PROPERTY INFO

PROPERTY ADDRESS	30XX 69TH AVE SE, MERCER ISLAND WA 98040
JURISDICTION ISLAND	CITY OF MERCER
PARCEL NUMBER	5093301316
ZONING LOT AREA	R-8.4 8,403 SF
LEGAL DESCRIPTION	MAPLE GROVE PARK SUBDIV E SEATTLE PARCEL "B" MERCER ISLAND LLA #SUB17-005 REC #20171019900001 SD LLA BEING POR OF LOTS 1-3 & 8-9 OF BLK 9 OF SD ADD PLat Block: 9 Plat Lot: 1-3

PROJECT DIRECTORY

OWNER

HUI HOME LLC 2112 22nd Ave S Seattle, WA 98144 206.910.1989 wenhu338@gmail.com Applicant/Contact: Zhanpeng Ma 509.491.7299 jamesmark2001@gmail.com

ARCHITECT

ATLAS ARCHITECTS 5280 Highland Drive Bellevue, WA 98006 Contact: Geng Tan, RA, LEED AP, NCARB 206.488.3688 gengtan@gmail.com

STRUCTURAL ENGINEER

DHS Engineering 1601 5th Ave., Suite 1100 Seattle, WA 98101 Contact: Dihong Shao, S.E. 206.906.9669 dihong.shao@dhsengineers.com

CIVIL ENGINEER TANDEM ENGINEERING 8822 NE 178th St Bothell, WA 98001 Contact: Steve Wu 206.795.5674 tandemengineering@outlook.com

GEOTECHNICAL ENGINEER NELSON GEOTECHNICAL ASSOCIATES, INC 17311-135th Ave. NE Suite A-500 Woodinville, WA 98072 Khaled M. Shawish, PE Contact: Katelyn Brower 425.486.1669 katelynb@nelsongeotech.com

SURVEYOR TERRANE

10801 Main St, Suite 102 Bellevue, WA 98004 425.458.4488 support@terrane.net

ARBORIST Root Down Consultants, LLC C.N. Floreani, PLA 360.399.6041 cfloreani@gmail.com

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SH1.0 SH1.1 SH2.0 SH3.0 SH3.1 SH3.2 SH3.3 SH3.3 SH3.A SH4.0 SH5.0 SH5.1 SH5.2	COVER & SHORING NOTES COVER & SHORING NOTES SHORING PLAN NORTH ELEVATION EAST ELEVATION SOUTH ELEVATION NORTH ELEVATION (PERIMETER) PILE AND ANCHOR SCHEDULE CROSS-SECTIONS DETAILS DETAILS DETAILS

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- S5.4 FOUNDATION DETAIL & SCHEDULE
- S5.5 LATERAL DETAIL AND SCHEDULE

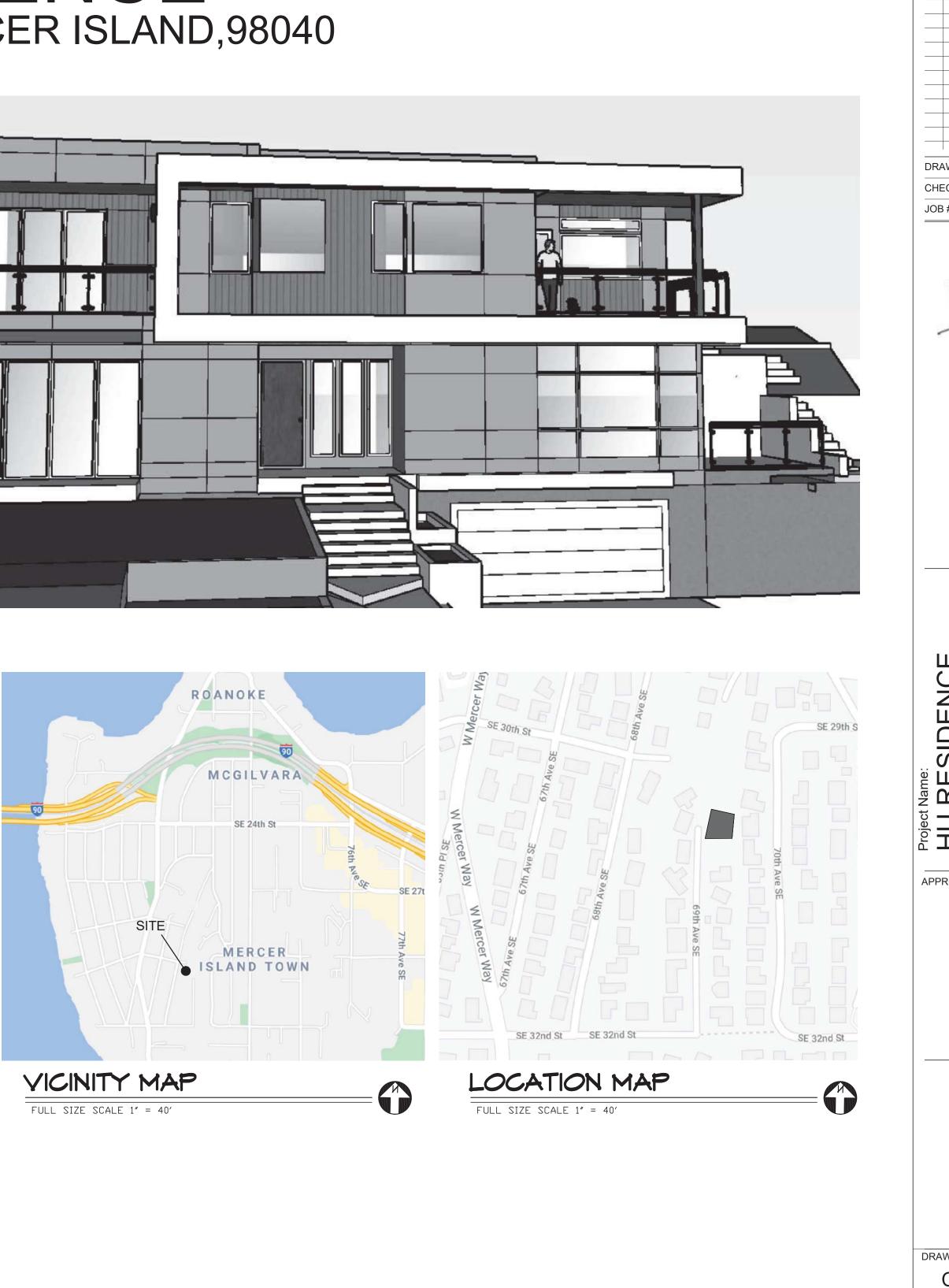
HU RESIDENCE 30XX 69TH AVE SE, MERCER ISLAND, 98040

ABBREVIATIONS AND SYMBOLS ABBREVIATIONS

	ABBREVIATIONS		
	AB	ANCHOR BOLTS	LONG
	ABV.	ABOVE	MFR
	ARCH	ARCHITECT	MSTR
	ASD	ALLOWABLE STRESS DESIGN	MK
	BLKG	BLOCKING	o/c, o.c.
	B & BOT	BOTTOM	OVHG
	BM	BEAM	PERM.
	СВ	CATCH BASIN	PERP.
	CONC	CONCRETE	P.L.
	CONT	CONTINUOUS	PL
	COV'D	COVERED	PW
	CRPT	CARPET	QTY
	CSMT	CASEMENT	REINF
	CL	CENTER LINE	RQMT
	DBL	DOUBLE	RBC
	DN	DOWN	RO
	EA	EACH	RW
	EG	EXISTING GRADE	R.Y.S.B.
	ELEV, EL.	ELEVATION	SG.
	EXIST'G, (E)	EXISTING	SHTG
	FF	FINISHED FLOOR	SCHD
	FG	FINISHED GRADE	STRUCT
	F.Y.S.B.	FRONT YARD SETBACK	S.Y.S.B.
	GALV	GALVANIZED	T
	HW	HARDWOOD	Т.В.
	HORZ	HORIZONTAL	T.B.D
	ITOW INSIDE	TOP OF WALL	TOF
			TW
			TRANS
	SYMBOLS		UNO
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)	w/	WITH	WIC
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	0/	OVER	

LONGITUDINAL MANUFACTURER MASTER MARK ON CENTER OVERHANG PERMEABLE PERPENDICULAR PROPERTY LINE PLATE PLYWOOD QUANTITY REINFORCING REQUIREMENT ROOF BOUNDARY CLIP ROUGH OPENING RETAINING WALL REAR WALL SETBACK SAFETY GLAZING SHEATHING SCHEDULE STRUCTURAL (ENGINEER) SIDE YARD SETBACK TOP THERMAL BREAK TO BE DETERMINED TOP OF FOOTING TOP OF WALL TRANSVERSE UNLESS NOTED OTHERWISE UNCOVERED VERTICAL WATER RESISTIVE BARRIER WINDOW WELL WALK-IN CLOSET





REVISION	S
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LEGAL DESCRIPTION

PARCEL B, CITY OF MERCER ISLAND LOT LINE REVISION NO. SUB17-005, RECORDED UNDER RECORDING NUMBER 20171019900001, RECORDS OF KING COUNTY, WASHINGTON.

BASIS OF BEARINGS

CENTERLINE OF SE 32ND STREET BEARS N 88°49'28" W BETWEEN FOUND MONUMENTS PER REFERENCE 3.

REFERENCES

- R1. SURVEY BOOK 72, PG. 15, RECORDS OF KING COUNTY. R2. SURVEY BOOK 216, PG. 145, RECORDS OF KING COUNTY. R3. SURVEY BOOK 248. PG. 12, RECORDS OF KING COUNTY. R4. SURVEY BOOK 71, PG. 9, RECORDS OF KING COUNTY. R5. MAPLE GROVE PARK SUBDIVISION, VOL. 8 OF PLATS, PG. 37, RECORDS OF KING COUNTY.
- R6. CITY OF MERCER ISLAND LOT LINE REVISION SUB17-005, BOOK 372, PG. 200-201, RECORDS OF KING COUNTY.

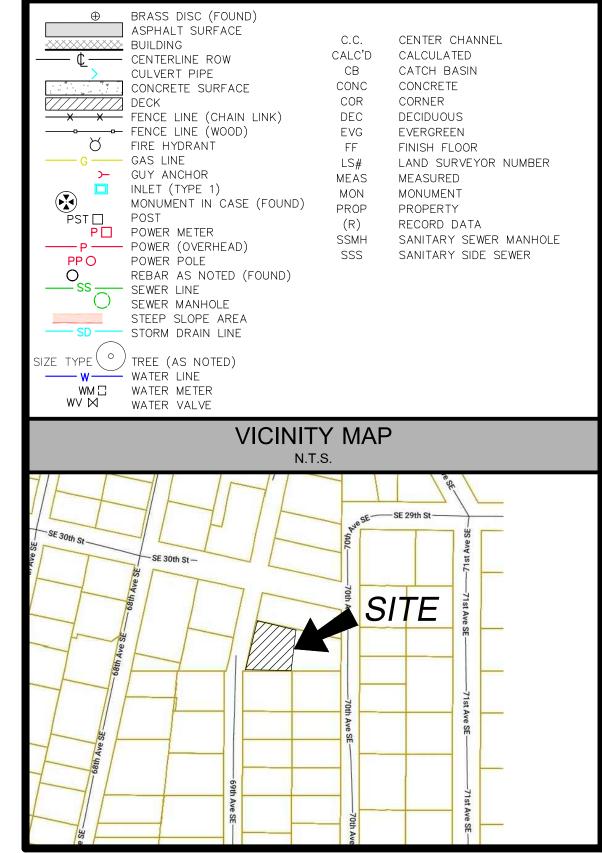
VERTICAL DATUM

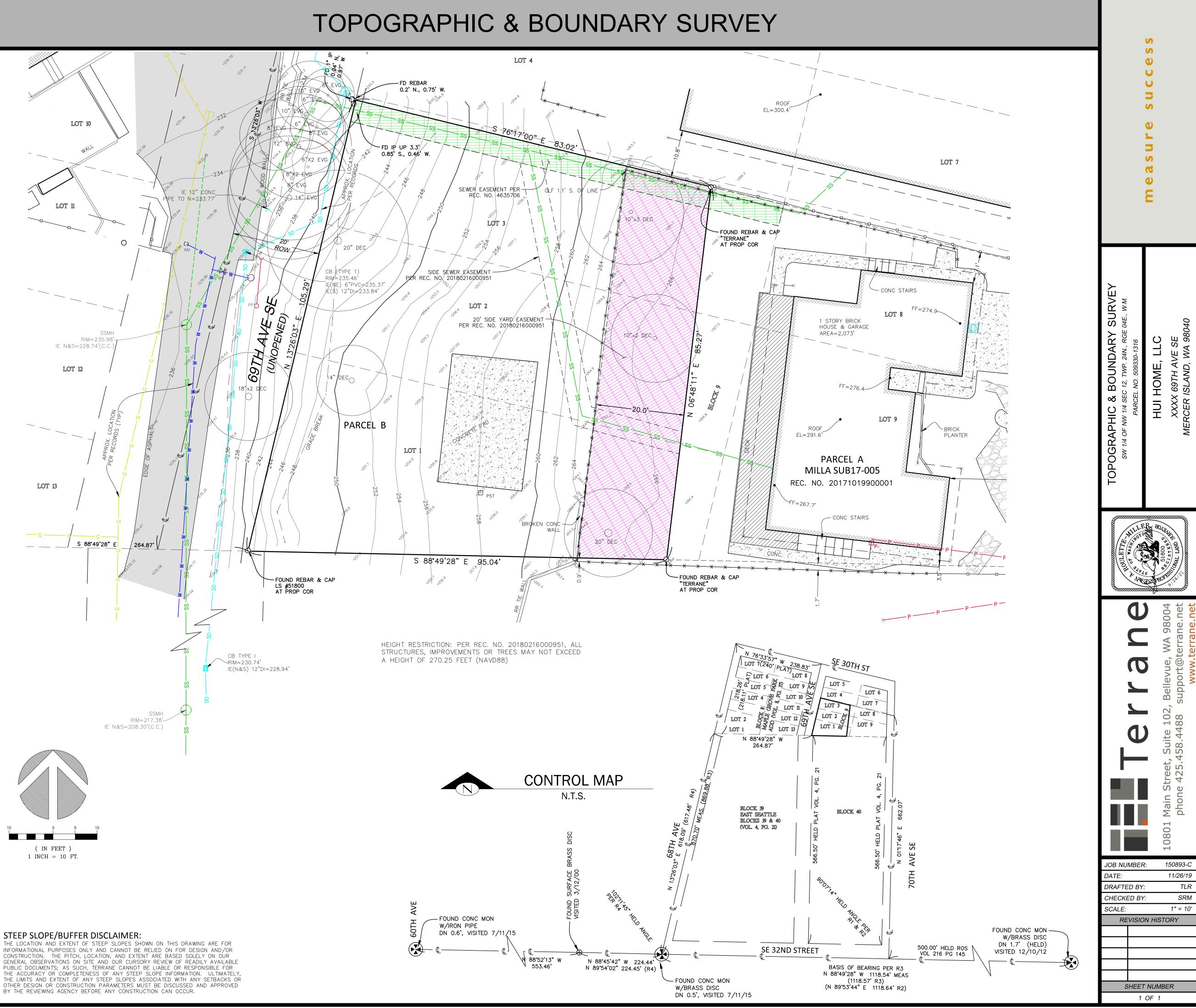
NAVD 88 PER CITY OF MERCER ISLAND BENCHMARK #502 BRASS CAP IN CONC MON AT INTX SE 32ND ST & 68TH AVE SE, ELEVATION = 112.57'

SURVEYOR'S NOTES

- . THE TOPOGRAPHIC SURVEY SHOWN HEREON WAS PERFORMED IN NOVEMBER OF 2019. THE FIELD DATA WAS COLLECTED AND RECORDED ON MAGNETIC MEDIA THROUGH AN ELECTRONIC THEODOLITE. THE DATA FILE IS ARCHIVED ON DISC OR CD. WRITTEN FIELD NOTES MAY NOT EXIST. CONTOURS ARE SHOWN FOR CONVENIENCE ONLY. DESIGN SHOULD RELY ON SPOT ELEVATIONS.
- 2. ALL MONUMENTS SHOWN HEREON WERE LOCATED DURING THE COURSE OF THIS SURVEY UNLESS OTHERWISE NOTED.
- 3. THE TYPES AND LOCATIONS OF ANY UTILITIES SHOWN ON THIS DRAWING ARE BASED ON INFORMATION PROVIDED TO US, BY OTHERS OR GENERAL INFORMATION READILY AVAILABLE IN THE PUBLIC DOMAIN INCLUDING, AS APPLICABLE, IDENTIFYING MARKINGS PLACED BY UTILITY LOCATE SERVICES AND OBSERVED BY TERRANE IN THE FIELD. AS SUCH, THE UTILITY INFORMATION SHOWN ON THESE DRAWINGS ARE FOR INFORMATIONAL PURPOSES ONLY AND SHOULD NOT BE RELIED ON FOR DESIGN OR CONSTRUCTION PURPOSES; TERRANE IS NOT RESPONSIBLE OR LIABLE FOR THE ACCURACY OR COMPLETENESS OF THIS UTILITY INFORMATION. FOR THE ACCURATE LOCATION AND TYPE OF UTILITIES NECESSARY FOR DESIGN AND CONSTRUCTION, PLEASE CONTACT THE SITE OWNER AND THE LOCAL UTILITY LOCATE SERVICE (800-424-5555).
- 4. SUBJECT PROPERTY TAX PARCEL NO. 509330-1316 5. SUBJECT PROPERTY AREA PER THIS SURVEY IS 8,403 S.F.
- (0.19 ACRES) 5. THIS SURVEY WAS PERFORMED WITHOUT THE BENEFIT OF A TITLE REPORT. EASEMENTS AND OTHER ENCUMBRANCES MAY EXIST THAT ARE NOT SHOWN HEREON.
- 7. FIELD DATA FOR THIS SURVEY WAS OBTAINED BY DIRECT FIELD MEASUREMENTS WITH A CALIBRATED ELECTRONIC 5-SECOND TOTAL STATION AND/OR SURVEY GRADE GPS OBSERVATIONS. ALL ANGULAR AND LINEAR RELATIONSHIPS ARE ACCURATE AND MEET THE STANDARDS SET BY WAC 332-130-090.

LEGEND





STEEP SLOPE/BUFFER DISCLAIMER:

CLEARING AND GRADING STANDARD NOTES

1. ALL CLEARING & GRADING CONSTRUCTION MUST BE IN ACCORDANCE WITH CITY OF MERCER ISLAND CLEARING & GRADING CODE; CLEARING & GRADING EROSION CONTROL STANDARD; LAND USE CODE; UNIFORM BUILDING CODE; PERMIT CONDITIONS; AND ALL OTHER APPLICABLE CODES, ORDINANCES, AND STANDARDS. THE DESIGN ELEMENTS WITHIN THESE PLANS HAVE BEEN REVIEWED ACCORDING TO THESE REQUIREMENT. ANY VARIANCE FROM ADOPTED EROSION STANDARDS IS NOT ALLOWED UNLESS SPECIFICALLYAPPROVED BY THE CITY OF MERCER ISLAND PUBLIC WORKS AND COMMUNITY DEVELOPMENT (PCD) PRIOR TO CONSTRUCTION.

IT IS THE SOLE RESPONSIBILITY OF THE APPLICANT AND THE PROFESSIONAL CIVIL ENGINEER TO CORRECT ANY ERROR, OMISSION, OR VARIATION FROM THE ABOVE REQUIREMENTS FOUND IN THESE PLANS. ALL CORRECTIONS WILL BE AT NO ADDITIONAL COST OR LIABILITY TO THE COB. ALL DETAILS FOR STRUCTURAL WALLS, ROCKERIES OVER FOUR FEET IN HEIGHT, GEOGRID REINFORCED ROCKERIES, AND GEOGRID REINFORCED MODULAR BLOCK WALLS MUST BE STAMPED BY A PROFESSIONAL ENGINEER.

2, A COPY OF THE APPROVED PLANS MUST BE ON-SITE DURING CONSTRUCTION. THE APPLICANT IS RESPONSIBLE FOR OBTAINING ANY OTHER REQUIRED OR RELATED PERMITS PRIOR TO BEGINNING CONSTRUCTION.

3. ALL LOCATIONS OF EXISTING UTILITIES HAVE BEEN ESTABLISHED BY FIELD SURVEY OR OBTAINED FROM AVAILABLE RECORDS AND SHOULD, THEREFORE, BE CONSIDERED ONLY APPROXIMATE AND NOT NECESSARILY COMPLETE. IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO INDEPENDENTLY VERIFY THE ACCURACY OF ALL UTILITY LOCATIONS AND TO DISCOVER AND AVOID ANY OTHER UTILITIES NOT SHOWN WHICH MAY BE AFFECTED BY THE IMPLEMENTATION OF THIS PLAN.

4. THE AREA TO BE CLEARED AND GRADED MUST FLAGGED BY THE CONTRACTOR AND APPROVED BY THE CLEARING & GRADING INSPECTOR PRIOR TO BEGINNING ANY WORK ON THE SITE.

5. A REINFORCED SILT FENCE MUST BE INSTALLED AS SHOWN ON THE APPROVED PLANS OR PER THE CLEARING & GRADING INSPECTOR, ALONG SLOPE CONTOURS AND DOWN SLOPE FROM THE BUILDING SITE.

6. A HARD-SURFACE CONSTRUCTION ACCESS PAD IS REQUIRED. THIS PAD MUST REMAIN IN THE PLACE UNTIL PAVING IS INSTALLED.

7. CLEARING WILL BE LIMITED TO THE AREAS WITHIN THE APPROVED DISTURBANCE LIMITS, EXPOSED SOILS MUST BE COVERED AT THE END OF EACH WORKING DAY WHEN WORKING FROM OCTOBER 1ST THROUGH APRIL 30. FROM MAY THROUGH SEPTEMBER 30, EXPOSED SOILS MUST BE COVERED AT THE END OF EACH CONSTRUCTION WEEK AND ALSO AT THE THREAT OF RAIN.

8. ANY EXCAVATED MATERIAL REMOVED FROM THE CONSTRUCTION SITE AND DEPOSITED ON THE PROPERTY WITHIN THE CITY LIMITS MUST BE DONE IN COMPLIANCE WITH VALID CLEARING & GRADING PERMIT. LOCATIONS FOR THE MOBILIZATION AREA AND STOCKPILED MATERIALS MUST APPROVED BY THE CLEARING & GRADING INSPECTOR AT LEAST 24 HOURS IN ADVANCE OF ANY STOCKIPLING.

9. TO REDUCE THE POTENTIAL FOR EROSION OF EXPOSED SOILS, OR WHEN RAINY SEASON CONSTRUCTION IS

- PERMITTED, THE FOLLOWING BEST MANAGEMENT PRACTICES (BMPS) ARE REQUIRED:
- * PRESERVED NATURAL VEGETATION FOR AS LONG AS POSSIBLE OR AS REQUIRED BY THE CLEARING & GRADING INSPECTOR. * PROTECT EXPOSED SOIL USING PLASTIC (EC-14), EROSION CONTROL BLANKETS, STRAW OR MULCH (COB GUIDE TO MULCH, RATES, AND USE CHART), OR AS DIRECTED BY THE CLEARING & GRADING INSPECTOR.
- * INSTALL CATCH BASIN INSERTS AS REQUIRED BY THE CLEARING & GRADING INSPECTOR OR PERMIT CONDITIONS OF APPROVAL. * INSTALL A TEMPORARY SEDIMENT POND, A SERIES OF SEDIMENTATION TANKS, TEMPORARY FILTER VAULTS, OR OTHER SEDIMENT CONTROL FACILITIES. ISTALLATION OF EXPOSED AGGREGATE SURFACES REQUIRES A SEPARATE EFFLUENT COLLECTION POND ON -SITE.

10. FINAL SITE GRADING MUST DIRECT DRAINAGE AWAY FROM ALL BUILDING STRUCTURES AT MINIMUM 2% SLOPE, PER UNIFORM BUILDING CODE.

11. THE CONTRACTOR MUST MAINTAIN A SWEEPER ON - SITE DURING EARTHWORK AND IMMEDIATELY REMOVE SOIL THAT HAS BEEN TRACKED ONTO PAVED AREAS AS RESULT OF CONSTRUCTION.

12. A PUBLIC INFORMATION SIGN LISTING 24-HOUR EMERGENCY NUMBER FOR THE CITY AND THE CONTRACTOR MAY BE PROVIDED TO THE APPLICANT AT THE TIME THE CLEARING & GRADING PERMIT IS ISSUED. THE APPLICANT MUST POST THE SIGN AT THE PROJECT SITE IN FULL VIEW OF THE PUBLIC AND THE CONTRACTORS, AND IT MUST REMAIN POSTED UNTIL FINAL SIGN -OFF BY THE CLEARING & GRADING INSPECTOR

13. TURBIDITY MONITORING MAY BE REQUIRED AS A OF CLEARING & GRADING PERMIT APPROVAL. IF REQUIRED, MONITORING MUST BE PERFORMED IN ACCORDANCE WITH THE APPROVED TURBIDITY MONITORING PLAN AND AS DIRECTED BY THE CLEARING & GRADING INSPECTOR. MONITORING MUST DURING SITE (EARTHWORK) CONSTRUCTION UNTIL THE FINAL SIGN - OFF BY THE CLEARING & GRADING INSPECTOR.

14. ANY PROJECT THAT IS SUBJECTED TO RAINY SEASON RESTRICTIONS WILL NOT BE ALLOWED TO PERFORM CLEARING & GRADING ACTIVITIES WITHOUT WRITTEN APPROVAL FROM THE CITY ENGINEER. THE RAINY SEASON EXTENDS FROM NOVEMBER 1ST THROUGH APRIL 30.

RESTORATION NOTES

1) Surface restoration of existing asphalt pavement shall be as required by the right-of-way use permit. 2) The Contractor shall restore the Right-of-Way and existing public storm drainage easement(s) after construction to a condition equal or better than condition prior to entry. The Contractor shall furnish a signed release from all affected property owners after restoration has been completed

FIELD BOOK:		A DO MIN H	TANDEM ENGINE
SURVEY BASE MAP:		State Contained	8822 NE 178TH
DESIGN ENTERED:	J.W		BOTHELL, WA 98
DESIGNED	S.W		(206) 795-5674
CHECKED:	S.W	LESS GISTERS	(200) $133-3014$
		PONAL CONTRACT	

UTILITY NOTES

- 1) The locations of all existing utilities shown hereon have been established by field survey or obtained from available records and should therefore be considered approximate
- only and not necessarily complete. It is the sole responsibility of the excavator to independently verify the accuracy of all utility locations shown, and to further discover and avoid any other utilities not shown here on which may be affected
- by the implementation of this plan. Immediately notify the responsible Professional Engineer if a conflict exists.
- 2) Call 1-800-424-5555, or 8-1-1, 72 hours before construction for utility locates.
- 3) The Contractor shall maintain a minimum of five feet (5) horizontal separation between all water and storm drainage lines. Any conflict shall be reported to the Utility and the Professional Engineer prior to construction.
- 4) Avoid crossing water or sewer mains at highly acute angles. The smallest angle measure between utilities should be 45 degrees.
- 5) It shall be the Contractors responsibility to ensure that no conflicts exist between storm drainage lines and proposed or existing utilities prior to construction.
- 6) At points where existing thrust blocking is found, minimum clearance between concrete blocking and other buried utilities or structures shall be 5 feet.
- 7) Where a new utility line crosses below an existing AC main, the AC pipe shall be replaced with DI pipe to 3 feet past each side of the trench as shown on Standard Detail W-8. Alternatively, where directed by the Utility, the trench shall be backfilled with controlled density fill (CDF, aka flowable fill) from bottom of trench to bottom of AC main.

STORM DRAINAGE NOTES

- 1) Storm pipe shall be PVC conforming to ASTM D-3034 SDR35 or ASTM F-679. Bedding and backfill shall be as shown in the Standard Details. 2) The footing drainage system and the roof downspout system shall not be interconnected and shall separately convey collected flows to the conveyance system or to on-site storm water facilities.
- Prior to final inspection and acceptance of storm drainage work, pipes and storm drain structures shall be cleaned and flushed. Any obstructions to flow within the storm drain system, (such as rubble, mortar and wedged debris), shall be removed at the nearest structure. Wash water of any sort shall not be discharged to the storm drain system or surface waters.
- Ends of each storm drain stub at the property line shall be capped and located with an 8' long 2" x 4" board, embedded to the stub cap and extending at least 3 feet above grade, and marked permanently "STORM". A copper 12 ga. locate wire firmly attached. The stub depth shall be indicated on the marker.
- 5) All grates in roadways shall be ductile iron, bolt-locking, vane grates per the Standard Details. Structures in traffic lanes outside of the curb line which do not collect runoff shall be fitted with round, bolt-locking solid covers. Off-street structures which do not collect runoff shall be fitted with bolt-locking solid covers.
- 6) Vegetation/landscaping in the detention pond, bioretention facility, vegetated roof and/or drainage swale(s) are an integral part of the runoff treatment system for the project. Such drainage facilities will not be accepted until plantings are established.
- 7) All new manholes shall have a minimum inside diameter of 48' and shall conform to the Standard Details. All new catch basins shall conform to the Standard Details.
- 8) Side storm stations are referenced from nearest downstream manhole/ catch basin.
- All testing and connections to existing mains shall be done in the presence of a representative of the City of Mercer Island Utilities Department.
- 10) All public storm drains shall be air tested and have a video inspection performed prior to acceptance (see #23 below). Storm main constructed with flexible pipe shall be deflection tested with a mandrel prior to acceptance.
- 11) Storm stubs shall be tested for acceptance at the same time the main storm is tested.
- 12) All manholes/ catch basins in unpaved areas shall include a concrete seal around adjustment rings per Standard Details.
- 13) All storm main extensions within the public right-of-way or in easements must be staked by a surveyor licensed in Washington State for line and grade and cut sheets provided to the Professional Engineer, prior to starting construction.
- 14) Storm drainage mainlines, stubs and fittings shall be constructed using the same pipe material and manufacturer. Connections stubs and the mainline will be made with a tee fitting. Tee fitting shall be from same manufacturer as pipe. Cut-in connections only allowed when connecting a new stub to an existing mainline.
- 15) Manholes, catch basins and vaults are considered to be permit-required confined spaces. Entry into these spaces shall be in acc with Chapter 296-809 WAC.
- 16) Placement of surface appurtenances (MH lids, valve lids, etc.) in tire tracks of traffic lanes shall be avoided whenever possible.
 - 17) The Contractor shall perform a video inspection and provide a DVD of the storm pipe interior for the Citys review. The video sl provide a minimum of 14 lines per millimeter resolution and cover the entire length of the applicable pipe. The camera shall be through the pipe at a uniform rate (=30 ft/min), stopping when necessary to ensure proper documentation of the pipe condition. video shall be taken after installation and cleaning to insure that no defects exist. The project will not be accepted until all defects have been repaired.
 - 18) Clearly label public and private systems on the plans. Private systems shall be marked private and shall be maintained by the property owner(s).
 - 19) All concrete structures (vaults, catch basins, manholes, oil/water separators, etc.) shall be vacuum tested.
 - 20) Manholes, catch basins and inlets in easements shall be constructed to provide a stable, level grade for a minimum radius of 2. around the center of the access opening to accommodate confined space entry equipment
 - 21) Tops of manholes/ catch basins within public right-of-way shall not be adjusted to final grade until after paving.
 - 22) Contractor shall adjust all manhole/ catch basin rims to flush with final finished grades, unless otherwise shown.
 - 23) Contractor shall install, at all connections to existing downstream manholes/catch basins, screens or plugs to prevent foreign mate entering existing storm drainage system. Screens or plugs shall remain in place throughout the duration of the construction and removed along with collected debris at the time of final inspection and in the presence of a representative of the City of Bellevu Utilities Department.
 - 24) Before commencement of trenching, the Contractor shall provide filter fabric for all downhill storm drain inlets and catch basins, whether the state of the s receive runoff from the project site. The contractor shall periodically inspect the condition of all filter fabric and replace as necess 25) Minimum cover over storm drainage pipe shall be 2 feet, unless otherwise shown.
 - 26) Redirect sheet flow, block drain inlets and/or curb openings in pavement and install flow diversion measures to prevent construct runoff and debris from entering excavations and finish surfaces for bioretention facilities and permeable pavements.
 - 27) Where amended soils, bioretention facilities, and permeable pavements are installed, these areas shall be protected at all times fro over-compacted. If areas become compacted, remediate and till soil in accordance with the Citys Project Representatives requirements at no additional cost in order to restore the systems ability to infiltrate.

EERING CONSULTANT INC ST 8011

GENERAL NOTES

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VICINITY MAP

NTS

LEGAL DESCRIPTION

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The	PARCEL NUMBER: 50	9330-1316	
S	PROPERTY OWNER:	HUI HOME LLC 30XX 69TH AVE SE MERCER ISLAND, WA	98040
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	PROJECT ENGINEER:	STEVE WU	
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HU'S RESIDENCE 30XX 69TH AVE SE **MERCER ISLAND WA 98040**

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GENERAL TESC NOTES

Temporary erosion and sedimentaiton control facilities (TESC) (including but not limited to temporary construction entrance, catch basin protection, silt fence installation, interceptor ditches, sedimentation ponds and straw bales) must be in place and Inspected by the City of Mercer Island prior to demolition, clearing/grading, etc. Spoil piles shall be kept covered. All City streets shall be kept free of mud and construction debris. TESC facilities shall be maintained until final landscaping is completed. No sediment-laden water shall enter Lake Washington, the public storm drain system, water courses, sensitive areas or the adjacent properties. Not all of these facilities may be identified on this plan but may be required during construction. Contractor will adhere to additional requirements as conditions warrant and the project progresses, including cleaning of downstream catch basins and drainage facilities of sediment from this project.

PLAN NOTES

- 1. Approval of this temporary erosion and sedimentation control (TESC) plan does not constitute an approval of permanent road or drainage design.
- 2. The implementation of these TESC plans and the construction, maintenance, replacement, and upgrading of these TESC facilities is the responsibility of the owner/agent and/or their contractor until all construction is approved.
- 3. The boundaires of the clearing limits shown on this plan shall be clearly flagged by a continuous length of survey tape (or fencing, if required) prior to construction. During the construction period, no disturbance beyond the clearing limits shall be permitted. The clearing limits shall be maintained by the owner/agent and/or their contractor for the duration of construction.
- 4. The TESC facilities shown on this plan must be constructed prior to or in conjunction with all clearing and grading so as to ensure that the transport of sediment to surface waters, drainage systems, and adjacent properties is minimized.
- 5. The TESC facilities shown on this plan are the minimum requirements for anticipated site conditions. During the construction period, these TESC facilities shall be upgraded as needed for unexpected storm events and modified to account for changing site conditions (e.g., additional sump pumps, relocation of ditches, hay bales and silt fences, etc.).
- 6. The TESC facilities shall be inspected daily by the owner/agent and/or their contractor and maintained to ensure continued proper functioning. Written records shall be kept of weekly reviews of the TESC facilities during the wet season (Oct. 1 to April 30) and of monthly reviews during the dry season (May 1 to Sept. 30).
- 7. Any areas of exposed soils, including roadway embankments, that will not be disturbed for two days during the wet season (Oct. 1 to April 30) or seven days during the dry season (May 1 to Sept. 30). shall be immediately stabilized with approved TESC methods (e.g., seeding, mulching, plastic covernig, etc.).
- 8. Any area needing TESC measures that do not require immediate attention shall be addressed within fifteen (15) days.
- 9. The TESC fa cilities on inactive sites shall be inspected and maintained a minimum of once a month or within forty- eight (48) hours following a storm event. 10. At no time shall more than one (1) foot of sediment be allowed to accumulate within a catch basin. All catch basins and conveyance lines shall be cleaned prior to final grading and/or paving. The cleaning operation shall not flu sh sedimentl-aden water into the downstream system.
- 11. Stabilized construction entrances and roads shall be installed at the beginning of construction and maintained for the duration of the project. Additional measures, such as wash pads and sediment traps, may be required to ensure that all paved areas are kept clean for the duration of the project.
- 12. Any permanent flow control facility used as a temporary settling basin shall be modified with the necessary temporary erosion control measures and shall provide adequate storage capacity.
- 13. Where straw mulch for temporary erosion control is required, it shall be applied at a minimum thickness of 2 to 3 inches.
- 14. Prior to the beginning of the wet season (Oct. 1), all disturbed areas shall be reviewed to identify which ones can be seeded in preparation for the winter rains. Disturbed areas shall be seeded within one week of the beginning of the wet season. The City can require seeding of additional areas in order to protect surface waters, adjacent properties, or drainage facilities.

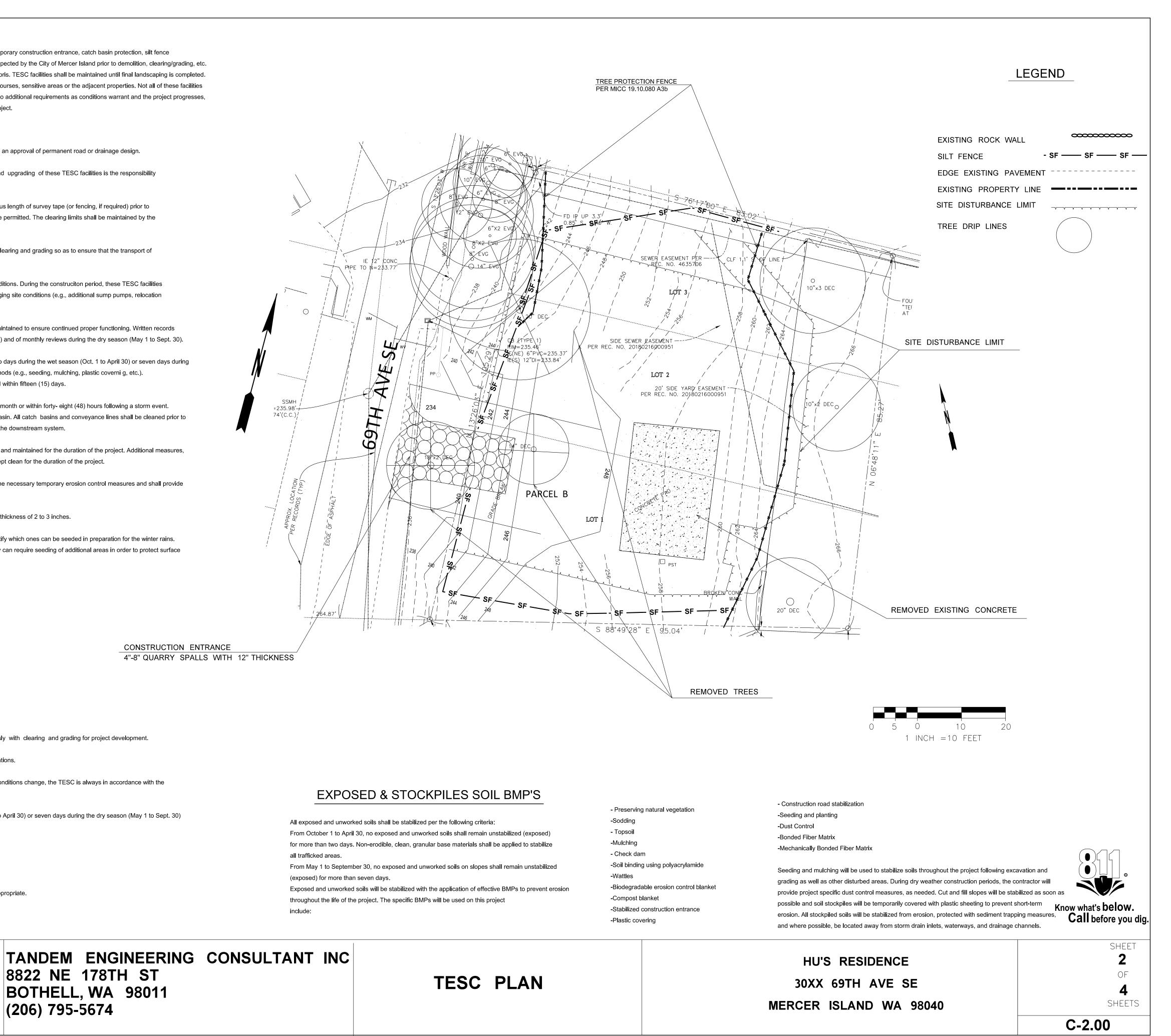
Construction Sequence:

- 1. Hold an onsite pre-construction meeting.
- 2. Flag or fence clearing limits.
- 3. Install catch basin protection, if required.
- 4. Grade and install construction entrance(s).
- 5. Install perimeter protection (silt fence, brush barrier, etc.).
- 6. Construct sediment pond(s) and/or trap(s).
- 7. Construct surface water controls (interceptor dikes, pipe slope drains, etc.) simultaneously with clearing and grading for project development.
- 8. Maintain TESC measures in accordance with City standards and manufacturer's recommendations.
- 9. Relocate surface water controls or TESC measures, or install new measures so that as site conditions change, the TESC is always in accordance with the City of Mercer Island Temporary Erosion and Sedimentation Control Requirements.
- 10. Cover all areas that will be un-worked for more than two days during the wet season (Oct. 1 to April 30) or seven days during the dry season (May 1 to Sept. 30) with straw, wood fiber mulch, compost, plastic sheeting, or equivalent.
- 11. Stabilize all areas within seven days of reaching final grade.
- 12. Seed or sod any areas to remain un-worked for more than 30 days.
- 13. Upon completion of the project, stabilize all disturbed areas and remove TESC measures if appropriate.

