat pump water heater meeting the standards for Tier III of NEEA's advanced water heating specification or For R-2 Occupancy, electric heat pump water heater(s), meeting the standards for Tier III of NEEA's advanced water heating specification, shall supply domestic hot water to all units. If one water heater is serving more than one dwelling unit, all hot water supply and recirculation piping shall be insulated with R-8 minimum pipe insulation.

7.1

All of the following appliances shall be new and installed in the dwelling unit and shall meet the following standards: Dishwasher Energy Star rated Refrigerator (if provided) Energy Star rated Washing machine Energy Star rated Dryer Energy Star rated, ventless dryer with minimum CEF rating of 5.2

To qualify to claim this credit, the building permit drawings shall specify the option being selected and shall show the appliance type and provide documentation of Energy Star compliance. At the time of inspection, all appliances shall be installed and connected to utilities. Dryer ducts and exterior dryer vent caps are not permitted to be installed in the dwelling unit.

ADU

Vindow, Skylight and Door Schedule oject Information			
ITHILA PRIMARY	Com	tact Information	
	Ref. U-factor	Width Height Qt. Feet ^{Inch} Feet ^{Inch}	Area UA
xempt Swinging Door (24 sq. ft. max.) Exempt Glazed Fenestration (15 sq. ft. max.)			0.0 0.00 0.0 0.00
ertical Fenestration (Windows and doors)			
Component Description	Ref. U-factor	Width Height Qt. Feet ^{Inch} Feet ^{Inch}	Area UA
ENTRY LR	0.28	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	45.0 12.60 109.4 30.63
	0.28	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	225.1 63.02 80.8 22.64
KITCHEN KITCHEN	0.28	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	40.0 11.20
G BATH	0.28	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	4.0 1.12
G BED GAR DOOR	0.28	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	64.0 17.92 25.3 7.09
STAIR BATH 2	0.28	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	42.2 11.82 12.0 3.36
BED 1 BED 1	0.28	1 6 ° 6 ° 1 10 ° 6 °	36.0 10.08 60.0 16.80
LAUNDRY M BED	0.28 0.28	1 2 0 6 0 1 12 0 6 0	12.0 3.36 72.0 20.16
M BED M BATH	0.28 0.28	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	36.010.0860.016.80
OFFICE OFFICE	0.28 0.28	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	42.0 11.76 128.0 35.84
MEDIA MEDIA	0.28	$\begin{array}{cccccccccccccccccccccccccccccccccccc$	128.0 35.84 71.8 20.09
BED2 STAIR	0.28	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	25.0 7.00 14.4 4.04
FLEX FLEX FLEX	0.28	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	<u>39.0</u> 10.91 <u>30.0</u> 8.40
FLEX	0.28	$1 6 ^{2} 8 ^{0}$	49.3 13.81
			0.0 0.00
			0.0 0.00
	Image: Constraint of the second sec	Image: second	0.0 0.00 0.0 0.00 0.0 0.00 0.0 0.00
	Sum of Vertic Vertical Fenestration	al Fenestration Area and UA Area Weighted U = UA/Area	1612.1 451.38 0.28
erhead Glazing (Skylights)		Width Height	
Component			
Component Description M BATH	Ref. U-factor 0.50	Qt. Feet ^{Inch} Feet ^{Inch}	Area UA 16.0 8.00
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Component Description M BATH M CLO Total Sum of Fenestration Area a	Ref. U-factor	Qt. Feet Inch 1 2 4 0 1 2 4 0 1 2 4 0 1 2 4 0 1 2 4 0 1 2 0 4 0 1 2 0 4 0 1 2 0 4 0 1 2 0 4 0 1 2 0 4 0 1 2 0 4 0 1 2 0 4 0 1 0 0 0 0 erhead Glazing Area and UA Area Width Height tact Information Vidth Height 0	Area UA 16.0 8.00 8.0 4.00 0.0 0.00 0.0 0.00 0.0 0.00 0.0 0.00 0.0 0.00 1636.1 463.38
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General Structural Notes (GSN's)

1. ALL MATERIALS, WORKMANSHIP, DESIGN, AND CONSTRUCTION SHALL CONFORM TO THE DRAWINGS, SPECIFICATIONS, AND THE INTERNATIONAL BUILDING CODE (IBC) WITH WASHINGTON STATE ADMINISTRATIVE CODE AMENDMENTS, 2018 EDITION.

2. DESIGN LOADING CRITERIA ROOF SNOW LOAD $\ldots \ldots 25$ PSF (I_S = 1.0) + 5 PSF RAIN ON SNOW SURCHARGE DECK LIVE LOAD 60 PSF $S_{S} = 1.403, S_{1} = 0.488, S_{DS} = 1.122, S_{D1} = 0.590$ EQUIVALENT LATERAL FORCE PROCEDURE

LIGHT FRAME (WOOD) WALLS AND ROOFS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR $R = 6.5, \Omega_0 = 2\frac{1}{2}, I_E = 1.0, C_d = 4, C_s = 0.172$ BASE SHEAR V = 50.9 K - LRFD

-60.0/-36.0 GROSS UPLIFT AT ROOF (LRFD/ASD) WIND PRESSURES BASED ON LESS THAN 10 SQUARE FOOT TRIBUTARY AREAS NEAR WALL CORNERS OR ROOF EDGES (EXCLUDING CORNER ZONES AT ROOF). REDUCED DESIGN PRESSURES MAY BE CALCULATED IN ACCORDANCE WITH ASCE 7–16 CHAPTER 30.

- STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ALL OTHER CONTRACT DOCUMENTS FOR BIDDING AND CONSTRUCTION. CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS FOR COMPATIBILITY AND SHALL NOTIFY ENGINEER OF ALL DISCREPANCIES PRIOR TO CONSTRUCTION. IT IS THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE BUILDING LAYOUT DIMENSIONS (GRID LAYOUTS, SITE COORDINATES, ETC.) AMONGST ALL TRADES, INCLUDING SHOP FABRICATED ITEMS.
- 4. CONTRACTOR SHALL PROVIDE TEMPORARY BRACING, BOTH FOR VERTICAL LOADS AND LATERAL STABILITY, FOR THE STRUCTURE AND STRUCTURAL COMPONENTS UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE DRAWINGS.
- 5. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS. TECHNIQUES. SEQUENCES OR PROCEDURES REQUIRED TO PERFORM THE WORK.
- 6. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER.
- 7. ALL STRUCTURAL SYSTEMS COMPOSED OF COMPONENTS TO BE FIELD ERECTED SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH INSTRUCTIONS PREPARED BY THE SUPPLIER.
- 8. SEISMIC BRACING AND/OR GRAVITY SUPPORT AND ANCHORAGE OF ALL MECHANICAL OR ELECTRICAL EQUIPMENT SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF WASHINGTON, EXCEPT FOR ELEMENTS SPECIFICALLY SHOWN AND DETAILED ON THE STRUCTURAL DRAWINGS. THE MECHANICAL/ELECTRICAL CONTRACTOR MUST HIRE THE ENGINEER AND IS RESPONSIBLE FOR ALL COSTS RELATED TO THE PURCHASE AND INSTALLATION OF NECESSARY SUPPORTS, BRACING AND ANCHORAGE. SEISMIC BRACING AND ANCHORAGE DESIGN AND CONSTRUCTION SHALL COMPLY WITH CHAPTER 13 OF ASCE 7-10.
- SHOP DRAWING REVIEW: SHOP DRAWINGS FOR TRUSSES SHALL BE SUBMITTED TO THE CONTRACTOR. ARCHITECT, AND ENGINEER OF RECORD FOR REVIEW PRIOR TO FABRICATION OF THESE ITEMS. DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE ENGINEER OF RECORD, AND THEREFORE MUST BE VERIFIED BY THE CONTRACTOR. THE CONTRACTOR SHALL REVIEW AND STAMP DRAWINGS PRIOR TO REVIEW BY ENGINEER OF RECORD. SUBMITTALS SHALL INCLUDE A REPRODUCIBLE AND ONE COPY. THE REPRODUCIBLE SHALL BE MARKED AND RETURNED. SHOP DRAWING SUBMITTALS PROCESSED BY THE ENGINEER ARE NOT CHANGE ORDERS. THE PURPOSE OF SHOP DRAWING SUBMITTALS BY THE CONTRACTOR IS TO DEMONSTRATE TO THE ENGINEER THAT THE CONTRACTOR UNDERSTANDS THE DESIGN CONCEPT BY INDICATING WHICH MATERIAL IS INTENDED TO BE FURNISHED AND INSTALLED AND BY DETAILING THE INTENDED FABRICATION AND INSTALLATION METHODS. IF DEVIATIONS, DISCREPANCIES, OR CONFLICTS BETWEEN SHOP DRAWING SUBMITTALS AND THE CONTRACT DOCUMENTS ARE DISCOVERED EITHER PRIOR TO OR AFTER SHOP DRAWING SUBMITTALS ARE PROCESSED BY THE ENGINEER, THE DESIGN DRAWINGS AND SPECIFICATIONS SHALL CONTROL AND SHALL BE FOLLOWED.
- 10. DEFERRED SUBMITTALS SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE OF WASHINGTON. THE COMPONENT DESIGNER SHALL BE A REGISTERED STRUCTURAL ENGINEER IF REQUIRED BY THE BUILDING OFFICIAL OF THE LOCAL JURISDICTION. BUILDING COMPONENT SUBMITTALS SHALL INCLUDE THE DESIGNING PROFESSIONAL ENGINEER'S STAMP AND SHALL BE APPROVED BY THE COMPONENT DESIGNER PRIOR TO CURSORY REVIEW BY THE ENGINEER OF RECORD FOR LOADS IMPOSED ON THE BASIC STRUCTURE. THE COMPONENT DESIGNER IS RESPONSIBLE FOR CODE CONFORMANCE INCLUDING ACCOMMODATION FOR STRUCTURAL DISPLACEMENT PER ASCE 7-10 SECTION 13.3.2. AND ALL NECESSARY CONNECTIONS NOT SPECIFICALLY CALLED OUT ON ARCHITECTURAL OR STRUCTURAL DRAWINGS. DEFERRED SUBMITTALS SHALL INDICATE MAGNITUDE AND DIRECTION OF ALL LOADS IMPOSED ON BASIC STRUCTURE. DESIGN CALCULATIONS SHALL BE INCLUDED IN THE SUBMITTAL. THE CONTRACTOR SHALL FORWARD DEFERRED SUBMITTALS TO THE BUILDING OFFICIAL AND HAVE THE DEFERRED SUBMITTALS ON SITE FOR THE GOVERNING JURISDICTIONS INSPECTORS USE AND REFERENCE. THE FOLLOWING BUILDING COMPONENTS SHALL BE DEFERRED SUBMITTALS FOR THIS PROJECT: - CONNECTOR PLATE WOOD TRUSSES

GEOTECHNICAL:

11. FOUNDATION NOTES: SUBGRADE PREPARATION INCLUDING DRAINAGE, EXCAVATION, COMPACTION, AND FILLING REQUIREMENTS, SHALL CONFORM STRICTLY WITH THE RECOMMENDATIONS GIVEN IN THE GEOTECHNICAL REPORT REFERENCED BELOW, THE SPECIFICATIONS, OR AS DIRECTED BY THE OWNER APPOINTED GEOTECHNICAL ENGINEER. FOOTINGS SHALL BEAR DIRECTLY ON SUBGRADE COMPACTED WITH A HOEPACK OR HAND-HELD PLATE COMPACTER. EXTERIOR FOOTINGS AND FOOTINGS IN UNHEADED AREAS SHALL BEAR AT LEAST 36" BELOW EXISTING GRADE AND AT LEAST 18" BELOW FINISHED EXTERIOR GRADES SHALLOWER THAN 5H:1V. THE OWNER APPOINTED GEOTECHNICAL ENGINEER SHALL APPROVE FOOTING EXCAVATION / PREPARATION PRIOR TO PLACEMENT OF ALL FOOTINGS. BACKFILL BEHIND ALL RETAINING WALLS WITH FREE DRAINING. GRANULAR FILL AND PROVIDE FOR SUBSURFACE DRAINAGE AS NOTED IN THE GEOTECHNICAL REPORT REFERENCED BELOW, THE SPECIFICATIONS, OR AS DIRECTED BY THE OWNER APPOINTED GEOTECHNICAL ENGINEER 2 000 DCE