Reference: King County Surface Water Design Manua, I Appendix D - 10.3

FIELD BOOK:		O MIN H
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8822 NE 178TH ST BOTHELL, WA 98011 (206) 795-5674



- STD.S-3, STD.S-4, STD. S-17, STD. S-18.

HMA PAVING ONLY WITHIN ROW

4" PERFORATED FOOTING DRAIN						
POINT	INVERT	LENGTH	S			
А	237.0	49.5	1%			
В	236.51	31.5	1%			
С	236.19	31	1%			
D	235.88	13.5	11.5%			
E	234.33					
A	237.0	35	1%			
F	236.65	24	1%			
D	235.88	49	1.6%			

4" PVC ROOF DRAIN

NVERT	LENGTH	S
244		
244		1
Z44	53	15.4%
235.84	16	11.5%
234.00		
258.00	34	2.94%
257.00	53.5	35.00%
238.28	4	61.38%
235.82		
	234.00 258.00 257.00 238.28	235.84 16 234.00

STORM DRAIN NOTES

(1) 4" FOOTING DRAIN MIN 2% GRADE (ASTM D-3034 PVC)

(2) 4" ROOF DRAIN MIN 2% GRADE (ASTM D-3034 PVC)

(3) 4" STORM DRAIN, PVC, L=11.0', S=15.5%, (ASTM D-3034 PVC)

(4) 6" STORM DRAIN, L=11, S=4.1% ((ASTM D-3034 PVC))

(5) 4" PVC STORM DRAIN, L=19.0, S=3.6%, (ASTM D-3034 PVC)

(6) 4" PVC STORM DRAIN, L=10.5 S=40%, (ASTM D-3034 PVC)

(7) 4" PVC STORM DRAIN, MIN 2% SLOPE (ASTM D-3034 PVC)

Post Construction Soil Quality

All areas subject to clearing and grading that have not been covered by impervious surface, incorporated into a drainage facility or engineered as structure fill or slope shall, at project completion, demonstrate the following:

1. A topsoil layer with a minimum organic matter content of 10% dry weight in planting beds, and 5% organic matter content in turf areas, and a pH from 6.0 to 8.0 or matching the pH of the undisturbed soil. The topsoil layer shall have a minimum depth of eight inches except where tree roots limit the depth of incorporation of amendments needed to meet the criteria. Subsoils below the topsoil layer should be scarified at least 4 inches with some incorporation of the upper material to avoid stratified layers, where feasible.

2. Mulch planting beds with 2 inches of organic material

3. Use compost and other materials that meet these organic content requirements:

a. The organic content for "pre-approved" amendment rates can be met only using compost meeting the compost specification for BMP T7.30: Bioretention Cells, Swales, and Planter Boxes (p.959), with the exception- that the compost may have up to 35% biosolids or manure. The compost must also have an organic matter content of 40% to 65%, and a carbon to nitrogen ratio below 25:1. The carbon to nitrogen ratio may be as high as 35:1 for plantings composed entirely of plants native to the Puget Sound Lowlands region.

b. Calculated amendment rates may be met through use of composted material meeting (a.) above; or other organic materials amended to meet the carbon to nitrogen ratio requirements, and not exceeding the contaminant limits identified in Table 220-B, Testing Parameters, in WAC 173-350-220.

Maintenance

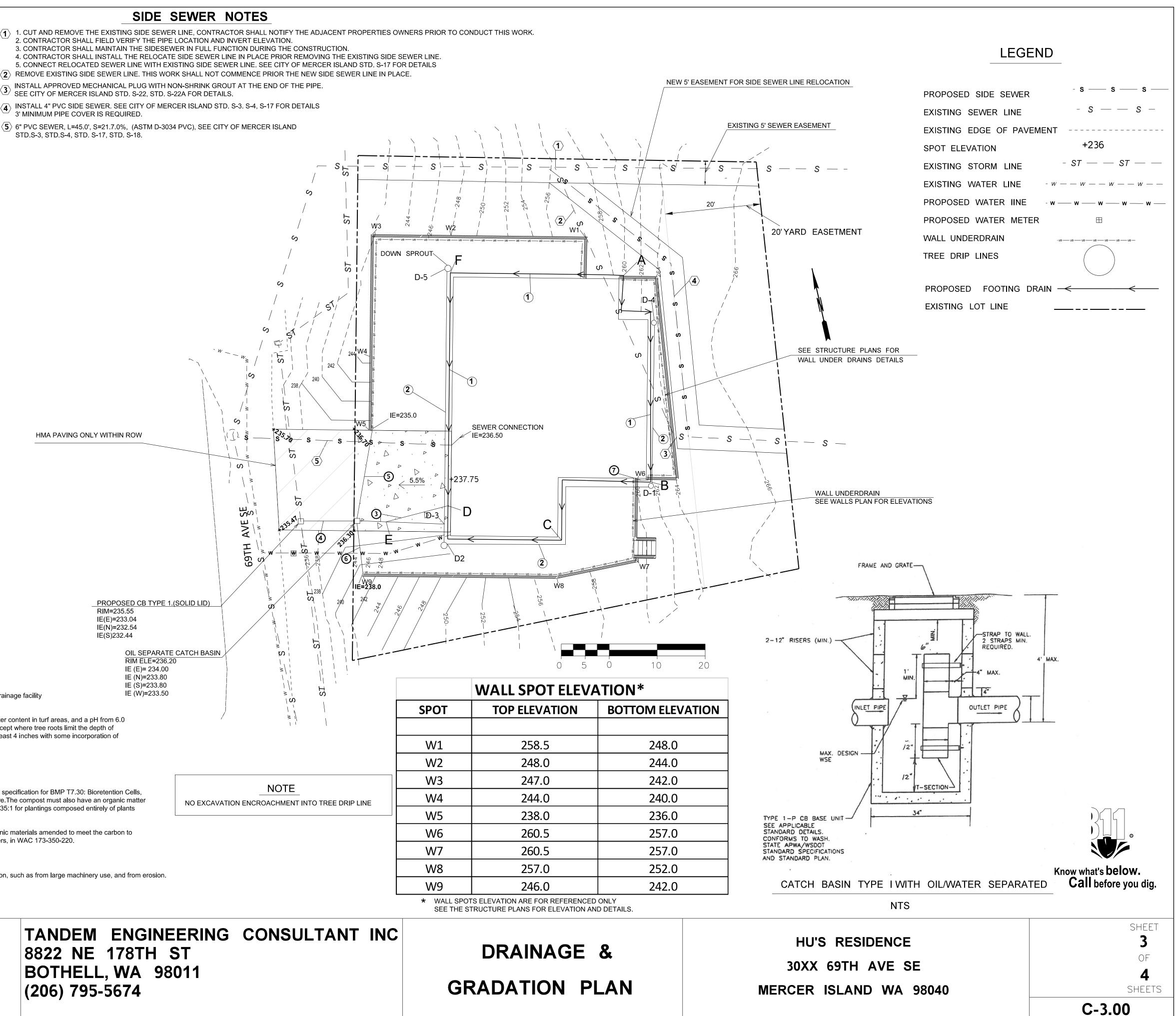
Establish soil quality and depth toward the end of construction and once established, protect from compaction, such as from large machinery use, and from erosion. Plant vegetation and mulch the amended soil area after installation.

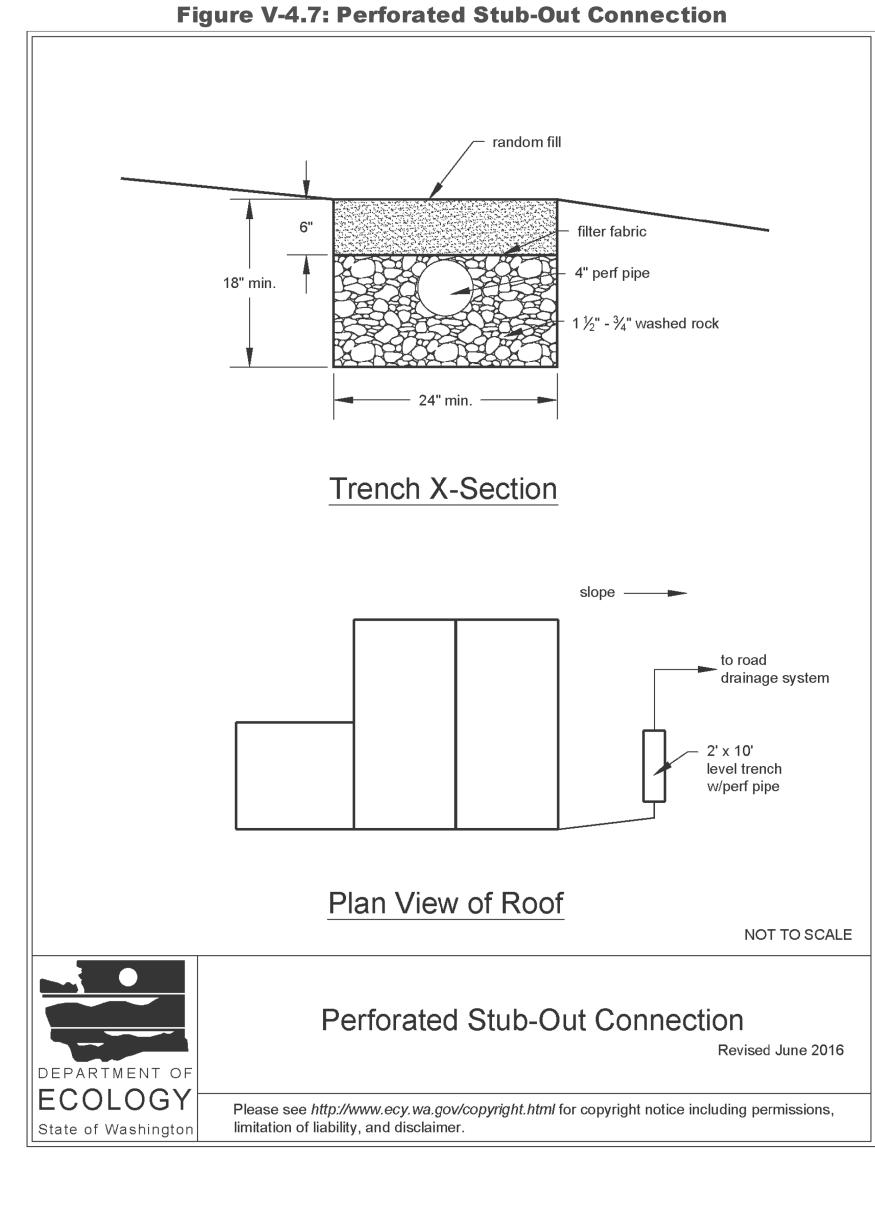
2. 3. Leave plant debris or its equivalent on the soil surface to replenish organic matter.

- 4. Reduce and adjust, where possible, the use of irrigation, fertilizers, herbicides and
- pesticides, rather than continuing to implement formerly established practices.

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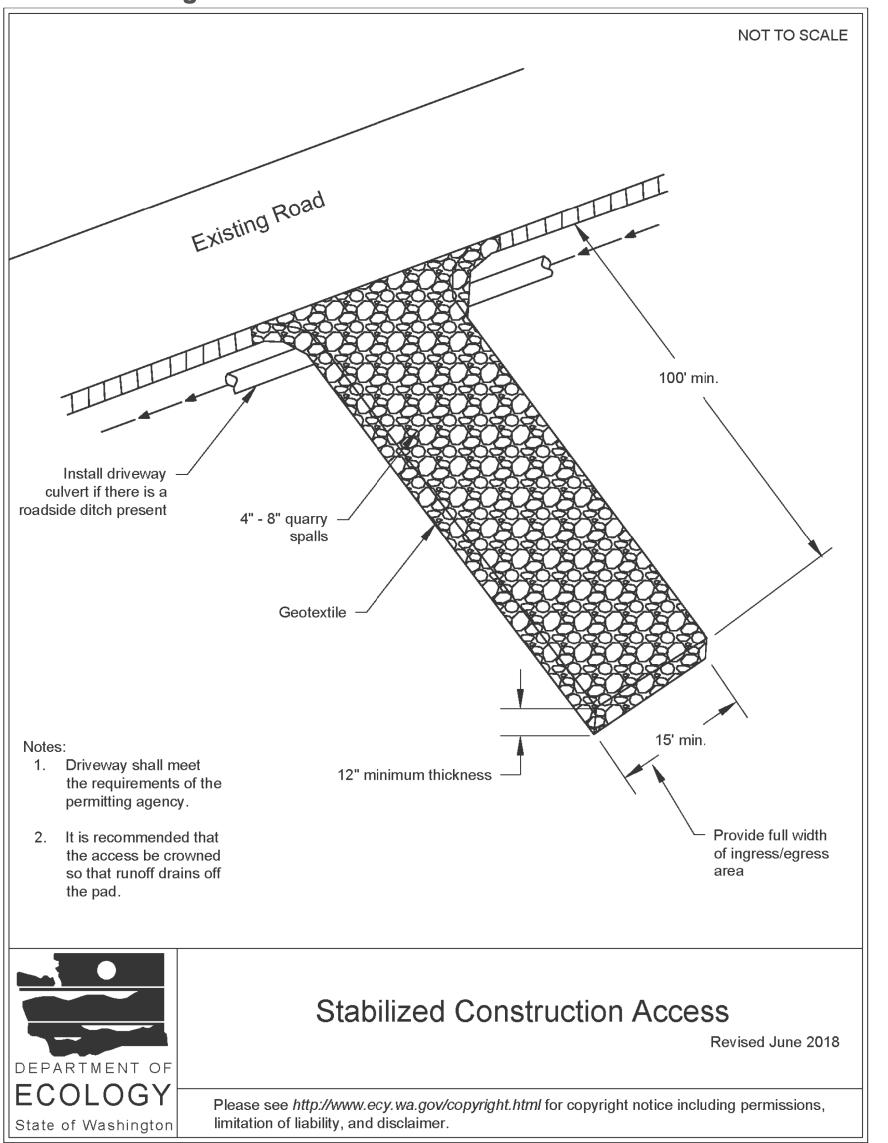




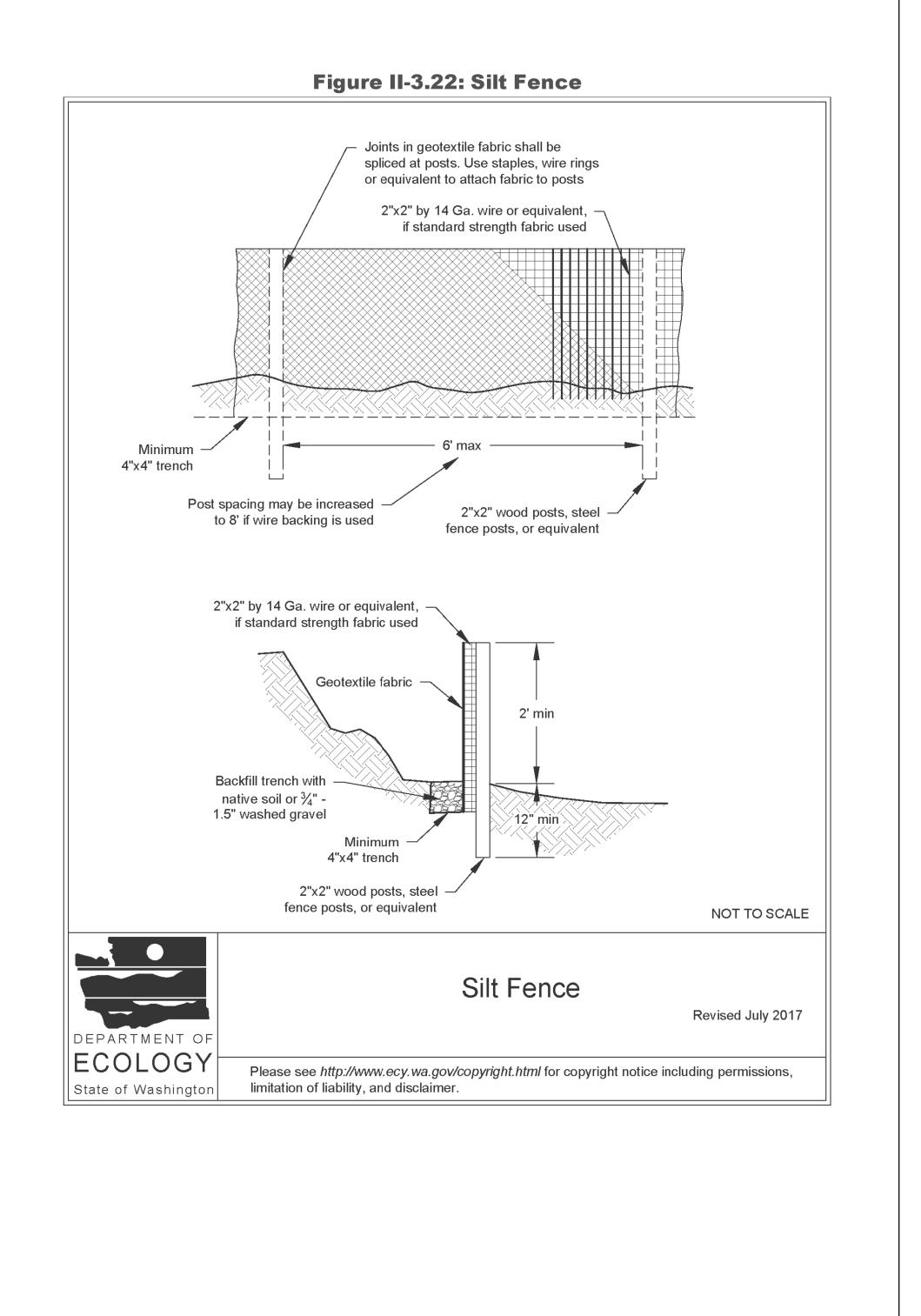




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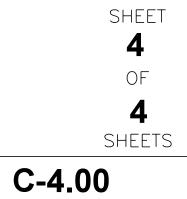








HU'S RESIDENCE 30XX 69TH AVE SE MERCER ISLAND WA 98040



TEMPORARY SHORING AND PERMANENT RETAINING WALL PLANS

SHEET NUMBER

SH O = 1SH2.0 SH3.0-3.4 SH3.A SH4.0-4.1

COVER & SHORING NOTES SHORING PLAN SHORING ELEVATION PILE AND ANCHOR SCHEDULE CROSS-SECTIONS AND DESIGN DIAGRAMS DETAILS

SHEET TITLE

SH5.0-5.2



SHORING WALL NOTES:

GENERAL:

THE GENERAL CONTRACTOR IS RESPONSIBLE FOR FIELD VERIFYING ALL EXISTING DIMENSIONS AND SITE CONDITIONS. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR DETERMINING ACTUAL LOCATIONS OF ALL EXISTING UTILITIES SHOWN ON THE PLANS AND THOSE UTILITIES OR UNDERGROUND OBSTRUCTIONS NOT SHOWN ON THE PLANS. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR REMOVAL OF ALL ABANDONED UTILITIES, OR OTHER UNDERGROUND OBSTRUCTIONS THAT INTERFERE WITH THE NEW CONSTRUCTION

THE GENERAL CONTRACTOR AND SUBCONTRACTORS ARE RESPONSIBLE FOR THE CONSTRUCTION PROCESS AND THE SAFETY OF THE WORKERS. THIS INCLUDES BUT IS NOT LIMITED TO, THE CONSTRUCTION SEQUENCE, TEMPORARY HANDRAILS, EXCAVATION ACCESS AND BARRIERS. IT ALSO INCLUDES LIFTING OF MATERIALS AND CONSTRUCTION EQUIPMENT INTO AND OUT OF THE EXCAVATION, TEMPORARY BRACING OF SINGLE-SIDED FORMWORK TEMPORARY SHORING OF EXCAVATIONS, AND STABILITY OF ALL TEMPORARY CUT SLOPES.

A PRE-CONSTRUCTION MEETING SHALL BE HELD PRIOR TO THE START OF THE WORK AND SHALL BE ATTENDED BY THE OWNER'S REPRESENTATIVES, THE ENGINEER, THE GENERAL CONTRACTOR, THE EXCAVATION SUBCONTRACTOR, THE SHORING SPECIALTY SUBCONTRACTOR, THE GEOTECHNICAL SPECIAL INSPECTOR, AND THE CITY REPRESENTATIVE. THE PRE-CONSTRUCTION MEETING SHALL BE CONDUCTED TO CLARIFY THE REQUIREMENTS FOR THE WORK, TO COORDINATE THE CONSTRUCTION ACTIVITIES, AND TO IDENTIFY CONTRACTUAL RELATIONSHIPS AND RESPONSIBILITIES.

PRE-CONSTRUCTION REVIEW:

SIX WEEKS PRIOR TO ORDERING SHORING SYSTEM MATERIALS, NOTIFY GROUND SUPPORT PLLC SO THAT THE EXCAVATION PLAN CAN BE CHECKED FOR CHANGES.

DHS ENGINEERS

HU RESIDENCE

30xx 69TH AVENUE SE, MERCER ISLAND, WASHINGTON

REFERENCE DATA:

ALL EXISTING SITE DATA, EXISTING AND PROPOSED TOPOGRAPHICAL DATA, AND EXISTING AND PROPOSED UTILITY DATA; AND PROPOSED SHORING WALL LOCATIONS ARE BASED ON:

• THE PLAN SET TITLED "HU_20201228_TO_DH.DWG" DECEMBER 28, 2020. • THE ELECTRONIC DRAWING FILE NAMED "HU RESIDENCE DHSI042IBW-A.PDF", PERMIT SET FILED DATED JANUARY 8, 2021, FILE PROVIDED TO GROUND SUPPORT PLLC BY DHS ENGINEERS.

BUILDING CODES, DESIGN MANUALS, AND SPECIFICATIONS:

2015 INTERNATIONAL BUILDING CODE, AS AMENDED BY THE CITY OF MERCER ISLAND.

1998 FHWA SUMMARY REPORT OF RESEARCH ON PERMANENT GROUND ANCHOR WALLS

GEOTECHNICAL ENGINEERING CIRCULAR NO. 4, "GROUND ANCHORS AND ANCHORED SYSTEMS", FHWA, DATED JUNE 1999.

DESIGN LIVE LOADS:

TRAFFIC/CONSTRUCTION SURCHARGE = SEE SH4.0

DESIGN CALCULATIONS:

THE SOLDIER PILE SHORING WALL DESIGN CALCULATIONS ARE CONTAINED IN THE REPORT TITLED: "DESIGN MEMORANDUM, HU RESIDENCE (PROJECT NO. 20-50), 30xx 69TH AVENUE SE, MERCER ISLAND, WASHINGTON", PREPARED BY GROUND SUPPORT PLLC FOR DHS ENGINEERS, Dihong Shao, DATED JANUARY 5, 2021, REVISED JULY 31, 2022.

SUBSURFACE DESIGN:

ALL SUBSURFACE DESIGN PARAMETERS USED IN THE SHORING DESIGN ARE BASED ON THE SUBSURFACE CHARACTERIZATION PRESENTED IN THE REPORT "GEOTECHNICAL ENGINEERING EVALUATION, HU RESIDENCE DEVELOPMENT, 30xx 69th AVENUE SE, MERCER ISLAND, WASHINGTON", PREPARED BY NELSON GEOTECHNICAL ASSOCIATES, INC., DATED JULY 10TH, 2020. THE SHORING DESIGN PARAMETERS AND EARTH PRESSURE DIAGRAMS ARE PRESENTED ON THE PLANS.

SEISMIC DESIGN PARAMETERS:

SEE SH4.0

PSI.

INCHES.

ADMIXTURES SHALL CONFORM TO THE REQUIREMENTS OF ASTM C494 / AASHTO MI94, SHALL BE USED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS, AND SHALL BE APPROVED BY THE ENGINEER.

AGGREGATES SHALL CONFORM TO THE REQUIREMENTS OF ASTM C33 / AASHTO M6 FOR FINE AGGREGATES AND AASHTO M80, CLASS B FOR COARSE AGGREGATES.

TIMBER LAGGING:

ALL LAGGING BOARDS SHALL BE PRESSURE-TREATED, IN GOOD CONDITION, AND SHALL BE HEM-FIR NO. I OR BETTER HEM-FIR NO. 2 OR BETTER, WITH AN ALLOWABLE FLEXURAL STRESS FB=1020 PSI (4-INCH LAGGING) AND FB-1050 PSI (6-INCH LAGGING) (WHICH INCLUDES ALL APPLICABLE FLAT-USE AND SIZE FACTORS).

ALL LAGGING BOARDS SHALL BE PRESSURE-TREATED IN ACCORDANCE WITH AWPA STANDARD UI-05 (FOR END USE CLASSIFICATION UC4), TO A MINIMUM RETENTION OF 0.40 PCF, USING THE CCA PROCESS (COMMERCIAL PRODUCT NAME OSMOSE OR APPROVED EQUAL). ALTERNATIVE TREATMENT PROCESSES MAY BE SUBMITTED TO GROUND SUPPORT PLLC FOR APPROVAL.

STRUCTURAL STEEL:

ALL STRUCTURAL STEEL SHAPES SHALL CONFORM TO ASTM A992 (f_{γ} =50 KSI (MIN)), AND PLATES SHALL CONFORM TO ASTM A572 (50KSI), UNLESS SHOWN OTHERWISE ON THE PLANS, OR APPROVED OTHERWISE BY THE ENGINEER.

LAYOUT OF SHORING ELEMENTS PERPENDICULAR TO THE BUILDING WALLS SHALL BE BASED ON THE ARCHITECTURAL PLANS TAKING INTO ACCOUNT PERTINENT BUILDING ELEMENTS (E.G., WATERPROOFING) NOT SHOWN ON THESE PLANS.

CONCRETE / CONTROLLED-DENSITY-FILL (CDF):

ALL STRUCTURAL CONCRETE SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH OF 3,000

ALL CONTROLLED-DENSITY-FILL (CDF) SHALL HAVE A MINIMUM OF 1.5 SACKS (141 LB) OF CEMENT PER CUBIC YARD OF CONCRETE.

TYPE I, II, OR III PORTLAND CEMENT CONFORMING TO ASTM CI50 / AASHTO M85 SHALL BE USED FOR CDF

SLUMP FOR ALL CONCRETE SHALL NOT BE LESS THAN 5 INCHES AND NO GREATER THAN 9

STRUCTURAL WELDING:

MINIMUM WELD SIZE 1/4 INCH CONTINUOUS FILLET. MINIMUM WELD LENGTH 2 INCHES. ALL WELDING TO CONFORM TO AWS DI.I. USE ETOXX ELECTRODES.

SHORING ELEMENT LAYOUT:

AND WAL TES NG NG C Ζ TAINII 2 \mathbf{O} Ο Ш SID ОЩΟ L T н N R Ŷ Hu Zw R TEMPOR ERMANE COVER Ш $\vdash \square$ Ω PROJ. NO. 20-50

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WATERPROOFING:

THE RETAINING WALL SYSTEM, IS CONSTRUCTED EXTERNAL TO THE BUILDING ENVELOPE OF THE PERMANENT STRUCTURE. DRAINAGE/WATER-PROOFING OF THE PERMANENT BASEMENT WALLS IS PROVIDED BY OTHERS.

DRILLED SOLDIER PILES:

THE MINIMUM REQUIRED STRUCTURAL STEEL SHAPES FOR THE SOLDIER PILES ARE INDICATED IN THE SCHEDULES. ALTERNATIVE STEEL SECTIONS MAY BE USED PROVIDED THAT THE CROSS-SECTIONAL AREA AND SECTION MODULUS OF EACH ALTERNATIVE STEEL SECTION ARE EQUAL TO OR GREATER THAN THE CROSS-SECTIONAL AREA AND SECTION MODULUS OF THE CORRESPONDING STEEL SECTION SHOWN ON THE PLANS.

SHAFTS SHALL BE CONSTRUCTED SO THAT THE CENTER AT THE TOP OF THE SHAFT IS WITHIN +/-3 INCHES OF THE PLAN LOCATION. SHAFT PLUMBNESS MAY VARY UP TO I PERCENT OF PILE LENGTH.

THE STEEL SOLDIER PILES SHALL BE PLACED SO THAT THE CENTER LINE OF THE PILE IS WITHIN +/- I INCH OF THE PLAN LOCATION. THE STEEL SOLDIER PILE SHALL BE PLUMB CONSISTENT WITH MAXIMUM DEVIATION INTO/OUT-OF THE EXCAVATION AS DEFINED BY THE STRUCTURAL ENGINEER AND GENERAL CONTRACTOR. THE TOP ELEVATION OF THE STEEL SOLDIER PILE SHALL BE WITHIN +/- 3 INCHES OF THE PLAN ELEVATION.

SHAFTS SHALL BE EXCAVATED TO THE REQUIRED DEPTH AS SHOWN ON THE PLANS. THE EXCAVATION SHALL BE COMPLETED IN A CONTINUOUS OPERATION USING EQUIPMENT CAPABLE OF EXCAVATING THROUGH THE TYPE OF MATERIAL EXPECTED TO BE ENCOUNTERED.

IF THE SHAFT EXCAVATION IS STOPPED WITH THE APPROVAL OF THE ENGINEER, THE SHAFT SHALL BE SECURED BY INSTALLATION OF A SAFETY COVER. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THE SAFETY OF THE SHAFT AND SURROUNDING SOIL AND THE STABILITY OF THE SIDE WALLS. A TEMPORARY CASING SHALL BE USED IF NECESSARY TO ENSURE SUCH SAFETY AND STABILITY.

WHERE CAVING CONDITIONS ARE ENCOUNTERED, THE CONTRACTOR SHALL SELECT A METHOD TO PREVENT GROUND MOVEMENT. THE CONTRACTOR MAY ELECT TO PLACE A TEMPORARY CASING.

THE CONTRACTOR SHALL USE APPROPRIATE MEANS (SUCH AS A CLEANOUT BUCKET), TO CLEAN THE BOTTOM OF THE EXCAVATION SUCH THAT NO MORE THAN 2 INCHES OF LOOSE OR DISTURBED MATERIAL IS PRESENT.

UNLESS SHOWN OTHERWISE ON THE PLANS, EXCAVATION OF SHAFTS SHALL NOT COMMENCE UNTIL A MINIMUM OF 12 HOURS AFTER THE CDF FOR THE ADJACENT SHAFTS HAS BEEN PLACED.

TEMPORARY CASINGS FOR THE SHAFTS SHALL BE REMOVED. A MINIMUM 5 FOOT HEAD OF CONCRETE MUST BE MAINTAINED TO BALANCE THE SOIL AND WATER PRESSURE AT THE BOTTOM OF THE CASING DURING REMOVAL. THE CASING SHALL BE SMOOTH.

SHAFT CONCRETE / CDF SHALL BE PLACED AS SHOWN ON THE PLANS.

THE CONCRETE / CDF SHALL BE DEPOSITED BY TREMIE PLACEMENT METHODS.

SUBMITTALS:

THE FOLLOWING SUBMITTALS SHALL BE PROVIDED BY THE CONTRACTOR, AT LEAST 15 DAYS PRIOR TO INITIATING THE WORK, FOR THE ENGINEER'S REVIEW AND APPROVAL:

- . WORKING DRAWINGS INDICATING STEEL PILE FABRICATION DETAILS AND MATERIALS.
- 2. CONCRETE MIX DESIGNS AND PLACEMENT PROCEDURES.
- 3. PAINT SYSTEM AND APPLICATION PROCEDURES.
- 4. SHAFT INSTALLATION PLAN, PROVIDING AT LEAST THE FOLLOWING INFORMATION:
- A. LIST AND DESCRIPTION OF PROPOSED EQUIPMENT TO BE USED, INCLUDING BUT NOT LIMITED TO, CRANES, DRILLS, AUGERS, BAILING BUCKETS, FINAL CLEANING EQUIPMENT, TREMIES, PUMPS, ETC.
- B. THE CONSTRUCTION SEQUENCE.
- C. DETAILS OF SHAFT EXCAVATION METHODS INCLUDING METHODS TO CLEAN THE SHAFT EXCAVATION.
- D. DETAILS OF THE SHAFTS AND CASING.
- E. DETAILS OF SOLDIER PILE PLACEMENT METHODS.

PAINTING OF SOLDIER PILES:

THE STEEL SOLDIER PILES AND ATTACHMENTS SHALL BE PAINTED AFTER FABRICATION TO THE LIMITS SHOWN ON THE PLANS WITH ONE COAT INORGANIC ZINC-RICH PRIMER CONFORMING TO EITHER AASHTO M 300 OR SSPC PAINT 20 TYPE I.

PAINTING SHALL BE IN ACCORDANCE WITH SECTION 6-07 OF THE 2012 WSDOT STANDARD SPECIFICATIONS FOR ROAD, BRIDGE, AND MUNICIPAL CONSTRUCTION, AS AMENDED BY THE CONTRACT PROVISIONS.

EACH COAT SHALL HAVE A MINIMUM DRY FILM THICKNESS OF 2.5 MILS.

EXCAVATION, LAGGING, BACKFILL, AND ANCHOR STRESSING:

THE CONTRACTOR SHALL EXCAVATE THE WALL FACE AND INSTALL LAGGING IN SUCH A MANNER AS TO MAINTAIN A SAFE WORK PLACE AND AVOID EXCESSIVE SLOUGHING AND OVERBREAK. AS A MINIMUM, PRIOR TO PLACING THE SUBSEQUENT SET OF TIMBER LAGGING, DO NOT EXCAVATE MORE THAN 4 FEET BELOW THE CURRENT DEPTH OF LAGGED WALL FACE. IF FACE STABILITY CONDITIONS REQUIRE, THIS HEIGHT MUST BE REDUCED.

DO NOT EXCAVATE TO A DEPTH GREATER THAN 2 FEET BELOW A LEVEL OF ANCHORS PRIOR TO INSTALLATION, TESTING, AND LOCKOFF (AS APPLICABLE) OF THOSE ANCHORS.

LAGGING SHALL BE INSTALLED FROM THE TOP OF THE PILE PROCEEDING DOWNWARD. THE TIMBER LAGGING SHALL MAKE DIRECT CONTACT WITH THE SOIL. VOIDS BEHIND THE LAGGING SHALL BE FILLED WITH FREE-DRAINING BACKFILL. LEAVE GAPS (1/4-INCH TYP) BETWEEN LAGGING FOR DRAINAGE. CDF MAY BE USED AS BACKFILL IN LOCALIZED AREAS.

PERMANENT GROUND ANCHORS:

GENERAL:

IA. THE CONTRACTOR SHALL SELECT THE INSTALLATION METHOD, THE ANCHOR DIAMETER, AND THE METHOD OF GROUTING, IN ORDER TO DEVELOP THE DESIGN LOADS INDICATED ON THE PLANS, AS VERIFIED IN ACCORDANCE WITH THE ANCHOR TESTING PROGRAM.

IB. THE CONTRACTOR SHALL PREPARE AND SUBMIT TO THE ENGINEER FOR REVIEW AND APPROVAL WORKING DRAWINGS AND A DESIGN SUBMISSION DESCRIBING THE GROUND ANCHOR WORK. THE WORKING DRAWING AND DESIGN SUBMISSION SHALL INCLUDE THE FOLLOWING:

- 2. GROUT MIX DESIGN AND THE PROCEDURES FOR GROUT PLACEMENT.
- 2. GROUND ANCHOR INSTALLATION:

LOCATION SHOWN ON THE PLANS. THE DRILLHOLE SHALL BE LOCATED SO THE STRUCTURE.

2B. AT THE POINT OF ENTRY, THE GROUND ANCHOR SHALL BE INSTALLED WITHIN +/- 3 DEGREES OF THE INCLINATION FROM HORIZONTAL SHOWN IN THE PLANS. AT THE POINT OF ENTRY, THE HORIZONTAL ANGLE MADE BY THE GROUND ANCHOR AND THE STRUCTURE SHALL BE WITHIN +/- 3 DEGREES OF A LINE DRAWN PERPENDICULAR TO THE PLANE OF THE STRUCTURE, UNLESS SHOWN OTHERWISE ON THE PLANS. AT ALL ANCHOR LOCATIONS WHERE TIEBACKS CROSS, THE INCLINATION AND ORIENTATION OF THE ANCHORS SHALL BE +/- I DEGREE.

2C. WHEN CAVING CONDITIONS ARE ENCOUNTERED, NO FURTHER DRILLING WILL BE ALLOWED UNTIL THE CONTRACTOR SELECTS A METHOD TO PREVENT GROUND MOVEMENT. THE CONTRACTOR MAY USE TEMPORARY CASING. THE CONTRACTOR'S METHOD TO PREVENT GROUND MOVEMENT SHALL BE APPROVED BY THE ENGINEER.

2D. THE TENDON SHALL BE INSERTED INTO THE DRILLHOLE TO THE DESIRED DEPTH WITHOUT DIFFICULTY. WHEN THE TENDON CANNOT BE COMPLETELY INSERTED, THE CONTRACTOR SHALL REMOVE THE TENDON FROM THE DRILLHOLE AND CLEAN OR REDRILL THE HOLE TO PERMIT INSERTION. PARTIALLY INSERTED TENDONS SHALL NOT BE DRIVEN OR FORCED INTO THE HOLE.

2E. THE CONTRACTOR SHALL USE A NEAT-CEMENT OR A SAND-CEMENT GROUT. THE CEMENT SHALL NOT CONTAIN LUMPS OR OTHER INDICATIONS OF HYDRATION. ADMIXTURES, IF USED, SHALL BE MIXED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

2F. THE GROUT EQUIPMENT SHALL PRODUCE A GROUT FREE OF LUMPS AND UNDISPERSED CEMENT. A POSITIVE DISPLACEMENT GROUT PUMP SHALL BE USED. THE PUMP SHALL BE EQUIPPED WITH A PRESSURE GAUGE TO MONITOR GROUT PRESSURES AND A STROKE COUNTER. THE PRESSURE GAUGE SHALL BE CAPABLE OF MEASURING PRESSURES OF AT LEAST 150 PSI OR TWICE THE ACTUAL GROUT PRESSURES USED BY THE CONTRACTOR, WHICHEVER IS GREATER. THE GROUTING EQUIPMENT SHALL BE SIZED TO ENABLE THE GROUT TO BE PUMPED IN ONE CONTINUOUS OPERATION. THE MIXER SHALL BE CAPABLE OF CONTINUOUSLY AGITATING THE GROUT.

2F. THE GROUT EQUIPMENT SHALL PRODUCE A GROUT FREE OF LUMPS AND UNDISPERSED CEMENT. A POSITIVE DISPLACEMENT GROUT PUMP SHALL BE USED. THE PUMP SHALL BE EQUIPPED WITH A PRESSURE GAUGE TO MONITOR GROUT PRESSURES AND A STROKE COUNTER. THE PRESSURE GAUGE SHALL BE CAPABLE OF MEASURING PRESSURES OF AT LEAST 150 PSI OR TWICE THE ACTUAL GROUT PRESSURES USED BY THE CONTRACTOR, WHICHEVER IS GREATER. THE GROUTING EQUIPMENT SHALL BE SIZED TO ENABLE THE GROUT TO BE PUMPED IN ONE CONTINUOUS OPERATION. THE MIXER SHALL BE CAPABLE OF CONTINUOUSLY AGITATING THE GROUT.

2G. THE GROUT SHALL BE INJECTED FROM THE LOWEST POINT OF THE DRILLHOLE. THE GROUT MAY BE PUMPED THROUGH GROUT TUBES, CASING, OR DRILL RODS. THE GROUT CAN BE PLACED BEFORE OR AFTER INSERTION OF THE TENDON. THE QUANTITY OF THE GROUT AND THE GROUT PRESSURES SHALL BE RECORDED. THE GROUT PRESSURES AND GROUT TAKES SHALL BE CONTROLLED TO PREVENT EXCESSIVE HEAVE IN SOILS OR FRACTURING OF ROCK FORMATIONS.

2H. NO GROUT SHALL BE PLACED UNDER PRESSURE ABOVE THE BOND LENGTH DURING INITIAL GROUTING OF THE ANCHOR BOND LENGTH. THE GROUT AT THE TOP OF THE DRILLHOLE SHALL NOT CONTACT THE BACK OF THE STRUCTURE OR THE BOTTOM OF THE TRUMPET.

21. AFTER GROUTING, THE TENDON SHALL NOT BE LOADED UNTIL THE GROUT HAS ATTAINED SUFFICIENT STRENGTH TO CARRY THE TEST LOAD.

2J. THE CORROSION PROTECTION SURROUNDING THE UNBONDED LENGTH OF THE TENDON SHALL EXTEND UP BEYOND THE BOTTOM SEAL OF THE TRUMPET OR I FOOT INTO THE TRUMPET IF NO TRUMPET SEAL IS PROVIDED. THE CORROSION PROTECTION SURROUNDING THE NO-LOAD ZONE LENGTH SHALL NOT CONTACT THE BEARING PLATE OR ANCHOR HEAD DURING STRESSING. TRUMPET GROUT SHALL BE PLACED AFTER THE GROUND ANCHOR HAS BEEN TESTED AND LOCKED OFF.

3. ANCHOR GROUT:

3A. THE GROUT SHALL BE A NEAT OR SAND/CEMENT MIXTURE WITH A MINIMUM 3-DAY COMPRESSIVE STRENGTH OF 1500 PSI AND A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 3000 PSI PER ASTM CIO9 / AASHTO TIO6.

3B. TYPE II CEMENT CONFORMING TO THE REQUIREMENTS OF ASTM CI50 / AASHTO M85 SHALL BE USED.

3C. FINE AGGREGATES SHALL CONSIST OF CLEAN, NATURAL SAND, CONFORMING TO THE REQUIREMENTS OF ASTM C33 / AASHTO M6. MANUFACTURED SAND IS ACCEPTABLE PROVIDED IT IS SUITABLE FOR PUMPING IN ACCORDANCE WITH ACI 304, SECTION 4.2.2.

3D. ADMIXTURES SHALL BE IN ACCORDANCE WITH ASTM C494 / AASHTO MI94. ADMIXTURES WHICH CONTROL BLEED, IMPROVE FLOW, REDUCE WATER CONTENT, AND RETARD SET MAY BE USED IN THE GROUT SUBJECT TO THE APPROVAL OF THE ENGINEER. EXPANSIVE ADMIXTURES SHALL ONLY BE ADDED TO THE GROUT USED FOR FILLING SEALED ENCAPSULATIONS, TRUMPETS AND ANCHORAGE COVERS. ACCELERATORS WILL NOT BE PERMITTED. ADMIXTURES SHALL BE COMPATIBLE WITH PRESTRESSING STEELS AND MIXED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATION.

4. ANCHOR TENDONS:

4A. THE BAR GROUND ANCHORS TENDONS SHALL CONSIST OF THE FOLLOWING:

- ASTM A722 / AASHTO M275, GRADE I50.

ANCHOR SYSTEM OR SYSTEMS INTENDED FOR USE. THE WORKING DRAWINGS AND DESIGN SUBMISSION SHALL BE SUBMITTED 15 DAYS PRIOR TO THE COMMENCEMENT OF THE GROUND

I. CERTIFIED MILL TEST RESULTS AND TYPICAL STRESS-STRAIN CURVES FOR THE PRESTRESSING STEEL. THE TYPICAL STRESS-STRAIN CURVE SHALL BE OBTAINED BY APPROVED STANDARD PRACTICES. THE GUARANTEED ULTIMATE STRENGTH, YIELD STRENGTH, ELONGATION, AND COMPOSITION SHALL BE SPECIFIED.

3. CALIBRATION DATA FOR EACH TEST JACK, PRESSURE GAUGE AND REFERENCE PRESSURE GAUGE TO BE USED. THE CALIBRATION TESTS SHALL HAVE BEEN PERFORMED BY AN INDEPENDENT TESTING LABORATORY AND TESTS SHALL HAVE BEEN PERFORMED WITHIN 60 DAYS OF THE DATE SUBMITTED

2A. AT THE GROUND SURFACE, THE DRILLHOLE SHALL BE LOCATED WITHIN 4 INCHES OF THE LONGITUDINAL AXIS OF THE DRILLHOLE AND THE LONGITUDINAL AXIS OF THE TENDON ARE PARALLEL. THE GROUND ANCHOR SHALL NOT BE DRILLED IN A LOCATION THAT REQUIRES THE TENDON TO BE BENT IN ORDER TO CONNECT THE BEARING PLATE TO THE SUPPORTED

I. THREADBARS CONFORMING TO ASTM A615 / AASHTO M31, GRADE 60 OR 75 OR

2. EPOXY COATING, WHERE REQUIRED ON THE PLANS, SHALL CONFORM TO AASHTO M284, MINIMUM 12 MIL ELECTROSTATICALLY APPLIED, BEND TEST REQUIREMENTS

SHALL BE WAIVED. 3. FOR DAMAGED EPOXY COATED NAILS, THE COATING SHALL BE REPAIRED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS USING AN EPOXY FIELD REPAIR KIT APPROVED BY THE EPOXY MANUFACTURER.

4B. BAR COUPLERS ARE ALLOWED.

5. CORROSION PROTECTION:

5A. THE BONDBREAKER SHALL BE FABRICATED FROM A SMOOTH PLASTIC TUBE OR PIPE HAVING THE FOLLOWING PROPERTIES:

- I. RESISTANCE TO CHEMICAL ATTACK FROM AGGRESSIVE ENVIRONMENTS, GROUT OR GREASE.
- RESISTANCE TO AGING BY ULTRAVIOLET LIGHT.
- 3. FABRICATED FROM MATERIAL NON-DETRIMENTAL TO THE TENDON. 4. CAPABLE OF WITHSTANDING ABRASION, IMPACT, AND BENDING DURING HANDLING AND INSTALLATION.
- 5. ENABLE THE TENDON TO ELONGATE DURING TESTING AND STRESSING. 6. ALLOW THE TENDON TO REMAIN UNBONDED AFTER LOCKOFF.

5B. CORROSION PROTECTION SHALL BE EPOXY COATING. EPOXY COATING SHALL CONFORM TO AASHTO M284, MINIMUM 12 MIL ELECTROSTATICALLY APPLIED, BEND TEST REQUIREMENTS SHALL BE WAIVED.

6. SPACERS AND CENTRALIZERS:

6A. SPACERS SHALL BE USED ALONG THE TENDON BOND LENGTH OF MULTI-ELEMENT TENDONS TO SEPARATE EACH OF THE INDIVIDUAL ELEMENTS OF THE TENDON SO THE PRESTRESSING STEEL WILL BOND TO THE GROUT. SPACERS SHALL BE POSITIONED SO THEIR CENTER-TO-CENTER SPACING DOES NOT EXCEED 10 FEET. IN ADDITION, THE UPPER SPACER SHALL BE LOCATED A MAXIMUM OF 5 FEET FROM THE TOP OF THE TENDON BOND LENGTH AND THE LOWER SPACER SHALL BE LOCATED A MAXIMUM OF 5 FEET FROM THE BOTTOM OF THE TENDON BOND LENGTH. SPACERS SHALL PERMIT GROUT TO FREELY FLOW UP THE DRILLHOLE OR BOND LENGTH ENCAPSULATION. SPACERS SHALL BE FABRICATED FROM PLASTIC.

6B. CENTRALIZERS SHALL PERMIT FREE GROUT FLOW AND SHALL PROVIDE A MINIMUM OF 0.5 INCHES OF COVER OVER THE TENDON BOND LENGTH ENCAPSULATION. CENTRALIZERS SHALL BE SECURELY ATTACHED TO THE ENCAPSULATION AND THE CENTER TO CENTER SPACING SHALL NOT EXCEED 10 FEET. THE UPPER CENTRALIZER SHALL BE LOCATED A MAXIMUM OF 5 FEET FROM THE TOP OF THE TENDON BOND LENGTH AND THE LOWER CENTRALIZER SHALL BE LOCATED A MAXIMUM OF 3 FEET FROM THE BOTTOM OF THE TENDON BOND LENGTH. CENTRALIZERS SHALL BE FABRICATED FROM PLASTIC.

7. ANCHOR TESTING:

8A. EACH GROUND ANCHOR SHALL BE TESTED. THE MAXIMUM TEST LOAD SHALL NOT EXCEED 80% OF THE MINIMUM GUARANTEED ULTIMATE TENSILE STRENGTH (GUTS) OF THE TENDON. THE TEST LOAD SHALL BE SIMULTANEOUSLY APPLIED TO THE ENTIRE TENDON. STRESSING OF SINGLE ELEMENTS OF MULTI-ELEMENT TENDONS WILL NOT BE PERMITTED.

7B. THE TESTING EQUIPMENT SHALL CONSIST OF:

- I. A DIAL GAUGE OR VERNIER SCALE CAPABLE OF MEASURING TO 0.001 INCHES SHALL BE USED TO MEASURE THE GROUND ANCHOR MOVEMENT. THE MOVEMENT-MEASURING DEVICE SHALL HAVE A MINIMUM TRAVEL EQUAL TO THE THEORETICAL ELASTIC ELONGATION OF THE TOTAL ANCHOR LENGTH AT THE MAXIMUM TEST LOAD PLUS I INCH. THE DIAL GAUGE OR VERNIER SCALE SHALL BE SUPPORTED INDEPENDENT OF THE JACKING SYSTEM AND RETAINED STRUCTURE AND SHALL BE ALIGNED SO THAT ITS AXIS IS WITHIN 5 DEGREES FROM THE AXIS OF THE GROUND ANCHOR.
- 2. A HYDRAULIC JACK AND PUMP SHALL BE USED TO APPLY THE TEST LOAD. THE JACK AND PRESSURE GAUGE SHALL BE CALIBRATED BY AN INDEPENDENT TESTING LABORATORY AS A UNIT. THE PRESSURE GAUGE SHALL BE GRADUATED IN 100 PSI INCREMENTS OR LESS. THE PRESSURE GAUGE WILL BE USED TO MEASURE THE APPLIED LOAD. THE RAM TRAVEL OF THE JACK SHALL NOT BE LESS THAN THE THEORETICAL ELASTIC ELONGATION OF THE TOTAL ANCHOR LENGTH AT THE MAXIMUM TEST LOAD PLUS ONE INCH. THE JACK SHALL BE INDEPENDENTLY SUPPORTED AND CENTERED OVER THE TENDON SO THAT THE TENDON DOES NOT CARRY THE WEIGHT OF THE JACK.

PERMANENT GROUND ANCHORS (CONT.):

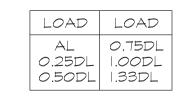
7C. PERFORMANCE TESTING SHALL BE PERFORMED ON 5 PERCENT OF THE GROUND ANCHORS OR A MINIMUM OF 3 ANCHORS, WHICHEVER IS GREATER. THE PERFORMANCE TEST SHALL BE MADE BY LOADING AND UNLOADING THE GROUND ANCHOR IN ACCORDANCE WITH THE FOLLOWING SCHEDULE. AT LOAD INCREMENTS OTHER THAN THE MAXIMUM TEST LOAD. THE LOAD SHALL BE HELD LONG ENOUGH TO OBTAIN A STABLE READING.

LOAD	HOLD TIME	LOAD	HOLD TIME	
AL 0.25DL 0.50DL 0.75DL 1.00DL 1.25DL 1.50DL 1.75DL 2.00DL	I MINUTE IO MINUTES IO MINUTES IO MINUTES IO MINUTES IO MINUTES IO MINUTES IO MINUTES IO MINUTES	I.75DL I.50DL I.25DL I.00DL 0.75DL 0.50DL 0.25DL AL	UNTIL STABLE UNTIL STABLE UNTIL STABLE UNTIL STABLE UNTIL STABLE UNTIL STABLE UNTIL STABLE UNTIL STABLE	AL = ALIGNMENT LOAD DL = DESIGN LOAD

THE ALIGNMENT LOAD (AL) SHALL BE THE MINIMUM LOAD REQUIRED TO ALIGN THE TESTING APPARATUS AND SHALL NOT EXCEED 0.05DL. DIAL GAUGES SHALL BE SET AT "ZERO" AFTER THE ALIGNMENT LOAD HAS BEEN APPLIED.

THE MAXIMUM TEST LOAD SHALL BE HELD FOR 10 MINUTES. THE LOAD-HOLD PERIOD SHALL START AS SOON AS THE MAXIMUM TEST LOAD IS APPLIED AND THE ANCHOR MOVEMENT SHALL BE MEASURED AND RECORDED AT 1, 2, 3, 5, 6, AND 10 MINUTES. IF THE ANCHOR MOVEMENT BETWEEN I AND IO MINUTES EXCEEDS 0.04 INCHES, THE MAXIMUM TEST LOAD SHALL BE HELD AN ADDITIONAL 50 MINUTES. IF THE LOAD HOLD IS EXTENDED, THE ANCHOR MOVEMENTS SHALL BE RECORDED AT 20, 30, 50, AND 60 MINUTES. IF AN ANCHOR FAILS IN CREEP, RETESTING WILL NOT BE ALLOWED.

7D. PROOF TESTS SHALL BE PERFORMED ON ALL PRODUCTION ANCHORS BY INCREMENTALLY LOADING THE GROUND ANCHOR IN ACCORDANCE WITH THE FOLLOWING SCHEDULE. AT LOAD INCREMENTS OTHER THAN MAXIMUM TEST LOAD, THE LOAD SHALL BE HELD LONG ENOUGH TO OBTAIN A STABLE READING.



I. THE GROUND ANCHOR CARRIES THE MAXIMUM TEST LOAD WITH LESS THAN 0.04 INCHES OF MOVEMENT BETWEEN THE I AND IO MINUTE READINGS. 2. THE TOTAL MOVEMENT AT THE MAXIMUM TEST LOAD EXCEEDS 80% OF THE THEORETICAL ELASTIC ELONGATION OF THE UNBONDED LENGTH.

7G. UPON SUCCESSFUL COMPLETION OF THE TEST, THE LOAD SHALL BE ADJUSTED TO THE SPECIFIED LOCK-OFF LOAD. AFTER TRANSFERRING THE LOAD TO THE ANCHORAGE DEVICE AND PRIOR TO REMOVING THE JACK, A LIFT-OFF READING SHALL BE MADE. THE LIFT-OFF READING SHALL BE WITHIN 10 PERCENT OF THE SPECIFIED LOCK-OFF LOAD.

7H. GROUND ANCHORS THAT HAVE A CREEP RATE GREATER THAN SPECIFIED CAN BE INCORPORATED IN THE FINISHED WORK AT A LOAD EQUAL TO ONE-HALF OF THE FAILURE LOAD. THE FAILURE LOAD IS THE MAXIMUM LOAD CARRIED BY THE ANCHOR AFTER THE LOAD HAS BEEN ALLOWED TO STABILIZE FOR TEN MINUTES.

71. WHEN A GROUND ANCHOR FAILS, THE CONTRACTOR SHALL MODIFY THE ANCHOR DESIGN, THE CONSTRUCTION PROCEDURES, OR BOTH. THESE MODIFICATIONS MAY INCLUDE, BUT ARE NOT LIMITED TO: INSTALLING REPLACEMENT GROUND ANCHORS, MODIFYING THE INSTALLATION METHODS, INCREASING THE BOND LENGTH, OR CHANGING THE GROUND ANCHOR TYPE. ANY MODIFICATION WHICH REQUIRES CHANGES TO THE STRUCTURE SHALL HAVE PRIOR APPROVAL OF THE ENGINEER.

IN ACCORDANCE WITH SECTION 1704 OF IBC (2015), SPECIAL INSPECTION IS REQUIRED FOR THE FOLLOWING SHORING ITEMS OR PROCESSES: SOIL NAIL INSTALLATION AND TESTING, SOLDIER PILE INSTALLATION, AND GROUND ANCHOR INSTALLATION AND TESTING.

PER THE REQUIREMENTS OF THE GEOTECHNICAL SPECIAL INSPECTOR, THE SHORING MONITORING PROGRAM SHALL CONSIST OF THE FOLLOWING:

• PRE-CONSTRUCTION SURVEY (VIDEO OR PHOTOGRAPHIC SURVEY) OF ADJACENT STREETS, UTILITIES, BUILDINGS, AND OTHER STRUCTURES.

• OPTICAL SURVEY OF MONITORING POINTS TO BE COMPLETED TWICE WEEKLY DURING CONSTRUCTION, AND AT LEAST EVERY OTHER WEEK (OR AS DETERMINED BY THE GEOTECHNICAL SPECIAL INSPECTOR) FOLLOWING COMPLETION OF THE EXCAVATION AND BEFORE THE INTERIOR BUILDING FLOORS REACH THE GROUND SURFACE. MONITORING SHALL INCLUDE VERTICAL AND HORIZONTAL SURVEY MEASUREMENTS TO AN ACCURACY OF O.OI FEET. BASELINE READINGS ARE TO BE TAKEN PRIOR TO THE START OF CONSTRUCTION. ALL RESULTS ARE TO BE SENT TO THE GEOTECHNICAL SPECIAL INSPECTOR WITHIN 24 HOURS. A LICENSED SURVEYOR (NOT THE CONTRACTOR) SHALL PERFORM THE MONITORING AT LEAST ONCE PER WEEK.

- ENGINEER.

THE ALIGNMENT LOAD (AL) SHALL BE THE MINIMUM LOAD REQUIRED TO ALIGN THE TESTING APPARATUS AND SHALL NOT EXCEED 0.05DL. DIAL GAUGES SHALL BE SET AT "ZERO" AFTER THE ALIGNMENT LOAD HAS BEEN APPLIED.

THE MAXIMUM TEST LOAD SHALL BE HELD FOR IO MINUTES. THE LOAD-HOLD PERIOD SHALL START AS SOON AS THE MAXIMUM TEST LOAD IS APPLIED AND THE ANCHOR MOVEMENT SHALL BE MEASURED AND RECORDED AT 1, 2, 3, 5, 6, AND 10 MINUTES. IF THE ANCHOR MOVEMENT BETWEEN I AND IO MINUTES EXCEEDS 0.04 INCHES, THE MAXIMUM TEST LOAD SHALL BE HELD OF AN ADDITIONAL 50 MINUTES. IF THE LOAD HOLD IS EXTENDED, THE ANCHOR MOVEMENTS SHALL BE RECORDED AT 20, 30, 50, AND 60 MINUTES. IF AN ANCHOR FAILS IN CREEP, RETESTING WILL NOT BE ALLOWED.

7E. A PERFORMANCE OR PROOF TESTED GROUND ANCHOR WITH A 10 MINUTE LOAD HOLD CREEP TEST IS CONSIDERED ACCEPTABLE WHEN:

7F. A PERFORMANCE OR PROOF TESTED GROUND ANCHOR WITH A 60 MINUTE LOAD HOLD CREEP TEST IS CONSIDERED ACCEPTABLE WHEN:

THE GROUND ANCHOR CARRIES THE MAXIMUM TEST LOAD WITH LESS THAN 0.08 INCHES OF MOVEMENT PER LOG CYCLE OF TIME AND THE CREEP RATE IS LINEAR OR DECREASING.

2. THE TOTAL MOVEMENT AT THE MAXIMUM TEST LOAD EXCEEDS 80% OF THE THEORETICAL ELASTIC ELONGATION OF THE UNBONDED LENGTH.

SPECIAL INSPECTION OF THE SHORING WALLS:

MONITORING:

• OPTICAL SURVEY POINTS SHALL BE ESTABLISHED AT THE TOP OF THE SHORING WALL AROUND THE PERIMETER OF THE EXCAVATION ON EVERY OTHER PILE ALONG THE LENGTH OF THE WALL.

 ADDITIONAL SURVEY POINTS SHALL BE ESTABLISHED ALONG THE CURBLINES AND CENTERLINES OF ADJACENT ROADWAYS, AND ON SETTLEM, ENT-SENSITIVE STRUCTURES, AND AT DISTANCES UP TO AT LEAST THE WALL HEIGHT ON PRIVATE PROPERTY ADJACENT THE EXCAVATION, AND SPACED AT 20 FEET HORIZONTALLY. THESE POINTS NEED BE MONITORED IF SHORING WALL MOVEMENTS EXCEED 0.5-INCH.

• SURVEY FREQUENCY CAN BE DECREASED AFTER THE SHORING SYSTEM HAS BEEN INSTALLED AND EXCAVATION IS COMPLETE IF THE DATA INDICATES LITTLE OR NO ADDITIONAL MOVEMENT. SURVEYING MUST CONTINUE UNTIL THE PERMANENT STRUCTURE (INCLUDING FLOOR SLABS AS BRACES) IS COMPLETE UP TO FINAL AND STREET GRADES. THE SURVEY FREQUENCY WILL BE DETERMINED BY THE GEOTECHNICAL

• THE GEOTECHNICAL ENGINEER SHALL REVIEW SURVEY DATA AND PROVIDE AN EVALUATION OF WALL PERFORMANCE ALONG WITH SURVEY DATA TO THE SHORING ENGINEER ON AT LEAST A WEEKLY BASIS. IMMEDIATELY AND DIRECTLY, NOTIFY THE SHORING ENGINEER IF ANY UNUSUAL OR SIGNIFICANTLY INCREASED MOVEMENT OCCURS.

 IMMEDIATELY AND DIRECTLY NOTIFY THE GEOTECHNICAL AND STRUCTURAL ENGINEERS, AND WALL DESIGNER IF 0.5 INCHES OF MOVEMENT OCCURS BETWEEN TWO CONSECUTIVE READINGS OR WHEN TOTAL MOVEMENTS REACH 0.5 INCH. AT THAT AMOUNT OF MOVEMENT, THE ENGINEERS AND DESIGNERS SHALL DETERMINE THE CAUSE OF DISPLACEMENT AND DEVELOP REMEDIAL MEASURES SUFFICIENT TO LIMIT TOTAL WALL MOVEMENTS TO I INCH. ALL EARTHWORK AND CONSTRUCTION ACTIVITIES MUST BE DIRECTED TOWARDS IMMEDIATE IMPLEMENTATION OF REMEDIAL MEASURES NECESSARY TO LIMIT TOTAL WALL MOVEMENTS TO WHAT HAS BEEN DEFINED AS ACCEPTABLE BY THE DESIGN TEAM.

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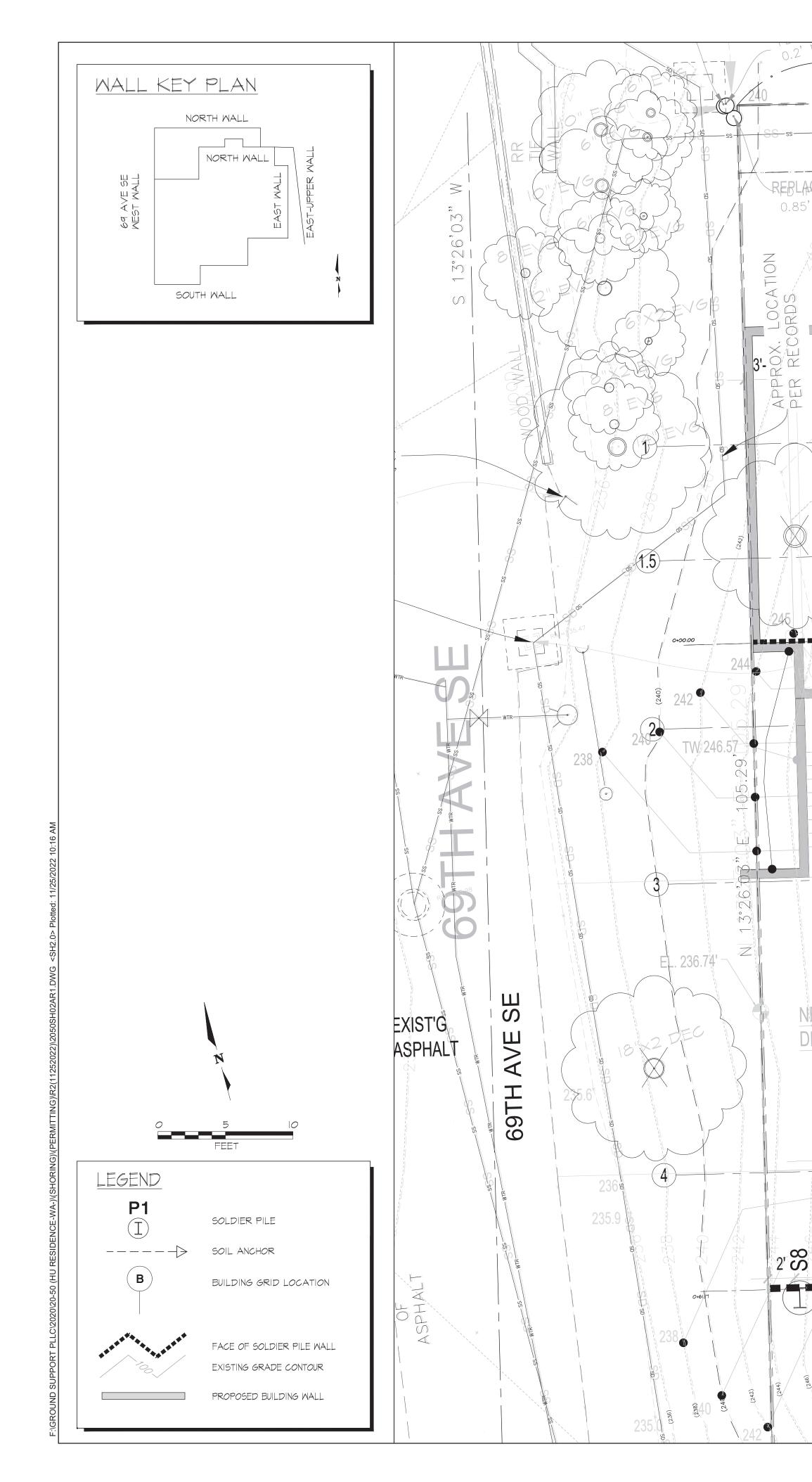
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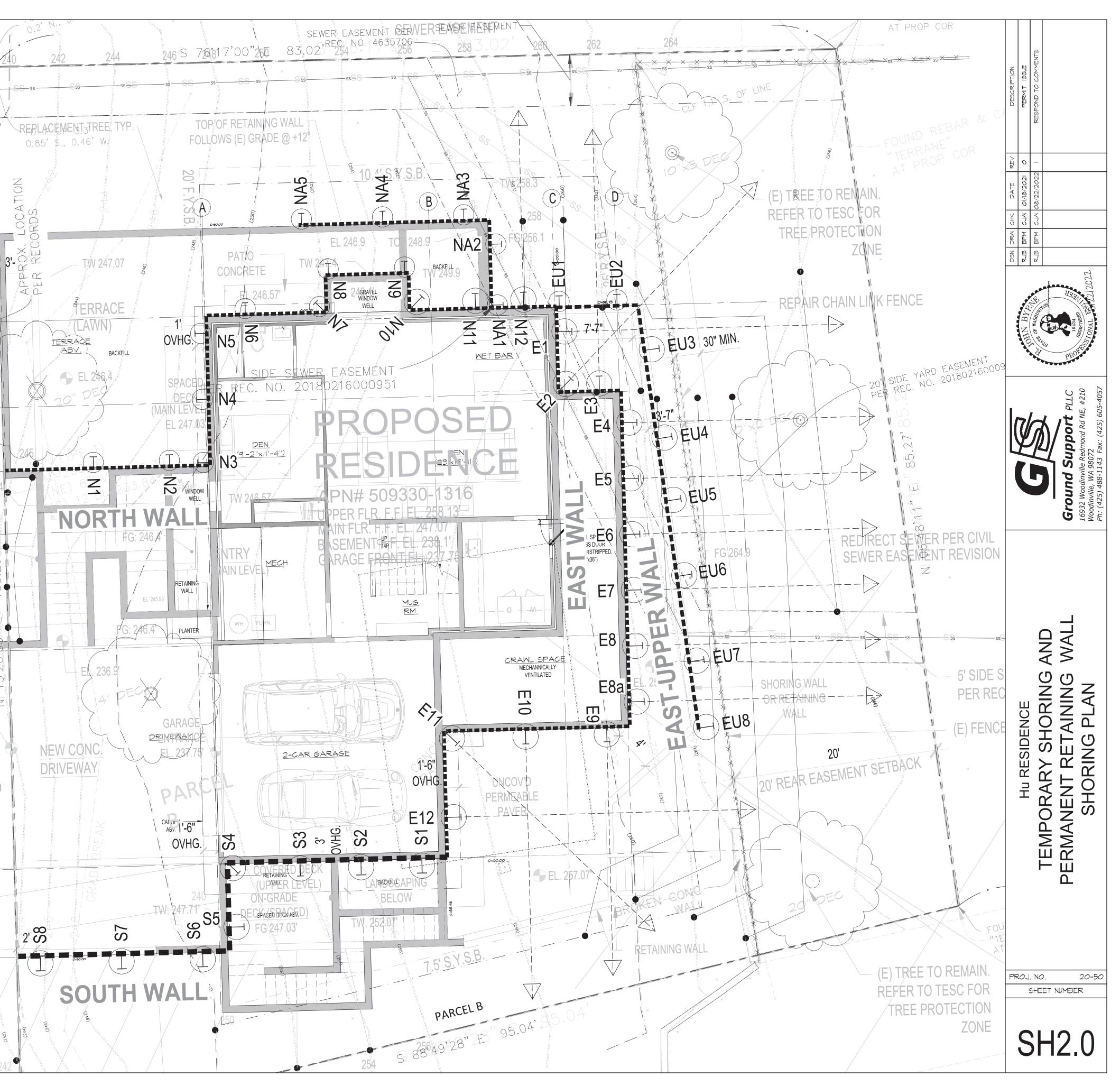
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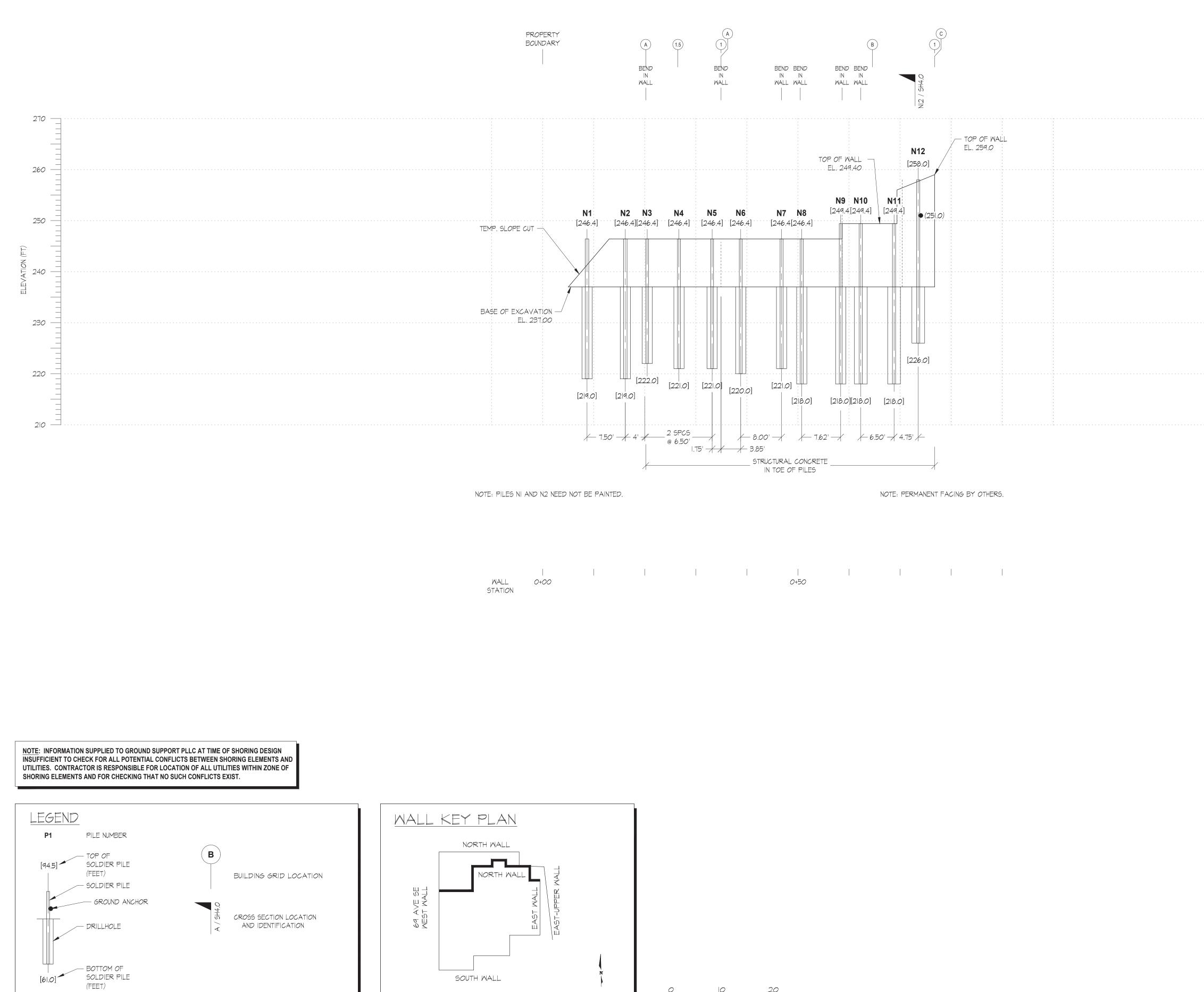
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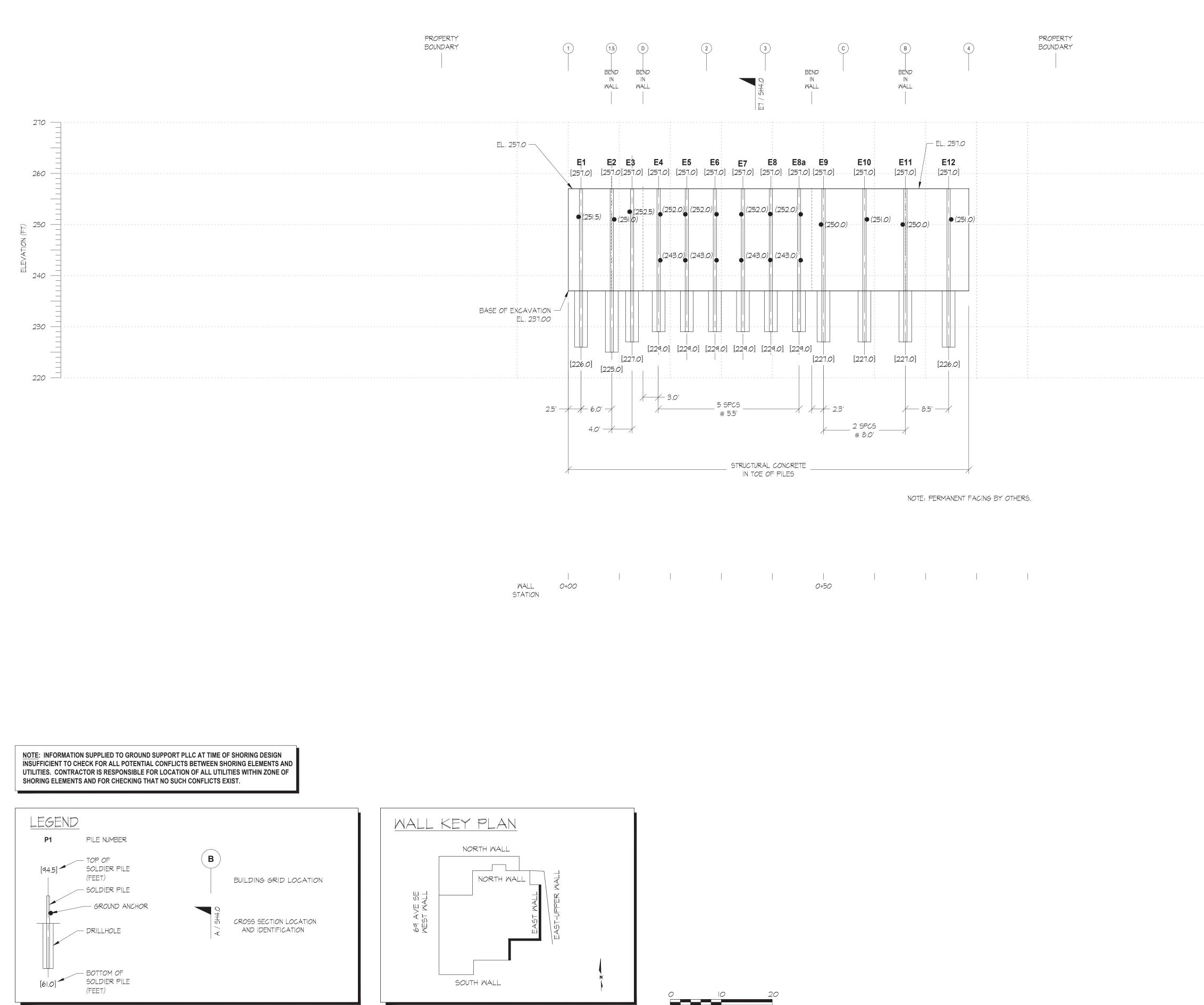