ALLOWABLE SUIL PRESSURE	2,000 PSF
LATERAL EARTH PRESSURE (UNRESTRAINED, LEVEL)	50 PCF
(RESTRAINED, LEVEL)	65 PCF
SEISMIC SURCHARGE PRESSURE	9H, UNIFORM
PASSIVE EARTH PRESSURE	300 PCF
BASE COEFFICIENT OF FRICTION	0.40
GEOTECHNICAL REFERENCE: "Geotechnical Evaluation Report; 3632 90th Ave SI	E; Mercer Island, W

American Geoservices; Project No. 0531-WA21; September 15, 2022"

12. PIPE PILES SHALL BE 4"\$ (12-TON CAPACITY) SCHEDULE 40, ASTM A-53 GRADE A GALVANIZED STEEL. INSTALLATION, FINAL PENETRATION RATE, FINISH, CONNECTION, ETC. SHALL CONFORM STRICTLY WITH THE RECOMMENDATIONS GIVEN IN THE ABOVE GEOTECHNICAL REPORT REFERENCE. PIPES SHALL BE DRIVEN TO REFUSAL PER THE GEOTECHNICAL REPORT REFERENCE AND REPLICATED BELOW. MINIMUM PILE LENGTH SHALL BE 10 FEET BELOW THE BOTTOM OF FOOTING, WITH MINIMUM 5 FEET EMBEDMENT INTO DENSE GLACIAL TILL. ALL PILES SHOULD BE PROOF LOADED IN THE PRESENCE OF THE PROJECT GEOTECHNICAL ENGINEER (OR REPRESENTATIVE). LOAD TESTS SHOULD BE PERFORMED ON 3% OF INSTALLED PILES OR A MAXIMUM OF 5 PILES. AT LEAST ONE LOAD TEST SHOULD BE PERFORMED. FULL TIME PILE LOAD TESTING AND PILE INSTALLATION SHOULD BE PERFORMED BY THE PROJECT GEOTECHNICAL ENGINEER (OR REPRESENTATIVE) AND ALL PILES SHOULD BE APPROVED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF CONCRETE. PILE COUPLINGS SHALL USE THREADED OR SLIP COUPLERS IF THEY FIT TIGHTLY AND TRANSFER LOAD WITHOUT LOSS OF CAPACITY. WELDING OF PIPE SECTIONS MAY ALSO BE SUED

ANCHORAGE:

13. DRIVE PINS AND OTHER POWDER-ACTUATED FASTENERS SHALL BE ONE OF THE FOLLOWING INSTALL STRICT ACCORDANCE WITH THE ICC-ES REPORTS INDICATED AND MANUFACTURER'S INSTRUCTIONS INCLUDING MINIMUM EMBED REQUIREMENTS: "TE SERIES" (0.157" DIAMETER) AS MANUFACTURED BY RAMSET (ICC-ES NO. 1799); OR "X-U" (0.157" DIAMETER) AS MANUFACTURED BY HILTI, INC. (ICC-NO. 2269); OR "STRONG-TIE PDPA" (0.157" DIAMETER) AS MANUFACTURED BY SIMPSON STRONG-T COMPANY, INC. (ICC–ES NO. 2138); OR "CSI PIN" (0.157" DIAMETER) AS MANUFACTURED BY DEWALT/POWERS (ICC-ES NO. 2024); OR AN APPROVED EQUIVALENT IN STRENGTH AND EMBEDMEN MINIMUM EMBEDMENT IN CONCRETE SHALL BE 1" UNLESS OTHERWISE NOTED. MAINTAIN AT LEAST 3-1/2" TO NEAREST CONCRETE EDGE.

CONCRETE: 14. CONCRETE SHALL BE MIXED, PROPORTIONED, CONVEYED AND PLACED IN ACCORDANCE WITH ACI 318 CHAPTER 26 AND ACI 301. CONCRETE SHALL ATTAIN A 28-DAY STRENGTH OF I'C = 2,500 PSI. 1 CONTRACTOR SHALL USE 5-1/2 SACK 2500 PSI CONCRETE MIXES PER CODE ALTERNATE R402.2 IN SEATTLE RESIDENTIAL CODE. IN ACCORDANCE WITH INTERNATIONAL BUILDING CODE SECTION 1904.2, 5-1/2 SACK 2500 PSI CONCRETE MIXES ARE EQUIVALENT TO 3000 PSI CONCRETE FOR WEATHERING POTENTIAL. IN ADDITION, AIR-ENTRAINMENT IS NOT REQUIRED TO ADDRESS WEATHERING.

- 15. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60, fy = 60,000 PSI. GRADE 60 REINFORCING BARS WHICH ARE TO BE WELDED SHALL CONFORM TO ASTM A706. REINFORCEMENT COMPLYING WITH ASTM A615(S1) MAY BE WELDED ONLY IF MATERIAL PROPERTY REPORTS INDICATING CONFORMANCE WITH WELDING PROCEDURES SPECIFIED IN A.W.S. D1.4 ARE SUBMITTED. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A1064.
- 16. REINFORCING STEEL SHALL BE DETAILED (INCLUDING HOOKS AND BENDS) IN ACCORDANCE WITH ACI 315-99 AND 318-14. LAP ALL CONTINUOUS REINFORCEMENT IN ACCORDANCE WITH "REINFORCEMEN SPLICE AND DEVELOPMENT LENGTH SCHEDULE" OF 10/S3.1. PROVIDE CORNER BARS AT ALL WALL FOOTING INTERSECTIONS. LAP ADJACENT MATS OF WELDED WIRE FABRIC A MINIMUM OF 12" AT SIE AND ENDS. NO BARS PARTIALLY EMBEDDED IN HARDENED CONCRETE SHALL BE FIELD BENT UNLESS OTHERWISE NOTED ON THE DRAWINGS OR APPROVED BY THE STRUCTURAL ENGINEER.
- 17. CONCRETE PROTECTION (COVER) FOR REINFORCING STEEL SHALL BE AS FOLLOWS: FOOTINGS AND OTHER UNFORMED SURFACES
- FORMED SURFACES EXPOSED TO EARTH
- (i.e. WALLS BELOW GROUND)OR WEATHER (#5 BARS OR SMALLER). $1\frac{1}{2}$ "
- 18. BONDING AGENT SHALL BE "MASTEREMACO ADH 326" BY BASF CORPORATION. OR EQUIVALENT, AND SHALL BE USED WHERE NEW CONCRETE IS PLACED AGAINST HARDENED CONCRETE. PLACE IN STRIC ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS, INCLUDING PREPARATION OF EXISTING SURFACE CONCRETE SHALL BE CONSIDERED HARDENED AFTER 56 DAYS.

19. NON-SHRINK GROUT SHALL BE FURNISHED BY AN APPROVED MANUFACTURER AND SHALL BE MIXED PLACED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. GROUT STRENGTH SHAL AT LEAST EQUAL TO THE MATERIAL ON WHICH IT IS PLACED (6,000 PSI MINIMUM). IBC TABLE 1705.3 REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION

REQUIRED?	VERIFICATION & INSPECTION	CONTINUOUS	PERIODIC	REF STD.
N/A	1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS AND VERIFY PLACEMENT.		Х	ACI 318 CH. 20, 25.2, 25.3, 26.5.1-26.5.3
N/A	 2. REINFORCING BAR WELDING: A. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A 706. B. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16" AND 		Х	AWSD1.4 ACI 318 26.5.4
	C. INSPECT ALL OTHER WELDS	Х	Х	
YES	3. INSPECT ANCHORS CAST IN CONCRETE.		х	ACI 318: 17.8.2
N/A	 4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS. A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS B. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.A 	X	х	ACI 318: 17.8.2.4 ACI 318:17.8.2
N*	5. VERIFY USE OF REQUIRED DESIGN MIX.		х	ACI 318: CH. 19, 26.4.3, 26.4.4
N*	6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	х		ASTM C 172 ASTM C 31 ACI 318: 26.4.5, 26.12
N*	7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	Х		ACI 318: 26.4.5
N*	8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.		Х	ACI 318: 26.4.7-26.4.9
N/A	9. INSPECT PRESTRESSED CONCRETE FOR: A. APPLICATION OF PRESTRESSING FORCES; AND B. GROUTING OF BONDED PRESTRESSING TENDONS	X X		ACI 318: 26.9.2.1 ACI 218: 26.9.2.3
N/A	10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.		х	ACI 318: CH. 26.8
N*	11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCT'L SLABS.		х	ACI 318: 26.10.2
N*	12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.		х	ACI318: 26.10.1(b)

20. FRAMING LUMBER SHALL BE KILN DRIED OR MC-19. AND GRADED AND MARKED IN CONFORMANCE N W.C.L.I.B. STANDARD GRADING RULES FOR WEST COAST LUMBER NO. 17 OR W.W.P.A. WESTERN LUMBE GRADING RULES. FURNISH TO THE FOLLOWING MINIMUM STANDARDS:

PLATES, LEDGERS & MISC.	DOUGLAS FIR NO. 3 OR STUD GRADE
LIGHT FRAMING:	MIN. BASIC DESIGN STRESS, $F_b = 525$ PSI, $E = 1400$ k
	F _c = 775 PSI, F _t = 325 I
JOISTS & RAFTERS:	DOUGLAS FIR NO. 2
	MIN. BASIC DESIGN STRESS, $F_b = 900$ PSI, $E = 1600$ k
	$F_{c} = 1350 \text{ PSI}, F_{t} = 575$
BEAMS:	DOUGLAS FIR NO. 1
4x_	MIN. BASIC DESIGN STRESS, $F_b = 1000$ PSI, $E = 1700$
	$F_{c} = 1500 \text{ PSI}, F_{t} = 675$
6x_	MIN. BASIC DESIGN STRESS, $F_b = 1350$ PSI, $E = 1600$
	$F_{c} = 925 \text{ PSI, } F_{t} = 675 \text{ F}$
COLUMNS:	DUUGLAS FIR NU. I
4x_	MIN. BASIC DESIGN STRESS, $F_b = 1000$ PSI, $E = 1/00$
	$F_{c} = 1500 \text{ PSI}, F_{t} = 6/5$
6x_	MIN. BASIC DESIGN STRESS, $F_b = 1200$ PSI, $E = 1600$
	$F_{c} = 1000 \text{ PSI}, F_{t} = 825$

20. MANUFACTURED LUMBER SHALL BE AS MANUFACTURED BY TRUS JOIST OR APPROVED EQUAL. REQ FOR APPROVAL AS EQUAL WILL REQUIRE SUBMITTAL OF ICC REPORT EQUIVALENT TO ESR-1387 FOR LAMINATED VENNER LUMBER (LVL, LAMINATED STRAND LUMBER (LSL), OR PARALLEL STRAND LUMBEF (PSL). THE MINIMUM ALLOWABLE DESIGN VALUES ARE AS FOLLOWS: $LVL - F_{h} = 2,600$ $F_{v} = 290$ PSI E = 2,000,000 PSI

LSL –	$F_{b} = 1,900$	$F_v = 150 \text{ PSI}$	E = 1,300,000 PSI

21. GLUED LAMINATED MEMBERS SHALL BE FABRICATED IN CONFORMANCE WITH ASTM AND A.I.T.C. STANDARDS IN ACCORDANCE WITH SBC SECTION 2303.1.3. EACH MEMBER SHALL BEAR AN A.I.T.C. IDENTIFICATION MARK AND SHALL BE ACCOMPANIED BY AN A.I.T.C. CERTIFICATE OF CONFORMANCE. HORIZONTAL MEMBERS AND INCLINED MEMBERS OF LESS THAN 1:1 SLOPE SHALL HAVE A RADIUSED CAMBER OF 3,500 FT. UNLESS OTHERWISE NOTED.