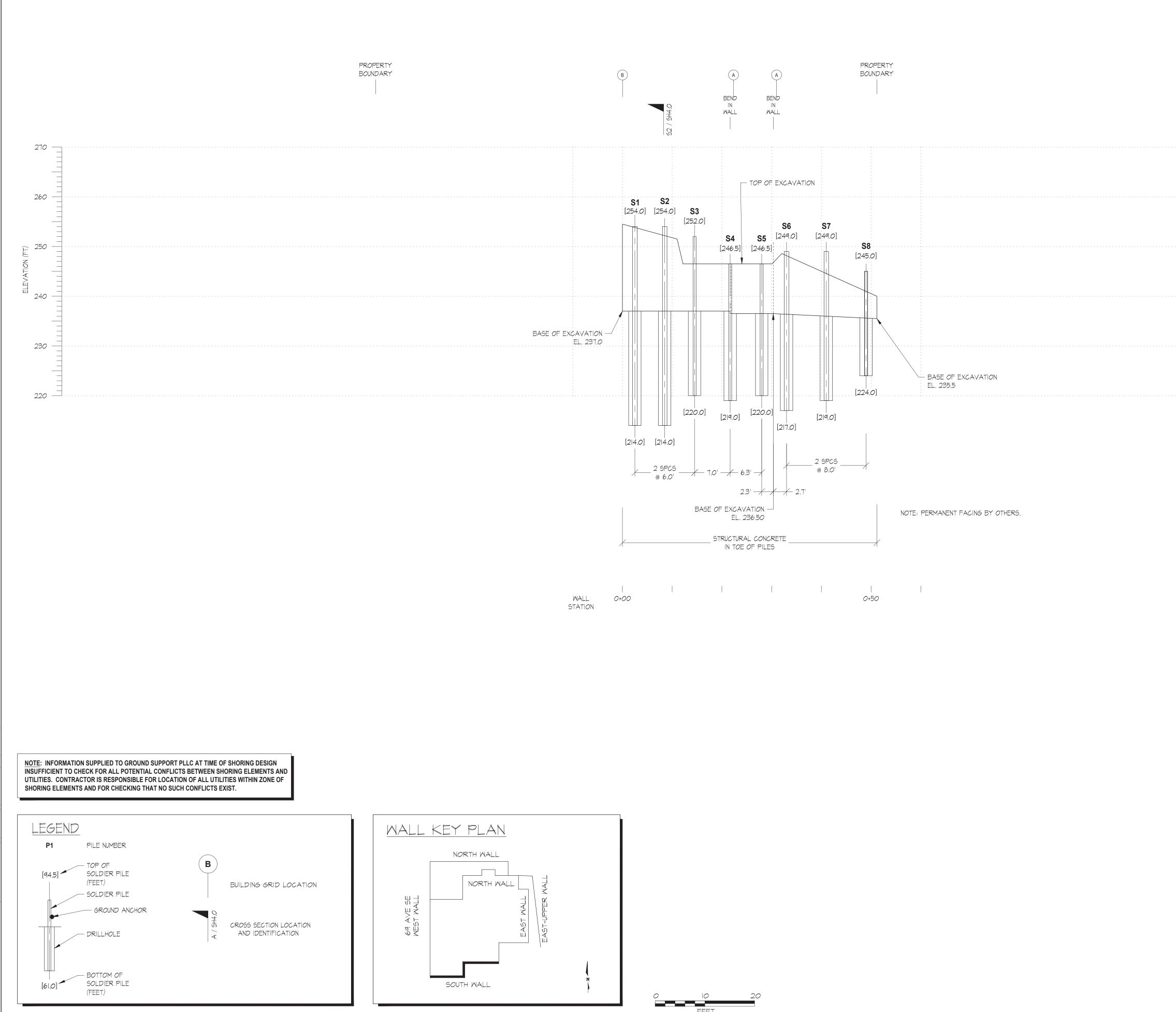
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- 220	Gound Support PLLC 16932 Woodinville Redmond Rd NE, #210 Woodinville, WA 98072 Ph: (425) 488-1143 Fax: (425) 605-4057
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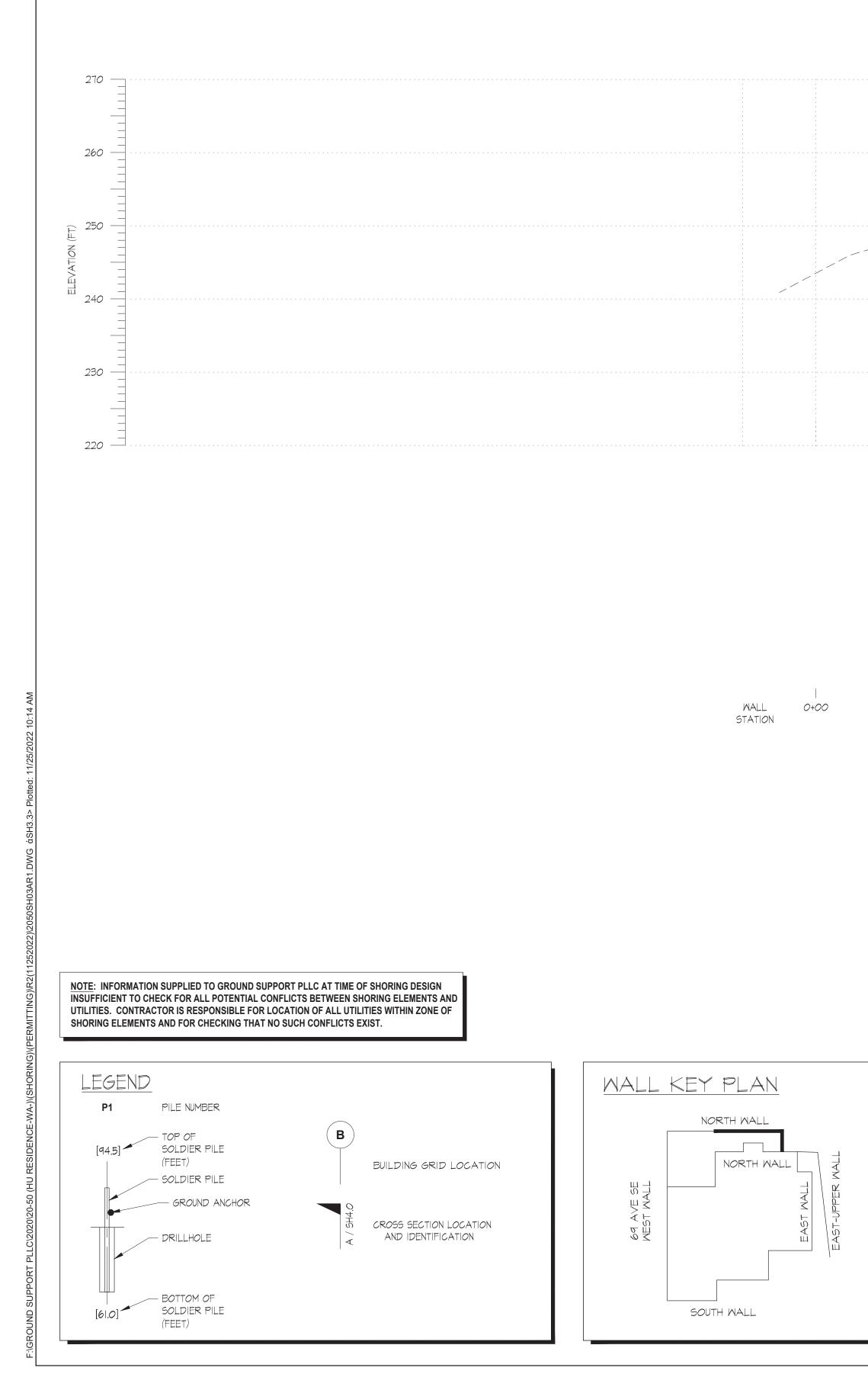
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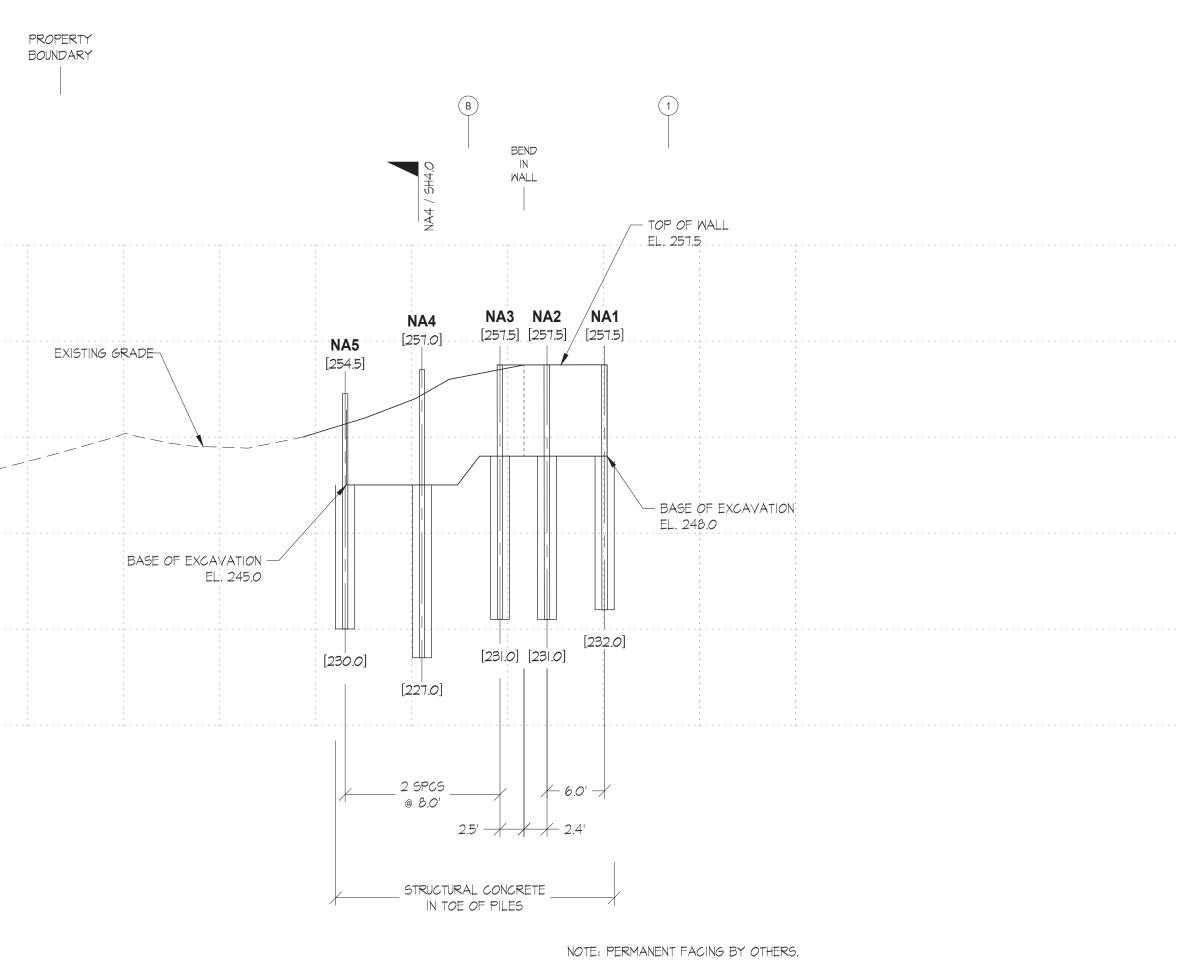
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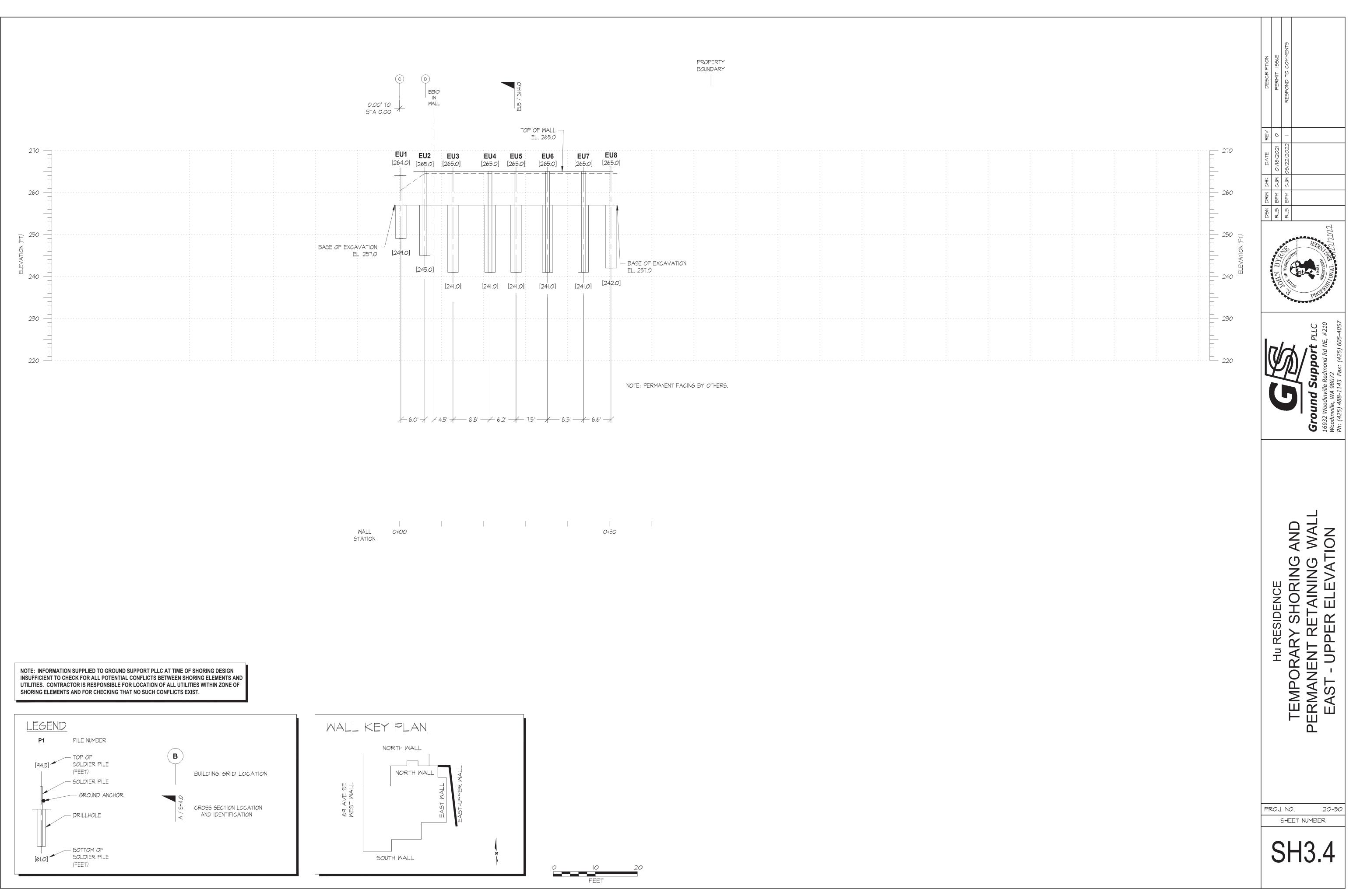
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PILE NUMBER	WALL STA	STEEL SECTION	PILE TOP ELEV (FT)	PILE BOT ELEV (FT)	PILE LENGTH (FT)	MIN. DRILL- HOLE DIA (FT)	ANCHOR ELEV (FT)	DECLI- NATION (DEG)	TOTAL LENGTH (FT)	UNBOND LENGTH (FT)	BOND LENGTH (FT)	
NI	0+07.5	WI4X48	246.4	219.0	27.4	2.0	-	-	-	-	-	
N2	0+15.5	WI4X48	246.4	219.0	27.4	2.0	-	-	-	-	-	
N3	0+19.5	WI4X38	246.4	222.0	24.4	2.0	-	-	-	-	-	
N4	0+26.0	WI4X38	246.4	.221.0	25.4	2.0	-	-	-	-	-	
N5	0+32.5	WI4X38	246.4	.221.0	25.4	2.0	-	-	-	-	-	
N6	0+38.1	WI4X38	246.4	220.0	26.4	2.0	-	-	-	-	-	
N7	0+46.1	WI4X38	246.4	.221.0	25.4	2.0	-	-	-	-	-	
NB	0+50.0	WI4X48	246.4	218.0	28.4	2.0	-	-	-	-	-	
N9	0+57.6	WI4X48	249.4	218.0	31.4	2.0	-	-	-	-	-	
NIO	0+61.5	WI8X50	249.4	218.0	31.4	2.5	-	-	-	-	-	
NII	0+68.0	WI8X55	249.4	218.0	31.4	2.5	-	-	-	-	-	
NI2	0+72.8	WI8X50	258.0	226.0	32.0	2.5	.251.0	45.0	27.5	11.5	16	

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										ANCHOR								ANCHOR 2				
PILE NUMBER	WALL STA	STEEL SECTION	PILE TOP ELEV (FT)	PILE BOT ELEV (FT)	PILE LENGTH (FT)	MIN. DRILL- HOLE DIA (FT)	ANCHOR ELEV (FT)	DECLI- NATION (DEG)	TOTAL LENGTH (FT)	UNBOND LENGTH (FT)	BOND LENGTH (FT)	BAR SIZE	DESIGN LOAD (K)	LOCKOFF LOAD (K)	ANCHOR ELEV (FT)	DECLI- NATION (DEG)	TOTAL LENGTH (FT)	UNBOND LENGTH (FT)	BOND LENGTH (FT)	BAR SIZE	DESIGN LOAD (K)	LOCKOFF LOAD (K)
EI E2 E3	0+02.5 0+08.5 0+12.5	WI8X65 WI8X55 WI4X43	257.0 257.0 257.0	226.0 225.0 227.0	31.0 32.0 30.0	2.5 2.5 2.5	251.5 251.0 252.5	45.0 20.0 20.0	40.0 30.0 27.5	12.5 12.5 13.0	27.5 17.5 15.0	- /4-inch No. 0 No. 9	80.0 50.0 40.0	80.0 50.0 40.0								
E4 E5 E6	0+17.7 0+23.2 0+28.7	WI4X34 WI4X34 WI4X34	257.0 257.0 257.0 257.0	229.0 229.0 229.0 229.0	28.0 28.0 28.0 28.0	2.5 2.5 2.5 2.5	252.0 252.0 252.0	25.0 25.0 25.0 25.0	27.5 27.5 27.5 27.5	I2.5 I2.5 I2.5	15.0 15.0 15.0	No. 9 No. 9 No. 9	40.0 40.0 40.0	40.0 40.0 40.0	243.0 243.0 243.0	25.0 25.0 25.0	27.5 27.5 27.5	10.0 10.0 10.0	17.5 17.5 17.5	No. 10 No. 10 No. 10	50.0 50.0 50.0	50.0 50.0 50.0
E7 E8 E8a	0+34.2 0+39.7 0+45.2	WI4X34 WI4X34 WI4X34	257.0 257.0 257.0	229.0 229.0 229.0	28.0 28.0 28.0	2.5 2.5 2.5	252.0 252.0 252.0	25.0 25.0 25.0	27.5 27.5 27.5	2.5 2.5 2.5	15.0 15.0 15.0	No. 9 No. 9 No. 9	40.0 40.0 40.0	40.0 40.0 40.0	243.0 243.0 243.0	25.0 25.0 25.0	27.5 27.5 27.5	10.0 10.0 10.0	17.5 17.5 17.5	No. 10 No. 10 No. 10	50.0 50.0 50.0	50.0 50.0 50.0
E9 EIO EII	0+50.0 0+58.0 0+66.0	WI8X50 WI8X50 WI8X50	257.0 257.0 257.0	227.0 227.0 227.0	30.0 30.0 30.0	2.5 2.5 2.5	250.0 251.0 250.0	40.0 35.0 40.0	32.5 32.5 37.0	10.0 10.5 10.5	20.0 22.0 26.5	l-inch l-inch l-l/4-inch	60.0 65.0 80.0	60.0 65.0 80.0								
El2	0+74.5	WI8X50	257.0	226.0	31.0	2.5	251.0	20.0	32.5	12.5	20.0	l-inch	60.0	60.0	-	-	-	-	-	-	-	-

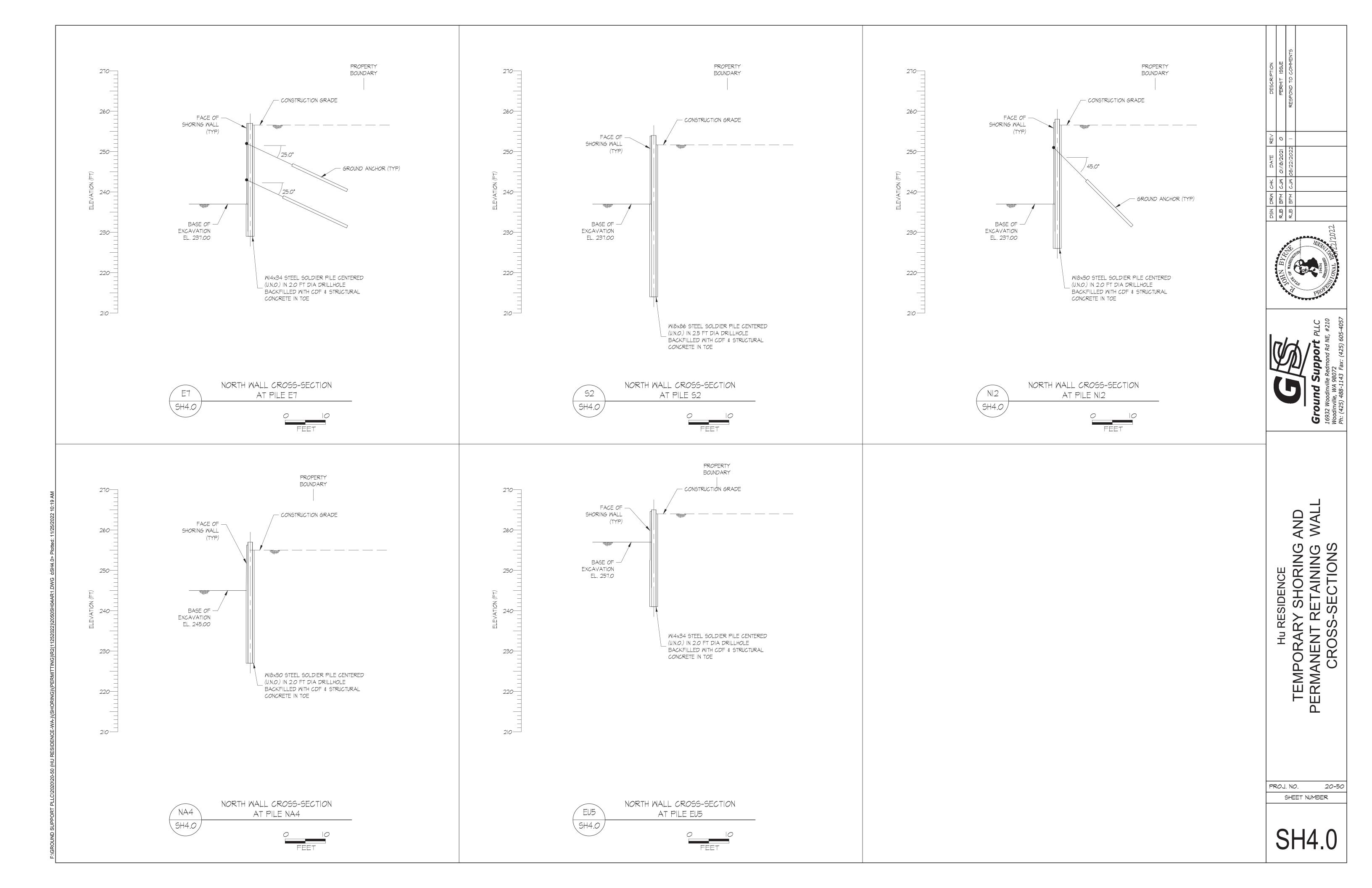
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PILE NUMBER	WALL STA	STEEL SECTION	PILE TOP ELEV (FT)	PILE BOT ELEV (FT)	PILE LENGTH (FT)	MIN. DRILL- HOLE DIA (FT)
SI	0+02.5	WI8X97	254.0	2 4.0	40.0	2.5
S2	0+08.5	WI8X86	254.0	2 4.0	40.0	2.5
S3	0+14.5	WI8X50	252.0	220.0	32.0	2.5
54	0+21.5	WI8X50	246.5	219.0	27.5	2.5
55	0+28.0	WI8X50	246.5	220.0	26.5	2.5
56	0+33.0	WI8X65	249.0	217.0	32.0	2.5
57	0+41.0	WI4X43	249.0	219.0	30.0	2.5
58	0+49.0	WI4X34	245.0	224.0	21.0	2.5

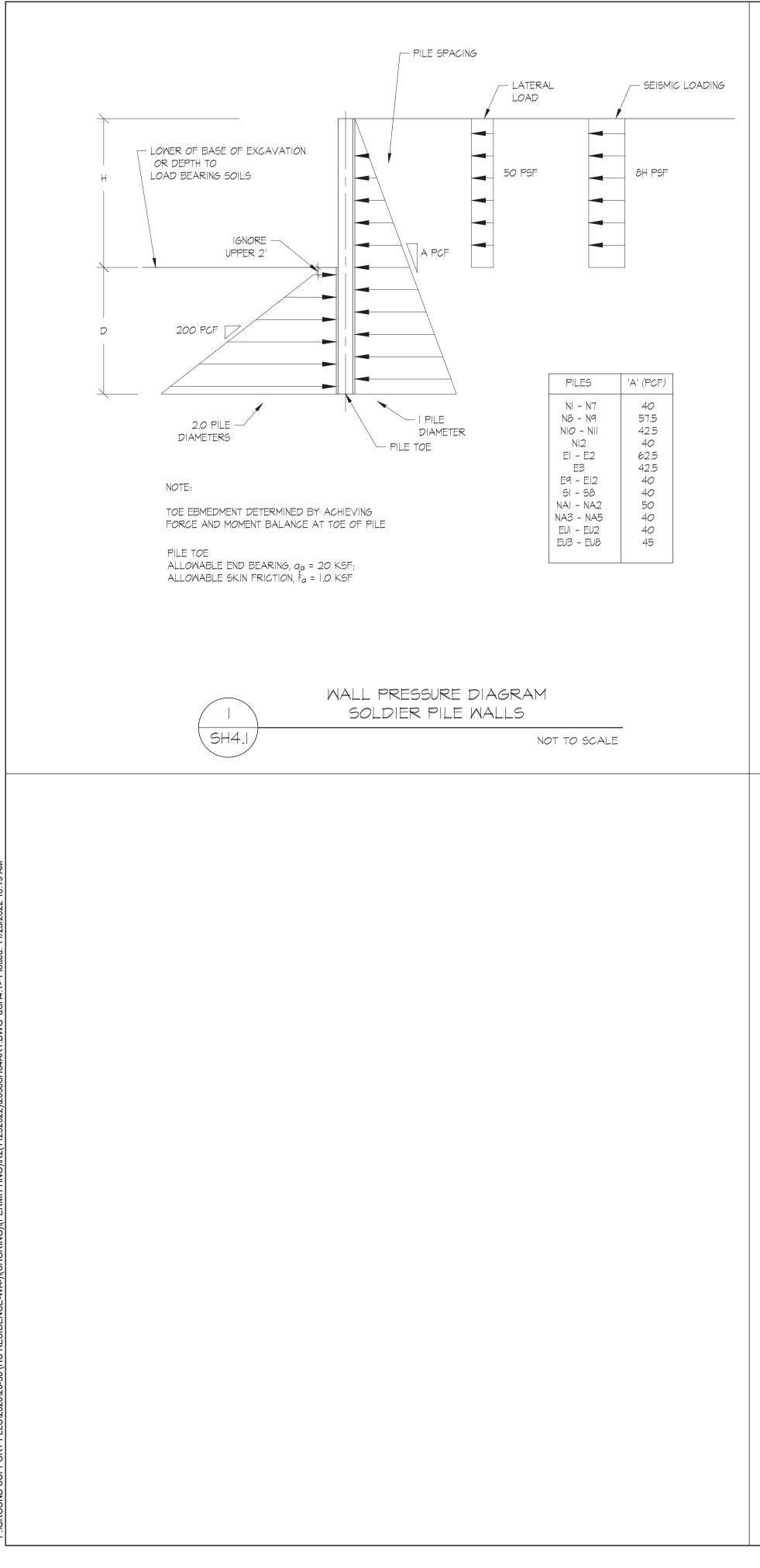
BAR SIZE	DESIGN LOAD (K)	LOCKOFF LOAD (K)
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EAST UPPER WALL - PILE SCHEDULE						
PILE NUMBER	WALL STA	STEEL SECTION	PILE TOP ELEV (FT)	PILE BOT ELEV (FT)	PILE LENGTH (FT)	MIN. DRILL- HOLE DIA (FT)
EUI EU2 EU3 EU4 EU5 EU6 EU7 EU8	0+00.3 0+06.0 0+12.3 0+21.1 0+27.3 0+34.8 0+43.3 0+49.9	MI4X34 MI4X34 MI4X34 MI4X34 MI4X34 MI4X34 MI4X34 MI4X34	264.0 265.0 265.0 265.0 265.0 265.0 265.0 265.0	249.0 245.0 241.0 241.0 241.0 241.0 241.0 241.0 242.0	15.0 20.0 24.0 24.0 24.0 24.0 24.0 23.0	2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0

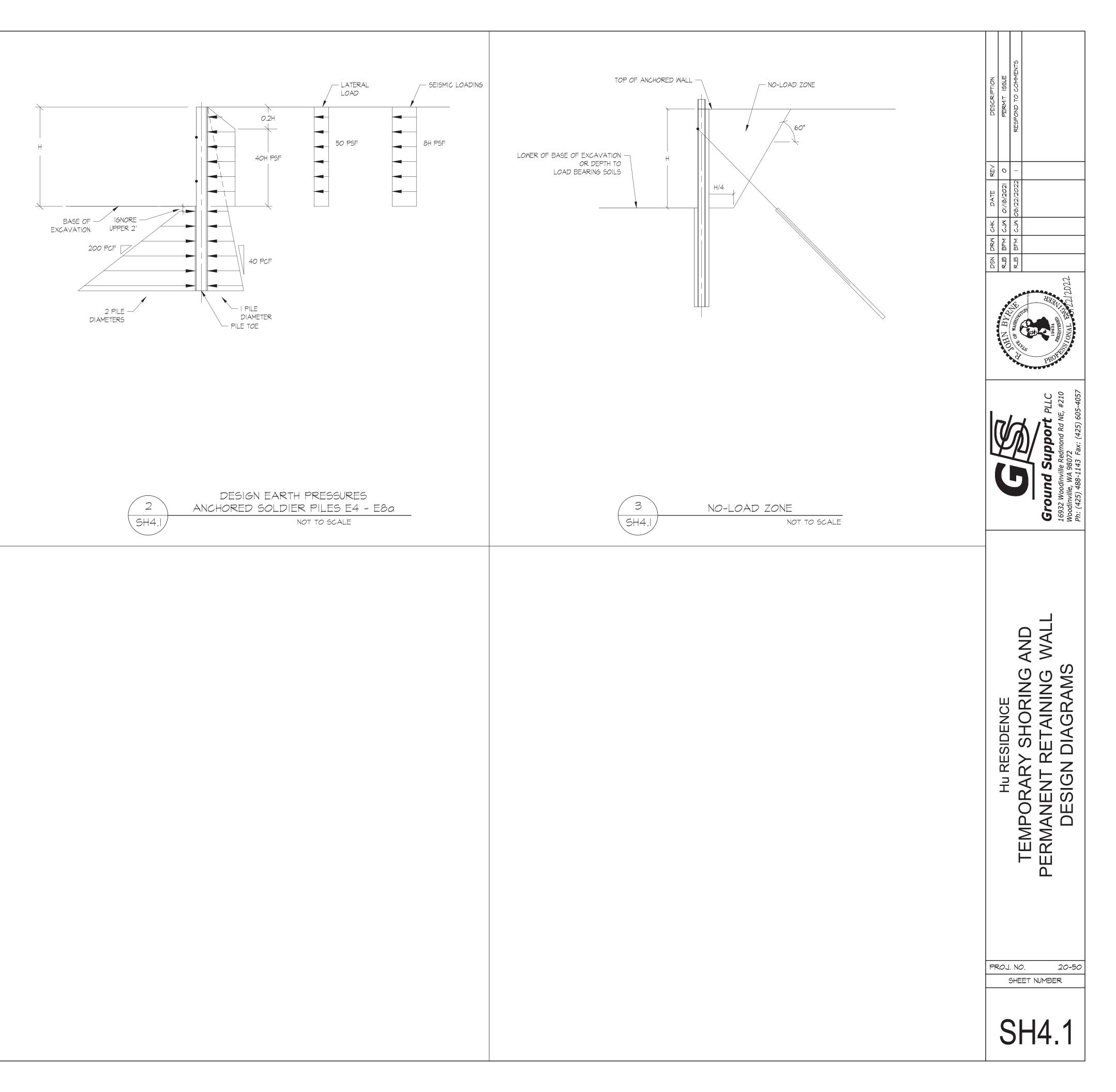
NOR	TH (00"	TSIDE ,			SCHEI	DULE
PILE NUMBER	WALL STA	STEEL SECTION	PILE TOP ELEV (FT)	PILE BOT ELEV (FT)	PILE LENGTH (FT)	MIN. DRILL- HOLE DIA (FT)
NAI NA2 NA3 NA4 NA5	0+70. 0+64. 0+59.2 0+51. 0+43.	WI4X34 WI4X38 WI4X38 WI8X50 WI8X50 WI4X34	257.5 257.5 257.5 257.0 254.5	232.0 231.0 231.0 231.0 227.0 230.0	25.5 26.5 26.5 30.0 24.5	2.0 2.0 2.0 2.0 2.0

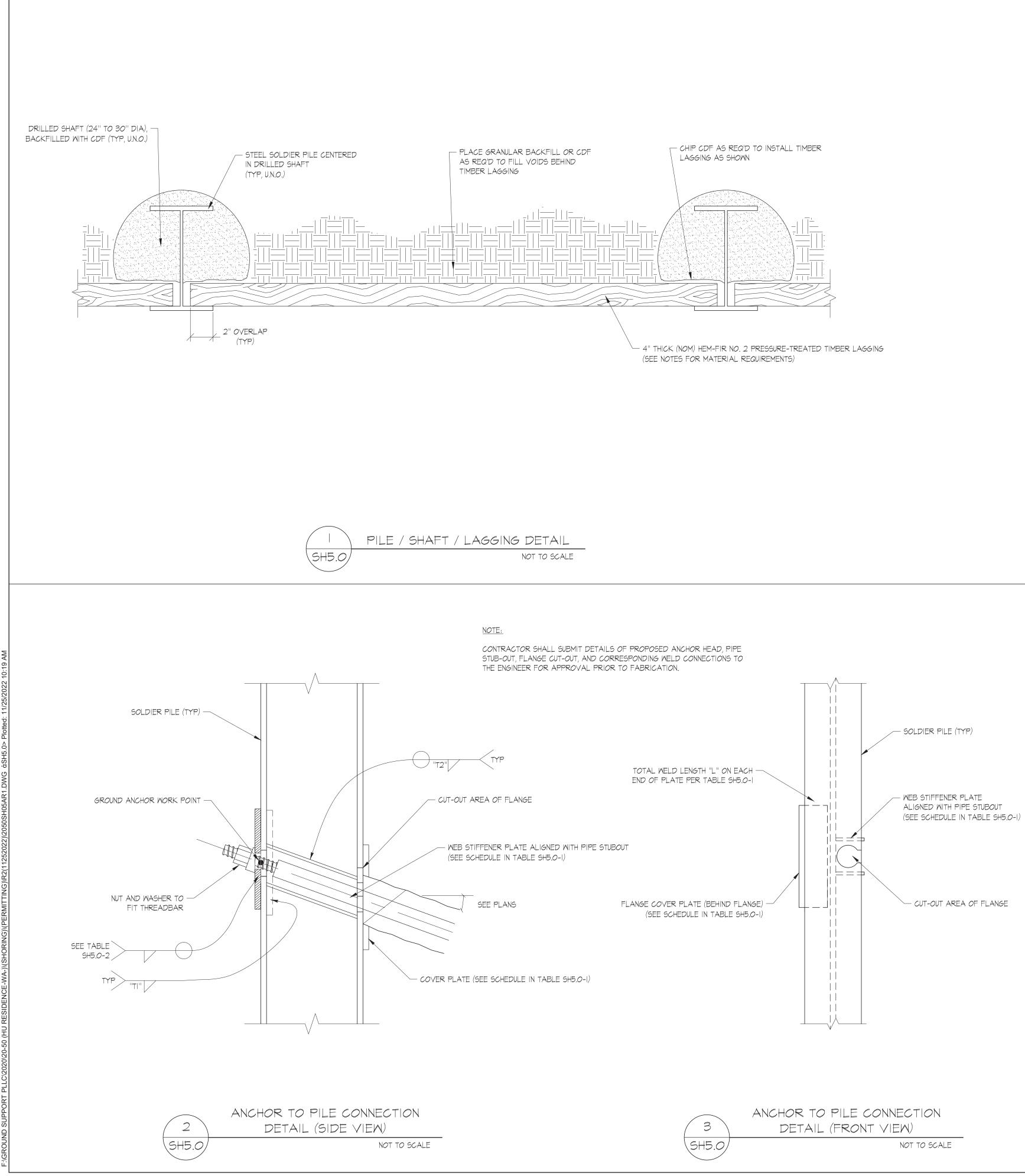


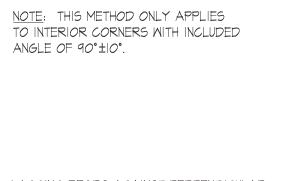




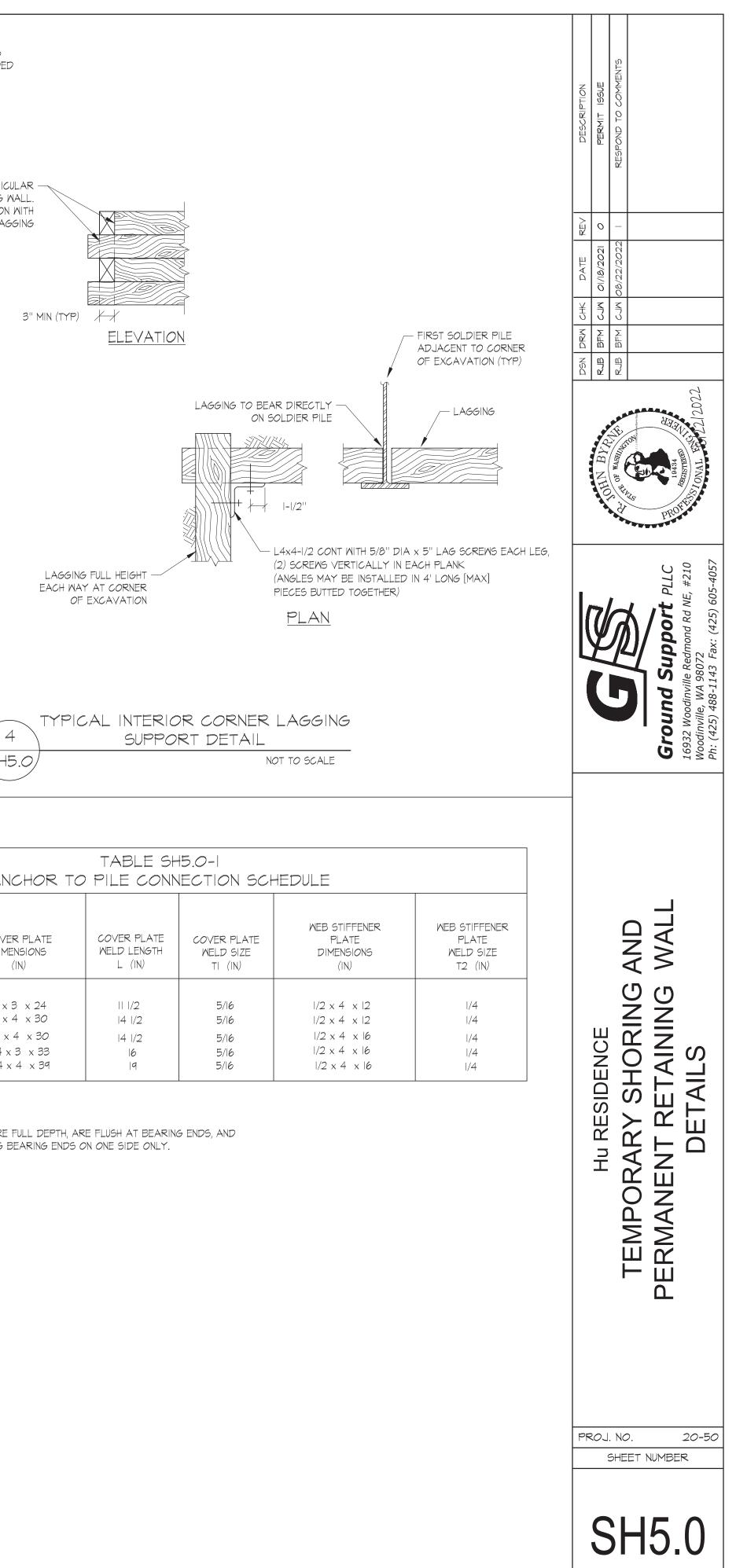
UND SUPPORT PLLC\2020\20-50 (HU RESIDENCE-WA-))(SHORING))(PERMITTING))R2(11252022))2050SH04AR1.DWG & SH4.1> Plotted: 11/25/2022 10:19







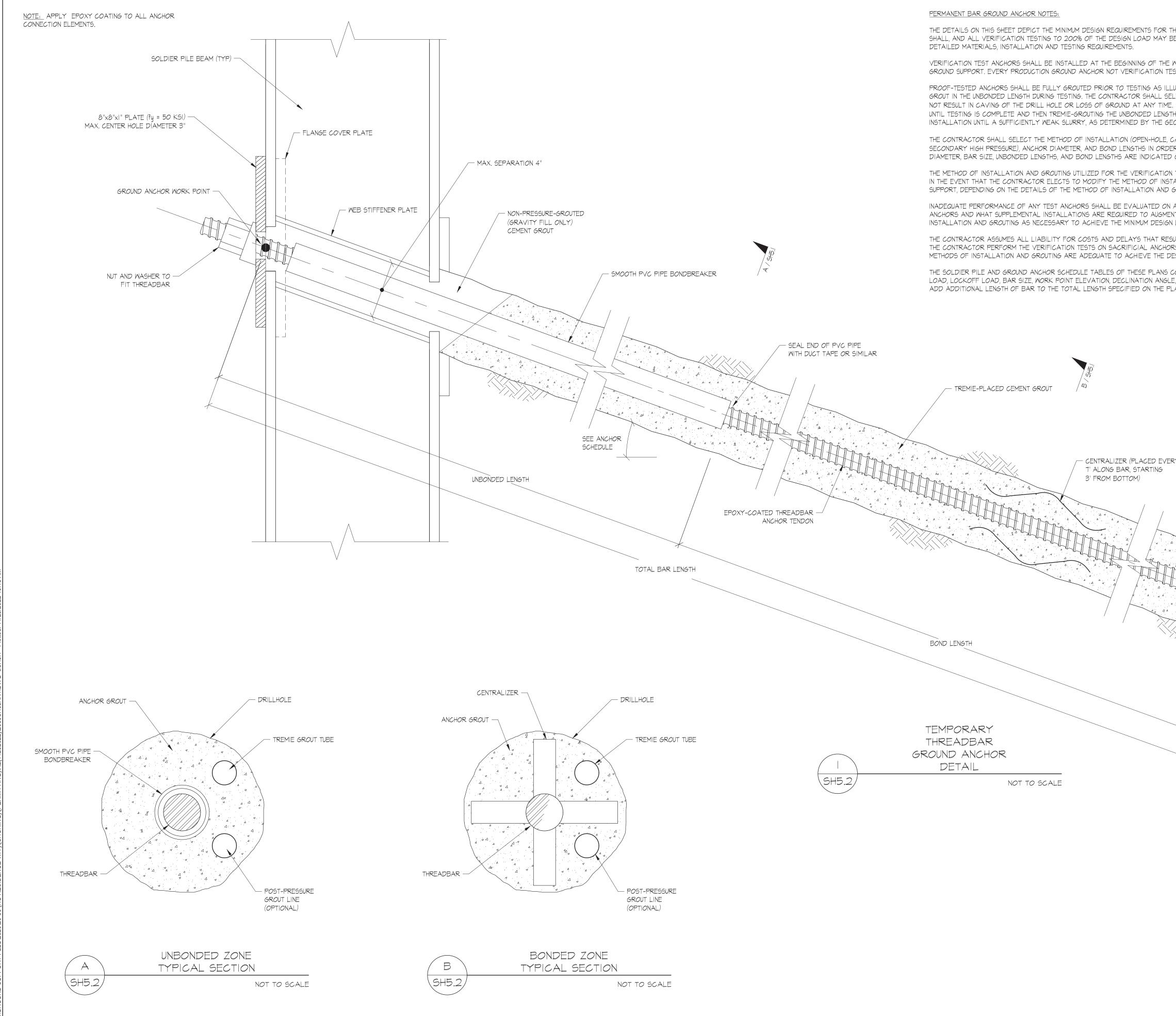
LAGGING BEARS AGAINST PERPENDICULAR -LAGGING ON ADJACENT SHORING WALL. ALTERNATE BEARING DIRECTION WITH EACH SUCCESSIVE LAYER OF LAGGING



4 \SH5.0

			_
		ANCHOR TO	
ANCHOR LOAD (KIPS)	PILE SECTI <i>O</i> N	COVER PLATE DIMENSIONS (IN)	
50 50 80 80 80	WI4x34 WI4x43 WI8x50 WI8x55 WI8x65	/2 x 3 x 24 /2 x 4 x 30 /2 x 4 x 30 3/4 x 3 x 33 3/4 x 4 x 39	

NOTE: WEB STIFFENER PLATES ARE FULL DEPTH, ARE FLUSH AT BEARING ENDS, AND WELDED FULL LENGTH AND ALONG BEARING ENDS ON ONE SIDE ONLY.