SIMPLE SPAN BEAMS DOUGLAS FIR COMBINATION 24F-V4 $F_{b} = 2400 \text{ PSI}; F_{v} = 265 \text{ PSI}; E = 1,800,000 \text{ PSI}$ CONTINUOUS OR DOUGLAS FIR COMBINATION 24F-V8 CANTILEVERED BEAMS $F_{b} = 2400 \text{ PSI}; F_{v} = 265 \text{ PSI}; E = 1,800,000 \text{ PSI}$

THESE MEMBERS ARE NOTED AS '*' IN PLAN

GLUED LAMINATED MEMBERS EXPOSED TO WEATHER OR MOISTURE SHALL BE

TRFATED WITH A NON-CORROSIVE. APPROVED PRESERVATIVE.

Minimum Connectors and Fasteners for

WING INSTALLED IN	23	PREFARRICATED CONNECTOR PLATE WOOD TRUSS	SES SHALL BE DESIGNED) BY THE MANUEACTURER IN		DESCRIPTION OF BUILDING ELEMENT	NUMBER AND TYPE OF FASTEN
STRUCTIONS ACTURED BY ITW TI, INC. (ICC-ES IN STRONG-TIE ED BY D EMBEDMENT. N AT LEAST	20.	ACCORDANCE WITH ANSI/TPI I-2007 AND IBC S ON THE DRAWINGS. DESIGN LOADS SHALL BE AS <u>ROOF TRUSSES</u> TOP CHORD LIVE LOAD BOTTOM CHORD LIVE LOAD TOP CHORD DEAD LOAD BOTTOM CHORD DEAD LOAD WIND UPLIFT (TOP CHORD)	ECTION 2303.4 FOR THE S FOLLOWS: 25 PSF, SNOW + 5 0 PSF 15 PSF 5 PSF SFF NOTE#2 COMP(5 PSF, RAIN ON SNOW SURCHARGE	1.	BLOCKING BETWEEN CEILING JOISTS, RAFTERS, OR TRUSSES TO TOP PLATE OR OTHER FRAMING BELOW BLOCKING BETWEEN RAFTERS OR TRUSS NOT AT THE WALL TOP	ROOF $3-8d$ COMMON ($2\frac{1}{2}$ " x 0.131"); or 3-10d BOX (3 " x 0.128"); or 3-3" x 0.131" NAILS; or $3-3$ " x 14 GAGE STAPLES, $\frac{7}{6}$ " Cl $2-8d$ COMMON ($2\frac{1}{2}$ " x 0.131")
WITH ACI 318-14 2,500 PSI. THE		FLOOR TRUSSES TOP CHORD LIVE LOAD BOTTOM CHORD LIVE LOAD TOP CHORD DEAD LOAD	40 PSF 0 PSF 20 PSF			PLATE, TO RAFTER OR TRUSS	2-3 x 0.131 NAILS 2-3" x 14 GAGE STAPLES 2-16d COMMON (3½" x 0.162") 3-3" x 0.131" NAILS 3-3" x 14 GAGE STAPLES
TIE R402.2 IN THE TION 1904.2, R WEATHERING G.		THE TRUSS MANUFACTURER SHALL COORDINATE PLUMBING, MECHANICAL UNITS, DUCTS, AND/OR	LOCATIONS AND SUPPO	RT CONFIGURATIONS OF ITEMS WITH THE CONTRACTOR		FLAT BLOCKING TO TRUSS AND WEB FILLER	16d COMMON (3½" x 0.162") @ 6' 3" x 0.131" NAILS @ 6" oc 3" x 14 GAGE STAPLES @ 6" oc
RADE 60 FORCEMENT RTS INDICATING WELDED WIRE		PRIOR TO TRUSS FABRICATION. THE TRUSS MAN LOADS ASSOCIATED WITH SUCH ITEMS. THE TRUS AND APPROVED HANGER CONNECTION DETAILS T SYSTEM COMPONENTS AS APPLICABLE.	UFACTURER SHALL DESH SS SHOP DRAWINGS SHA TO TRUSS CHORDS FOR	GN TRUSSES TO SUPPORT ALL ALL INCLUDE ALL DESIGN LOADS SUPPORT OF HUNG MECHANICAL	2.	CEILING JOISTS TO TOP PLATE	3-8d COMMON (2½" x 0.131"); or 3-10d BOX (3" x 0.128"); or 3-3" x 0.131" NAILS; or 3-3" x 14 GAGE STAPLES, 7/6" CI
ICE WITH ACI REINFORCEMENT IT ALL WALL AND		WOOD TRUSSES SHALL UTILIZE APPROVED CONN AND CALCULATIONS SHALL BE PROVIDED AS A I STRUCTURAL ENGINEER OF RECORD PER GENERA INDICATE SHAPES, BEARING POINTS, INTERSECTIO SPECIAL HIP, VALLEY, AND INTERSECTION AREAS	ECTOR PLATES (GANGNA DEFERRED SUBMITTAL TO AL STRUCTURAL NOTE 13 DNS, HIPS, VALLEYS, ETO G (USE OF GIRDER TRUS	AIL OR EQUAL). SHOP DRAWINGS D THE CONTRACTOR AND 3. SHOP DRAWINGS SHALL C. EXACT COMPOSITION OF SES, JACK TRUSSES, STEP-DOWN	3.	CEILING JOIST NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITION (NO THRUST) (SEE 2308.7.3.1, TABLE 2308.7.3.1)	3–16d COMMON (3½" x 0.162"); c 4–10d BOX (3" x 0.128"); or 4–3" x 0.131" NAILS; or 4–3" x 14 GAGE STAPLES, ½6" CI
12" AT SIDES BENT UNLESS		TRUSSES, ETC.) SHALL BE DETERMINED BY THE DRAWINGS. THE TRUSS MANUFACTURER SHALL P DETAILS AND REQUIRED CONNECTION MATERIALS	MANUFACTURER UNLESS PROVIDE ALL TRUSS-TO- . THE TRUSS MANUFACT	S OTHERWISE NOTED ON THE -TRUSS BEAM/JOIST CONNECTION URER SHALL DESIGN AND PROVIDE	4.	CEILING JOIST ATTACHED TO PARALLEL RAFTER (HEEL JOINT)	PER TABLE 2308.7.3.1
. 3"	24.	ROOF & WALL SHEATHING SHALL BE APA RATED STRAND BOARD (OSB) IN CONFORMANCE WITH S), EXTERIOR OR EXPOSU BC SECTION 2303.1.5. S	RIDGING. RE 1 PLYWOOD OR ORIENTED SHEATHING SHALL BE	0.	GOLLAR HE TO RAFIER	4-10d BOX (3" x 0.128"); or 4-3" x 0.131" NAILS; or $4-3$ " x 14 GAGE STAPLES, $7/_6$ " Cl
. 1½" IVALENT, AND LACE IN STRICT	05	DRAWINGS FOR THICKNESS, SPAN RATING, AND I	ND POLICIES FOR STRUC NAILING REQUIREMENTS.	IDARDS DOC PS 1-09, PS 2-10, ITURAL USE PANELS. SEE	6.	RAFTER OR ROOF TRUSS TO TOP PLATE (SEE 2308.7.5, TABLE 2308.7.5)	3–10d COMMON (3" x 0.148"); or 3–16d BOX (3½" x 0.135"); or 4–10d BOX (3" x 0.128"); or
ING SURFACES.	25.	AT NON-SHEAR WALL EXTERIOR WALLS, UNLESS (NOMINAL) WITH SPAN RATING OF 24 ; WITH 8d PANEL EDGES); AND 8d @ 12" oc TO INTERN	©IHERWISE NOIED, WAI @ 6" oc PANEL NAILIN IEDIATE FRAMING.	IL SHEATHING SHALL BE ½ IG (APPLIES TO ALL SHEATHING	7.	ROOF RAFTERS TO RIDGE VALLEY	4-3" x 0.131" NAILS; or $4-3$ " x 14 GAGE STAPLES, $\frac{7}{16}$ " CI 2-16d COMMON (3%" x 0.162"):
IBC REF.	26.	ALL PRESSURE-TREATED (P.T.) WOOD MEMBERS GROUND AND CONTINUOUSLY PROTECTED FROM PRESSURE-TREATED WITH DOT SODIUM BORATE EXPOSED TO WEATHER OR IN CONTACT WITH TH PRESSURE-TREATED WITH ALKALINE COPPER QU OTHERWISE NOTED. AMMONIACAL COPPER ZINC A PRESERVATIVES WITH AMMONIA CARRIERS, SHALI GLUED LAMINATED MEMBERS EXPOSED TO WEATH NON-CORROSIVE, APPROVED PRESERVATIVE. SEE NOTE #27 FOR MATERIAL REQUIREMENTS OF PRESSURE-TREATED MEMBERS	SPECIFIED ON THE DRA MOISTURE (INTERIOR LOO (SBX) WITHOUT NoSIO ₂ . E GROUND, WOOD MEMB AT (ACQ-C FOR DOUGL ARSENATE (ACZA) PRESI _ NOT BE USED. HER OR MOISTURE SHAL F CONNECTORS AND FAS	WINGS THAT OCCUR ABOVE CATIONS) SHALL BE AT LOCATIONS PERMANENTLY ERS SHALL BE AS-FIR) PRESERVATIVE UNLESS ERVATIVE OR OTHER L BE TREATED WITH A STENERS IN CONTACT WITH		OR HIP RAFTERS; OR ROOF RAFTER TO 2" RIDGE BEAM	$3-10d \text{ BOX } (3" \times 0.128"); \text{ or}$ $3-3" \times 0.131 \text{ NAILS; or}$ $3-3" \times 14 \text{ GAGE STAPES, \frac{7}{16}" \text{ CR}3-10d \text{ COMMON } (3\frac{1}{2}" \times 0.148"); \text{ or}3-10d \text{ BOX } (3\frac{1}{2}" \times 0.135"); \text{ or}4-10d \text{ BOX } (3" \times 0.128"); \text{ or}4-3" \times 0.131 \text{ NAILS; or}4-3" \times 14 \text{ GAGE STAPES, \frac{7}{16}" \text{ CR}}WALL$
	27.	TIMBER CONNECTORS CALLED OUT BY LETTERS , COMPANY, AS SPECIFIED N THEIR WOOD CONSTR	AND NUMBERS SHALL BI RUCTION CONNECTORS C.	E "STRONG-TIE" BY SIMPSON ATALOG NO. C-C-2017-18.	8.	STUD TO STUD (NOT AT SHEARWALL CHORDS)	16d COMMON $(3\frac{1}{2}^{"} \times 0.162")"$
+ , 1904.1, 1904.2, 1908.2, 1908.3		INSTALL NUMBER AND SIZE OF FASTENERS AS S INSTALLED IN ACCORDANCE WITH THE MANUFAC CONNECT TWO MEMBERS, CENTER STRAP ON JOI SPECIFIED BY MANUFACTURER, WITH EQUAL NUM BOLTS IN WOOD MEMBERS SHALL CONFORM TO NUTS OF ALL BOLTS AND LAG SCREWS BEARING AND THE SAME GRADE (MINIMUM) AS MEMBERS	SPECIFIED BY MANUFACT TURER'S INSTRUCTIONS. INT AND INSTALL NUMBE IBER AND SIZE OF FAST ASTM A307. INSTALL WA ON WOOD. ALL SHIMS CONNECTED.	URER. CONNECTORS SHALL BE WHERE CONNECTOR STRAPS ER AND SIZE OF FASTENERS AS ENERS IN EACH MEMBER. ALL ASHERS UNDER THE HEADS AND SHALL BE SEASONED AND DRIED	9.	STUD TO STUD AND ABUTTING STUDS AT INTERSECTION WALL CORNERS	3" x 0.131" NAILS; or $3-3$ " x 14 GAGE STAPLES, 7_{6} " CI 16d COMMON ($3\frac{1}{2}$ " x 0.162")"; or 16d BOX ($3\frac{1}{2}$ " x 0.135")"; or
1908.10 1908.6, 1908.7, 1908.8 1908.9		ALL TIMBER CONNECTORS IN CONTACT WITH PRE CHEMICALS OTHER THAN DOT SODIUM BORATE (Z_{MAX} STEEL BY SIMPSON (G185 STEEL PER ASTM ALTERNATIVELY, CONNECTORS CAN BE POST HO GALVANIZED PER ASTM B695, CLASS 55 OR GRI WITH STAINLESS STEEL CONNECTORS, AND HOT	SSURE-TREATED WOOD SBX) WITHOUT NoSIO ₂ S A A653), OR TYPE 304 T DIP GALVANIZED PER EATER. STAINLESS STEEI DIP GALVANIZED FASTEN	THAT USED PRESERVATIVE HALL BE MANUFACTURED FROM OR 316 STAINLESS STEEL. ASTM A123 OR MECHANICALLY L FASTENERS SHALL BE USED WERS PER ASTM A153 SHALL BE	10.	BUILT-UP HEADER (2" TO 2" HDR.)	3" x 0.131" NAILS; or 3–3" x 14 GAGE STAPLES, 7_6 " Cl 16d COMMON ($3\frac{1}{2}$ " x 0.162")"; or 16d BOX ($3\frac{1}{2}$ " x 0.135")
8	28.	USED WITH GALVANIZED CONNECTORS.	APPLY UNLESS OTHERWI	SE NOTED ON THE DRAWINGS:	11.	CONTINUOUS HEADER TO STUD	4-8d COMMON (2½" x 0.131"); or 4-10d BOX (3" x 0.128")
)		A. ALL WOOD TRAMING DETAILS STALL E SBC. MINIMUM NAILING SHALL CONFOR NER-272. COORDINATE THE SIZE AND ARCHITECTURAL DRAWINGS. INSTALL N AND LAG SCREWS BEARING ON WOOD 2012 NDS SECTION 11.1.4, AND INSTA SECTION 11.1.3	RM TO SBC TABLE 2304 D LOCATION OF ALL OPE WASHERS UNDER THE HE INSTALLATION OF LAG ALLATION OF BOLTS SHA	2.9.1 OR CURRENT ICC-ES REPORT ENINGS WITH MECHANICAL AND EADS AND NUTS OF ALL BOLTS SCREWS SHALL CONFORM TO LL CONFORM TO 2012 NDS	12.	TOP PLATE TO TOP PLATE	16d COMMON (3½" x 0.162"); or 10d BOX (3" x 0.128"); or 3" x 0.131" NAILS; or 3" x 14 GAGE STAPLES, ½6" CRON
NFORMANCE WITH ESTERN LUMBER		B. WALL FRAMING: TWO STUDS MINIMUM UNLESS NOTED OTHERWISE NOTED. IN FLOOR SPACES TO SUPPORTS BELOW ALL STUD WALLS SHALL HAVE THEIR	SHALL BE INSTALLED A ISTALL SOLID BLOCKING LOWER WOOD PLATES A	T THE ENDS OF ALL WALLS, FOR WOOD COLUMN THROUGH	13.	TOP PLATE TO TOP PLATE, AT END JOINTS	8–16d COMMON (3½" x 0.162"); a 12–10d BOX (3" x 0.128"); or 12–3" x 0.131" NAILS; or 12–3" x 14 GAGE STAPLES, 7/6" (
SI, E = 1400 KSI SI, F _t = 325 PSI		BELOW WITH TED NAILS @ 12 oc ST BOLTS @ 4'-O" oc PER SBC SECTION x 0.229" PLATE WASHERS SHALL BE INSTALLED PER AF&PA SDPWS-2008 STUD POSTS SHALL BE NAILED TO F	AGGERED OR BOLIED IC I 2308.6 (EMBED 7"), U USED WITH ALL SILL PL SECTION 4.3.6.4.3. INDI ACH OTHER WITH 16d @	NLESS OTHERWISE NOTED. 3" x 3" ATE ANCHOR BOLTS AND VIDUAL MEMBERS OF BUILT-UP	14.	BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING NOT AT SHEARWALL	16d COMMON (3½" x 0.162")"; or 16d BOX (3½" x 0.135")"; or 3" x 0.131" NAILS; or 3" x 14 GAGE STAPLES. ½6" CRON
SI, E = 1600 KSI SI, F _t = 575 PSI SI, E = 1700 KSI SI, F _t = 675 PSI SI, F _t = 675 PSI SI = - 1600 KSI		C. FLOOR AND ROOF FRAMING: INSTALL JOISTS TO SUPPORTS WITH (2)16d N/ BEAMS WITH SIMPSON METAL JOIST H MULTI-JOIST BEAMS TOGETHER WITH	SOLID BLOCKING AT ALI AILS. ATTACH TIMBER JC IANGERS IN ACCORDANC 16d@12"oc STAGGERED.	L BEARING POINTS. TOENAIL DISTS TO FLUSH HEADERS OR E WITH NOTES ABOVE. NAIL ALL	15.	BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST, OR BLOCKING AT SHEARWALL	2–16d COMMON $(3\frac{1}{2}^{"} \times 0.162")$; or 3–16d BOX $(3\frac{1}{2}^{"} \times 0.135")$; or 4–3" x 0.131" NAILS; or 4–3" x 14 GAGE STAPLES, $\frac{7}{16}$ " Cl
SI, $F_t = 675$ PSI SI, $F_t = 675$ PSI SI, $F_t = 675$ PSI SI, $F_t = 675$ PSI SI, $F_t = 1600$ KSI PSI, $F_t = 825$ PSI EQUAL. REQUESTS SR-1387 FOR		ROOF AND FLOOR SHEATHING SHALL AND NAILED AS SHOWN ON THE DRA BETWEEN JOISTS/TRUSSES AT UNBLO EDGES SHALL HAVE APPROVED TONG SOLID BLOCKING ALLOW '&" SPACING SHEATHING. TOENAIL BLOCKING TO SU SECTION 1604.8.3, DECKS SHALL BE OTHER THAN NAILS SUBJECT TO WITH EACH END ATTACHED TO DECK JOIST	BE LAID UP WITH GRAIN WINGS. INSTALL APPROV CKED ROOF SHEATHING UE-AND-GROOVE JOINT AT ALL PANEL EDGES A JPPORTS WITH 16d@12"c POSITIVELY ANCHORED IDRAWAL. ANCHOR WITH S AND TO A SOLID BLO	N PERPENDICULAR TO SUPPORTS ED PANEL EDGE CLIPS CENTERED EDGES. ALL FLOOR SHEATHING S OR SHALL BE SUPPORTED WITH AND ENDS OF LOOR AND ROOF DOC. IN ACCORDANCE WITH SBC TO THE STRUCTURE BY MEANS MINIMUM (1) CS16 STRAP AT CKING MEMBER WITHIN THE	16.	STUD TO TOP OR BOTTOM PLATE	4-8d COMMON $(2\frac{1}{2}$ " x 0.131"); or 4-10d BOX $(3" \times 0.128")$; or 4-3" x 0.131" NAILS; or 4-3" x 14 GAGE STAPLES, $\frac{7}{16}$ " CH 2-16d COMMON $(3\frac{1}{2}" \times 0.162")$; or 3-10d BOX $(3" \times 0.128")$; or 3-3" x 0.131" NAILS; or 3-3" x 14 GAGE STAPLES, $\frac{7}{16}$ " CH
TRAND LUMBER		duilding. D. <u>Nailing</u> : A Minimum nail diameter ,	AND LENGTH SHALL BE SIZE ON DRAWINGS	AS FOLLOWS:	17.	TOP OR BOTTOM PLATE TO STUD	2−16d COMMON (3½" x 0.162"); c 3−10d BOX (3" x 0.128"); or 3−3" x 0.131" NAII S: or
A.I.T.C. AN A.I.T.C. NFORMANCE.		SHEATHING NAILS	8d 10d 10d	$\begin{array}{r} 0.131'' \times 2\frac{1}{4}'' \\ 0.148'' \times 2\frac{1}{2}'' \\ 0.148'' \times 3'' \end{array}$			3-3" x 14 GAGE STAPLES, 7/6" CH
a radiused			16d	0.148" x 3¼"			

	NUMBER AND TYPE OF FASTENERS	SPACING & LOCATION	DESCRIPTION OF BLDG. ELEMEN
	3-8d COMMON (2½" x 0.131"); or 3-10d BOX (3" x 0.128"); or 3-3" x 0.131" NAILS; or 3-3" x 14 GAGE STAPLES, 7/6" CROWN	EACH END, TOENAIL	18. TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS
	2–8d COMMON (2½" x 0.131") 2–3" x 0.131" NAILS 2–3" x 14 GAGE STAPLES	EACH END, TOENAIL	19. 1" BRACE TO EACH STUD AND PLATE
	2−16d COMMON (3½" x 0.162") 3−3" x 0.131" NAILS 3−3" x 14 GAGE STAPLES	END NAIL	20. 1" x 6" SHEATHING TO
	16d COMMON (3½" x 0.162") @ 6" oc 3" x 0.131" NAILS @ 6" oc 3" x 14 GAGE STAPLES @ 6" oc	FACE NAIL	EACH BEARING
	3-8d COMMON ($2\frac{1}{2}$ " x 0.131"); or 3-10d BOX (3" x 0.128"); or 3-3" x 0.131" NAILS; or 3 3" x 14 CACE STADLES 7(" CROWN	EACH JOIST, TOENAIL	22. JOIST TO SILL. TOP
	$3-16d$ COMMON ($3\frac{1}{2}$ " x 0.162"); or 4-10d BOX (3 " x 0.128"); or 4-3" x 0.131" NAILS: or	FACE NAIL	PLATE, OR GIRDER
	$4-3" \times 14$ GAGE STAPLES, $\frac{7}{6}"$ CROWN PER TABLE 2308.7.3.1	FACE NAIL	23. RIM JOIST, BAND JOIST, OR BLOCKING TO TOP PLATE, SILL, OR OTHER FRAMING BELOW
	3–10d COMMON (3" x 0.148"); or 4–10d BOX (3" x 0.128"); or 4–3" x 0.131" NAILS; or	FACE NAIL	24. 1" x 6" SUBFLOOR OR LESS TO EACH JOIST
	4-3" x 14 GAGE STAPLES, 16" CROWN 3-10d COMMON (3" x 0.148"); or	TOENAIL	25. 2" SUBFLOOR TO JOIST OR GIRDER
	3-16d BOX (3½" x 0.135"); or 4-10d BOX (3" x 0.128"); or 4-3" x 0.131" NAILS; or		26. 2" PLANKS (PLANK & BEAM – FLOOR & ROOF)
	4-3" x 14 GAGE STAPLES, $\frac{7}{6}$ " CROWN 2-16d COMMON ($\frac{3}{2}$ " x 0.162"); or 3-10d BOX (3 " x 0.128"); or 3-3" x 0.131 NAILS; or	END NAIL	27. BUILT-UP GIRDERS AND BEAMS, 2"LUMBER LAYERS
	$3-3'' \times 14$ GAGE STAPES, $\%_{16}''$ CROWN $3-10d$ COMMON ($3\%_2'' \times 0.148''$); or $3-16d$ BOX ($3\%_2'' \times 0.135''$); or $4-10d$ BOX ($3'' \times 0.128''$); or $4-3'' \times 0.131$ NAILS; or $4-3'' \times 14$ GAGE STAPES, $\%_{16}''$ CROWN	TOENAIL	
		24" oc FACE NAIL	
	10d BOX (3" x 0.128"); or 3" x 0.131" NAILS; or 3-3" x 14 GAGE STAPLES, 7/6" CROWN	16" oc FACE NAIL	28. LEDGER STRIP SUPPORTING JOISTS OR RAFTERS
S	16d COMMON (3½" x 0.162")"; or 16d BOX (3½" x 0.135")"; or 3" x 0.131" NAILS; or 3–3" x 14 GAGE STAPLES, 7/6" CROWN	16" oc FACE NAIL 12" oc FACE NAIL 12" oc FACE NAIL	29. JOIST TO BAND JOIST OR RIM JOIST
	16d COMMON (3½" x 0.162")"; or 16d BOX (3½" x 0.135")	16" oc EA. EDGE, FACE NAIL 12" oc EA. EDGE, FACE NAU	30. BRIDGING OR BLOCKING TO JOIST, RAFTER, OR TRUSS
	4-8d COMMON (2½" x 0.131"); or 4-10d BOX (3" x 0.128")	TOENAIL	
	16d COMMON (3 $\frac{1}{2}$ " x 0.162"); or	16" oc FACE NAIL	
	10d BOX (3" x 0.128"); or 3" x 0.131" NAILS; or 3" x 14 GAGE STAPLES, 7/6" CROWN	12" oc FACE NAIL	
	8–16d COMMON (3½" x 0.162"); or 12–10d BOX (3" x 0.128"); or 12–3" x 0.131" NAILS; or 12–3" x 14 GAGE STAPLES, 7⁄16" CROWN	EACH SIDE OF END JOINT, FACE NAIL (MINIMUM 24" LAP SPLICE LENGTH EA. SIDE OF END JOINT	
	16d COMMON (3½" x 0.162")"; or	16" oc FACE NAIL	
	16d BOX (3½" x 0.135")"; or 3" x 0.131" NAILS; or 3" x 14 GAGE STAPLES, ½6" CROWN	12" oc FACE NAIL	
	2–16d COMMON (3½" x 0.162"); or 3–16d BOX (3½" x 0.135"); or 4–3" x 0.131" NAILS; or 4–3" x 14 GAGE STAPLES, 7⁄6" CROWN	16" oc FACE NAIL	MULTIPLE LVL PIECE NUMBER WIDTH OF PLIES T
	4-8d COMMON $(2\frac{1}{2}$ " x 0.131"); or 4-10d BOX (3" x 0.128"); or 4-3" x 0.131" NAILS; or 4-3" x 14 GAGE STAPLES, $\frac{7}{16}$ " CROWN 2-16d COMMON ($3\frac{1}{2}$ " x 0.162"); or 3-10d BOX (3" x 0.128"); or 3-3" x 0.131" NAILS; or 3-3" x 14 GAGE STAPLES $\frac{7}{16}$ " CROWN	TOENAIL END NAIL	$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$
	2-16d COMMON ($3\frac{1}{2}$ " x 0.162"); or 3-10d BOX (3 " x 0.128"); or 3- 3 " x 0.131" NAILS; or 3- 3 " x 14 GAGE STAPLES, $\frac{7}{16}$ " CROWN	END NAIL	4 100 4 Sc