THE DETAILS ON THIS SHEET DEPICT THE MINIMUM DESIGN REQUIREMENTS FOR THE GROUND ANCHORS ON THIS PROJECT. ALL PROOF TESTING TO 133% OF THE DESIGN LOAD SHALL, AND ALL VERIFICATION TESTING TO 200% OF THE DESIGN LOAD MAY BE PERFORMED ON PRODUCTION GROUND ANCHORS. SEE THE NOTES ON SHEET SHI. FOR

VERIFICATION TEST ANCHORS SHALL BE INSTALLED AT THE BEGINNING OF THE WORK AT THE LOCATIONS SELECTED BY THE CONTRACTOR AND APPROVED IN WRITING BY GROUND SUPPORT. EVERY PRODUCTION GROUND ANCHOR NOT VERIFICATION TESTED SHALL BE PROOF TESTED.

PROOF-TESTED ANCHORS SHALL BE FULLY GROUTED PRIOR TO TESTING AS ILLUSTRATED ON THIS SHEET. VERIFICATION TEST ANCHORS SHALL NOT HAVE STRUCTURAL GROUT IN THE UNBONDED LENGTH DURING TESTING. THE CONTRACTOR SHALL SELECT A METHOD FOR OBTAINING NO STRUCTURAL GROUT IN THE UNBONDED LENGTH THAT DOES NOT RESULT IN CAVING OF THE DRILL HOLE OR LOSS OF GROUND AT ANY TIME. SUCH METHODS INCLUDE BUT ARE NOT LIMITED TO: (A) LEAVING THE UNBONDED LENGTH OPEN UNTIL TESTING IS COMPLETE AND THEN TREMIE-GROUTING THE UNBONDED LENGTH WITH STRUCTURAL GROUT; AND (B) WASHING OUT THE UNBONDED LENGTH AFTER INSTALLATION UNTIL A SUFFICIENTLY WEAK SLURRY, AS DETERMINED BY THE GEOTECHNICAL SPECIAL INSPECTOR, IS ALL THAT REMAINS IN THE UNBONDED LENGTH.

THE CONTRACTOR SHALL SELECT THE METHOD OF INSTALLATION (OPEN-HOLE, CASED, AUGER-CAST, ETC.), METHOD OF GROUTING (TREMIE, PRIMARY LOW-PRESSURE, SECONDARY HIGH PRESSURE), ANCHOR DIAMETER, AND BOND LENGTHS IN ORDER TO DEVELOP THE DESIGN LOADS SPECIFIED ON THE PLANS. THE MINIMUM REQUIRED ANCHOR DIAMETER, BAR SIZE, UNBONDED LENGTHS, AND BOND LENGTHS ARE INDICATED ON THE PLANS.

THE METHOD OF INSTALLATION AND GROUTING UTILIZED FOR THE VERIFICATION TEST ANCHORS SHALL BE UTILIZED FOR ALL PRODUCTION ANCHORS INSTALLED THEREAFTER. IN THE EVENT THAT THE CONTRACTOR ELECTS TO MODIFY THE METHOD OF INSTALLATION AND GROUTING, ADDITIONAL VERIFICATION TESTING MAY BE REQUIRED BY GROUND SUPPORT, DEPENDING ON THE DETAILS OF THE METHOD OF INSTALLATION AND GROUTING.

INADEQUATE PERFORMANCE OF ANY TEST ANCHORS SHALL BE EVALUATED ON A CASE BY CASE BASIS BY GROUND SUPPORT TO DETERMINE THE REMAINING VALUE OF THE ANCHORS AND WHAT SUPPLEMENTAL INSTALLATIONS ARE REQUIRED TO AUGMENT THE INADEQUATE ANCHORS. THE CONTRACTOR SHALL MODIFY THE METHOD OF INSTALLATION AND GROUTING AS NECESSARY TO ACHIEVE THE MINIMUM DESIGN LOADS SPECIFIED ON THE PLANS.

THE CONTRACTOR ASSUMES ALL LIABILITY FOR COSTS AND DELAYS THAT RESULT FROM INADEQUATE PERFORMANCE OF TEST ANCHORS. IT IS HIGHLY RECOMMENDED THAT THE CONTRACTOR PERFORM THE VERIFICATION TESTS ON SACRIFICIAL ANCHORS PRIOR TO PRODUCTION ANCHOR INSTALLATION IN ORDER TO CONFIRM THAT THE CHOSEN METHODS OF INSTALLATION AND GROUTING ARE ADEQUATE TO ACHIEVE THE DESIGN LOADS SPECIFIED ON THE PLANS.

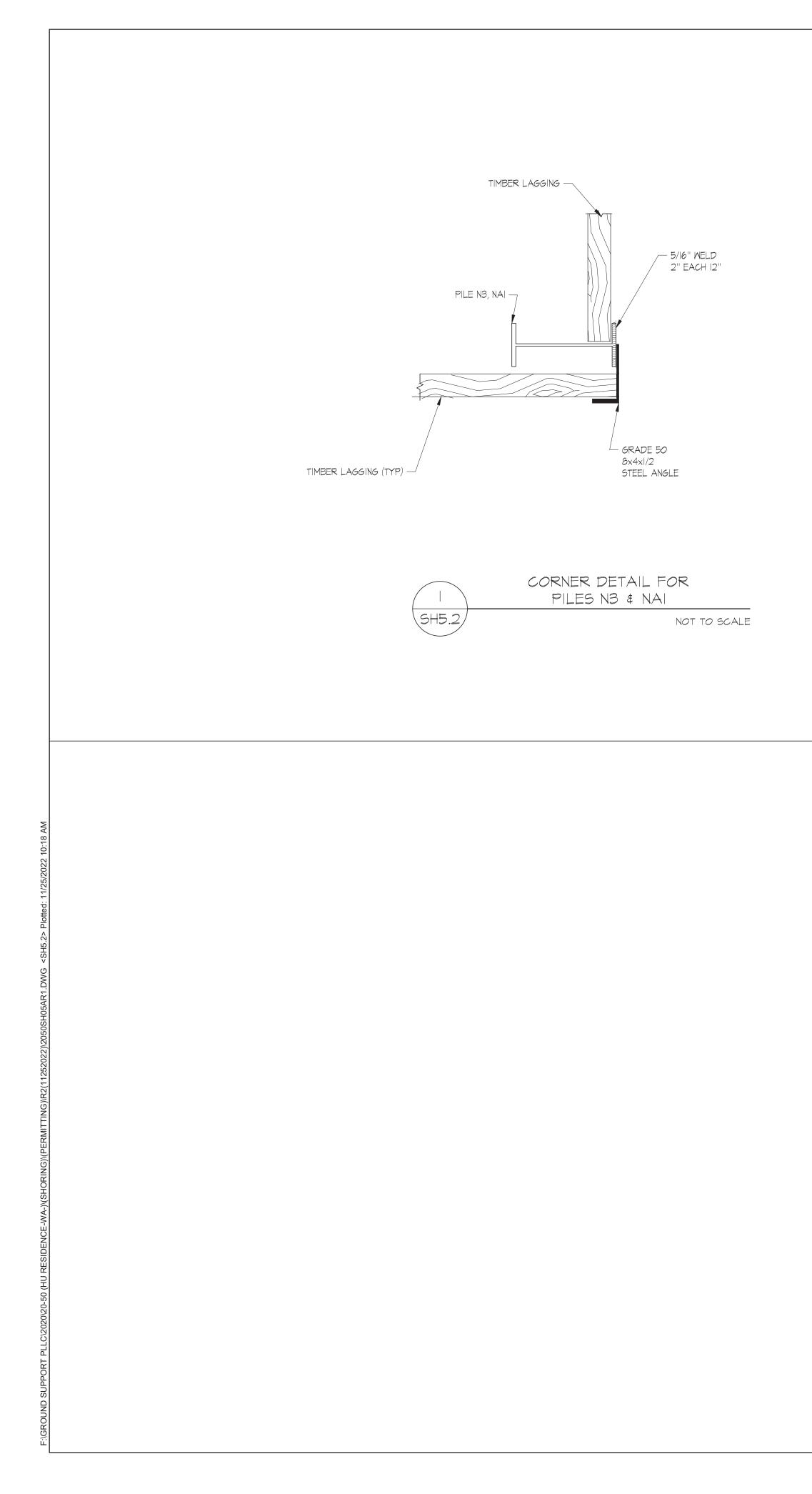
THE SOLDIER PILE AND GROUND ANCHOR SCHEDULE TABLES OF THESE PLANS CONTAIN ALL REMAINING MINIMUM GROUND ANCHOR DESIGN PARAMETERS INCLUDING: DESIGN LOAD, LOCKOFF LOAD, BAR SIZE, WORK POINT ELEVATION, DECLINATION ANGLE, BOND LENGTH, UNBONDED LENGTH, AND TOTAL LENGTH. NOTE THAT THE CONTRACTOR MUST ADD ADDITIONAL LENGTH OF BAR TO THE TOTAL LENGTH SPECIFIED ON THE PLANS TO EXTEND OUT FROM THE FACE OF THE DRILL HOLE.

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DRILLED HOLE
5-I/4" DIA (MIN) DRILLHOLE

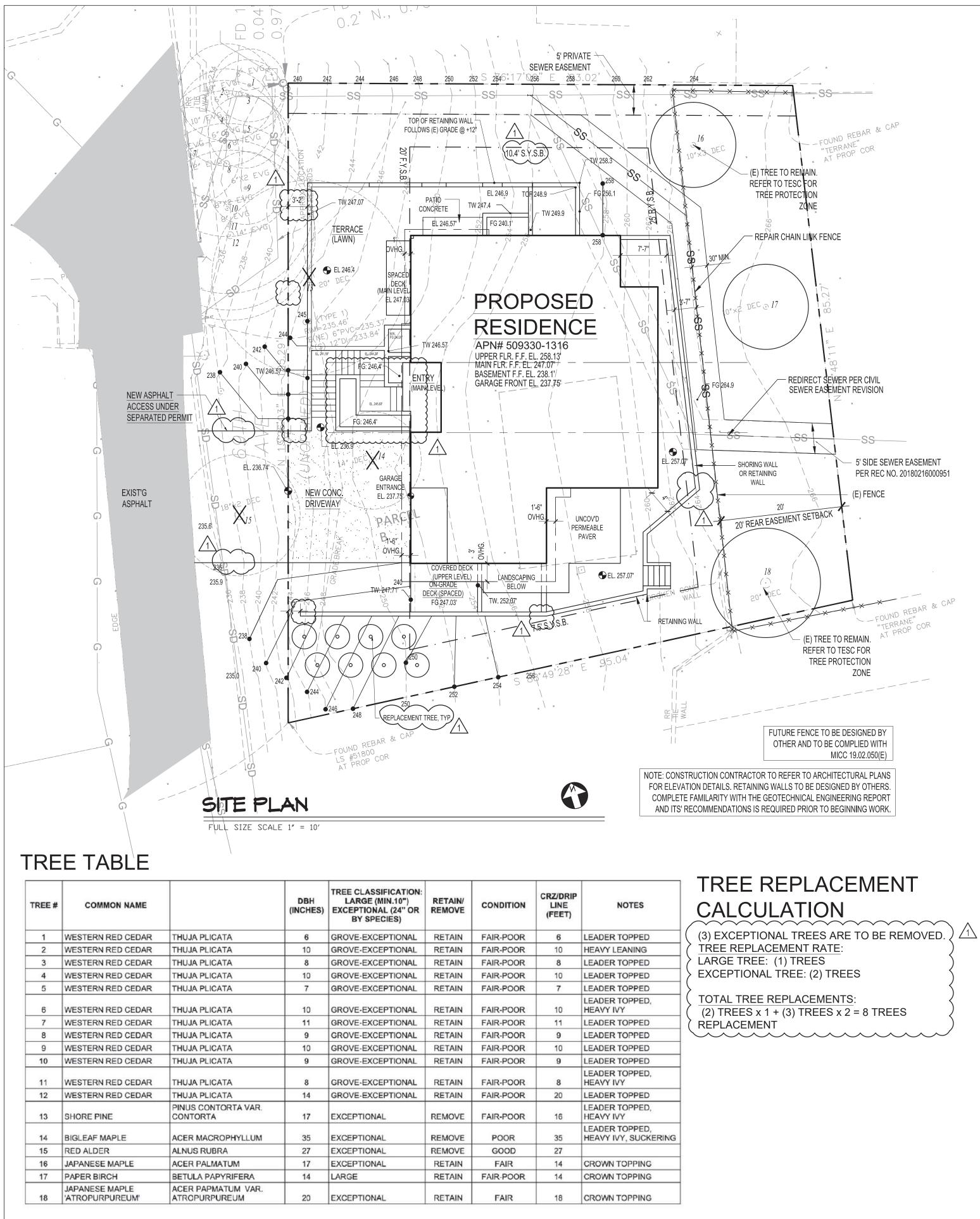
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d Support PLLC	19434 EF					
inville Redmond Rd NE, #210 WA 98072	CAL REGISTERED CS					
8-1143 Fax: (425) 605-4057						

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Hu RESIDENCE TEMPORARY SHORING AND PERMANENT RETAINING WALL DETAILS
PROJ. NO. 20-50 SHEET NUMBER
SH5.1
PROJ. NO. 20-50 SHEET NUMBER



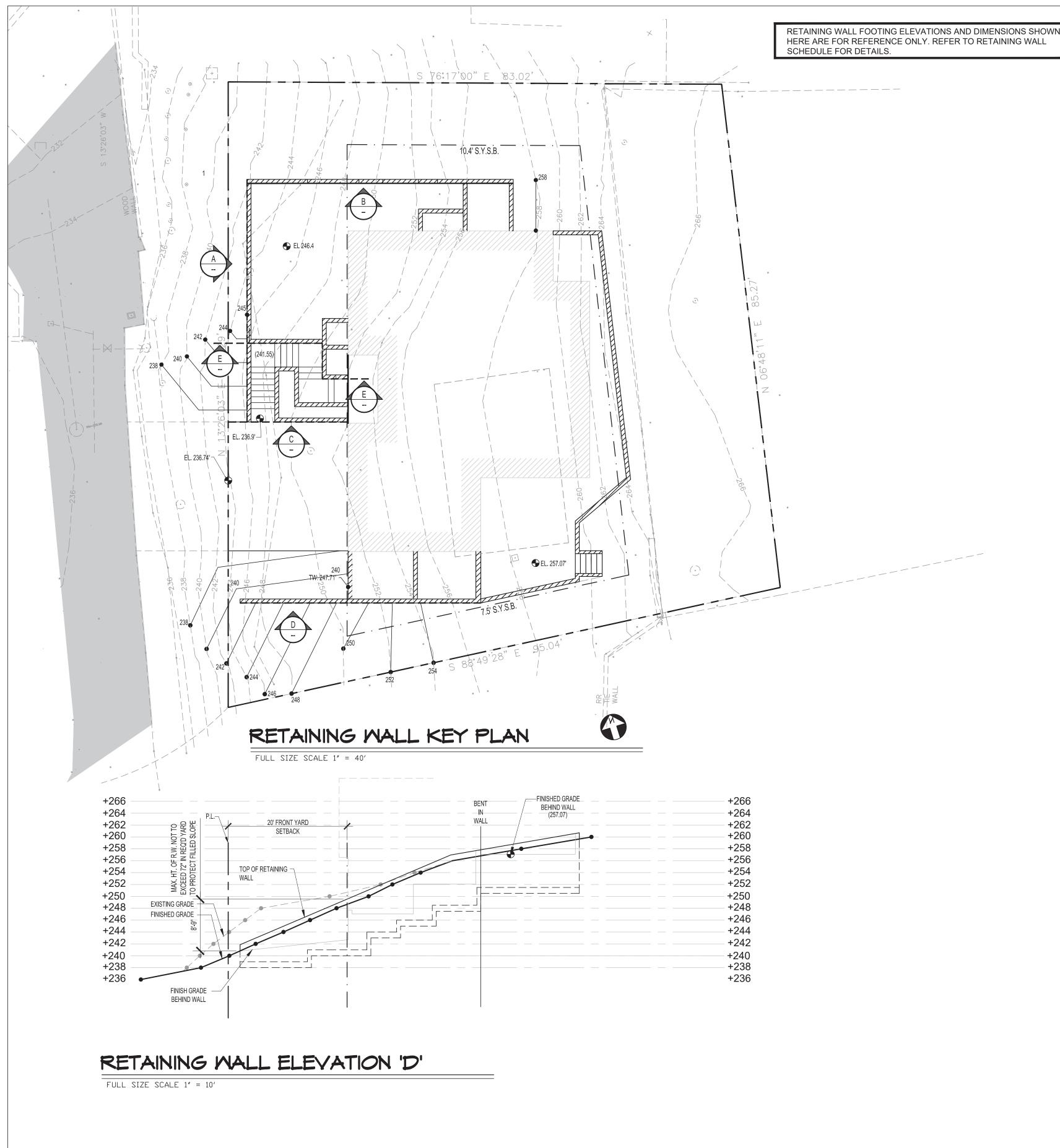
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				Ground Support PLLC	16932 Woodinville Redmond Rd NE, #210 Woodinville, WA 98072 Ph: (425) 488-1143 Fax: (425) 605-4057
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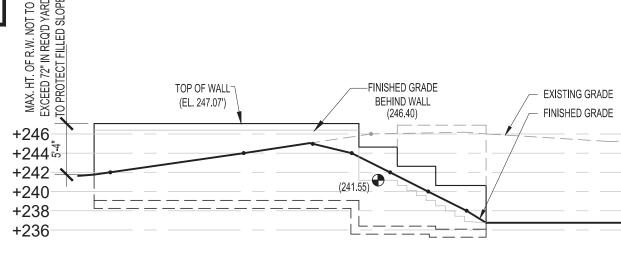


FRONT YARD SETBACK: 20 FT **REAR YARD SETBACKS:** 25 FT SIDE YARD SETBACKS: 105.29 * 17% = 17.9 FEET TOTAL $\checkmark \checkmark \checkmark \checkmark \checkmark \checkmark$ PER 19.02.020(C)(1)(c)(iii)(a) & (b) /1\ SOUTH EXTERIOR WALL FACADE HEIGHT IS 23'-6", SEE SOUTH ELEVATION ON A3.00 / FOR HEIGHT MEASUREMENT. SOUTH MIN. SIDE YARD S.B. = 7.5 FT NORTH MIN. SIDE YARD S.B. = 10.4FT

SETBACKS

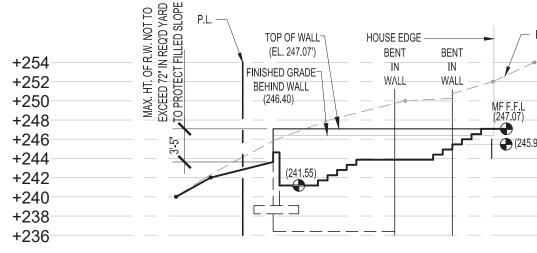
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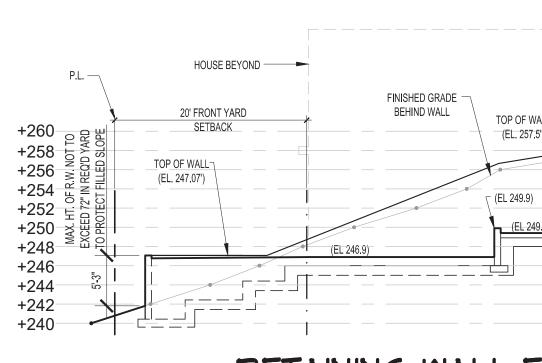
RETAINING WALL ELEVATION 'A'

FULL SIZE SCALE 1" = 10'

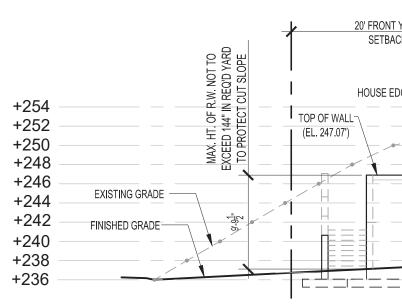


RETAINING MALL ELEVATION 'E'

FULL SIZE SCALE 1" = 10'



FULL SIZE SCALE 1" = 10'



RETAINING MALL ELEVATION 'C'

 	+266
 	+264
 	+262
 	+260
 	+258
 	+256
 	+254
 	+252
 	+250
 	+248
 	+246
 	+244
 	+242
	+240
 	+238
	+236

FULL SIZE SCALE 1" = 10'

t yard ACK	*		
EDGE		+25 +25	
•	FINISHED GRADE BEHIND WALL (246.40)	+25 +24	0 8
/		+24 +24	
		+24	2
		+24	0
		+23	8
		+230	6

RETAINING WALL ELEVATION 'B'

		- FINIS	GHED GRADE	 	
VALL .5')				 	+252
	-			 	+258
		 		 	+256
		 		 	+254
		 		 	+252
19.4)		 		 	+250
				 	+248
		 		 	+246
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5.93)	 	 	+246
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- EXISTING GRADE

/- FINISHED G	GRADE			
				+246
				+244
				+242
				+240
				+238
		_	-	+236

Project Name: HU RESIDENCE	30XX 69TH AVE SE MERCER ISLAND, WA 98040
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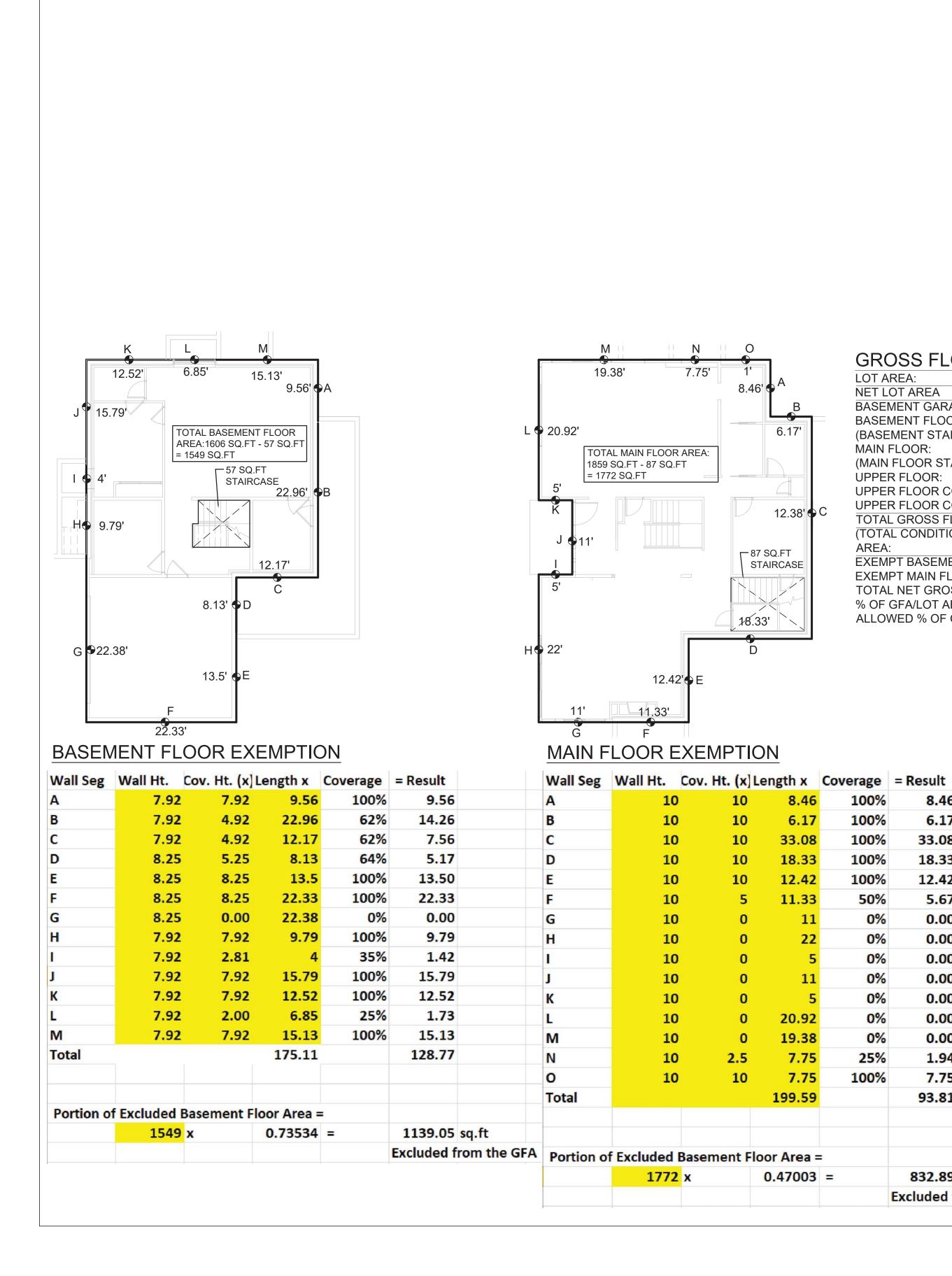
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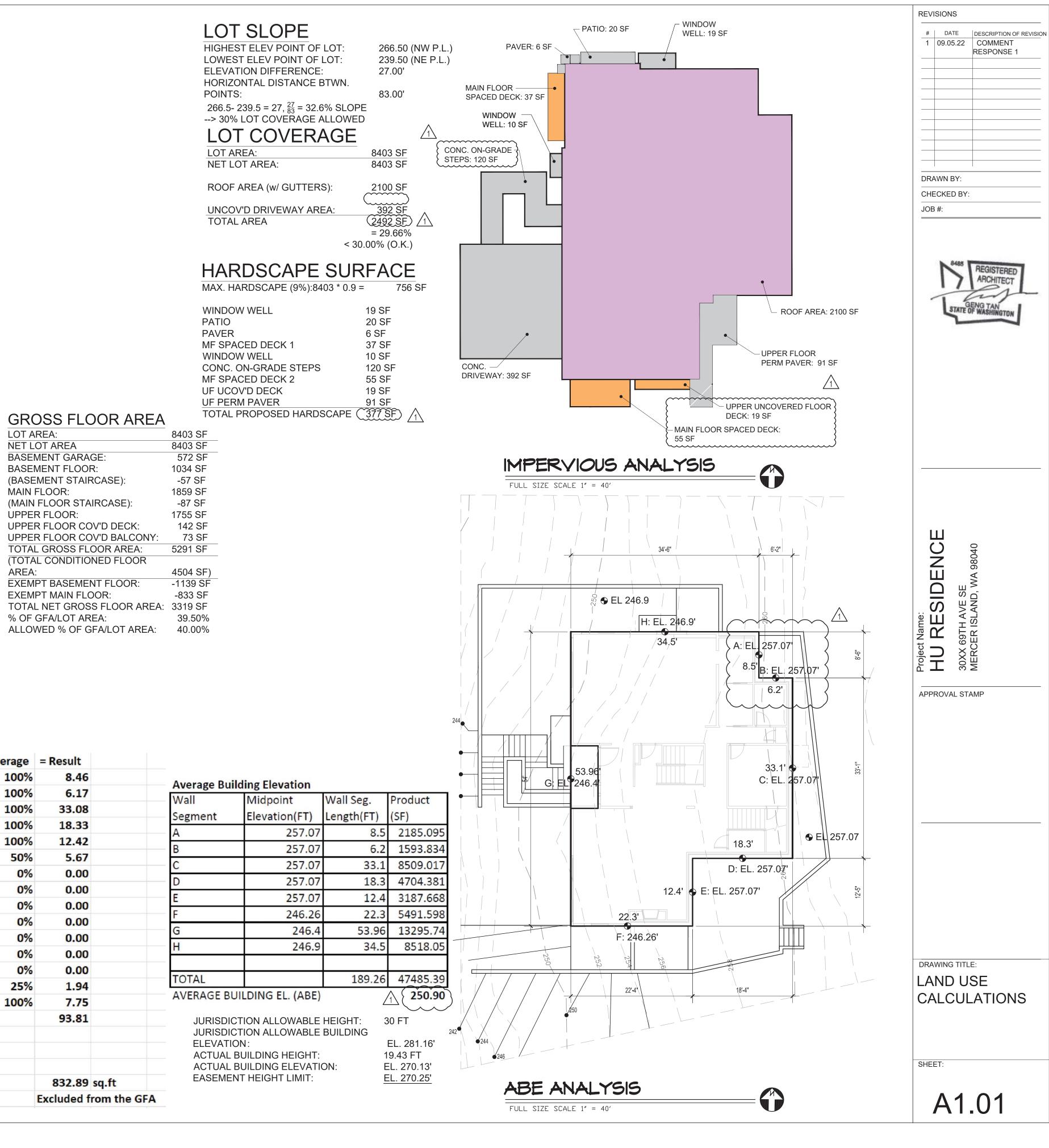
REVISIONS

| DATE | DESCRIPTION OF REVISION

RESPONSE 1

1 09.05.22 COMMENT





GENERAL NOTES

CODES:

WORK SHALL COMPLY WITH THE FOLLOWING CODES: 2018 WASHINGTON STATE RESIDENTIAL CODE 2018 WASHINGTON ENERGY CODE, RESIDENTIAL PROVISIONS (WSEC). WAC 51-11R 2018 UNIFORM PLUMBING CODE (UPC) WITH WASHINGTON STATE AMENDMENTS. WAC 51-56 2018 INTERNATIONAL FIRE CODE WITH WASHINGTON STATE AMENDMENTS. WAC 51-54A. OTHER CODES APPLICABLE BY JURISDICTION.

- All applicable codes, ordinances and minimum structural 1 requirements take precedence over all drawings, notes and specifications.
- 2. Do not scale drawings. Use printed dimensions only. Notify architect of any omissions or discrepancies before proceeding with work in question. Dimensions take precedence over scaled drawings.
- Contractor shall visit site and familiarize himself with all aspects of the work prior to contracting with the owner to perform the work.
- 4. Before commencing work, the contractor shall verify all dimensions and notes shown on drawings, verify dimensions with existing conditions by taking field measurements as required, and report grades and existing conditions prior proceeding with work. It is the contractor's responsibility to identify all the discrepancies to the architect at the time they are noted. Any work done by the contractor after discovery of such discrepancy shall be done at the contractor's risk.
- Contractor shall verify conformance of actual soil conditions with soils report and design assumptions.
- Contractor shall be responsible for acquiring all necessary permits for the work, except for the building permit which is the responsibility of the architect.
- Dimensions are to face of concrete, face of stud, or centerline of columns, U.N.O.
- Repetitive notes may be called out only once and indicated as typical. Repetitive features may be drawn only once, but shall be provided as if drawn in full.
- Separate mechanical, electrical, and plumbing permits are required in addition to the basic building permit, u.n.o. 10. The contractor is responsible for coordinating mechanical,
- electrical and plumbing contractors and notifying the architect of any discrepancies in framing prior to proceeding with work.

SITE SAFETY

- Contractor shall be responsible for all required safety 1. precautions and the methods, techniques, sequences, or procedures required to perform the work.
- 2. The architect has not been retained or compensated to provide design and/or construction review services relating to the contractor's safety precautions.
- Periodic site visits performed by the architect shall not be construed as supervision of actual construction safety precautions.
- 4. The architect is not responsible for providing a safe place for the performance of work by the contractor or the contractor's employees or employees of suppliers or subcontractors, or for access, visits, use, work, travel or occupancy by any person.
- Contractor shall maintain a trash bin in an area designated by the owner's representative for the collection of all construction debris. Contractor shall dispose of all debris and remove trash bin prior to occupancy. All surfaces shall be cleaned prior to occupancy.

TREE PROTECTION

- Preconstruction. Trees in the protection areas should be pruned, fertilized watered and bedded as recommended by the arborist or landscape architect before construction starts.
- Fencing. Construction fencing either orange fiberglass or chain link shall be placed according to the Approved Plan
- Signage. Signage shall indicate that no material storage, grade disturbance, or construction traffic shall occur within the tree protection areas.
- 4. Inspection. Trees should be inspected by the general contractor at least monthly during construction to ensure that they are being properly preserved.
- Tree damage. Damage or stress noticed to a tree or trees in the protection areas should be referred to the project arborist or landscape architect for recommended action.

MOISTURE PROTECTION

- 1. Provide pressure treated plates between concrete and framing
- 2. Provide a minimum of 12" clear between wood girders and earth.
- 3. Provide a minimum of 18" clear between wood joists and earth.
- 4. Provide a minimum of 8" clear between wood posts and earth.
- 5. Provide a minimum of 1" clear between wood posts and concrete floors.
- 6. Caulk all openings thoroughly.
- 7. Flash all openings with a minimum of 26 gauge galvanized steel to acceptable industry standards.
- Metal coping at parapet to be a minimum of 22 gauge galvanized steel

EGRESS & SECURITY

1. Stairways to meet the following requirements: cies less than 10)

(occupancies less that	Π
STAIR WIDTH	
TREAD WIDTH	
RISER HEIGHT	
HEADROOM	
HANDRAIL HEIGHT	
HANDRAIL GRASP	

36" MIN. 10" MIN., 6" MIN. FOR WINDERS 7-3/4" MAX. 80" MIN. 34" TO 38" ABOVE NOSING 1-1/4" MIN. TO 2" MAX.

- Egress Openings. Emergency escape and rescue openings shall have a minimum net clear opening for emergency escape and rescue grade-floor openings shall be 5 sq.ft. Where provided, they shall have a sill no greater than 44" above the adjacent floor. The minimum net clear opening height shall be 24". The minimum net clear opening width shall be 20". (R310.1)
- 2. Handrail intermediate members shall be configured as to passing a 4"-diameter sphere through any opening.
- Guardrails shall be a minimum of 36" above finish floor. 4. Guardrail intermediate members shall be configured as to
- passing a 4" diameter sphere through any opening. 5. Deadbolts with a minimum throw of $\frac{1}{2}$ " and a view port are required at all exterior doors

GLASS AND GLAZING

Safety Glazing. Install in areas subject to human impact (R308.4). Such hazardous locations include:

- 1. Glazing in fixed and operable panels of winging, sliding and bifold doors.
- 2. Glazing in a fixed or operable panel adjacent to a door where the nearest vertical edge is within a 24 inch arc of the door in a closed position and whose bottom edge is less than 60 inches above the floor or walking surface except for:
- 2.1. decorative glazing;
- where there is an intervening wall; 2.2. glazing in the wall perpendicular to the latch side of the 2.3.
- door: 2.4. adjacent to a closet door less than 3 feet deep (must comply with R308.4.3)
- adjacent to the fixed panel of patio doors. 2.5.
- 3. Glazing in an individual or fixed panel that meets all of the following conditions:
- Exposed area of an individual pane greater than 9 square 3.1
- 3.2 Bottom edge less than 18 inches above the floor.
- 3.3 Top edge greater than 36 inches above the floor. 3.4 One or more walking surfaces within 36 inches
- horizontally of the glazing.
- 4. All glazing in railings, regardless of an area or height above walking surface. Included are structural baluster panels and nonstructural in-fill panels.
- Glazing in walls, enclosures, or fences for hot tubs, spas, whirlpools, saunas, steam rooms, bathtubs, showers, and indoor or outdoor pools where the bottom exposed edge of the glazing is less than 60 inches above any standing or walking surface and within 60 inches horizontally of the water's edge.
- Glazing adjacent to sstairways, landings, and ramps within 36 inches horizontally of a walking surface when the bottom exposed edge of the glass is less than 36 inches above the adjacent walking surface. Except when a rail is installed on the accessible side of the glazing 34" to 38" above the walking surface.
- Glazing adjacent to the landing at the bottom of a stairway within 60 inches horizontally of the bottom tread when the exposed surface of the glazing is less than 36 inches above the nose of the tread. Except when the glazing is protected by a guard complying with section R312 and the glass is more than 18" from the guard.