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PTION OF BLDG. ELEMENT	NUMBER AND TYPE OF FASTENERS	SPACING & LOCATION
OP PLATES, LAPS AT CORNERS AND NTERSECTIONS	WALL (CONTINUED) 2–16d COMMON (3½" x 0.162"); or 3–10d BOX (3" x 0.128"); or 3–3" x 0.131" NAILS; or 3–3" x 14 GAGE STAPLES, ½6" CROWN	FACE NAIL
" BRACE TO EACH STUD ND PLATE	2-8d COMMON (2½" x 0.131"); or 2-10d BOX (3" x 0.128"); or 2-3" x 0.131" NAILS; or 2-3" x 14 GAGE STAPLES, 7/6" CROWN	FACE NAIL
" x 6" SHEATHING TO TACH BEARING	2-8d COMMON (2½" x 0.131"); or 2-10d BOX (3" x 0.128"); or	FACE NAIL
" x 8" AND WIDER GHEATHING TO EACH BEARING	3-8d COMMON (2½" x 0.131"); or 3-10d BOX (3" x 0.128"); or	FACE NAIL
	FLOOR	
OIST TO SILL, TOP PLATE, OR GIRDER	3–8d COMMON (2½" x 0.131"); or 3–10d BOX (3" x 0.128"); or 3–3" x 0.131" NAILS; or 3–3" x 14 GAGE STAPLES, ½6" CROWN	TOENAIL
RIM JOIST, BAND JOIST, OR BLOCKING TO TOP PLATE, SILL, OR OTHER RAMING BELOW	8d COMMON (2½" x 0.131"); or 10d BOX (3" x 0.128"); or 3" x .131" NAILS; r 3" x 14 GAGE STAPLES, 7/6" CROWN	6" o.c., TOENAIL
" x 6" SUBFLOOR OR ESS TO EACH JOIST	2-8d COMMON (2½" x 0.131"); or 2-10d BOX (3" x 0.128")	FACE NAIL
" SUBFLOOR TO OIST OR GIRDER	2−16d COMMON (3½" x 0.162")	FACE NAIL
2" PLANKS (PLANK & BEAM – FLOOR & ROOF)	2-16d COMMON (3½" x 0.162")"	EA. BEARING, FACE NAIL
BUILT-UP GIRDERS AND BEAMS, 2"LUMBER AYERS	20d COMMON (4" x 0.192")	32" o.c., FACE NAIL TOP & BO STAGGERED ON OPPOSITE SIDES
	10d BOX (3" x 0.128"); or 3" x 0.131" NAILS; or 3" x 14 GAGE STAPLES, 7/6" CROWN	24" o.c., FACE NAIL AT TOP & BOT. STAGGEREE ON OPP. SIDES
	AND: 2-20d COMMON (4" x 0.192"); or 3-10d BOX (3" x 0.128"); or 3-3" x 0.131" NAILS; or 3-3" x 14 GAGE STAPLES, 7/6" CROWN	ENDS AND AT EACH SPLICE, FACE NAIL
EDGER STRIP SUPPORTING JOISTS DR RAFTERS	3–16d COMMON (3½" x 0.162"); or 4–10d BOX (3" x 0.128"); or 4–3" x 0.131" NAILS; or 4–3" x 14 GAGE STAPLES, 7/6" CROWN	EACH JOIST OR RAFTER, FACE NAIL
IOIST TO BAND JOIST OR RIM JOIST	$3-16d$ COMMON ($3\frac{1}{2}$ " x 0.162"); or 4-10d BOX (3 " x 0.128"); or 4-3" x 0.131" NAILS; or $4-3$ " x 14 GAGE STAPLES, $\frac{7}{16}$ " CROWN	end nail
BRIDGING OR BLOCKING O JOIST, RAFTER, OR RUSS	2-8d COMMON (2½" x 0.131"); or 2-10d BOX (3" x 0.128"); or 2-3" x 0.131" NAILS; or 2-3" x 14 GAGE STAPLES, ½6" CROWN	EACH END, TOENAIL

	MULTIPLE	. LVL MEMBER	FASIEN	ING PE	R WEYERHAU	JSER
				F	ASTENER	
PIECE WIDTH	NUMBER OF PLIES	TYPE ⁽¹⁾	MIN. LENGTH	# ROWS	O.C. SPACING	LOCATION
		10d NAILS	3"	3(2)	10"	
	2	12d - 16d NAILS	31⁄4"	2 ⁽²⁾	12	ONE SIDE
		SCREWS	$3\frac{3}{8}$ " or $3\frac{1}{2}$ '	2	24"	
		10d NAILS	3"	ح(2)	10"	
	7	12d - 16d NAILS	31⁄4"	2 ⁽²⁾	ΙZ	BOTH SIDES
13⁄4"		SCREWS	$3\frac{3}{8}$ " or $3\frac{1}{2}$ '		24"	BOTH SIDES
	4		5"	2		ONE SIDE
		10d NAILS	3"	3(2)	10"	ONE SIDE
		12d - 16d NAILS	31⁄4"	2 ⁽²⁾	12	(PER PLY)
		SODEWS	5" or 6"	2	24"	BOTH SIDES
		JUNE WS	6¾"	2	ΣT	ONE SIDE
3½"		2 SCREWS 5" or 6" 2 24"	0	E E	BOTH SIDES	
	2		Ζ4	ONE SIDE		
		½"ø BOLTS	8"	2	24"	_
(1) 10	Dd NAILS ARE	0.128" DIAMETER; 1	2d - 16d N	AILS ARE	0.148" - 0.162" [DIAMETER;

(2) AN ADDITIONAL ROW OF NAILS IS REQUIRED WITH DEPTHS OF 14" OR GREATER (3) WHEN CONNECTING 4-PLY MEMBERS, NAIL EACH PLAY TO THE OTHER AND OFFSET NAIL ROWS BY 2" FROM ROWS IN THE PLY BELOW

SCREWS ARE SDS, USP WP, TrussLOK, OR SDW

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CONTENTS General Strucural Notes
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S1.1

<u>LEGEND</u>						
	CONCRETE FOOTING	F <u>X.X</u>	DENOTES SPREAD FOOTING PER 5/S3.1			
	CONCRETE WALL	×	POST ABOVE			
low phigh	STEP IN FOOTING PER 9/S3.1	SW	DENOTES EXTENT OF SHEARWALL TYPE SW DEP 1/S6.6			
(<u></u>	DENOTES TOP OF FOOTING ELEVATION	SW*	DENOTES STRAPPED SHEARWALL PER			
1===1	STRUCTURAL WOOD STUDWALL BELOW		7/S6.6, WITH ≌ DENOTING STRAP PER SCHEDULE ABOVE & BELOW OPENING			
	STRUCTURAL WOOD	HDU	DENOTES SHEARWALL TENSION TIE PER 4/S6.6 * – denotes transfer tie from tie above			
			^ – DENOTES TIE ATOP FRAMING MEMBER			
						1
					T.O. CONC. WAI EL. = 264.9	
				T. <u>P. CONC. WALL /</u> EL. = 266.17'		
						4" SLAB−ON−GRAE PER 3/S3.1 ↓T/slab eL=256'-0"
					 	4
						4" SLAB-ON-GRADE
						PER 3/S3.1
						T.O. CONC. WALL
						EL. = 266.17'
					T.O. CONC. WALL	
				 	EL. = 266.50 [°]	SEE 8/S6. TIES A
						SHE
					1 3.1	
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	CONNECTOR	TABLE
	SIMPSON DESIGNATION	NOTES
$\langle A \rangle$	ECCLQ, ECCRQ	L-POST CAP
$\langle B \rangle$	HUS ~or~ BU	HANGER
$\langle \hat{O} \rangle$	HGU ~or~ EGQ	HANGER
$\langle D \rangle$	CCT	T-POST CAP
Æ	IUS ~or~ ITS	HANGER
$\langle E \rangle$	CCQ	COLUMN CAP
G	HUCQ	CONCEALED FLANGE HANGER
$\langle H \rangle$	IUS ~or~ MIT	HANGER
$\langle \rangle$	LUS ~or~ HWPH	HANGER
$\langle \mathbb{J} \rangle$	HHUS	HANGER





	CONNECTOR	TABLE
	SIMPSON DESIGNATION	NOTES
$\langle A \rangle$	ECCLQ, ECCRQ	L-POST CAP
$\langle B \rangle$	HUS ~or~ BU	HANGER
$\langle 0 \rangle$	HGU ~or~ EGQ	HANGER
$\langle D \rangle$	CCT	T-POST CAP
Æ>	IUS ~or~ ITS	HANGER
$\langle E \rangle$	CCQ	COLUMN CAP
$\langle G \rangle$	HUCQ	CONCEALED FLANGE HANGER
$\langle H \rangle$	IUS ~or~ MIT	HANGER
$\langle \rangle$	LUS ~or~ HWPH	HANGER
$\langle J \rangle$	HHUS	HANGER

SEE PLAN NOTE 5, TYP. AT ALL HEADERS SUPPORTING FRAMING

ALL EXTERIOR EXPOSED WOOD MEMBERS SHALL BE PRESSURE TREATED (OR APPROVED ALT.), SEE GENERAL STRUCTURAL NOTES #26 & 27



DENOTES STRAP TYPE BY LENGTH, CENTERED ON ABUTTING ELEMENTS



ROOF FRAMING PLAN 1/4" = 1'-0"

S2.4





	CONNECTOR	TABLE
	SIMPSON DESIGNATION	NOTES
$\langle A \rangle$	ECCLQ, ECCRQ	L-POST CAP
$\langle B \rangle$	HUS ~or~ BU	HANGER
$\langle C \rangle$	HGU ~or~ EGQ	HANGER
$\langle D \rangle$	CCT	T-POST CAP
$\langle E \rangle$	IUS ~or~ ITS	HANGER
$\langle F \rangle$	CCQ	COLUMN CAP
$\langle G \rangle$	HUCQ	CONCEALED FLANGE HANGER
$\langle H \rangle$	IUS ~or~ MIT	HANGER
$\langle \rangle$	LUS ~or~ HWPH	HANGER
$\langle J \rangle$	HHUS	HANGER

– SEE PLAN NOTE 3, TYP. AT ALL TRUSSES

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TENSI	<u>ON TIE SCH</u>	IEDULE			TENSION TIE	e above
ie (†) Ark	Min. # of studs	CLEAR SPAN AND ^② TOTAL FASTENERS	ASD ⁽³⁾ CAPACITY	BUILT-UP STUD FACE NAILS or SCREWS ④	TIE ① MARK	Min. (of stud
TC28	(2)2x	18" - (12)0.148"ø x 3¼"	1,150#	10d @ 6"oc	HDU2^	(2)2x
FC40	(2)2x	18" - (28)0.148"ø x 3¼"	2,690#	10d @ 4" oc	HDU4^	(3)2x
rc52	(3)2x	18" - (44)0.148"ø x 3¼"	4,225#	(8)¼"øx4½" SDS	HDU8^	(4)2x
TC66	(3)2x	18" - (64)0.148"ø x 3¼"	5,850#	(12)¼"øx6" SDS	HDU11^	6x6
STC52	(4)2x	18" - (64)0.148"ø x 3¼"	7,750#	(14)¼"øx6" SDS	HDU14^	6x6
STC66	6x6	18" - (64)0.148"ø x 3¼"	9,800#	(12)¼"øx6" SDS		

	IENSIUN HE SUTE	DULE			
tie ① Mark	MIN. NUMBER® OF STUDS	ANCHOR (Ø x EMBEDMENT) ⁽³⁾ and No. OF HAIRPIN DOWELS	FASTENERS FROM TIE TO STUD	ASD CAPACITY	BUILT-UP STUD FACE NAILS or SCREWS ④
HDU2	(2)2x	5∕8"ø x 10" − (2)#4 HAIRPIN	(6)¼"ø x 2½" SDS SCREWS	3,075#	10d @ 4" oc
HDU4	(3)2x	5⁄8"ø x 10" − (2)#4 HAIRPIN	(10)¼"ø x 2½" SDS SCREWS	4,565#	(9)¼"øx4½" SDS
HDU5	(3)2x	7∕8"ø x 10" − (2)#4 HAIRPIN	(14)¼"ø x 2½" SDS SCREWS	5,645#	(10)¼"øx4½" SDS
HDU8	(4)2x	⅛"ø x 10" − (4)#4 HAIRPIN	(20)¼"ø x 2½" SDS SCREWS	7,870#	(15)¼"øx6" SDS
HDU11	6x6	1"ø x 10" – (4)#4 HAIRPIN	(30)¼"ø x 2½" SDS SCREWS	11,175#	N/A
HDU14	6x6	1"ø x 10" – (6)#4 HAIRPIN	(36)¼"ø x 2½" SDS SCREWS	14,445#	N/A

				CONN	I. OF BLKG. OR F	RAMING	8	9	
		2	STUD/BLKG. AT ABUTTING PANEL	ro F	'LATE TO SILL PL/	ATE	BOLT	S TO	ASD CAPACITY.
SHEARWALL	SHEATHING	0.148" x 2½"	EDGES & SILL	(⊈) ¼"ø x 3½"	(5) A 35 CLIPS	6 I TP4 PLATES	CO	NC.	PLF
PANEL TIPE	THICKNESS	PANEL NAILING	PLATE THICKNESS	SDS SCREWS	O NOO OEII O		%"ø	3∕4"ø	
SW-6	1/2"	6"ос	2x	15" oc	25"ос	24" oc	48"oc	48"oc	310
SW-4	1/2"	4" oc	Зx	10" oc	16" oc	16" oc	38" oc	48" oc	460
SW-3	1/2"	3" ос	Зx	8" oc	13" oc	12" oc	29" oc	40" oc	600
SW-2	1/2"	2" oc	Зx	6" ос	10" oc	9" oc	23" oc	31" oc	770
SW-44	1/2"	4" oc EA. SIDE	Зx	5" ос	8" oc	8" oc	19"oc	26"oc	920
SW-33	1/2"	3" oc EA. SIDE	Зx	4" oc	6" ос	6" oc	14" oc	20" oc	1200
SW-22	1/2"	2" oc EA. SIDE	Зx	3" ос	5" ос	4" oc	11" oc	15"oc	1540
1 SHEATHING	SHALL CONSI	IST OF ½" PLYWO	OD AND HAVE A M	INIMUM SPAN RAT	ING OF ²⁴ %. <u>At in</u>	ITERIOR SHEARWALL	<u>.s only</u> ,	¹⁵ ⁄32" OSE	B MAY BE USED
2 PANEL NAII INTERMEDIA	LING APPLIES ATE STUDS/BL	TO ALL SHEATHI OCKING WITH PAN	NG PANEL EDGES. NEL NAILS AT 12"oc	INSTALL BLOCKING	G AT ALL UNFRAM	IED PANEL EDGES.	ENSURE	SHEATHI	NG IS NAILED TO
(3) DOUBLE 2x WITH 8d F/	: MEMBERS MA ACE: @ 4" oc	AY BE SUBSTITUT FOR SW-6, @ 3	ED FOR 3x MEMBER "oc FOR SW-4, @	S AT WALLS WITH 2" oc FOR SW-3	ONLY ONE LAYEF , AND (2)@ 3" od	R OF SHEATHING. 2 c FOR SW—2 (116#,	x MEMBE /NAIL)	RS SHALL	. BE NAILED TOGE
(4) ROWS OF N OF MEMBER	NAILS AND SD RS SHALL BE	S SCREWS SHALL %" (400#/SCREV	.BE OFFSET AT LE <i>H</i> V)	AST ½" AND STAG	GERED. MINIMUM	EDGE DISTANCE FO	OR NAILS	AND SDS	S SCREWS INTO ED
5 A35 CLIPS	SHALL BE INS	STALLED w/ (12)	0.131 x 1½ " NAILS	(650#/CLIP)					
	DAL TE DIAT	C MAY DE INISTA	IIEN AVED SUEATL	$W = \frac{1}{2} \sqrt{(12)} = \frac{1}{21}$	V 21/2" NAUS (62				

- SHEATHING.
- THE "NO REINFORCING" HOLE SIZE PER 2/S6.1.

S6.6 / 1" = 1' - 0"

(6) LIP4 LATERAL THE PLATES MAY BE INSTALLED OVER SHEATHING W/ (12)0.131 X 2½ NAILS (623#/CLIP)

(7) CONTRACTOR SHALL USE A35 CLIPS TO CONNECT ROOF TRUSS TO DOUBLE TOP PLATE SDS SCREWS or LTP4 CLIPS TO CONNECT SOLE PLATE TO FLOOR TRUSS RIM BOARD

A35 or LT4P CLIPS TO CONNECT FLOOR TRUSS TIM BOARD TO DOUBLE TOP PLATE (8) PLATE WASHERS IN 2x4 STUD WALLS AND <u>ALL</u> SINGLE SIDED SHEAR WALLS SHALL BE 3"x3"x0.229". DOUBLE SIDED 2x6 SHEAR WALLS SHALL HAVE $4\frac{1}{2}$ "x3"0.229" PLATE WASHERS. THE EDGE OF PLATE WASHERS SHALL BE LOCATED WITHIN $\frac{1}{2}$ " OF THE EDGE OF BOTTOM PLATE ON THE SIDE WITH

(9) CAST ANCHORS A MINIMUM OF 7" INTO CONCRETE. INSTALL ADDITIONAL ANCHOR BOLTS AT EACH SIDE OF PLATE BREAKS AND PENETRATIONS EXCEEDING

SHEARWALL SECTION AND SCHEDULE

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- SEDIMENT TO SURFACE WATERS, DRAINAGE SYSTEMS, AND ADJACENT PROPERTIES IS MINIMIZED. 5. THE ESC FACILITIES SHOWN ON THIS PLAN ARE THE MINIMUM REQUIREMENTS FOR ANTICIPATED SITE CONDITIONS. DURING THE CONSTRUCTION PERIOD, THESE ESC FACILITIES SHALL BE UPGRADED AS NEEDED FOR UNEXPECTED STORM EVENTS AND MODIFIED TO ACCOUNT FOR
- CHANGING SITE CONDITIONS (E.G., ADDITIONAL SUMP PUMPS, RELOCATION OF DITCHES AND SILT FENCES, ETC.). 6. THE ESC FACILITIES SHALL BE INSPECTED DAILY BY THE APPLICANT/ESC SUPERVISOR AND
- MAINTAINED TO ENSURE CONTINUED PROPER FUNCTIONING. WRITTEN RECORDS SHALL BE KEPT OF WEEKLY REVIEWS OF THE ESC FACILITIES DURING THE WET SEASON (OCT. 1 TO APRIL 30) AND OF MONTHLY REVIEWS DURING THE DRY SEASON (MAY 1 TO SEPT. 30).
- ANY AREAS OF EXPOSED SOILS, INCLUDING ROADWAY EMBANKMENTS, THAT WILL NOT BE DISTURBED FOR TWO DAYS DURING THE WET SEASON OR SEVEN DAYS DURING THE DRY SEASON SHALL BE IMMEDIATELY STABILIZED WITH THE APPROVED ESC METHODS (E.G., SEEDING, MULCHING, PLASTIC COVERING, ETC.).
- 8. ANY AREA NEEDING ESC MEASURES NOT REQUIRING IMMEDIATE ATTENTION SHALL BE ADDRESSED WITHIN FIFTEEN (15) DAYS. 9. THE ESC FACILITIES ON INACTIVE SITES SHALL BE INSPECTED AND MAINTAINED A MINIMUM OF
- ONCE A MONTH OR WITHIN FORTY-EIGHT (48) HOURS FOLLOWING A STORM EVENT.
- 10. AT NO TIME SHALL MORE THAN ONE (1) FOOT OF SEDIMENT BE ALLOWED TO ACCUMULATE WITHIN A CATCH BASIN. ALL CATCH BASINS AND CONVEYANCE LINES SHALL BE CLEANED PRIOR TO PAVING. THE CLEANING OPERATION SHALL NOT FLUSH SEDIMENT-LADEN WATER INTO THE DOWNSTREAM SYSTEM.
- 11. STABILIZED CONSTRUCTION ENTRANCES AND ROADS SHALL BE INSTALLED AT THE BEGINNING OF CONSTRUCTION AND MAINTAINED FOR THE DURATION OF THE PROJECT. ADDITIONAL MEASURES, SUCH AS WASH PADS, MAY BE REQUIRED TO ENSURE THAT ALL PAVED AREAS ARE KEPT CLEAN FOR THE DURATION OF THE PROJECT.
- 12. ANY PERMANENT FLOW CONTROL FACILITY USED AS A TEMPORARY SETTLING BASIN SHALL BE MODIFIED WITH THE NECESSARY EROSION CONTROL MEASURES AND SHALL PROVIDE ADEQUATE STORAGE CAPACITY. IF THE FACILITY IS TO FUNCTION ULTIMATELY AS AN INFILTRATION SYSTEM, THE TEMPORARY FACILITY MUST BE GRADED SO THAT THE BOTTOM AND SIDES ARE AT LEAST THREE FEET ABOVE THE FINAL GRADE OF THE PERMANENT FACILITY.
- 13. WHERE STRAW MULCH FOR TEMPORARY EROSION CONTROL IS REQUIRED. IT SHALL BE APPLIED AT A MINIMUM THICKNESS OF 2 TO 3 INCHES.
- 14. PRIOR TO THE BEGINNING OF THE WET SEASON (OCT. 1), ALL DISTURBED AREAS SHALL BE REVIEWED TO IDENTIFY WHICH ONES CAN BE SEEDED IN PREPARATION FOR THE WINTER RAINS. DISTURBED AREAS SHALL BE SEEDED WITHIN ONE WEEK OF THE BEGINNING OF THE WET SEASON. A SKETCH MAP OF THOSE AREAS TO BE SEEDED AND THOSE AREAS TO REMAIN UNCOVERED SHALL BE SUBMITTED TO THE DDES INSPECTOR. THE DDES INSPECTOR CAN REQUIRE SEEDING OF ADDITIONAL AREAS IN ORDER TO PROTECT SURFACE WATERS, ADJACENT PROPERTIES, OR DRAINAGE FACILITIES.