ENERGY NOTES

TABLE R402.1.1 INSULATION AND FENESTRATION REQUIREMENTS BY COMPONENT (a)				
CLIMATE ZONE	5 AND MARINE-4			
FENESTRATION U-FACTOR (b)	0.30			
SKYLIGHT U-FACTOR (b)	0.50			
GLAZED FENESTRATION SHGC	NR			
CEILING R-VALUE (e)	49			
WOOD FRAME WALL R-VALUE (g,h)	21 INT			
MASS WALL R-VALUE (i)	21/21			
FLOOR R-VALUE	30			
BELOW-GRADE WALL R-VALUE (c,h)	10/15/21 INT + TB			
SLAB R-VALUE & DEPTH (d,f)	10, 2FT			

TABLE R402.1.1 FOOTNOTES

- FOR SI: 1 FOOT = 304.8 MM, CI = CONTINUOUS INSULATION, INT = INTERMEDIATE FRAMING. R-VALUES ARE MINIMUMS. U-FACTORS AND SHGC ARE MAXIMUMS. WHEN INSULATION IS INSTALLED IN A CAVITY WHICH IS LESS THAN THE LABEL OR DESIGN THICKNESS OF THE
- INSULATION. THE COMPRESSED R-VALUE OF THE INSULATION FROM APPENDIX TABLE A101.4 SHALL NOT BE LESS THAN THE R-VALUE SPECIFIED IN THE TABLE
- THE FENESTRATION U-FACTOR COLUMN EXCLUDES SKYLIGHTS
- "10/15/21 +TB" MEANS R-10 CONTINUOUS INSULATION ON THE EXTERIOR OF THE WALL, OR R-15 CONTINUOUS INSULATION ON THE INTERIOR OF THE WALL, OR R-21 CAVITY INSULATION PLUS A THERMAL BREAK BETWEEN THE SLAB AND THE BASEMENT WALL AT THE INTERIOR OF THE BASEMENT WALL. "10/15/21 +TB" SHALL BE PERMITTED TO BE MET WITH R-13 CAVITY INSULATION ON THE INTERIOR OF THE BASEMENT WALL PLUS R-5 CONTINUOUS INSULATION ON THE INTERIOR OR EXTERIOR OF THE WALL. "5TB" MEANS R-5 THERMAL BREAK BETWEEN FLOOR SLAB AND BASEMENT WALL
- d. R-10 CONTINUOUS INSULATION IS REQUIRED UNDER HEATED SLAB ON GRADE FLOORS. SEE SECTION R402.2.9.1.
- FOR SINGLE RAFTER- OR JOIST-VAULTED CEILINGS, THE INSULATION MAY BE REDUCED TO R-38 IF THE FULL INSULATION DEPTH EXTENDS OVER THE TOP PLATE OF THE EXTERIOR.
- R-7.5 CONTINUOUS INSULATION INSTALLED OVER AN EXISTING SLAB IS DEEMED TO BE EQUIVALENT OT THE REQUIRED PERIMETER SLAB INSULATION WHEN APPLIED TO EXISTING SLABS COMPLYING WITH SECTION R503.1.1. IF FOAM PLASTIC IS USED, IT SHALL MEET THE REQUIREMENTS FOR THERMAL BARRIERS PROTECTING FOAM PLASTICS
- FOR LOG STRUCTURES DEVELOPED IN COMPLIANCE WITH STANDARD ICC 400, LOG WALLS SHALL MEET THE REQUIRMENTS FOR CLIMATE ZONE 5 OF ICC 400.
- INT. (INTERMEDIATE FRAMING) DENOTES FRAMING AND INSULATION AS DESCRIBED IN SECTION A103.2.2 INCLUDING STANDARD FRAMING 16 INCHES ON CENTER, 78 PERCENT OF THE WALL CAVITY INSULATED AND HEADERS INSULATED WITH A MINIMUM OF R-10 INSULATION.
- 1. A certificate complying with 2015 WSEC R401.3 is required to be completed by the design professional or builder and permanently posted within 3' of the electrical panel prior to final inspection.
- 2. The building shall be tested and verified as having an air leakage rate not exceeding 5 air changes per hour. Testing shall be conducted with a blower door at a pressure of 0.2 inches w.g.
- Each dwelling unit is required to be provided with at least one programmable thermostat for the regulation of temperature.
- 4. Ducts shall be leak tested in accordance with WSU RS-33 using the max. duct leakage rates specified.
- 5. A minimum of 75% of permanently installed lamps in lighting fixtures shall be high-efficacy lamps.
- 6. Caulk all joints around exterior opening
- Seal all tears and joints in insulation with approved tape.
- 8. All crawlspaces shall have a minimum of 6 Mil black Visqueen ground cover extended over the top of the footings. Lap all joints 12" minimum.
- 9. Fireplaces shall have tight fitting dampers and shall be provided with a minimum of 6 square inches of outside combustible air supply.
- 10. Metal ducts outside the conditioned space shall be insulated to R-8 minimum per the 2015 WSEC, Section R403.2.1. Provide weather barrier if located on the exterior of the building.
- 11. Hot water shall meet 1987 National Appliance Energy Conservation Act.

VENTILATION

- Source specific fans shall be located in all kitchens bathrooms, water closets, and laundry facilities in compliance with the 2015 WRC, Section M1507.4. Ventilation capacity shall be at least 50 cfm for bathrooms. Water closets, and laundry rooms (Intermittent use) and 100 cfm for kitchens (Intermittent use). Range hoods shall be exhausted in accordance with Section M1503. 2. Whole House Ventilation System shall comply with the
- 2015 WRC, Section M1507.3 2.1. Per 2015 WRC Table M1507.3.3(1) continuous whole
- house mechanical ventilation system airflow rate requirements: Provide 120 CFM airflow (4,501-6,000 SF dwelling unit with 6 bedrooms = 120 CFM airflow)
- 2.2. System components include whole-house ventilation using exhaust fans and integrated with a forced-air system. Timer, intake grill & ducting (exterior), motorized damper, intake blower, electric air tampering, exhaust ducting & port with back draft damper. Distribution ducting & grills (habitable rooms shall be provided with outdoor air), electric exhaust fan.
- 2.3. The whole house ventilation fan shall meet the requirements of Section M1507.3.2 and M1507.3.2.1 Manufacturer's installation, operating instructions, and a whole house ventilation system operation description shall be provided by installer.

2.4. Controls for all ventilation systems shall be readily accessible by the occupant. Local exhaust systems shall be controlled by manual switches, dehumidistats, timers, or other approved means. Continuous whole house

- ventilation system shall comply with the following: A. Continuous whole house ventilation system shall operate continuously and be equipped with an override control. A "Fan On" switch shall be permitted as an override control.
- B. Controls shall be capable of operating the ventilation system exhaust fans, forced-air system fans, or supply fans without energizing other energy consuming appliances. C. A label shall be affixed to the control that reads

"Whole House Ventilation (See operating instructions)"

- 2.5. Intermittent ventilation shall occur at least 25% of each 4-Hour segment. Ventilation rate shall be not less than as specified by Table M1507.3.3(1), multiplied by the rate factor indicated on Table M1507.3.3(2). Fan shall have a sone rating of 1.0 or less measured at 0.1 inches W.G.
- Clothes dryers shall be exhausted in accordance with the 3 2015 WRC, Section M1502. Duct length shall not exceed 35 feet, plus the length of the transition duct, less the equivalent length of fittings per Table M1502.4.4.1.
- 4. Exhaust duct work shall conform to 2015 WRC, Chapter 16. Exhaust ducting terminations shall be outside the building, shall be located in compliance with Section M1506.2. And shall be equipped with backdraft dampers.
- Supply ducts within conditioned space shall be insulated 5. to a minimum of R-4.
- Provide a minimum net area of 1 square foot of ventilation area for each 300 square feet of crawlspace area. Place openings as near as to corners as practicable and shall provide cross ventilation.
- 7. All crawlspace vents shall be provided with $\frac{1}{4}$ "
- non-corrosive wire mesh.
- Provided a minimum net area of 1 square foot of ventilation area for every 150 square feet of attic area. Provide a continuous 1 inch minimum air space above insulation for cross ventilation.
- 9. All attic vents shall be provided with $\frac{1}{4}$ " non-corrosive wire mesh or approved soffit vents.

ALARM SCHEDULE			VENTILATION SCHEDULE			
2015 WRC SECTION R314 & R315						
SYMBOL	DESCRIPTION	REQUIREMENTS		C SECTION M1507		
	SMOKE ALARM *110 V INTERCONNECTED W/ BATTERY BACKUP	SYMBOL	LOCATION(S)	MINIMUM FAN REQUIREMENTS		
SASMOKE ALARM*110 V INTERCONNECTED W/ BATTERY BACKUP *INSTALLED ON EACH FLOOR, IN EACH SLEEPING AREA, AND OUTISDE EACH SEPARATE SLEEPING AREA *LISTED IN ACCORDANCE WITH UL 217 AND INSTALLED PER THE HOUSEHOLD FIRE WARNING EQUIPMENT PROVISIONS OF NFPA 72		BATH/TOILET, POWDER, LAUNDRY	MIN. 50 CFM @ 0.25" WG			
	LISTED IN ACCORDANCE WITH UL 217 AND INSTALLED PER THE HOUSEHOLD FIRE WARNING EQUIPMENT	B	KITCHEN	MIN. 100 CFM @ 0.25" WG (RANGE HOOD OR DOWN DRAFT EXHAUST FAN RATED AT MIN. 100 CFM @ 0.10" WG MAY BE USED FOR EXHAUST FAN REQUIREMENTS.)		
(CD)SA)	COMBINATION SMOKE ALARM & CARBON MONOXIDE ALARM	*INSTALLED ON EACH FLOOR AND OUTSIDE OF EACH SEPARATE SLEEPING AREA IN THE IMMEDIATE VICINITY OF THE BEDROOMS *SMOKE ALARM REQUIREMENTS PER ABOVE *CARBON MONOXIDE ALARMS LISTED AS COMPLYING WITH UL 2075 AND INSTALLED PER MANUFACTURER'S INSTALLATION INSTRUCTIONS	C	WHOLE HOUSE FAN	MIN. CFM = 120 @ 0.25" WG (WRC TABLE M1507.3.3(1)) (BASED ON 5291 S.F. FLOOR AREA & 6 BEDROOMS) (DAILY FRACTIONAL OPERATION TIME = 75 %) *WHOLE HOUSE FANS LOCATED 4 FT. OR LESS FROM INTERIOR GRILLE TO HAVE A SONE RATING OF 1.0 OR LESS MEASURED @ 0.1" WG	

- 1.2. 1.3.

- lumber. places: 8.1.
 - 8.2.
 - 8.3.

FIRE PROTECTION

1. The garage shall be separated from the residence and its attic by no less than the following:

1.1. 5/8" gypsum wallboard required at all walls separating garage and dwelling. Not less than (1)layer of 5/8" Type "X" gypsum wallboard at ceilings.

1-3/8" minimum thick, solid core, or honeycomb core steel door, or a 20-min. fire-rated door.

Ducts in the garage and ducts penetrating the separation assemblies shall be min. 26 gauge sheet steel and shall have no openings into the garage. 2. Fire separation to be horizontal and vertical including all structural members supporting the fire separation.

3. All enclosed useable space under stairways shall be (1) layer of 5/8" Type 'X' Gypsum wallboard on enclosed side. 4. Smoke alarms shall meet 2015 IFC 907.2.11.2. Smoke alarms shall be hardwired, provided a battery backup, and interconnected within each dwelling unit. In order to

reduce the chances of nuisance activations, smoke alarms should not be located near kitchen appliances. 5. Smoke detectors shall be audible in all sleeping rooms,

and outside each sleeping area in the immediate vicinity of the bedrooms.

6. A minimum of (1) smoke detector shall be installed on each floor including the garage.

7. Firestopping and draftstopping shall consist of 2"nominal 8. Firestopping and draftstopping is required in the following

> Concealed spaces at all floor and ceiling levels and at 10 feet intervals along the length of the wall

Interconnections between concealed vertical and horizontal spaces(i.e. Soffits)

Concealed spaces between stair stringers at top and bottom of the run.

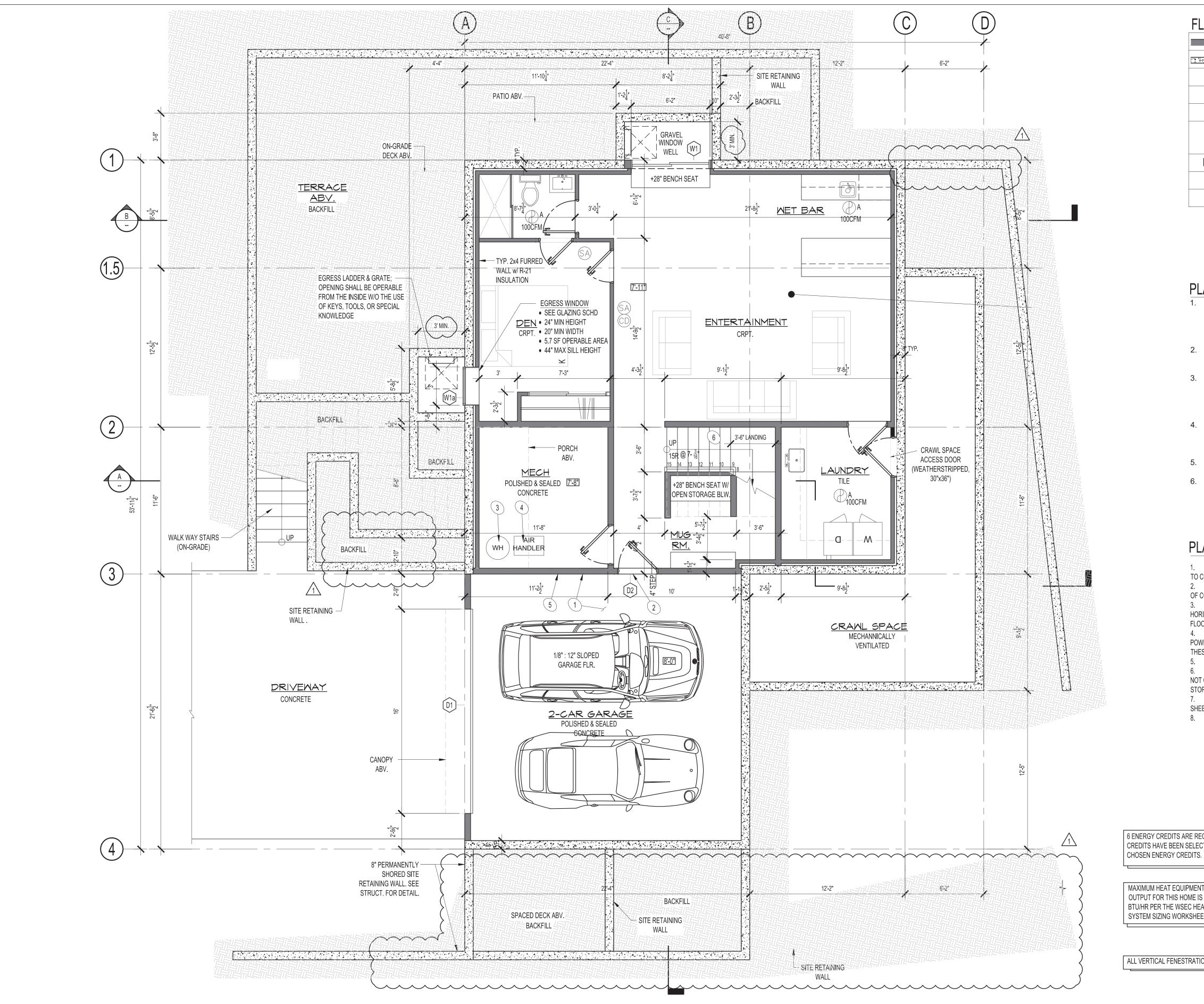
9. Rock wool around all openings for vents, pipes, ducts, etc. 10. Emergency egress windows shall meet the following requirements

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DRAWING TITLE: GENERAL NOTES & REQUIREMENTS
SHEET:
A1.02

WSEC ENERGY CREDIT OPTIONS

WASHINGTON STATE ENERGY CREDIT CALCULATION: 4504 SF OF NEW ENCLOSED FLOOR AREA FOR EACH UNIT. 6.0 CREDITS REQ'D. 6.0 CREDITS PROVIDED FUEL NORMALIZATION CREDITS (TABLE R406.2) 1.0 CREDIT SYSTEM TYPE 2 HEAT PUMP **OPTION 1.3: EFFICIENT BUILDING ENVELOPE OPTION:** 0.5 CREDITS VERTICAL FENESTRATION U = 0.28, FLOOR R-38, SLAB ON GRADE R-10 PERIMETER AND UNDER ENTIRE SLAB BELOW GRADE SLAB R-10 PERIMETER AND UNDER ENTIRE SLAB. OPTION 2.1 - AIR LEAKAGE CONTROL AND EFFICIENT VENTILATION: 0.5 CREDITS COMPLIANCE BASED ON R402.4.1.2: REDUCE THE TESTED AIR LEAKAGE TO 3.0 AIR CHANGES PER HOUR MAXIMUM AND ALL WHOLE HOUSE VENTILATION REQUIREMENTS AS DETERMINED BY SECTION M1507.3 OF TH∉ INTERNATIONAL RESIDENTIAL CODE SHALL BE MET WITH A HIGH EFFICIENCY FAN (MAXIMUM 0.35 WATTS/CFM), NOT INTERLOCKED WITH THE FURNACE FAN. VENTILATION SYSTEMS USING A FURNACE INCLUDING AN ECM MOTOR ARE ALLOWED, PROVIDED THAT THEY ARE CONTROLLED TO OPERATE AT LOW SPEED IN VENTILATION ONLY MODE. OPTION 3.5: HIGH EFFICIENCY HVAC: 1.5 CREDITS AIR-SOURCE, CENTRALLY DUCTED HEAT PUMP WITH MINIMUM HSPF OF 11.0. OPTION 4.1: HIGH EFFICIENCY HVAC DISTRIBUTION SYSTEM OPTIONS: 0.5 CREDITS ALL SUPPLY AND RETURN DUCTS LOCATED IN AN UNCODITIONED ATTIC SHALL BE DEEPLY BURIED IN CEILING INSULATION IN ACCORDANCE WITH SECTION R403.3.7. FOR MECHANICAL EQUIPMENT LOCATED OUTSIDE THE CONDITIONED SPAEC, A MAXIMUM OF 10 LINEAR FEET OF RETURN DUCT AND 5 LINEAR FEET OF SUPPLY DUCT CONNECTIONS TO THE EQUIPMENT MAY BE OUTSIDE THE DEEPLY BURIED INSULATION. ALL METALLIC DUCTS LOCATED OUTSIDE THE CONDITIONED SPACE MUST HAVE BOTH TRANSVERSE AND LONGITUDINAL JOINTS SEALED WITH MASTIC. IF FLEX DUCTS ARE USED, THEY CANNOT CONTAIN SPLICES. DUCT LEAKAGE SHALL BE LIMITED TO 3 CFM PER 100 SQUARE FEET OF CONDITONED FLOOR AREA. AIR HANDLER(S) SHALL BE LOCATED WITHIN THE CONDITIONED SPACE. OPTION 5.5: EFFICIENT WATER HEATING: 2.0 CREDITS ELECTRIC HEAT PUMP WATER HEATER MEETING THE STANDARDS FOR TIER III OF NEEA'S ADVANCED WATER HEATING SPECIFICATION

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BASEMENT FLOOR PLAN

22×34: SCALE 1/4" = 1'-0"

FLOOR PLAN LEGEND

	WOOD STUD WALL
	CONCRETE WALL
CD	CARBON MONOXIDE ALARM/DETECTOR
SA	SMOKE ALARM/DETECTOR
₩ ₩CFM	EXHAUST FAN LOCATION AND TYPE. REFER TO VENTILATION SCHEDULE ON A1.01 FOR REQUIREMENT DETAILS AND MIN. FLOW RATE, UNO
X	INDICATES NEW EXTERIOR DOOR/WINDOW. SEE DOOR/ WINDOW SCHEDULE ON SHEET A1.0
#	INDICATES PLAN KEYNOTES. SEE THIS SHEET.
	WALL TYPE. REFER TO WALL ASSEMBLY SCHEDULE ON SHEET

PLAN KEYNOTE

- 1. PROVIDE 5/8" GYPSUM WALLBOARD AT ALL WALLS SEPARATING GARAGE AND DWELLING. NOT LESS THAN (1)LAYER OF 5/8" TYPE "X" GYPSUM WALLBOARD AT CEILINGS.
- 2. PROVIDE TIGHT FITTING, 20-MIN. FIRE-RATED DOOR WITH SELF CLOSING DEVICE.
- 3. WATER HEATER, SHALL MEET EFFICIENCY REQUIREMENT PER WSEC ENERGY CREDIT OPTIONS ON SHEET A1.03. AND PROVIDE 26 GA. STRAP AROUND WATER HEATER TO WALL @ TOP & BOTTOM.
- 4. HVAC EQUIPMENT, SHALL MEET EFFICIENCY REQUIREMENT PER WSEC ENERGY CREDIT OPTIONS ON SHEET A1.03
- 5. 2x6 STUDS W/ R-21 INSULATION MIN.
- 6. SEE A4.03 & A4.04 FOR STAIR DETAILS

PLAN NOTES

CONTRACTOR SHALL VERIFY ALL NOTES, DIMENSIONS & CONDITIONS PRIOR TO CONSTRUCTION.

2. ALL DIMENSIONS ARE TO FACE OF FRAMING, CENTER OF COLUMN, OR FACE OF CONCRETE, UNO.

3. FIRE BLOCK ALL PLUMBING PENETRATIONS. FIRE BLOCK AT 10'-0" o.c. HORIZONTALLY AND VERTICALLY IN ALL FRAMED WALLS. FIRE BLOCK ALL WALL TO FLOOR, WALL TO CEILING, AND FLOOR TO FIREPLACE ASSEMBLIES. 4. PROVIDE ACOUSTICAL BATT INSULATION AT ALL INTERIOR BATHROOM AND

POWDER ROOM WALLS, BEDROOM WALLS, AND IN THE FLOOR AND CEILINGS WHEN THESE ROOMS OCCUR ABOVE OR BELOW A HABITABLE SPACE. 5. ALL EXTERIOR WALLS TO BE 2x6 STUDS @ 16" o.c. UNO.

6. PROVIDE A VENTED WINDOW IN EACH HABITABLE ROOM. THE FOLLOWING ARE NOT CONSIDERED HABITABLE ROOMS: BATHROOMS, TOILET ROOMS, CLOSETS, HALLS, STORAGE OR UTILITY SPACES AND SIMILAR.

7. SEE SHEET A1.01 FOR ADDITIONAL GENERAL NOTES APPLICABLE TO THIS PLAN SHEET 8. SEE SHEET A1.01 FOR VENTILATION AND ALARM SCHEDULES.

6 ENERGY CREDITS ARE REQUIRED AND 6 ENERGY CREDITS HAVE BEEN SELECTED. SEE A1.03 FOR ALL

ALL VERTICAL FENESTRATION U-FACTOR = 0.28 U.N.O

0' 1' 2' 3' 4'



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REVISIONS

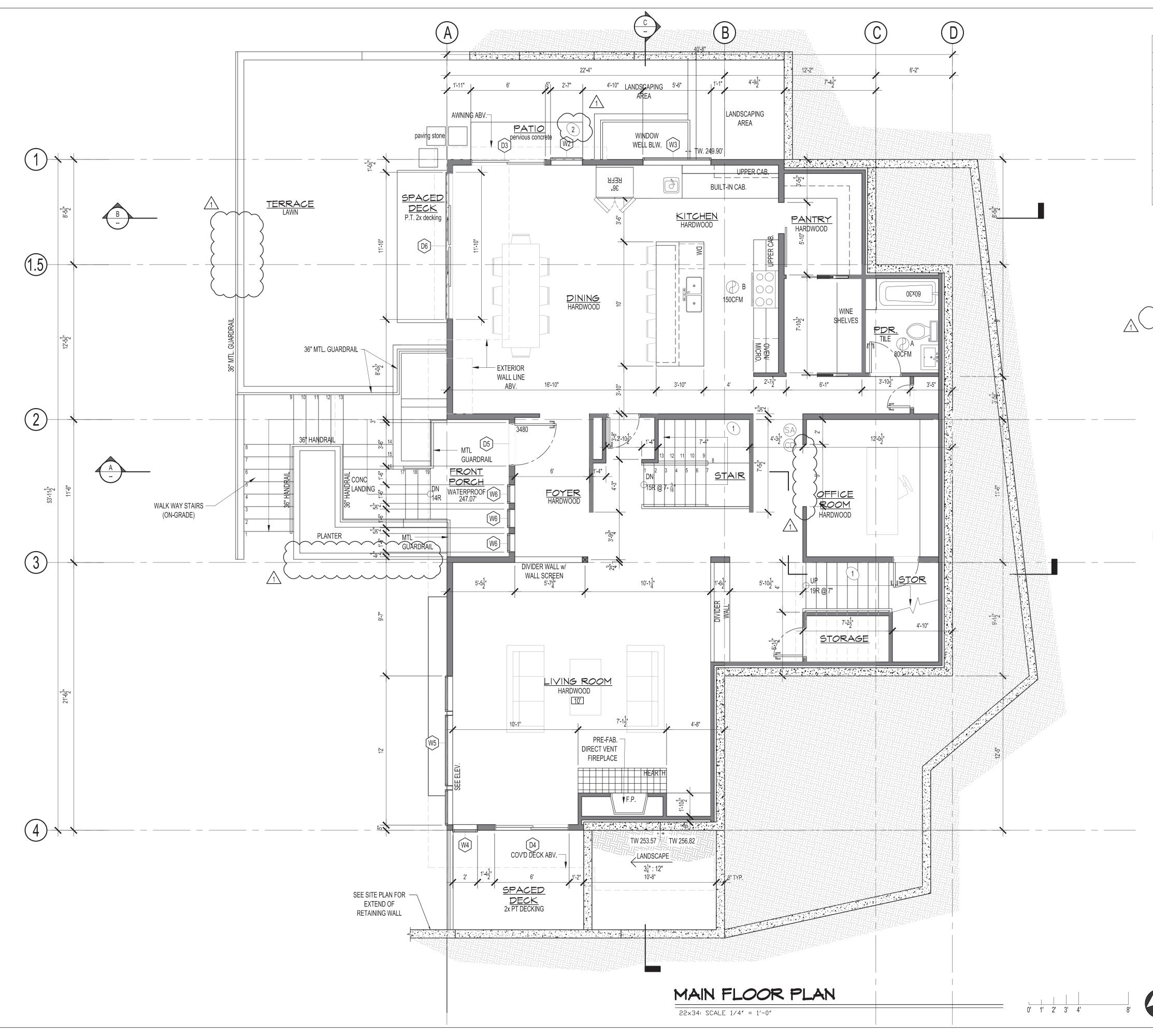
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BASEMENT FLOOR PLAN

A2.00

SHEET:



FLOOR PLAN LEGEND

	WOOD STUD WALL
	CONCRETE WALL
CD	CARBON MONOXIDE ALARM/DETECTOR
SA	SMOKE ALARM/DETECTOR
₩ ₩CFM	EXHAUST FAN LOCATION AND TYPE. REFER TO VENTILATION SCHEDULE ON A1.01 FOR REQUIREMENT DETAILS AND MIN. FLOW RATE, UNO
X	INDICATES NEW EXTERIOR DOOR/WINDOW. SEE DOOR/ WINDOW SCHEDULE ON SHEET A1.0
#	INDICATES PLAN KEYNOTES. SEE THIS SHEET.
	WALL TYPE. REFER TO WALL ASSEMBLY SCHEDULE ON SHEET

PLAN KEYNOTE

1.	SEE A4.03 & A4.04 FOR STAIR DETAILS
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2.	DESIGNATED CONDENSER LOCATION
\sim	

PLAN NOTES

1. CONTRACTOR SHALL VERIFY ALL NOTES, DIMENSIONS & CONDITIONS PRIOR TO CONSTRUCTION.

2. ALL DIMENSIONS ARE TO FACE OF FRAMING, CENTER OF COLUMN, OR FACE OF CONCRETE, UNO.

 FIRE BLOCK ALL PLUMBING PENETRATIONS. FIRE BLOCK AT 10'-0" o.c.
 HORIZONTALLY AND VERTICALLY IN ALL FRAMED WALLS. FIRE BLOCK ALL WALL TO FLOOR, WALL TO CEILING, AND FLOOR TO FIREPLACE ASSEMBLIES.
 PROVIDE ACOUSTICAL BATT INSULATION AT ALL INTERIOR BATHROOM AND POWDER ROOM WALLS, BEDROOM WALLS, AND IN THE FLOOR AND CEILINGS WHEN

THESE ROOMS OCCUR ABOVE OR BELOW A HABITABLE SPACE. 5. ALL EXTERIOR WALLS TO BE 2x6 STUDS @ 16" o.c. UNO.

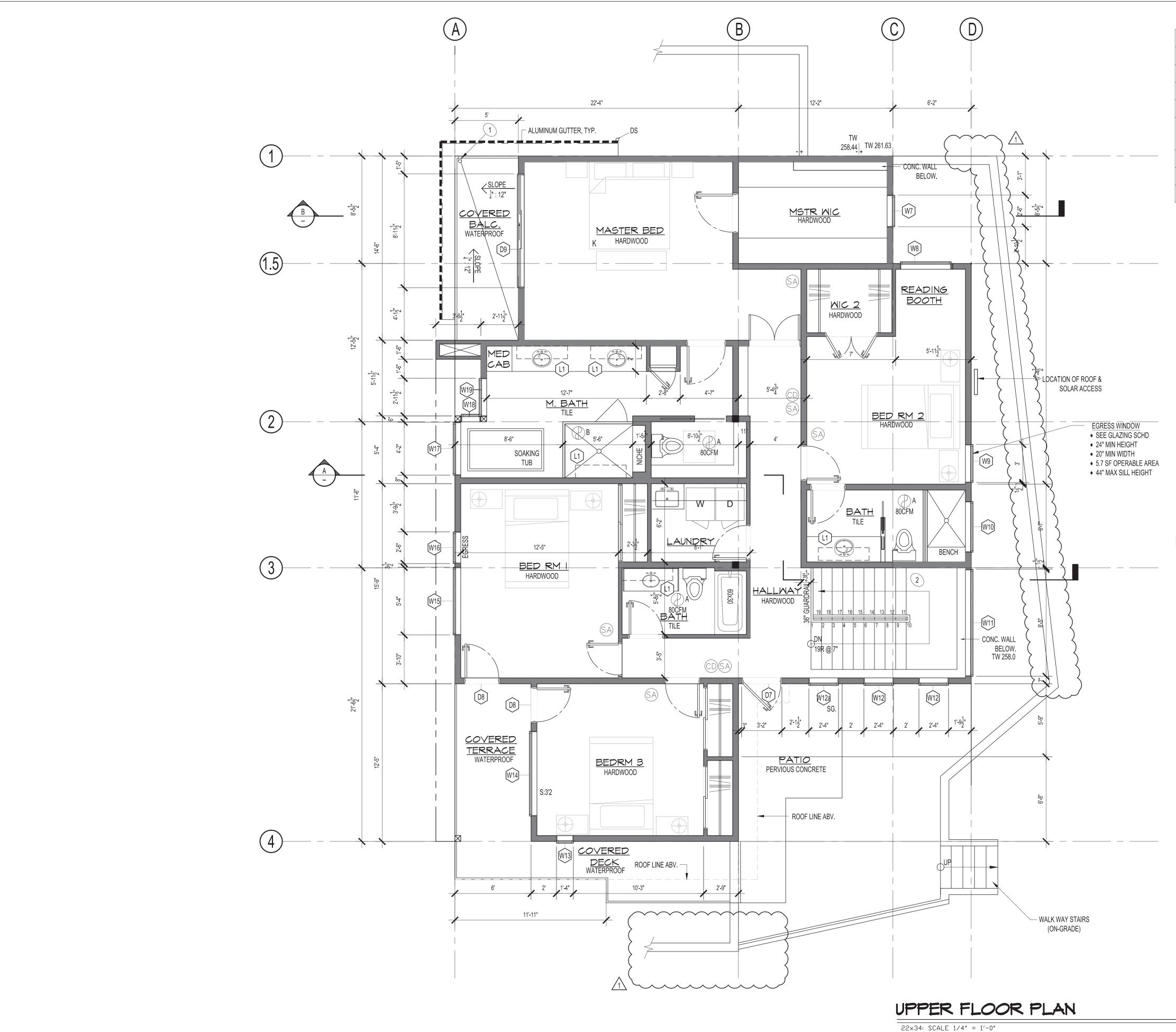
6. PROVIDE A VENTED WINDOW IN EACH HABITABLE ROOM. THE FOLLOWING ARE NOT CONSIDERED HABITABLE ROOMS: BATHROOMS, TOILET ROOMS, CLOSETS, HALLS, STORAGE OR UTILITY SPACES AND SIMILAR.

7. SEE SHEET A1.01 FOR ADDITIONAL GENERAL NOTES APPLICABLE TO THIS PLAN SHEET

8. SEE SHEET A1.01 FOR VENTILATION AND ALARM SCHEDULES.

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FLOOR PLAN LEGEND

	WOOD STUD WALL		
	CONCRETE WALL		
DS o	INDICATES GUTTER w/ DOWNSPOUT LOCATION		
CD	CARBON MONOXIDE ALARM/DETECTOR		
SA	SMOKE ALARM/DETECTOR		
₩CFM	EXHAUST FAN LOCATION AND TYPE. REFER TO VENTILATION SCHEDULE ON A1.01 FOR REQUIREMENT DETAILS AND MIN. FLOW RATE, UNO		
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#	INDICATES PLAN KEYNOTES. SEE THIS SHEET.		
	WALL TYPE. REFER TO WALL ASSEMBLY SCHEDULE ON SHEET		

PLAN KEYNOTE

 PROVIDE INTERNAL DRAIN AT LOW POINT w/ 3" DIA. ALUMINUM DOWNSPOUT, TIGHTLINE TO STORMWATER SYSTEM, TYP.

2. SEE A4.03 & A4.04 FOR STAIR DETAILS

PLAN NOTES

1. CONTRACTOR SHALL VERIFY ALL NOTES, DIMENSIONS & CONDITIONS PRIOR TO CONSTRUCTION.

 ALL DIMENSIONS ARE TO FACE OF FRAMING, CENTER OF COLUMN, OR FACE OF CONCRETE, UNO.
 FIRE BLOCK ALL PLUMBING PENETRATIONS. FIRE BLOCK AT 10'-0" o.c.

 FIRE BLOCK ALL FLOMBING FENETRATIONS. FIRE BLOCK AT 10-0 0.0.
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 PROVIDE ACOUSTICAL BATT INSULATION AT ALL INTERIOR BATHROOM AND POWDER ROOM WALLS, BEDROOM WALLS, AND IN THE FLOOR AND CEILINGS WHEN

THESE ROOMS OCCUR ABOVE OR BELOW A HABITABLE SPACE. 5. ALL EXTERIOR WALLS TO BE 2x6 STUDS @ 16" o.c. UNO.

6. PROVIDE A VENTED WINDOW IN EACH HABITABLE ROOM. THE FOLLOWING ARE NOT CONSIDERED HABITABLE ROOMS: BATHROOMS, TOILET ROOMS, CLOSETS, HALLS, STORAGE OR UTILITY SPACES AND SIMILAR.

 SEE SHEET A1.01 FOR ADDITIONAL GENERAL NOTES APPLICABLE TO THIS PLAN SHEET
 SEE SHEET A1.01 FOR VENTILATION AND ALARM SCHEDULES.

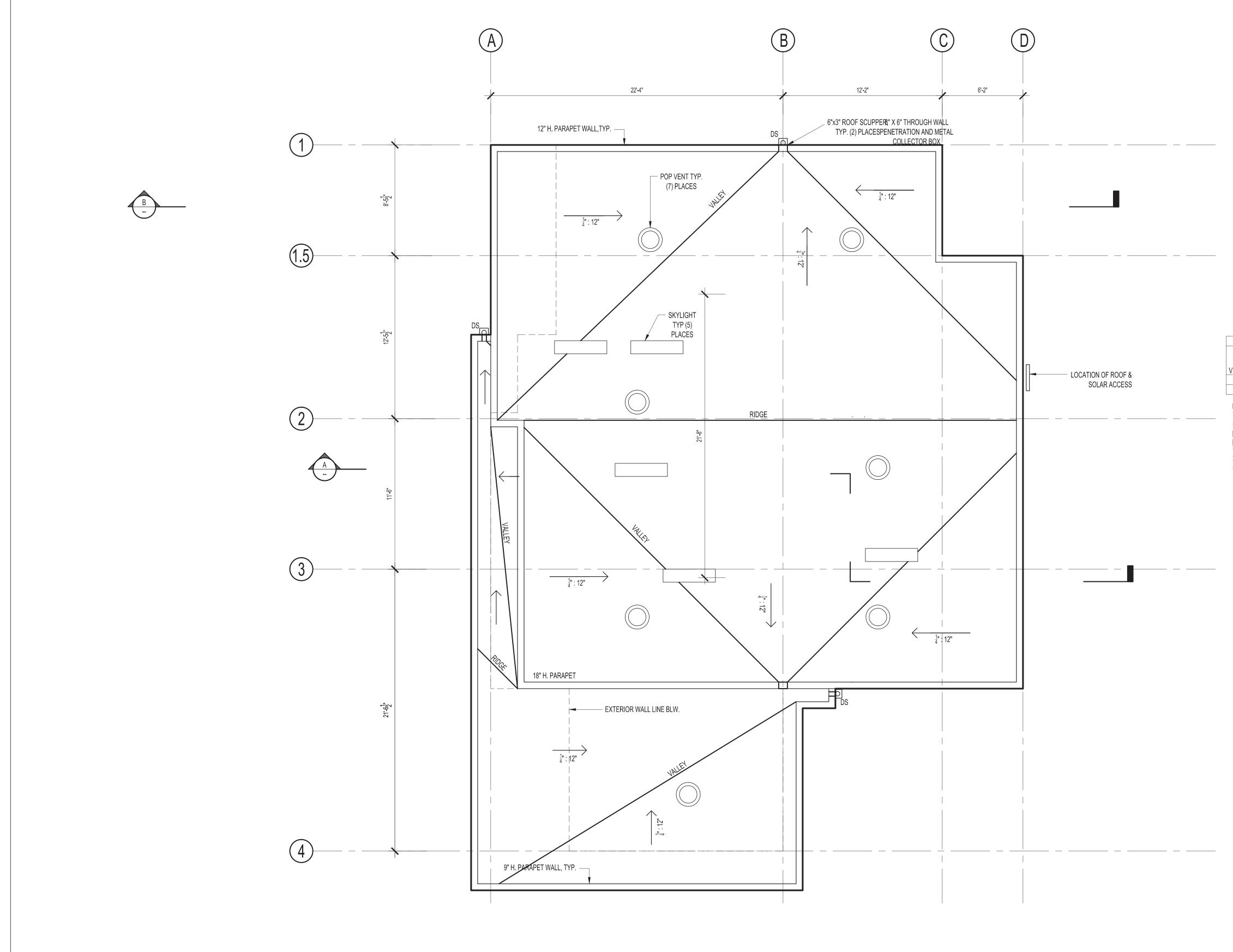
 SEE SHEET AT OF FOR VENTILATION AND ALARM SCHEDULES.
 SEE ROOF VENTILATION CALCULATIONS ON A2.03 FOR REQUIRED ROOF VENTILATION (WHERE APPLICABLE)

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A2.02





22×34: SCALE 1/4" = 1'-0"

PLAN NOTES

1. CONTRACTOR SHALL VERIFY ALL NOTES, DIMENSIONS & CONDITIONS PRIOR TO CONSTRUCTION.

ROOF SLOPE SHALL IN NO CASE BE LESS THAT 1/4":12" AT ANY LOCATION.
 PLUMBING RISERS AND VENTS NOT SHOWN ON PLANS FOR CLARITY. PLUMBING

PENETRATIONS TO BE KEPT TO A MINIMUM AND LOCATED ON ROOF SLOPES NOT VISIBLE FROM THE ENTRY ACCESS

4. FLASH AND COUNTERFLASH ALL ROOF PENETRATIONS.

 SEE ROOF VENTILATION CALCULATIONS ON THIS SHEET FOR REQUIRED ROOF VENTILATION (WHERE APPLICABLE)
 PROVIDE BALANCED ROOF VENTILATION. IT IS RECOMMENDED THAT THE

6. PROVIDE BALANCED ROOF VENTILATION. IT IS RECOMMENDED THAT THE CONTRACTOR COORDINATE WITH VENTING PRODUCT SUPPLIERS TO PREVENT IMBALANCED VENTILATION UNDER SEVERE WEATHER CONDITIONS WHERE INFILATRATION COULD OCCUR. WEATHER SHIELD FLASHING MAY BE REQUIRED AT THESE CONDITIONS.

7. DOWNSPOUTS TO BE LOCATED IN EXTERIOR WALLS, UNO.

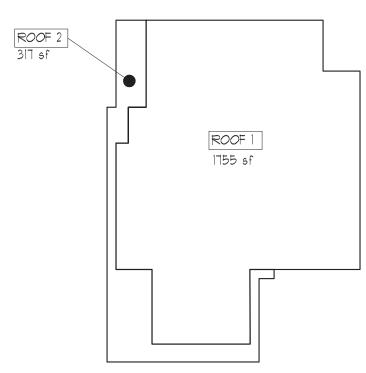
ROOF VENTING CALCULATION					
			PROPOSED	PROPOSED	TOTAL
	SQ.FT	SQ.IN	POP VENTS,	SOFFIT	VENTING
/ENT AREA	AREA	VENTING REQ'D	UNITS	VENTS, LF	PROVIDED
ROOF 1	1755	1685	7	0	1788
ROOF 2	317	304	0	40	400

NOTE: 1. Vent types:

soffit vent = AirVent Inc, continuous soffit vent, 10 sq.in. per linear ft pop vent = Active Ventilation Products Inc, Pop Vent,

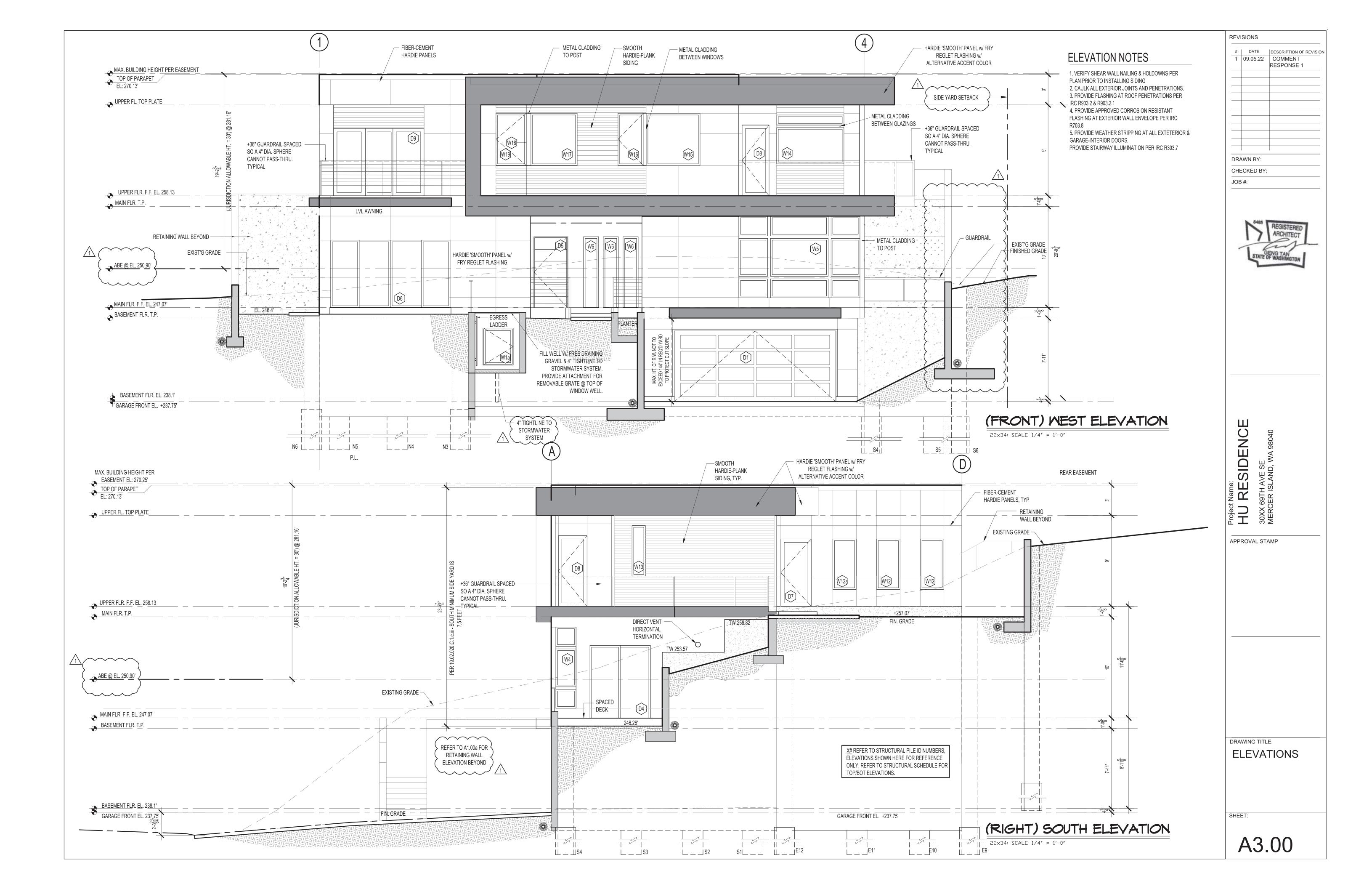
PV-18, 254 sq.in per unt 2. All roof venting area below Roof Level to provide

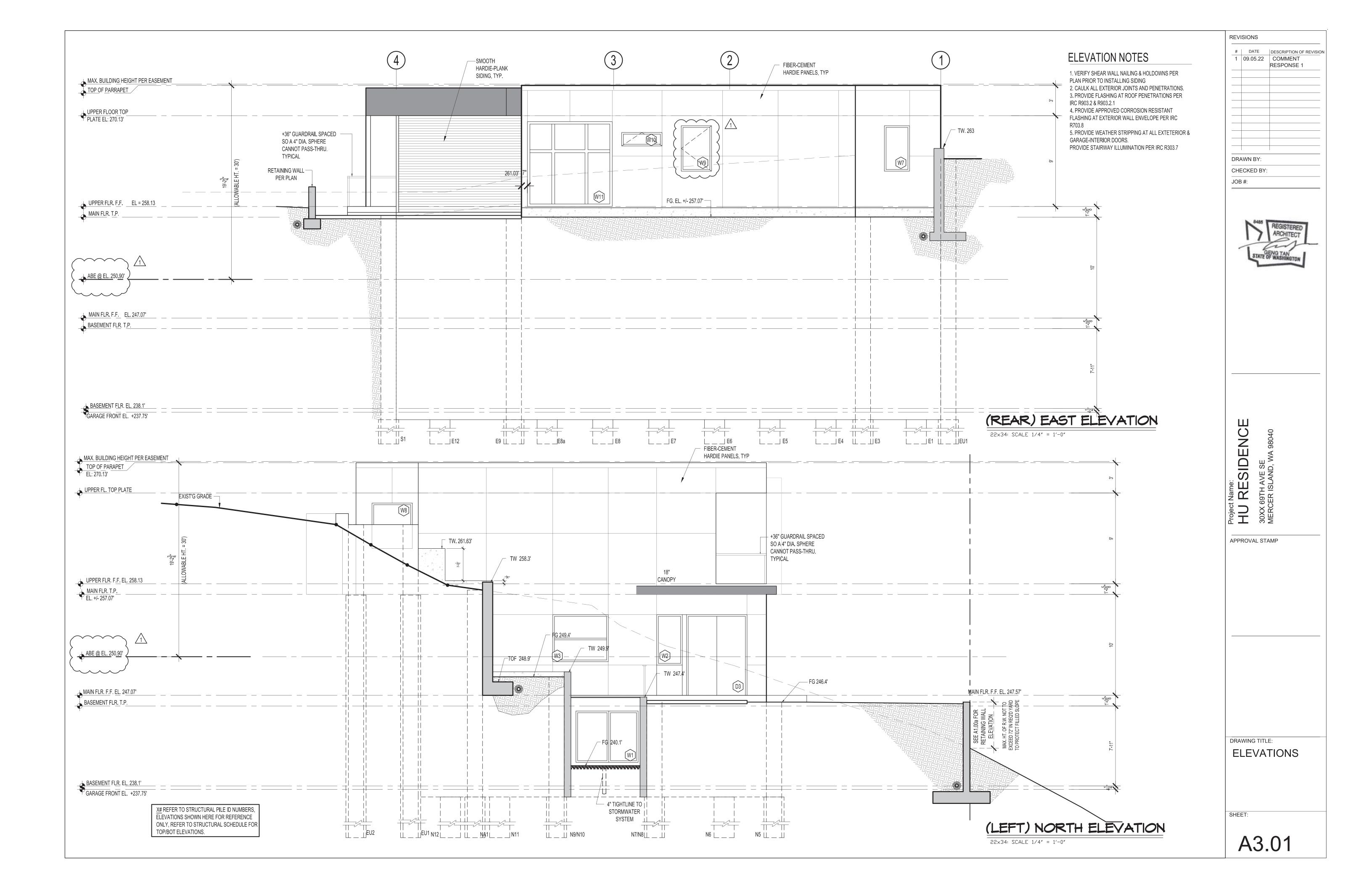
2. All roof venting area below Roof Level to provide soffit vent, uno.

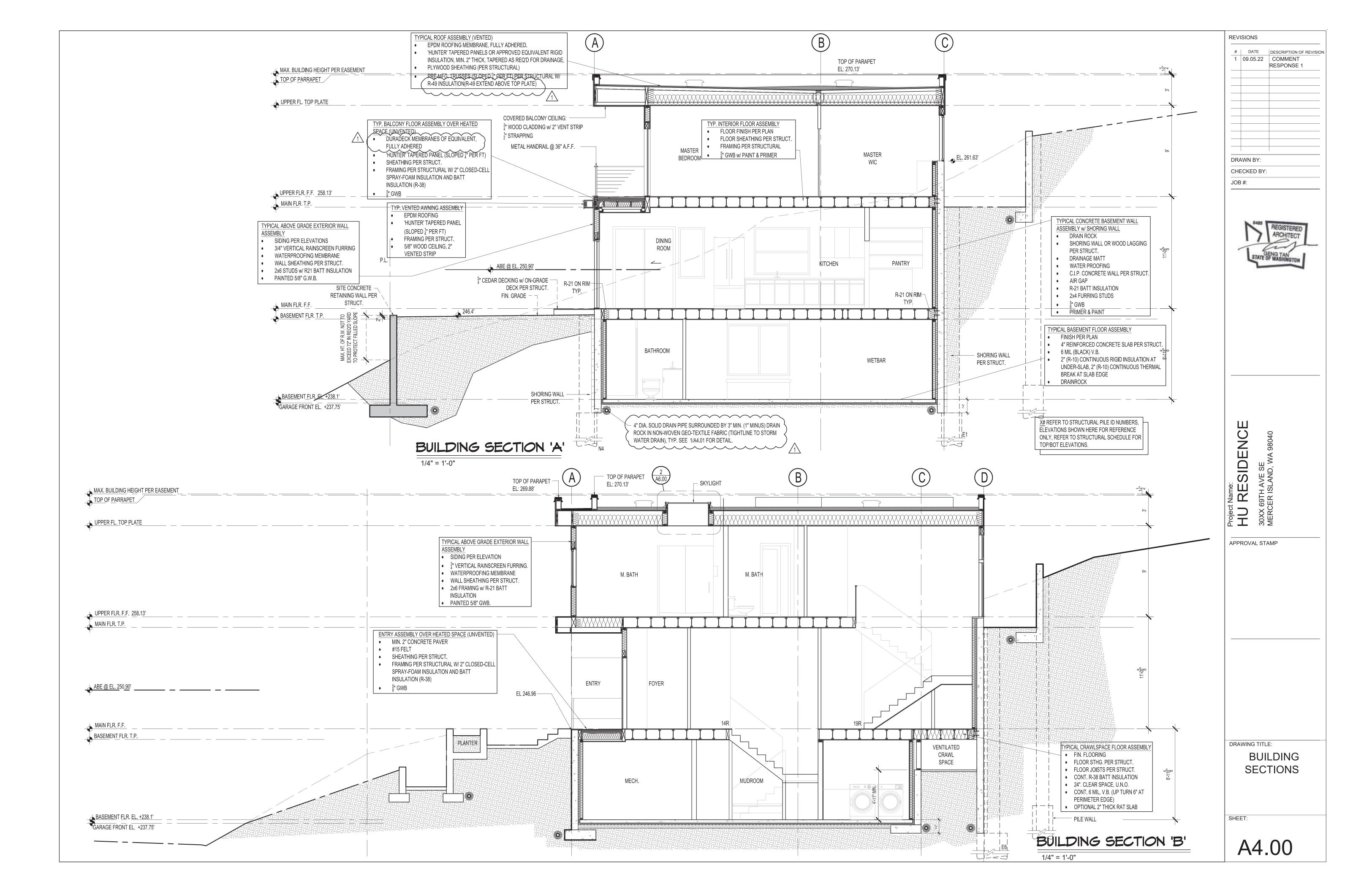


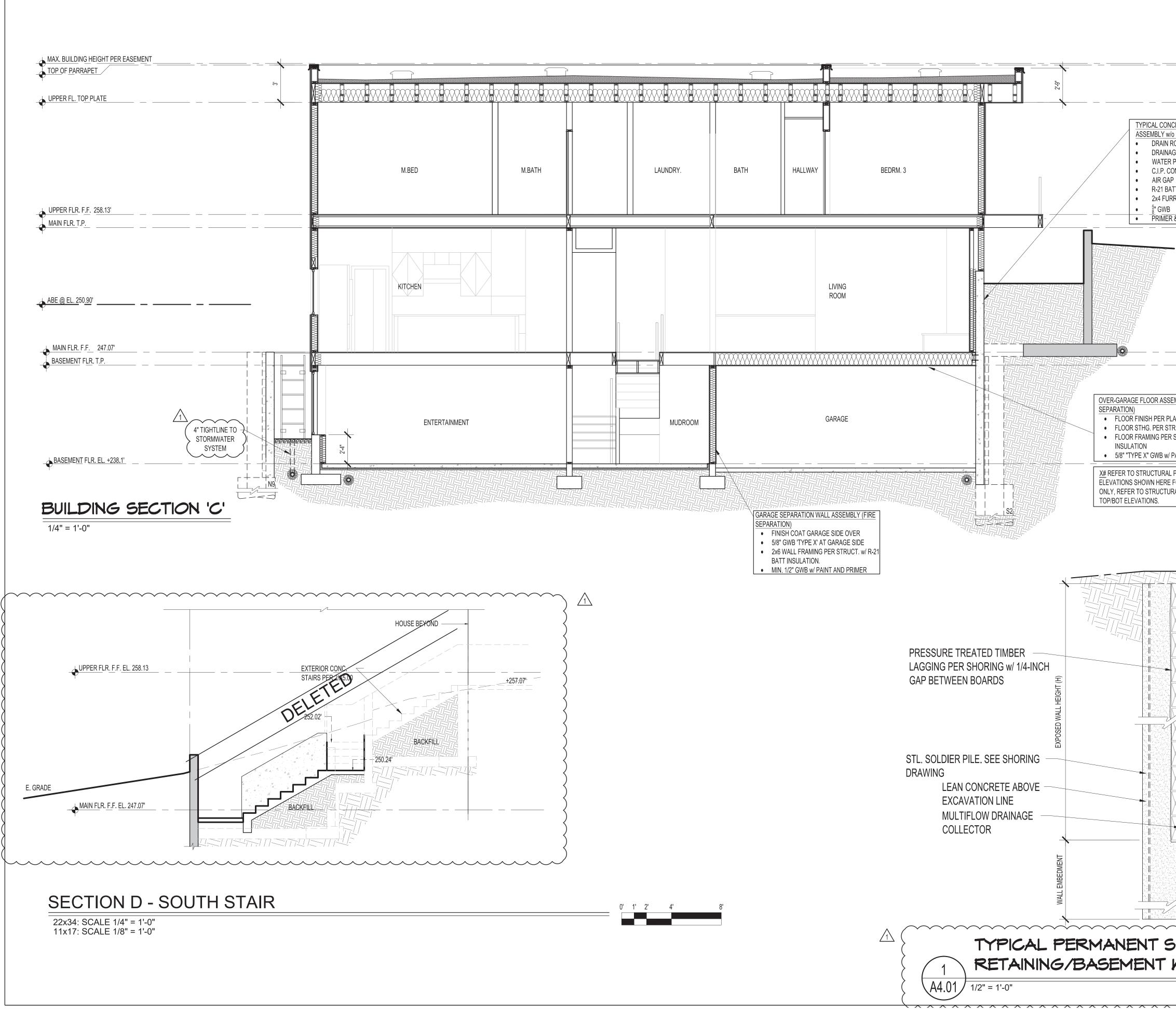
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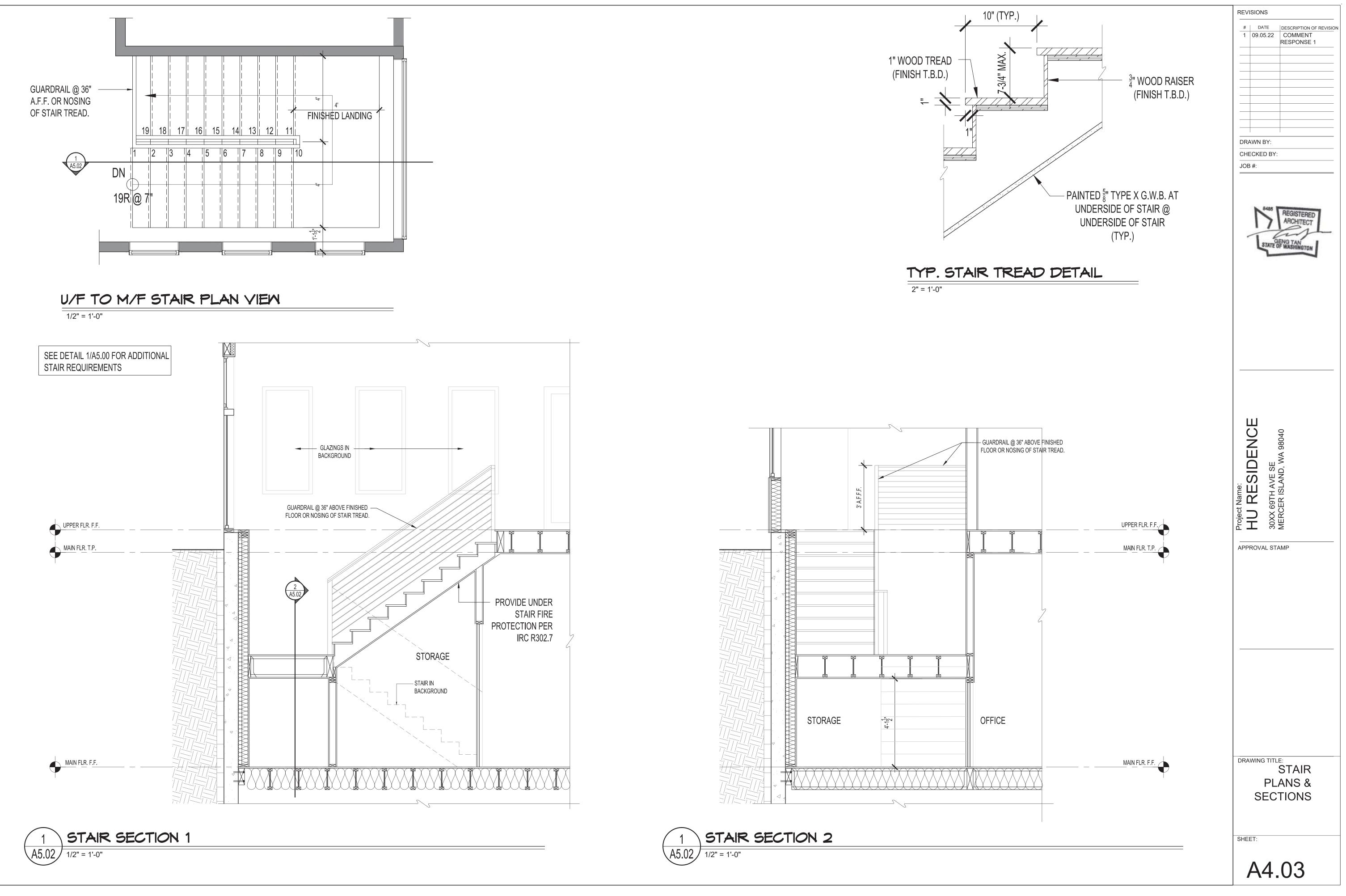


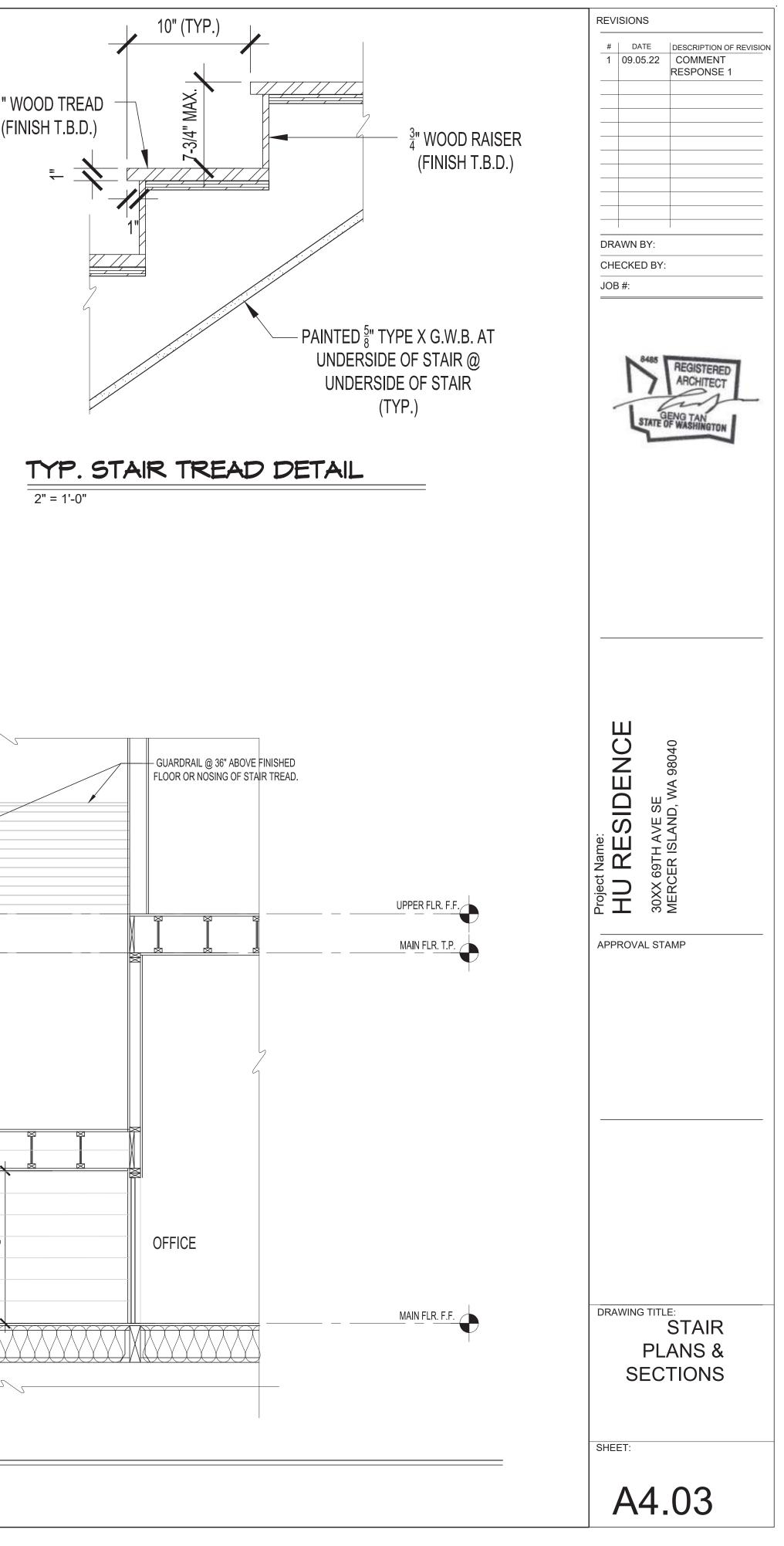


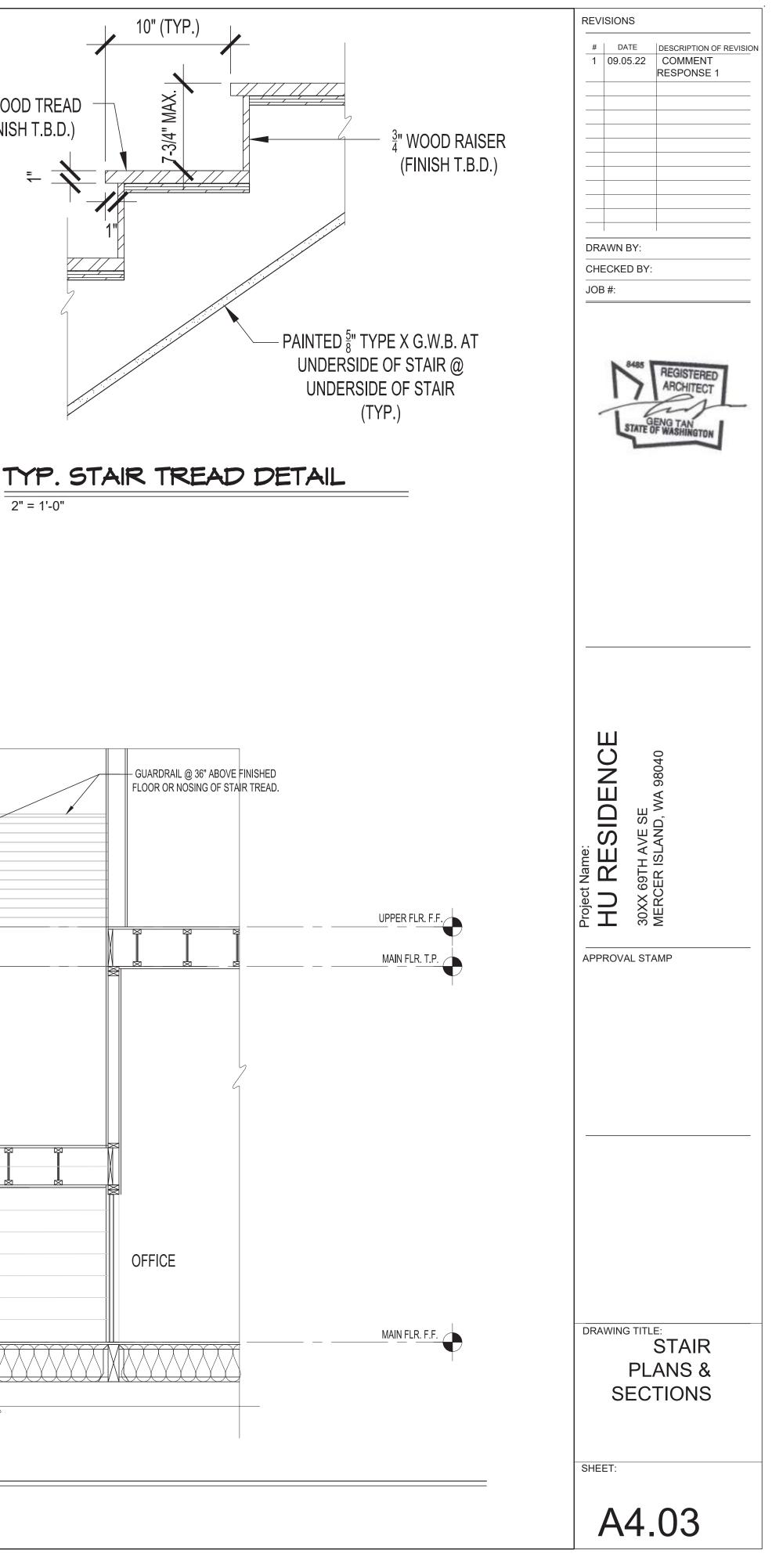


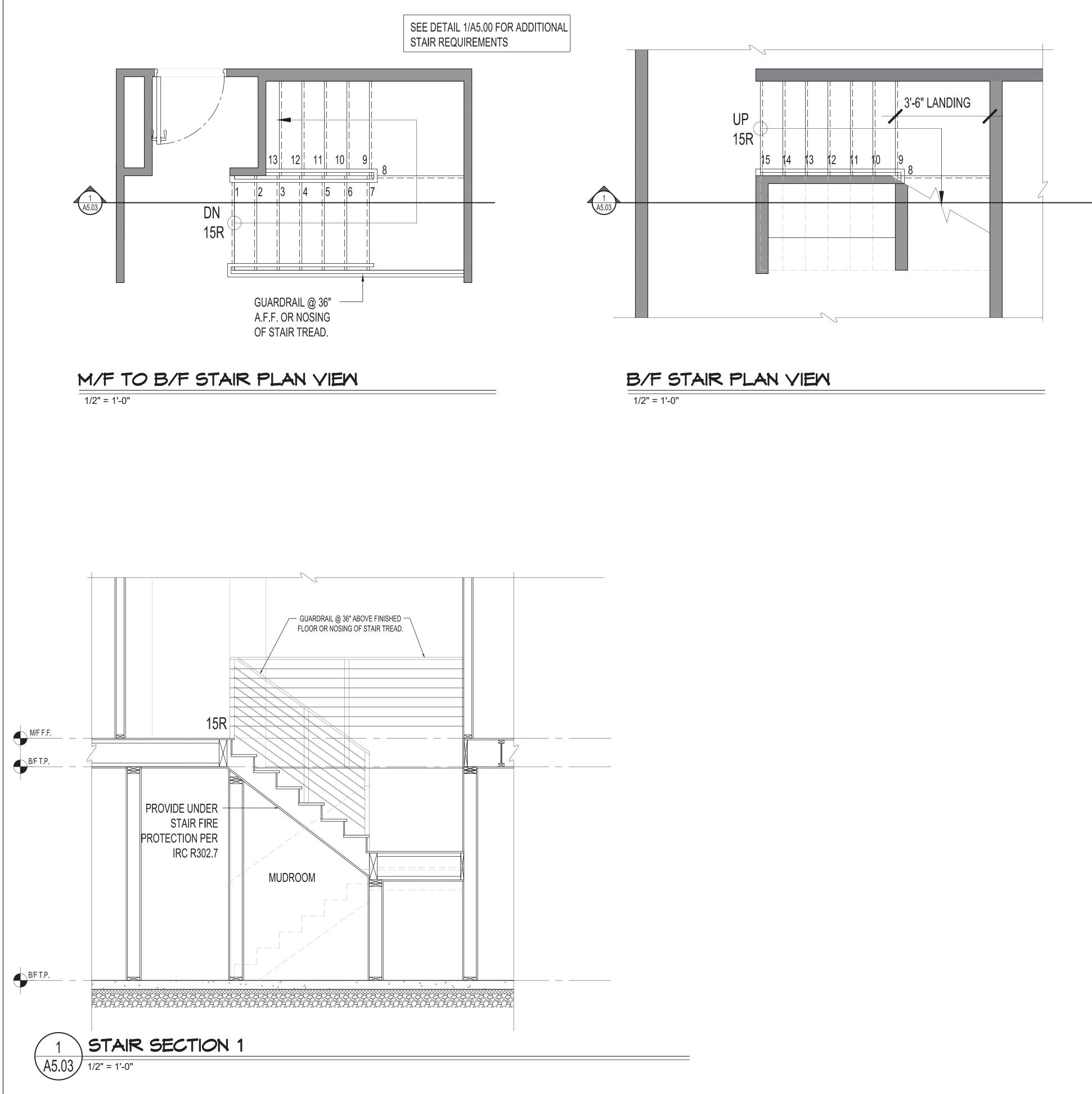


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_ PILE ID NUMBERS, FOR REFERENCE IRAL SCHEDULE FOR		38040 NC
	SEE STRUCTURAL DRAWINGS FOR RETAINING WALL & BASEMENT WALL DETAILS SEE SHORING DRAWINGS FOR SOLDIER PILE DETAILS REFER TO GEOTECH REPORT FOR DRAINAGE RECOMMENDATION	Project Name: HU RESIDENC 30XX 69TH AVE SE MERCER ISLAND, WA 98040
	MIRADRAIN DRAINAGE MATTING FULL HEIGHT & WIDTH CENTERED BETWEEN PIELS, INSTALLED WITH FABRIC TO LAGGING	APPROVAL STAMP
	- WATERPROOFING MEMBRANE ALONG LENGTH OF WALL ITERIOR	
	WELDED STUD PER STRUCT.	
	SLAB PER PLAN 4" DIA. PVC PIPE TIGHTLINED STORM DRAINAGE SYSTEM	ТО
	4" DIA. WEEP HOLE W/ 8' O.C. MAX THROUGH SHORING & CONC. WALL AND CONNECT TO 4" PVC PIPE PER SOILS REPORT	
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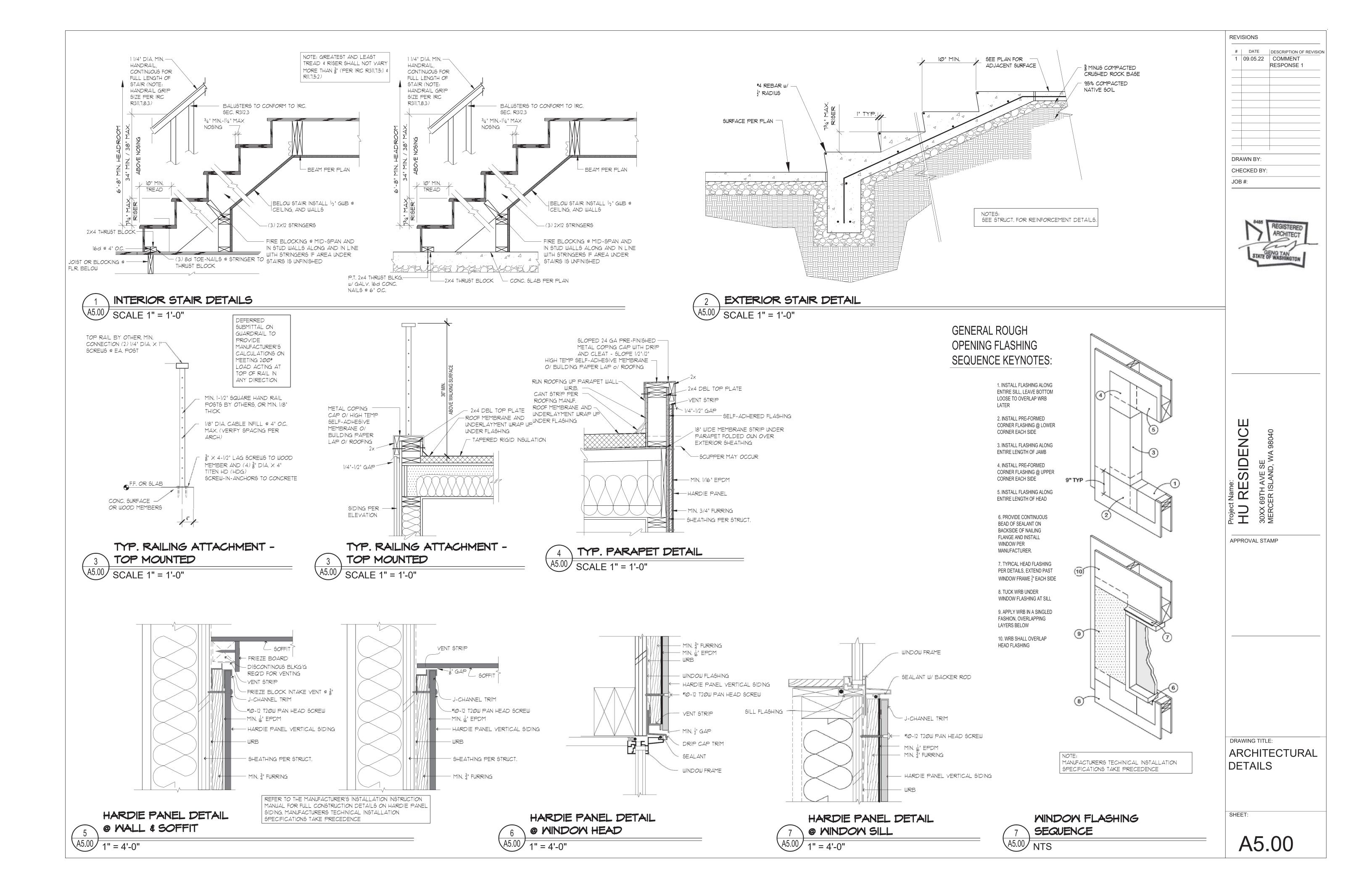


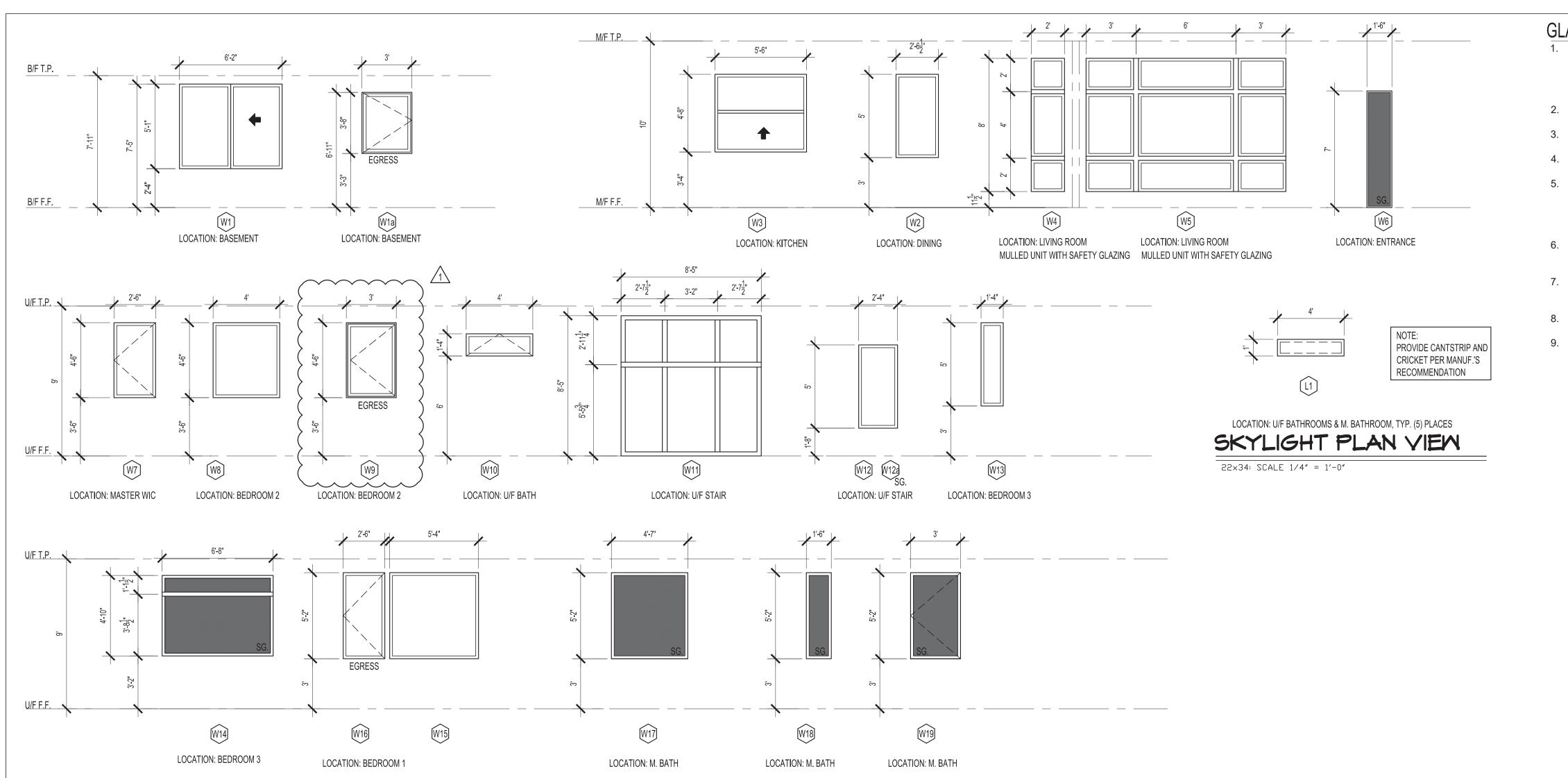






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SECTIONS		
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GLAZING NOTES

1. SAFETY GLAZING (SG.) TO BE PROVIDED WHERE REQUIRED BY IRC. REFER TO PLANS AND ELEVATIONS FOR SAFETY GLAZING LOCATIONS. EACH PANE OF SAFETY GLAZING SHALL BE IDENTIFIED BY A LABEL IN ACCORDANCE WITH IRC.