- POTENTIAL DRIP AND SPILL LOCATIONS DURING FILLING AND UNLOADING OF CONTAINERS. ANY COLLECTED
- 7. STORE AND MAINTAIN ABSORBENT PADS OR APPROPRIATE SPILL CLEANUP MATERIALS NEAR THE CONTAINER STORAGE AREA, IN A LOCATION KNOWN TO ALL. ENSURE THAT EMPLOYEES ARE FAMILIAR WITH THE SITE'S SPILL PLAN AND/OR PROPER SPILL CLEANUP PROCEDURES.
- 8. CHECK CONTAINERS (AND ANY CONTAINMENT SUMPS) DAILY FOR LEAKS AND SPILLS. REPLACE CONTAINERS CONTAINERS MADE OF COMPATIBLE MATERIALS MUST BE USED INSTEAD OF METAL DRUMS. NEW OR SECONDARY CONTAINERS MUST BE LABELED WITH THE PRODUCT NAME AND HAZARDS.
- 9. PLACE DRIP PANS OR ABSORBENT MATERIALS BENEATH A CONTAINER THAT IS FOUND TO BE LEAKING. OR PROPERLY DISPOSED OF.

<u>FUELING</u>

- 1. LOCATE THE FUELING OPERATION TO ENSURE LEAKS OR SPILLS WILL NOT DISCHARGE, FLOW, OR BE WASHED INTO THE STORM DRAINAGE SYSTEM, SURFACE WATER, OR GROUNDWATER.
- 3. IF FUELING IS DONE DURING EVENING HOURS, LIGHTING MUST BE PROVIDED.
- THAT EMPLOYEES ARE FAMILIAR WITH PROPER SPILL CONTROL AND CLEANUP PROCEDURES.
- SOILED ABSORBENT MATERIALS MUST BE REUSED, RECYCLED, OR PROPERLY DISPOSED OF. CONCRETE SAW CUTTING, SLURRY, AND WASHWATER DISPOSAL 1. SLURRY FROM SAW CUTTING THE SIDEWALK SHALL BE VACUUMED SO THAT IT DOES NOT ENTER NEARBY STORM DRAINS.
- 2. CONCRETE TRUCK CHUTES, PUMPS, AND INTERNALS SHALL BE WASHED OUT ONLY INTO FORMED AREAS AWAITING INSTALLATION OF CONCRETE.
- 3. UNUSED CONCRETE REMAINING IN THE TRUCK AND PUMP SHALL BE RETURNED TO THE ORIGINATING BATCH PLANT FOR RECYCLING.
- WASHED OFF ONLY INTO FORMED INTO FORMED AREAS AWAITING INSTALLATION OF CONCRETE OR IMPERMEABLE ASPHALT.
- 5. EQUIPMENT THAT CANNOT BE EASILY MOVED, SUCH AS CONCRETE PAVERS, SHALL ONLY BE WASHED IN
- NATURAL OR CONSTRUCTED STORMWATER CONVEYANCES. 7. WHEN NO FORMED AREAS ARE AVAILABLE, WASHWATER AND LEFTOVER PRODUCT SHALL BE CONTAINED IN
- A LINED CONTAINER. CONTAINED CONCRETE SHALL BE DISPOSED OF IN A MANNER THAT DOES NOT
- VIOLATE GROUNDWATER OR SURFACE WATER QUALITY STANDARDS. 8. CONTAINERS SHALL BE CHECKED FOR HOLES IN THE LINER DAILY DURING CONCRETE POURS AND REPLACED THE SAME DAY.

LIQUIDS OR SOILED ABSORBENT MATERIALS MUST BE REUSED, RECYCLED, OR PROPERLY DISPOSED OF.

THAT ARE LEAKING, CORRODED, OR OTHERWISE DETERIORATING. IF THE LIQUID CHEMICALS ARE CORROSIVE,

REMOVE THE DAMAGED CONTAINER AS SOON AS POSSIBLE. MOP UP THE SPILLED LIQUID WITH ABSORBENT PADS OR RAGS. ANY COLLECTED LIQUIDS OR SOILED ABSORBENT MATERIALS MUST BE REUSED, RECYCLED,

2. USE DRIP PANS OR ABSORBENT PADS TO CAPTURE DRIPS OR SPILLS DURING FUELING OPERATIONS.

4. STORE AND MAINTAIN APPROPRIATE SPILL CLEANUP MATERIALS IN THE MOBILE FUELING VEHICLE. ENSURE 5. IMMEDIATELY MOP UP ANY SPILLED FUEL WITH ABSORBENT PADS OR RAGS. ANY COLLECTED LIQUIDS OR

4. HAND TOOLS INCLUDING, BUT NOT LIMITED, SCREEDS, SHOVELS, RAKES, FLOATS, AND TROWELS SHALL BE

AREAS THAT DO NOT DIRECTLY DRAIN TO NATURAL OR CONSTRUCTED STORMWATER CONVEYANCES. 6. WASHDOWN FROM AREAS SUCH AS CONCRETE AGGREGATE DRIVEWAY SHALL NOT DRAIN DIRECTLY TO

BASIS OF BEARINGS

BEARINGS AND COORDINATES USED FOR THIS SURVEY ARE BASED ON THE NORTH AMERICAN DATUM OF 1983 (NAD83) WASHINGTON NORTH ZONE AND WERE ESTABLISHED USING RTK GPS WITH SMARTNET REFERENCE NETWORK.

LEGAL DESCRIPTION

LOT 5, BLOCK 4 OF MADRONA CREST ADDITION ACCORDING TO THE PLAT THEREOF RECORDED IN VOLUME 42 OF PLATS, PAGES 12-14, RECORDS OF KING COUNTY WASHINGTON. SITUATE IN COUNTY OF KING. STATE OF WASHINGTON

VERTICAL DATUM

/ GALLÁGHER/ HILL JÓPEN JŚPACE

ELEVATIONS SHOWN ON THIS DRAWING ARE BASE ON THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88) AND WERE ESTABLISHED USING RTK GPS.

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- STOCKPILE THE DUFF LAYER AND TOPSOIL ON SITE IN A DESIGNATED. CONTROLLED AREA. NOT ADJACENT TO PUBLIC RESOURCES AND CRITICAL AREAS. TO BE REAPPLIED TO OTHER PORTIONS OF THE SITE WHERE FEASIBLE.
- B. SOIL QUALITY. ALL AREAS SUBJECT TO CLEARING AND GRADING THAT HAVE NOT BEEN COVERED BY IMPERVIOUS SURFACE, INCORPORATED INTO A DRAINAGE FACILITY OR ENGINEERED AS STRUCTURAL FILL OR SLOPE SHALL, AT PROJECT COMPLETION, DEMONSTRATE THE FOLLOWING: 1. A TOPSOIL LAYER WITH A MINIMUM ORGANIC MATTER CONTENT OF 10% DRY WEIGHT IN PLANTING BEDS, AND 5% ORGANIC MATTER CONTENT IN TURF AREAS, AND A PH FROM 6.0 TO 8.0 OR MATCHING THE PH OF THE UNDISTURBED SOIL. THE TOPSOIL LAYER SHALL HAVE A MINIMUM DEPTH OF EIGHT INCHES EXCEPT WHERE TREE ROOTS LIMIT THE DEPTH OF INCORPORATION OF AMENDMENTS NEEDED TO MEET THE CRITERIA. SUBSOILS BELOW THE TOPSOIL LAYER SHOULD BE SCARIFIED AT LEAST 4 INCHES WITH SOME INCORPORATION OF THE UPPER MATERIAL TO AVOID STRATIFIED LAYERS, WHERE FEASIBLE.
- MULCH PLANTING BEDS WITH 2 INCHES OF ORGANIC MATERIAL 3. USE COMPOST AND OTHER MATERIALS THAT MEET THESE ORGANIC CONTENT REQUIREMENTS:
- A. THE ORGANIC CONTENT FOR "PRE-APPROVED" AMENDMENT RATES CAN BE MET ONLY USING COMPOST MEETING THE DEFINITION OF "COMPOSTED MATERIALS" IN WAC 173-350-220, WITH THE EXCEPTION THAT THE COMPOST MAY HAVE UP TO 35% BIOSOLIDS OR MANURE. THE COMPOST MUST ALSO HAVE AN ORGANIC MATTER CONTENT OF 40% TO 65%, AND A CARBON TO NITROGEN RATIO BELOW 25:1. THE CARBON TO NITROGEN RATIO MAY BE AS HIGH AS 35:1 FOR PLANTINGS COMPOSED ENTIRELY OF PLANTS NATIVE TO THE PUGET SOUND LOWLANDS REGION.
- B. CALCULATED AMENDMENT RATES MAY BE MET THROUGH USE OF COMPOSTED MATERIAL MEETING (A.) ABOVE; OR OTHER ORGANIC MATERIALS AMENDED TO MEET THE CARBON TO NITROGEN RATIO REQUIREMENTS, AND NOT EXCEEDING THE CONTAMINANT LIMITS IDENTIFIED IN TABLE 220-B, TESTING PARAMETERS, IN WAC 173- 350-220.
- THE RESULTING SOIL SHOULD BE CONDUCIVE TO THE TYPE OF VEGETATION TO BE ESTABLISHED. C. IMPLEMENTATION OPTIONS: THE SOIL QUALITY DESIGN GUIDELINES LISTED ABOVE CAN BE MET BY USING ONE OF THE METHODS LISTED BELOW:
- LEAVE UNDISTURBED NATIVE VEGETATION AND SOIL AND PROTECT FROM COMPACTION DURING CONSTRUCTION.
- AMEND EXISTING SITE TOPSOIL OR SUBSOIL EITHER AT DEFAULT "PREAPPROVED" RATES. OR AT CUSTOM CALCULATED RATES BASED ON TESTS OF THE SOIL AND AMENDMENT. 3. STOCKPILE EXISTING TOPSOIL DURING GRADING AND REPLACE IT PRIOR TO PLANTING. STOCKPILED TOPSOIL MUST ALSO BE AMENDED IF NEEDED TO MEET THE ORGANIC MATTER OR DEPTH REQUIREMENTS, EITHER AT A DEFAULT "PRE-APPROVED" RATE OR AT A CUSTOM CALCULATED RATE.
- 4. IMPORT TOPSOIL MIX OF SUFFICIENT ORGANIC CONTENT AND DEPTH TO MEET THE REQUIREMENTS. MORE THAN ONE METHOD MAY BE USED ON DIFFERENT PORTIONS OF THE SAME SITE. SOIL THAT ALREADY MEETS THE DEPTH AND ORGANIC MATTER QUALITY STANDARDS, AND IS NOT COMPACTED, DOES NOT NEED TO BE AMENDED.

ADDITIONAL NOTES:

- ALL CONSTRUCTION MATERIALS AND PRACTICE SHALL CONFORM TO THE CITY OF MERCER ISLAND STANDARDS AND THE WASHINGTON STATE DEPARTMENT OF TRANSPORTATION STANDARDS. EXISTING UTILITIES AS SHOWN ARE FROM CITY RECORDS AND ARE APPROXIMATE. IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO IDENTIFY, LOCATE AND PROTECT ABOVE AND BELOW
- GRADE UTILITIES. CONTRACTOR SHALL NOTIFY THE ENGINEER PRIOR TO CONSTRUCTION IF A CONFLICT EXISTS BETWEEN EXISTING UTILITIES AND THE PROPOSED IMPROVEMENTS. THE CONTRACTOR IS RESPONSIBLE FOR EROSION AND SEDIMENTATION CONTROL AND SHALL MAINTAIN THE NECESSARY SAFEGUARDS AND MANAGE THE CONSTRUCTION SO AS TO PREVENT WATERBORNE SEDIMENTS FROM LEAVING THE SITE.
- 4. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ADEQUATE SAFEGUARDS, SAFETY DEVICES, PROTECTIVE EQUIPMENT, FLAGGERS, AND ANY OTHER NEEDED ACTIONS TO PROTECT THE LIFE, HEALTH, AND SAFETY OF THE PUBLIC, AND TO PROTECT PROPERTY IN CONNECTION WITH THE PERFORMANCE OF WORK COVERED BY THE CONTRACTOR.
- 5. ON-SITE PRIVATE STORM AND SEWER PIPE SHALL BE SOLVENT WELDED SCHEDULE 40 PVC OR PVC ASTM D3034 SDR35 UNLESS SHOWN OTHERWISE. PVC PIPE LAID AT A SLOPE IN EXCESS OF 20% SHALL BE SOLVENT WELDED SCHEDULE 40 PVC. STORM PIPE IN THE RIGHT-OF-WAY SHALL BE HIGH-DENSITY POLYETHYLENE DOUBLE-WALLED SMOOTH INTERIOR PIPE SUCH AS ADS N-12 OR EQUIVALENT.
- 6. FOOTING DRAINS SHALL BE INSTALLED AROUND THE BASE OF ALL FOUNDATION FOOTINGS THAT ENCLOSE A CRAWL SPACE, CELLAR, BASEMENT, GARAGE OR OTHER BUILDING SPACE. FOOTING DRAINS SHALL BE PERFORATED 4-INCH DIAMETER PVC CONFORMING TO D2729, PERFORATIONS DOWN. GRANULAR BACKFILL SHALL BE PLACED AROUND AND ABOVE THE DRAIN TO A DEPTH OF 2/3 OF THE WALL HEIGHT. FILTER FABRIC (MIRAFI 140N OR EQUIVALENT) SHALL BE PLACED BETWEEN THE GRANULAR BACKFILL AND NATIVE SOILS. THE THE FOOTING DRAIN INTO THE STORM LINE AT A LOCATION WHERE THE FOOTING DRAIN ELEVATION IS AT LEAST 12-INCHES ABOVE THE STORM LINE.
- EXISTING SIDE SEWER AND STORM DRAIN DEPTH AND LOCATION SHALL BE DETERMINED PRIOR TO ANY CONSTRUCTION, INCLUDING BUILDING CONSTRUCTION. REPORT CONFLICTS WITH PROPOSED CONSTRUCTION TO ENGINEER. NEW SIDE SEWER CONNECTION TO MAIN OR SEWER EJECTOR PUMP MAY BE NECESSARY FOR BASEMENT.
- 8. PROPOSED METER LOCATION, IF SHOWN, IS APPROXIMATE. CONTRACTOR TO COORDINATE EXACT LOCATION OF NEW SERVICE/METER/ SUPPLY LINE WITH CITY WATER DEPARTMENT DURING CONSTRUCTION. 9. EACH DOWNSPOUT SHALL CONNECT TO A RIGID NON-PERFORATED PIPE AT THE BUILDING PERIMETER. UNDER NO CIRCUMSTANCES SHALL DOWNSPOUTS CONNECT DIRECTLY TO THE
- PERFORATED FOOTING DRAIN.
- 10. USE SAND COLLARS FOR PVC PIPE CONNECTIONS TO MANHOLES.
- 11. VERTICAL BENDS ON THE STORM DRAINS MAY BE NECESSARY TO MAINTAIN MIN. 1.5' SOIL COVER OVER PIPE. MAX. PIPE BENDS TO BE 45'. 12. DOWNSPOUT LOCATIONS SHOWN ARE PRELIMINARY. REFER TO ARCHITECTURAL PLANS FOR FINAL DOWNSPOUT LOCATIONS.
- 13. AN UNDERSLAB DRAINAGE SYSTEM MAY BE NECESSARY DEPENDENT ON GEOTECHNICAL EVALUATION BY OTHERS.
- 14. WINDOW WELLS SHALL BE DESIGNED FOR PROPER DRAINAGE BY CONNECTING TO THE BUILDING'S FOUNDATION DRAINAGE SYSTEM REQUIRED PER SECTION R310.2.3.2 OF THE INTERNATIONAL RESIDENTIAL CODE. A DRAINAGE SYSTEM FOR WINDOW WELLS IS NOT REQUIRED WHERE THE FOUNDATION IS ON WELL-DRAINED SOIL OR SAND-GRAVEL MIXTURE SOILS IN ACCORDANCE WITH THE UNITED SOIL CLASSIFICATION SYSTEM, GROUP I SOILS, AS DETAILED IN TABLE R405.1 OF THE IRC

/ GALLAGHER/ HILL JÓPEN JŚPACE

TREE PROTECTION DURING CONSTRUCTION

- 1. 6-FT. HIGH TEMPORARY CHAIN LINK FENCE SHALL BE PLACED AT THE DRIPLINE OF THE TREE TO BE SAVED. FENCE SHALL COMPLETELY ENCIRCLE THE TREE(S). INSTALL FENCE POSTS USING PIER BLOCKS ONLY. AVOID DRIVING POSTS OR STAKES INTO MAJOR ROOTS.
- 2. FOR ROOTS OVER 1-IN DIA. THAT ARE DAMAGED DURING CONSTRUCTION, MAKE A CLEAN, STRAIGHT CUT TO REMOVE THE DAMAGED PORTION. ALL EXPOSED ROOTS SHALL BE TEMPORARILY COVERED WITH DAMP BURLAP TO PREVENT DRYING, AND SHALL BE COVERED WITH SOIL AS SOON AS POSSIBLE 3. WORK WITHIN PROTECTION FENCE SHALL BE DONE MANUALLY. NO STOCKPILING OF MATERIALS, VEHICULAR TRAFFIC, OR STORAGE OF EQUIPMENT OR MACHINERY SHALL BE ALLOWED WITHIN THE

MAINTENANCE STANDARDS

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 ADDRESS:	3632 90TH AVE	SE	PREPARED BY:	NICK BOSSOFF ENG	
	MERCER ISLAND		PHONE:	(425) 881-5904	
			DATE:		
PIPE DIA (INCH): <u>60"</u>	PIPE LENGTH	(FT): <u>46</u>	ORIFICE #1 DIA 0.5 INCH, ELEV 258.75	
 PIPE MATE	rial: <u>ADS N-12</u>			ORIFICE #2 DIA 1.6 INCH, ELEV 263.35	

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SCALE: NTS

-THE SYSTEM IS TO BE AN ALTERNATING DUPLEX SYSTEM. -LOCATE CONTROL PANEL AND ALARM ON EXTERIOR BUILDING WALL. USE HYDROMATIC PANEL OR APPROVED EQUIVALENT. -SYSTEM TO BE FULLY AUTOMATIC WITH MANUAL OVERRIDE. -ALARM TO BE AUDIO (BELL) AND VISUAL (LIGHT). -BOTH PUMPS TO OPERATE AT "LAG PUMP ON" FLOAT LEVEL. -SCH 80 PVC PIPE INSIDE MANHOLE. -FOLLOW MANUFACTURER'S INSTRUCTIONS FOR ALL

INSTALLATION. -PROVIDE ELECTRICAL SUPPLY TO PANEL AND LIFT STATION PER MANUFACTURER'S SPECIFICATIONS. POWER TO PANEL AND PUMP SHALL BE ON A DEDICATED CIRCUIT. -ELECTRICAL CONNECTIONS AND SERVICES WITHIN THE PUMP

WETWELL SHOULD BE WATERTIGHT. -THE PRIVATE PROPERTY OWNER(S) SHALL BE RESPONSIBLE FOR ANY AND ALL CLAIMS FOR INJURIES AND DAMAGE DUE TO THE OPERATION OR NON-OPERATION OF THE PUMP SYSTEM.

LEGAL DE LEGAL DE LOT 5, BLO THEREOF R KING COUN SITUATE IN	SHOWN ON THIS DRAWING ARE BASE ON THE NORTH ATUM OF 1988 (NAVD88) AND WERE ESTABLISHED US UR INTERVAL – THE EXPECTED VERTICAL ACCURACY IS ONTOUR INTERVAL OR ± 1.0' FOR THIS PROJECT. SCRIPTION ICK 4 OF MADRONA CREST ADDITION ACCORDING TO T ECORDED IN VOLUME 42 OF PLATS, PAGES 12–14, R TY WASHINGTON. COUNTY OF KING, STATE OF WASHINGTON.	AMERICAN ING RTK S EQUAL TO PROPERTY OWNER: ELIZA 3632 MERC TAX PARCEL NUMBER: 5021 PROJECT ADDRESS: 3632 MERC PARCEL AREA: 11,2 AS 5	G ENGINEERING, PLLC BOX 412 INSDALE, WA 98051 (206) 420-7130 ABETH TUBBS 2 90TH AVE SE CER ISLAND, WA 98040 2 90TH AVE SE CER ISLAND, WA 98040 CO S.F. (0.257 ACRES ±) SURVEYED
HORIZONT BEARINGS A NORTH AME WERE ESTAF	AL DATUM & BASIS OF BEARINGS ND COORDINATES USED FOR THIS SURVEY ARE BASED RICAN DATUM OF 1983 (NAD83) WASHINGTON NORTH BLISHED USING RTK GPS WITH SMARTNET REFERENCE	D ON THE ZONE AND NETWORK. REFERENCE SURVEYS P - MADRONA CREST ADDITION R1 - AF# 2006022790005 BLA1 - AF# 20180806900000	DN VOL 42, PGS 12-14
SSS 230	SSS 10" SSS 10" SSS 10" SSS 10"	5-54 .22 36.49 .6.22 <u>AC SEWER</u> SSSSSSSS	SSMH 45-37 RIM=189.08 IE(W)=180.24 IE(NE)=180.03 IE(SE)=180.39 SDMH RIM=191.60
LLACHER HILL EN SPACE	SYMBOL LEGEND Image: Section corner Image:	Image: Signal Constraints and the second	12" CMP IE(SW)=186.57
	 G — GAS LINE CABLE RISER ■CTV CABLE BOX 	CONIFER	

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PROJECT NO.: REVISION DATE REVISION NO.: SHEET

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