 DIMENSIONS INDICATED ON GLAZING ELEVATIONS REFER TO PRELIMINARY RO DIMENSIONS.
 ALL VERTICAL GLAZING TO HAVE AN AREA WEIGHTED AVERAGE U-FACTOR OF 0.30 PER WSEC, UNO.
 PROVIDE EXTERIOR TRIM AND MULL COVERS AS SHOWN

ON THE DIAGRAM.
5. CONTRACTOR TO FIELD VERIFY ALL WINDOW/DOOR ROUGH OPENINGS, DIMENSIONS AND CONDITIONS
DRIOP TO PROCEEDING WITH WINDOW MANUFACTURER

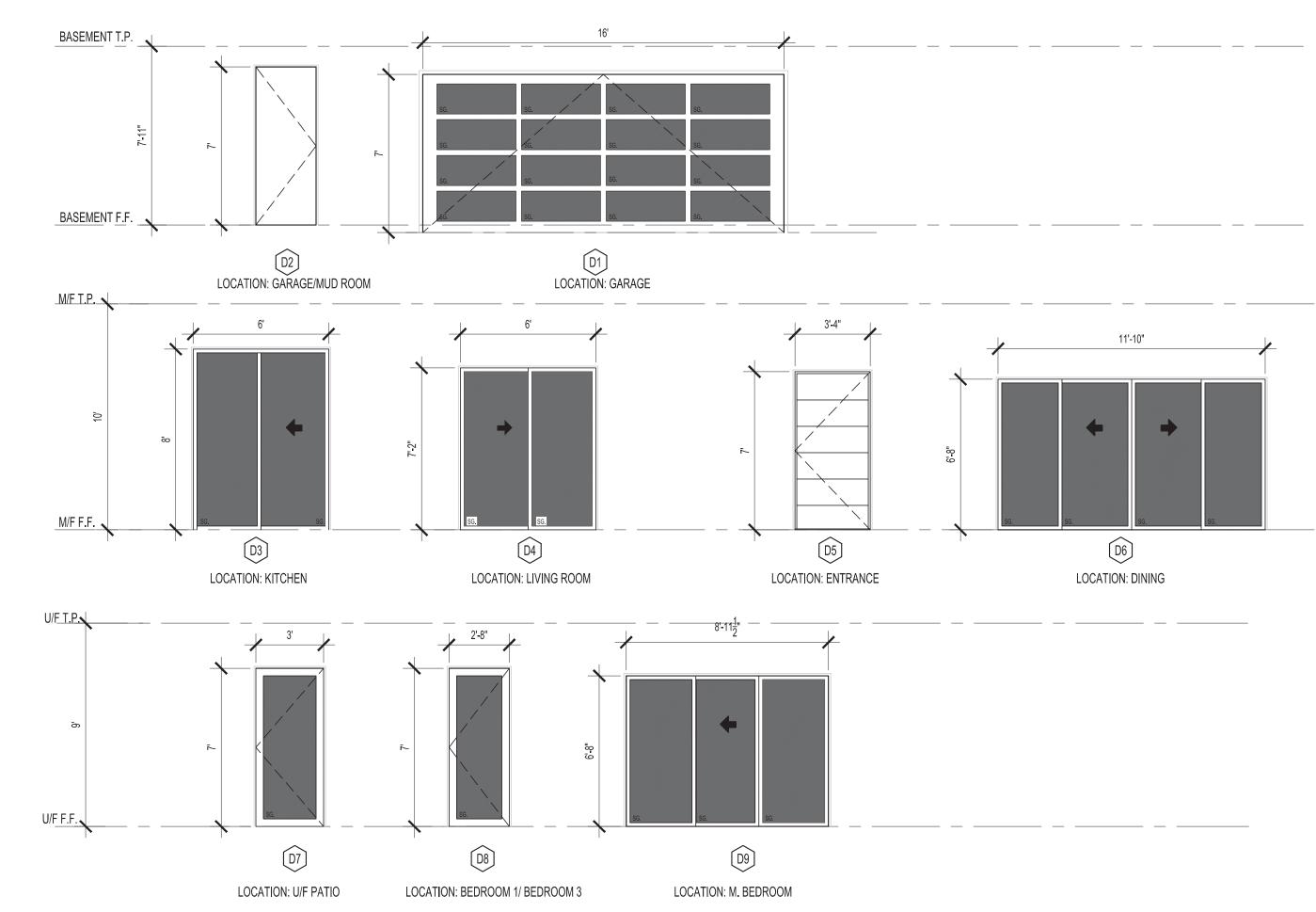
PRIOR TO PROCEEDING WITH WINDOW MANUFACTURER. NOTIFY ARCHITECT IMMEDIATELY OF ANY DISCREPANCIES.

6. WINDOW SUPPLIER/MANUFACTURER TO FIELD VERIFY ALL ROUGH OPENINGS, WINDOW DIVISIONS, AND OPERATION PRIOR TO FABRICATION.

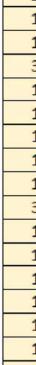
 ALL WINDOW FINISHES PER ARCHITECT. WINDOW SUPPLIER TO SUBMIT COLOR SAMPLE FOR APPROVAL BY ARCHITECT/OWNER PRIOR TO FABRICATION.
 ALL OPERABLE WINDOWS TO BE PROVIDED WITH SCREENS.

9. ALL WINDOWS TO BE NFRC CERTIFIED

REVI	SIONS	
#	DATE 09.05.22	DESCRIPTION OF REVISION
		RESPONSE 1
DRA	WN BY:	
	CKED BY:	
JOB	#.	
	No state o	REGISTERED ARCHITECT ENG TAN FWASHINGTON
	30XX 69TH AVE SE	MERCER ISLAND, WA 98040
		AERO
<u> </u>	പ്രതി	2
APPF	ROVAL STA	AMP
EL	EVAT	IUN
SHEE	ET:	
	46.	$\cap \cap$



			GLAZI	NG	SCHEDI	JLE										
		CONDI	TIONED F	LO	OR AREA:	4504				Sum	of UA	for H	leatir	ng Syster	n Sizing:	251.27
		SUM OF ALL GLAZING	AREAS FR	ON	A BELOW:	837.3										
		GLAZING TO	FLOOR	AR	EA RATIO:	0.186										
						\wedge										
EXTERIO	OR DOORS	(INCLUDE SOLID AND GLAZE	D DOOR)	1	\sim	$\sqrt{1}$		Width	ı	Heig	ht			Glazing	Door	Door
TAG	ROOM	ТҮРЕ	Ref.	5	U-factor	\langle	Qt.	Feet	nch	Feet		Feet	Inch	Area	Area	UA
D1	Garage	Metal w/ Glass Panels		Ś		<	1	16	0	7	0	0	0	0	112.0	0.00
D2	Garage	20-Min./Single Solid	WSEC	5	0.28	<	1	3	0	7	0	0	0	0	21.0	
D3	Kitchen	Double Glass Panels Slider	WSEC	5	0.28	<	1	6	0	8	0	0	1	48.0	48.0	
D4	Kitchen	Double Glass Panels Slider	WSEC	5	0.28	<	1	6	0	7	2	0		in the second second	43.0	
D5	Entry	Single Solid	WSEC	\mathbf{x}	0.28	\langle	1	3	4	7	0	0	-		23.3	6.53
D6	Dining	Four Panels Slider	WSEC	\mathbf{x}	0.28	<	1	11	10	6	8				78.9	22.09
D7	U/F Patio	Single Swing Lite	WSEC	\rightarrow	0.28	5	1	3	0	7	0	0		-	21.0	
D8	Bed 1&3	Sing Swing Lite	WSEC	\rightarrow	0.28	5	2	2	8	7	0	0		1	37.3	10.45
				\rightarrow		5	2	8	11.5	6	8					
D9	M. BED	(3) Glass Panels Slider	WSEC	\rightarrow	0.28	5	1	8		6	v	0	, v	59.7	59.7	16.72
		(1) Exempt Opaque Swinging Door (N		<u> </u>	0.30	5								207.0	222.2	02.0
					DODR ARE	A AND	UA (d	oes no	ot in				•		332.3	93.04
		SUM OF GLAZ	LING ARE	7	DOONANL	9					D					
				{		3		AREA						-		0.28
VERTICA	AL GLAZING	SUM OF GLAZ		{		3	M SIZ		Y (in Act		e exen			-	444.3	
VERTICA	AL GLAZING					3	M SIZ	E ONL	Y (in ACT	clude UAL S	e exer SIZE nt	npt d	oor):		-	
		SUM OF AREA	AND UA		OR HEATING	3	M SIZ	E ONL	Y (in ACT	clude UAL S Heigh	e exer SIZE nt	npt d Sill	oor):			93.04
TAG	ROOM	SUM OF AREA	AND UA Ref.		OR HEATING	3	M SIZ	E ONL Width Feet	Y (in ACT	Clude UAL S Heigh Feet	e exer SIZE nt	npt d Sill	oor):		Area	93.04 UA
TAG W1	ROOM Wet Bar	SUM OF AREA TYPE OX	AND UA Ref. WSEC		DR HEATING U-factor 0.28	3	M SIZ	E ONL Width Feet	Y (in ACT nch	UAL S Heigh Feet	e exer SIZE nt	Sill Feet	oor):		Area 31.3	93.04 UA 8.78
TAG W1 W1a	ROOM Wet Bar Guest	SUM OF AREA TYPE OX Casement Fixed Fixed	AND UA Ref. WSEC WSEC		DR HEATING U-factor 0.28 0.28 0.28 0.28	3	M SIZ Qt.	E ONL Width Feet	Y (in ACT nch 2 0 0 0	UALS Heigh Feet 5 3 5 4	e exen	npt d Sill Feet 2 3	00r):		Area 31.3 11.0 30.0 33.3	93.04 UA 8.78 3.08
TAG W1 W1a W3 W2 W4	ROOM Wet Bar Guest Kitchen Dining Living	SUM OF AREA TYPE OX Casement Fixed Fixed Fixed. Mulled Unit. SG.	AND UA Ref. WSEC WSEC WSEC WSEC WSEC		DR HEATIN U-factor 0.28 0.28 0.28 0.28 0.28 0.28	3	M SIZ	E ONL Width Feet	Y (in ACT nch 2 0 0 0	UALS Heigh Feet 5 3 5 4 8	e exen	sill Feet 2 3 3 2 0	OOT): Inch 4 3 0 6 11.5		Area 31.3 11.0 30.0 33.3 16.0	93.04 UA 8.78 3.08 8.40 9.33 4.48
TAG W1 W1a W3 W2 W4 W5	ROOM Wet Bar Guest Kitchen Dining Living Living	SUM OF AREA TYPE OX Casement Fixed Fixed Fixed. Mulled Unit. SG. Fixed. Mulled Unit. SG.	AND UA Ref. WSEC WSEC WSEC WSEC WSEC		DR HEATIN U-factor 0.28 0.28 0.28 0.28 0.28 0.28 0.28	3	Ot.	Width Feet	Y (in ACT nch 2 0 0 0 0 0	UAL S Heigh Feet 5 3 5 4 8 8 8	e exen SIZE Int Inch 1 8 0 2 0 0 0	Sill Feet 2 3 2 0 0 0	00r):		Area 31.3 11.0 30.0 33.3 16.0 96.0	93.04 UA 8.78 3.08 8.40 9.33 4.48 26.88
TAG W1 W1a W3 W2 W2 W4 W5 W6	ROOM Wet Bar Guest Kitchen Dining Living Living Living Entry	SUM OF AREA TYPE OX Casement Fixed Fixed Fixed Fixed. Mulled Unit. SG. Fixed. Mulled Unit. SG. Fixed. SG.	AND UA Ref. WSEC WSEC WSEC WSEC WSEC WSEC		DR HEATINO U-factor 0.28 0.28 0.28 0.28 0.28 0.28 0.28 0.28	3	Ot.	E ONL Width Feet	Y (in ACT nch 2 0 0 0 0 0 0 0	UAL S Heigh Feet 5 3 5 4 8 8 8 7	e exen	sill Feet 2 3 3 2 0 0 0 0	OOT): Inch 4 3 0 6 11.5		Area 31.3 11.0 30.0 33.3 16.0 96.0 31.5	93.04 UA 8.78 3.08 8.40 9.33 4.48 26.88 8.82
TAG W1 W1a W3 W2 W4 W5 W6 W7	ROOM Wet Bar Guest Kitchen Dining Living Living Living Entry MSTR WIC	SUM OF AREA TYPE OX Casement Fixed Fixed Fixed Fixed. Mulled Unit. SG. Fixed. SG. Fixed. SG. Casement.	AND UA Ref. WSEC WSEC WSEC WSEC WSEC WSEC WSEC		DR HEATINO U-factor 0.28 0.28 0.28 0.28 0.28 0.28 0.28 0.28	3	Ot.	E ONL Width Feet	Y (in ACT nch 2 0 0 0 0 0 6 6 6	CUAL S Heigh Feet 5 3 5 4 8 8 7 4 8 7 4	e exen SIZE Int Inch 1 8 0 2 0 0 0	sill Feet 2 3 3 2 0 0 0 3	OOT): Inch 4 3 0 6 11.5		Area 31.3 11.0 30.0 33.3 16.0 96.0 31.5 11.3	93.04 UA 8.78 3.08 8.40 9.33 4.48 26.88 8.82 3.15
TAG W1 W1a W3 W2 W4 W5 W6 W7 W8	ROOM Wet Bar Guest Kitchen Dining Living Living Entry MSTR WIC Bedroom2	SUM OF AREA TYPE OX Casement Fixed Fixed Fixed. Mulled Unit. SG. Fixed. Mulled Unit. SG. Fixed. SG. Casement. Fixed.	AND UA Ref. WSEC WSEC WSEC WSEC WSEC WSEC WSEC WSEC		DR HEATINO U-factor 0.28 0.28 0.28 0.28 0.28 0.28 0.28 0.28	3	Ot.	E ONL Width Feet 4 6 3 6 4 2 12 12 1 2 4	Y (in ACT nch 2 0 0 0 0 0 6 6 6 0	Clude UAL S Heigh Feet 5 3 5 4 8 8 8 7 4 4 4 4 4	e exen	sill Feet 2 3 3 2 0 0 0 3 3 3 3	OOT): Inch 4 3 0 6 11.5		Area 31.3 11.0 30.0 33.3 16.0 96.0 31.5 11.3 18.0	93.04 UA 8.78 3.08 8.40 9.33 4.48 26.88 8.82 3.15 5.04
TAG W1 W1a W3 W2 W4 W5 W6 W7 W8 W9	ROOM Wet Bar Guest Kitchen Dining Living Living Entry MSTR WIC Bedroom2 Bedroom2	SUM OF AREA TYPE OX Casement Fixed Fixed Fixed Fixed. Mulled Unit. SG. Fixed. Mulled Unit. SG. Fixed. SG. Casement. Fixed. Casement	AND UA Ref. WSEC WSEC WSEC WSEC WSEC WSEC WSEC WSEC		DR HEATINO U-factor 0.28 0.28 0.28 0.28 0.28 0.28 0.28 0.28	3	Ot.	E ONL Width Feet	Y (in ACT nch 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Clude UAL S Heigh Feet 5 3 5 4 8 8 8 8 7 4 4 4 4 4 4	e exen	npt d Sill Feet 2 3 3 2 0 0 0 0 0 3 3 3 3 3	OOT): Inch 4 3 0 6 11.5		Area 31.3 11.0 30.0 33.3 16.0 96.0 31.5 11.3 18.0 13.5	93.04 UA 8.78 3.08 8.40 9.33 4.48 26.88 8.82 3.15 5.04 3.78
TAG W1 W1a W3 W2 W4 W5 W6 W7 W6 W7 W8 W9 W10	ROOM Wet Bar Guest Kitchen Dining Living Living Living Entry MSTR WIC Bedroom2 Bedroom2 U/F Bath	SUM OF AREA OX Casement Fixed Fixed Fixed Fixed. Mulled Unit. SG. Fixed. Mulled Unit. SG. Fixed. SG. Casement. Fixed. Casement Awning	AND UA Ref. WSEC WSEC WSEC WSEC WSEC WSEC WSEC WSEC		DR HEATINO U-factor 0.28 0.28 0.28 0.28 0.28 0.28 0.28 0.28	3	Ot.	E ONL Width Feet	Y (in ACT nch 2 0 0 0 0 0 6 6 6 0	Clude UAL S Heigh Feet 5 3 5 4 8 8 8 7 4 4 4 4 4 1	e exen	npt d Sill Feet 2 3 3 2 0 0 0 0 3 3 3 6	OOT): Inch 4 3 0 6 11.5 11.5 0 6 6 6 6		Area 31.3 11.0 30.0 33.3 16.0 96.0 31.5 11.3 18.0 13.5 5.3	93.04 UA 8.78 3.08 8.40 9.33 4.48 26.88 8.82 3.15 5.04 3.78 1.49
TAG W1 W1a W3 W2 W4 W5 W6 W7 W6 W7 W8 W9 W9 W10 W11	ROOM Wet Bar Guest Kitchen Dining Living Living Entry MSTR WIC Bedroom2 Bedroom2 U/F Bath U/F Stair	SUM OF AREA OX Casement Fixed Fixed Fixed Fixed. Mulled Unit. SG. Fixed. Mulled Unit. SG. Fixed. SG. Casement. Fixed. Casement Awning Fixed. Mulled Unit. SG.	AND UA Ref. WSEC WSEC WSEC WSEC WSEC WSEC WSEC WSEC		DR HEATINO U-factor 0.28 0.28 0.28 0.28 0.28 0.28 0.28 0.28	3	Ot. 1 1 1 1 1 1 1 1 1 1 1 1 1	E ONL Feet	Y (in ACT nch 2 0 0 0 0 0 0 6 6 6 0 0 0 0 0 0 0 0 0 0	Clude UAL S Heigh Feet 5 3 5 4 8 8 7 4 4 4 4 4 4 4 1 8	e exen SIZE Inch 1 8 0 2 0 0 0 0 6 6 6 6 4	npt d Sill Feet 2 3 3 2 0 0 0 0 0 3 3 3 3 3	OOT): Inch 4 3 0 6 11.5 11.5 0 6 6 6 6		Area 31.3 11.0 30.0 33.3 16.0 96.0 31.5 11.3 18.0 13.5 5.3 70.8	93.04 UA 8.78 3.08 8.40 9.33 4.48 26.88 8.82 3.15 5.04 3.78 1.49 19.84
TAG W1 W1a W3 W2 W4 W5 W6 W7 W6 W7 W8 W7 W8 W9 W10 W11 W12	ROOM Wet Bar Guest Kitchen Dining Living Living Entry MSTR WIC Bedroom2 Bedroom2 U/F Bath U/F Stair	SUM OF AREA TYPE OX Casement Fixed Fixed Fixed. Mulled Unit. SG. Fixed. Mulled Unit. SG. Fixed. SG. Casement. Fixed. Casement Awning Fixed. Mulled Unit. SG. Fixed. Casement Awning Fixed. Mulled Unit. SG.	AND UA Ref. WSEC WSEC WSEC WSEC WSEC WSEC WSEC WSEC		DR HEATINO U-factor 0.28 0.28 0.28 0.28 0.28 0.28 0.28 0.28	3	Ot.	E ONL Width Feet	Y (in ACT nch 2 0 0 0 0 0 6 6 6 0 0 0 5	Clude UAL S Heigh Feet 5 3 5 4 8 8 8 7 4 4 4 4 4 4 1 8 5 5	e exen SIZE Inch 1 8 0 2 0 0 0 0 6 6 6 4 5	npt d Sill Feet 2 3 3 2 0 0 0 0 0 3 3 3 6 0 1	OOT): Inch 4 3 0 6 11.5 11.5 0 6 6 6 6		Area 31.3 11.0 30.0 33.3 16.0 96.0 31.5 11.3 18.0 13.5 5.3 70.8 35.0	93.04 UA 8.78 3.08 8.40 9.33 4.48 26.88 8.82 3.15 5.04 3.78 1.49 19.84 9.80
TAG W1 W1a W3 W2 W4 W5 W6 W7 W6 W7 W8 W9 W10 W11 W12 W13	ROOM Wet Bar Guest Kitchen Dining Living Living Living Entry MSTR WIC Bedroom2 Bedroom2 U/F Bath U/F Stair U/F Stair Bedroom3	SUM OF AREA OX Casement Fixed Fixed Fixed Fixed. Mulled Unit. SG. Fixed. Mulled Unit. SG. Fixed. SG. Casement. Fixed. Casement Awning Fixed. Mulled Unit. SG. Fixed.	AND UA Ref. WSEC WSEC WSEC WSEC WSEC WSEC WSEC WSEC		DR HEATINO U-factor 0.28 0.28 0.28 0.28 0.28 0.28 0.28 0.28	3	Ot. 1 1 1 1 1 1 1 1 1 1 1 1 1	E ONL Width Feet	Y (in ACT nch 2 0 0 0 0 0 0 6 6 6 0 0 0 0 5 4	Clude UAL S Heigh Feet 5 3 5 4 8 8 7 4 8 8 7 4 4 4 4 4 1 8 5 5 5 5 5	e exen SIZE Inch 1 8 0 2 0 0 0 0 0 6 6 6 6 6 6 6 4 5 0	npt d Sill Feet 2 3 3 2 0 0 0 0 3 3 3 6 0 1 3 3 6 0 1 3	OOT): Inch 4 3 0 6 11.5 11.5 0 6 6 6 6 6 0 0 8		Area 31.3 11.0 30.0 33.3 16.0 96.0 31.5 11.3 18.0 13.5 5.3 70.8 35.0 6.7	93.04 UA 8.78 3.08 8.40 9.33 4.48 26.88 8.82 3.15 5.04 3.78 1.49 19.84 9.80 1.87
TAG W1 W1a W3 W2 W4 W5 W6 W7 W6 W7 W6 W7 W8 W9 W10 W11 W11 W12 W13 W14	ROOM Wet Bar Guest Kitchen Dining Living Living Living Entry MSTR WIC Bedroom2 Bedroom2 U/F Bath U/F Stair U/F Stair Bedroom3	TYPE OX Casement Fixed Fixed Fixed Unit. SG. Fixed. Mulled Unit. SG. Fixed. SG. Casement. Fixed. Casement Awning Fixed. Mulled Unit. SG. Fixed. Casement Awning Fixed. Fixed. Casement Fixed. Casement Casement Fixed. Fixed. Casement Fixed. Fixed. Casement Fixed.	Ref. WSEC WSEC WSEC WSEC WSEC WSEC WSEC WSEC		U-factor 0.28 0.28 0.28 0.28 0.28 0.28 0.28 0.28	3	Ot. 1 1 1 1 1 1 1 1 1 1 1 1 1	E ONL Width Feet	Y (in ACT nch 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Clude UAL S Heigh Feet 5 3 5 4 8 8 7 4 8 8 7 4 4 4 4 1 8 5 5 5 4	e exen SIZE Inch 1 8 0 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	npt d Sill Feet 2 3 3 2 0 0 0 0 0 0 3 3 3 6 0 0 1 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	OOT): Inch 4 3 0 6 11.5 11.5 0 6 6 6 6 6 0 0 8		Area 31.3 11.0 30.0 33.3 16.0 96.0 31.5 11.3 18.0 13.5 5.3 70.8 35.0 6.7 32.2	93.04 UA 8.78 3.08 8.40 9.33 4.48 26.88 8.82 3.15 5.04 3.78 1.49 19.84 9.80 1.87 9.02
TAG W1 W1a W3 W2 W4 W5 W6 W7 W6 W7 W8 W9 W10 W11 W12 W13	ROOM Wet Bar Guest Kitchen Dining Living Living Living Entry MSTR WIC Bedroom2 Bedroom2 U/F Bath U/F Stair U/F Stair Bedroom3 Bedroom3	SUM OF AREA OX Casement Fixed Fixed Fixed Fixed. Mulled Unit. SG. Fixed. Mulled Unit. SG. Fixed. SG. Casement. Fixed. Casement Awning Fixed. Mulled Unit. SG. Fixed.	AND UA Ref. WSEC WSEC WSEC WSEC WSEC WSEC WSEC WSEC		DR HEATINO U-factor 0.28 0.28 0.28 0.28 0.28 0.28 0.28 0.28	3	Ot. 1 1 1 1 1 1 1 1 1 1 1 1 1	E ONL Width Feet	Y (in ACT nch 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Clude UAL S Heigh Feet 5 3 5 4 8 8 7 4 8 8 7 4 4 4 4 4 1 8 5 5 5 5 5	e exen SIZE Inch 1 8 0 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	npt d Sill Feet 2 3 3 2 0 0 0 0 3 3 3 6 0 1 3 3 6 0 1 3	OOT): Inch 4 3 0 6 11.5 11.5 0 6 6 6 6 6 0 0 8		Area 31.3 11.0 30.0 33.3 16.0 96.0 31.5 11.3 18.0 13.5 5.3 70.8 35.0 6.7	93.04 UA 8.78 3.08 8.40 9.33 4.48 26.88 8.82 3.15 5.04 3.78 1.49 19.84 9.80 1.87
TAG W1 W1a W3 W2 W4 W5 W6 W7 W6 W7 W8 W9 W10 W10 W11 W12 W11 W12 W13 W14 W15	ROOMWet BarGuestKitchenDiningLivingLivingBedroom2Bedroom2U/F StairU/F StairBedroom3Bedroom3Bedroom4	TYPE OX Casement Fixed Fixed Fixed Unit. SG. Fixed. Mulled Unit. SG. Fixed. SG. Casement. Fixed. Casement Awning Fixed. Mulled Unit. SG. Fixed. Casement Awning Fixed. Fix	Ref. WSEC WSEC WSEC WSEC WSEC WSEC WSEC WSEC		U-factor 0.28 0.28 0.28 0.28 0.28 0.28 0.28 0.28	3	Ot. 1 1 1 1 1 1 1 1 1 1 1 1 1	E ONL Width Feet 4 6 3 6 4 2 12 1 2 1 2 4 3 4 3 4 3 4 3 4 3 4 3 4 3 4 3 4 5 5	Y (in ACT nch 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	clude UAL S Heigh Feet 5 3 5 4 8 8 7 4 8 8 7 4 4 4 4 4 4 4 4 1 8 5 5 5 4 5 5 4 5 5 5 4 5 5 5 5 5 5 5 5	e exen SIZE Int Inch 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	npt d Sill Feet 2 3 3 2 0 0 0 0 0 0 3 3 3 6 0 0 1 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	OOT): Inch 4 3 0 6 11.5 11.5 0 6 6 6 6 6 0 0 8		Area 31.3 11.0 30.0 33.3 16.0 96.0 31.5 11.3 18.0 13.5 5.3 70.8 35.0 6.7 32.2 27.6	93.04 UA 8.78 3.08 8.40 9.33 4.48 26.88 8.82 3.15 5.04 3.78 1.49 19.84 9.80 1.87 9.80 1.87 9.02 7.72
TAG W1 W1a W3 W2 W4 W5 W6 W7 W6 W7 W8 W9 W10 W11 W12 W11 W12 W11 W12 W13 W14 W15 W16	ROOMWet BarGuestGuestKitchenDiningLivingLivingEntryMSTR WICBedroom2U/F BathU/F StairU/F StairBedroom3Bedroom1Bedroom1	TYPE OX Casement Fixed Fixed Fixed Fixed. Mulled Unit. SG. Fixed. Mulled Unit. SG. Fixed. SG. Casement. Fixed. Casement Awning Fixed. Casement Awning Fixed. Casement Fixed. Casement. Fixed. F	AND UA Ref. WSEC WSEC WSEC WSEC WSEC WSEC WSEC WSEC		U-factor 0.28 0.28 0.28 0.28 0.28 0.28 0.28 0.28	3	Ot. 1 1 1 1 1 1 1 1 1 1 1 1 1	E ONL Width Feet 1 6 3 6 4 2 12 1 2 4 3 4 3 4 3 4 3 4 3 4 3 4 3 4 3 4 3 4 5 2 1 6 5 2	Y (in ACT nch 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	clude UAL S Heigh Feet 5 3 5 4 8 8 7 4 8 8 7 4 4 4 4 4 4 1 8 5 5 4 5 5 4 5 5 4 5 5 5 5 5 5 5 5 5 5	e exen SIZE Inch 1 8 0 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	npt d Sill Feet 2 3 3 2 0 0 0 0 0 0 0 0 0 3 3 3 6 0 0 1 3 3 6 0 0 1 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3 3	OOT): Inch 4 3 0 6 11.5 11.5 0 6 6 6 6 6 0 0 8		Area 31.3 11.0 30.0 33.3 16.0 96.0 31.5 11.3 18.0 13.5 5.3 70.8 35.0 6.7 32.2 27.6 12.9	93.04 UA 8.78 3.08 8.40 9.33 4.48 26.88 8.82 3.15 5.04 3.78 1.49 19.84 9.80 1.87 9.02 7.72 3.62



OVERHEAD GLAZING

TAG	ROOM	TYPE	Ref.	U-factor
SL1	U/F PLAN	Skylight	WSEC	0.50

1. ALL DIAGRAMS ARE SHOWN FROM THE EXTERIOR SIDE. 2. CONTRACTOR TO CONFIRM ALL REQUIRED ROUGH OPENING SIZES WITH MANUFACTURER PRIOR TO FRAMING.

3. SHOP DRAWING APPROVAL BY ARCHITECT REQUIRED PRIOR TO FABRICATION.

4. MANUFACTURER TO REVIEW INSTALLATION LOCATIONS AND DETERMINE WHICH LITES ARE REQUIRED TO BE SAFETY GLAZING.

5. MANUFACTURER TO REVIEW INSTALLATION LOCATIONS AND SIZES TO DETERMINE IF OPERABLE DOORS MEET EGRESS REQUIREMENTS.

6. ALL DOORS TO BE NFRC CERTIFIED.

Qt.	Feet	Inch	Feet	Inch	Feet	Inch
5	1	0	4	0		

SUM OF AREA AND UA: AREA WEIGHTED U = UA/AREA

Area	UA
20.0	10.00
2	
20.0	10.00

2010	10.00
- 5.	0.50

REVISIONS # DATE DESCRIPTION OF REVISION 1 09.05.22 COMMENT **RESPONSE 1** ____

DRAWN BY: CHECKED BY: JOB #:



t Name: RESIDENCE 0 600 69TH AVE SE CER ISLAND, WA (Project HU 30XX 6 MERCE

APPROVAL STAMP

DRAWING TITLE: GLAZING ELEVATION

SHEET:

A6.01

GENERAL STRUCTURAL NOTES:

(THE FOLLOWING NOTES APPLIES TO THE PROPOSED PROJECT UNLESS OTHERWISE NOTED ON THE PLANS AND DETAILS)

ALL DESIGN AND CONSTRUCTION SHALL COMPLY WITH THE 2015 INTERNATIONAL BUILDING CODE

DESIGN LOADING CRITERIA:

1. DESIGN LOADS:

ROOF SNOW LOAD: FLOOR LIVE LOAD: DECK LIVE LOAD: WIND: SEISMIC: 25 PSF 40 PSF 60 PSF 110-MPH (3-SECOND GUST), EXPOSURE B, Kzt=2.00 SEISMIC USER GROUP I, I=1.0, SITE CLASS SD R=6.5 (WOOD SHEAR WALL) $\Omega o=3.0$ Cd=4.0 Ss=1.476; S1=0.501, Fa=1.000; Fv=1.500, SDS=0.984; SDI=0.501

DESIGN SOIL PRESSURE:

2,000 PSF MAXIMUM DEAD+LIVE LOAD WITH A ONE-THIRD INCREASE ALLOWED UNDER THE SHORT-TERM WIND OR SEISMIC LOADS. CAST FOOTING ON NATIVE SITE SOILS OR STRUCTURAL FILL THAT EXTENDS DOWN TO THESE SOILS.

0.35 (SF=1.5)

COEFFICIENT OF FRICTION FOR FOUNDATIO BASE FRICTION EQUIVALENT FLUID PRESSURE EARTH PRESSURE FOR YIELDING/ACTIVE CONDITION WALLS EARTH PRESSURE FOR NON-YIELDING/AT-REST CONDITION WALLS

SEISMIC EARTH PRESSURE FOR PERMANENT SHORING WALLS

200 PCF (SF=2.0) EXCLUSING THE TOP 2FT OF SOIL FOR SHORING PILES 40 PCF 60 PCF

8H; WHERE H: EXPOSED HEIGHT OF THE SHORING WALL

REFER TO GEOTECHNICAL REPORT AND FOLLOW UP MEMO DATED JULY 10, 2020 AND NOVEMBER 11, 2020 RESPECTIVELY BY NELSON GEOTHCNICAL ASSOCIATES, INC. FOR SPECIFICS.

CONCRETE AND FOUNDATION CONSTRUCTIONS:

1. ALL CONCRETE f'c=3,000 PSI (2,500 PSI USED FOR THE DESIGN), MAXIMUM WATER/CEMENT RATIO =0.45, MINIMUM 5-1/2 SACKS OF CEMENT PER CUBIC YARD. NO SPECIAL INSPECTION REQUIRED. CONCRETE BATCH TICKET OR DELIVERY RECEIPT FOR 3,000 PSI MINIMUM CONCRETE ON SITE FOR BUILDING INSPECTOR VERIFICATION. CONCRETE SHALL BE AIR ENTRAINED. TOTAL AIR CONTENT (PERCENT BY VOLUME OF CONCRETE) SHALL NOT BE LESS THAN 5 PERCENT OR MORE THAN 7 PERCENT.

2. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60. NO SPECIAL INSPECTION REQUIRED. ASTM A706, GRADE 60, REINFORCING STEEL SHALL BE USED FOR WELDED OR FIELD-BENT BARS, SHEAR WALL BOUNDARY MEMBER REINFORCING, MAIN REINFORCING, SPIRALS, TIES AND STIRRUPS IN THE FRAME MEMBERS (BEAMS AND COLUMNS) COMPRISING THE LATERAL FORCE RESISTING SYSTEM.

3. WELDED WIRE FABRIC PER ASTM A185. FURNISH IN FLAT SHEETS, NOT ROLLS. LAP EDGES 1-1/2 MESH MINIMUM.

4. PROVIDE CONCRETE COVER AS FOLLOWS: FOOTINGS 3", WALLS 1-1/2", AND SLAB ON GRADE 1-1/2".

5. PROVIDE 2#4 LONGITUDINAL BOTTOM BARS IN WALL FOOTINGS. PROVIDE CORNER BARS OF SAME SIZE AND NUMBER AT CORNERS AND INTERSECTIONS, 42 BAR DIAMETERS EACH LEG. PROVIDE VERTICAL DOWELS OF SAME SIZE, NUMBER AND SPACING AS CONCRETE STEM WALL VERTICAL BARS WITH A 90 DEGREE STANDARD HOOK AT THE BOTTOM OF THE FOOTING.

6. REINFORCING CONCRETE WALLS AS FOLLOWS"

6" WALLS, #4 @ 12" HORIZONTAL AND VERTICAL AT CENTER OF WALL,

- 8" WALLS, #5 @ 15" OR #4 @ 12" HORIZONTAL AND VERTICAL AT CENTER OF WALL,
- 10" WALLS, #4 @ 16" HORIZONTAL AND VERTICAL AT EACH FACE, 12" WALLS, #4 @ 12" HORIZONTAL AND VERTICAL AT EACH FACE.

AT OPENINGS OVER 12" SQUARE, PROVIDE 2#5 BARS AT CENTER OF WALL ALL FOUR SIDES, EXCEPT 10" WALLS OR OVER PROVIDE 1#6 BAR EACH FACE ALL FOUR SIDES, EXTENDING 42 BAR DIAMETERS PAST OPENING. PROVIDE 1#5X4'-0" DIAGONAL BAR AT CENTER OF WALL ALL FOUR CORNERS.

AT CORNERS, PROVIDE CORNER BARS IN OUTSIDE FACE OF SAME SIZE AND SPACING AS HORIZONTAL BARS, 42 BAR DIAMETER EACH LEG.

AT INTERSECTIONS, PROVIDE CORNER BARS OF SAME SIZE, NUMBER AND SPACING AS HORIZONTAL BARSOF INTERSECTING WALL, 42 BAR DIAMETER EACH LEG.

PROVIDE 2#4 LONGITUDINAL BARS AT TOP OF WALLS. PROVIDE KEYWAY OR ROUGHENED SURFACE AT CONSTRUCTION JOINTS.

PROVIDE VERTICAL DOWELS OF SAME SIZE, NUMBER AND SPACING AS VERTICAL BARS.

7. GROUT – 5000 PSI MINIMUM 7-DAY CUBE STRENGTH PER ASTM C1157-00. GROUT TO BE PREMIXED, NON-SHRINK "MASTERFLOW 928 GROUT" BY MASTER BUILDERS OR APPROVED EQUAL. ICC CERTIFICATION REQUIRED. USE SPECIFIC GROUT MIX RECOMMENDED BY MANUFACTURER FOR EACH GROUT APPLICATION AND FOLLOW MANUFACTURER'S INSTRUCTIONS.

8. ANCHOR BOLTS, ASTM A307. NO SPECIAL INSPECTION REQUIRED. SET ALL ANCHOR BOLTS BY TEMPLATE WHEREVER POSSIBLE.

9. DRILL-IN EXPANSION BOLTS, "KWIK-BOLT TZ" BY HILTI FASTENING SYSTEMS BY HILTI FASTENING SYSTEM OR APPROVED EQUAL. ICC CERTIFICATION REQUIRED (ERS-1917). SPECIAL INSPECTION REQUIRED.

10. DRILL-IN ADHESIVE BOLTS, "HIT RE-500" ADHESIVE ANCHOR SYSTEM BY HILTI FASTENING SYSTEM OR APPROVED EQUAL. ICC CERTIFICATION REQUIRED (ESR-2322). SPECIAL INSPECTION REQUIRED.

CONSTRUCTION REQUIREMENTS:

1. CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS FOR COMPATIBILITY AND SHALL NOTIFY OWNER OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN IN THE DRAWINGS ARE INTENDED AS GUIDELINES ONLY AND MUST BE VERIFIED. THE CONTRACTOR SHALL BRING ALL DISCREPANCIES TO THE OWNER.

2. CONTRACTOR SHALL PROVIDE TEMPORARY SHORING AND BRACING FOR THE STRUCTURE AND STRUCTURAL COMPONENTS UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDING WITH THE PLANS AND DETAILS. THIS INCLUDES EXISTING STRUCTURE.

3. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SAFETY AND HEALTH PRECAUTIONS INCLUDING HAZARDOUS CONDITIONS AND MATERIALS EXISTED OR CREATED BY OTHER PARTIES THAT WORKING ON THE PROJECT. CONTRACTOR SHALL ALSO BE RESPONSIBLE FOR CONSTRUCTION METHODS, TECHNIQUES, AND SEQUENCES OR PROCEDURES REQUIRED TO PERFORM THE WORK.

4. CONTRACTOR SHALL BE RESPONSIBLE FOR KEEPING ALL EXISTING COMPONENTS, WHICH ARE REQUIRED TO REMAIN, IN THEIR ORIGINAL CONDITION. THIS INCLUDES WEATHER PROTECTIONS FOR THESE COMPONENTS UNTIL SUCH TIME THAT THE ENTIRE DWELLING INCLUDING THE NEW ADDITION ITSELF IS WEATHER PROTECTED.

5. CONTRACTOR INITIATED CHANGES SHALL BE SUBMITTED IN WRITING TO THE OWNER FOR APPROVAL PRIOR FABRICATION OR CONSTRUCTION. CHANGES SHOWN IN SHOP DRAWINGS ONLY WILL NOT SATISFY THIS REQUIREMENT.

6. CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS BEFORE COMMENCING ANY DEMOLITION. SHORING SHALL BE INSTALLED TO SUPPORT EXISTING CONSTRUCTION AS REQUIRED AND IN A MANNER SUITABLE TO THE WORK SEQUENCES. DEMOLITION DEBRIS SHALL NOT BE ALLOWED TO DAMAGE OR OVERLOAD THE EXISTING STRUCTURAL. LIMIT CONSTRUCTION LOADING (INCLUDING DEMOLITION DEBRIS) ON EXISTING CEILING FAMING TO 10 PSF AND ON EXISTING FLOOR FRAMING TO 40 PSF. PROVIDE TEMPORARY PLANKS OR STRUCTURAL SHEATHING OVER THE EXISTING CEILING JOISTS AS REQUIRED TO PROTECT THE EXISTING SOFFIT.

7. CONTRACTOR SHALL CHECK FOR DRY-ROT FOR ALL EXISTING STRUCTURAL COMPONENTS AT EXTERIOR WALLS, EXISTING TOILET ROOM FLOORS AND WALLS, AREAS SHOWN WATER STAINS, WOOD IN CONTACT WITH EARTH AND CONCRETE, AND ALL WOOD MEMBERS IN CRAW SPACES. ALL ROTTEN WOOD SHALL BE REMOVED AND DAMAGED MEMBERS SHALL BE REPLACED OR REPAIRED AS DIRECTED BY THE OWNER.

8. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO REVIEW AND APPROVAL BY THE OWNER.

9. ALL STRUCTURAL SYSTEMS, WHICH ARE TO BE COMPOSED OF COMPONENTS TO BE FIELD ERECTED, SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE, AND ERECTION IN ACCORDANCE WITH INSTRUCTIONS PREPARED BY THE SUPPLIER.

10. THE MANUFACTURER'S INSTRUCTIONS SHALL BE AVAILABLE ON THE JOB SITE AT THE TIME OF INSPECTION, FOR THE INSPECTOR'S USE AND REFERENCE.

STRUCTURAL FRAMING REQUIREMENTS:

1. ALL LUMBER SHALL BE KILN DRIED OR MC-19 WITH WWPA GRADED OR APPROVED EQUAL. ALL STRUCTURAL FLOOR, ROOF, AND SHEAR WALL SHEARING SHALL BE APA RATED. ALL SPECIFIED INDUSTRIAL LUMBERS, NAMELY PARALLAM PSL, MICROLLAM LVL, TIMBERSTRAND LSL, AND TJI SHALL BE MADE BY TRUS-JOIST CORPORATION OR OWNER APPROVED EQUAL.

ALL GLUED LAMINATED MEMBERS SHALL BE FABRICATED IN ACCORDANCE WITH AITC 110, AITC 117 AND ANSI/AITC A190.1. EACH MEMBER SHALL BEAR AN AITC IDENTIFICATION MARK AND SHALL BE ACCOMPANIED BY AITC CERTIFICATE OF CONFORMANCE. USE EXTERIOR TYPE ADHESIVES. BEAMS SHALL BE INDUSTRIAL APPEARANCE GRADE, UON BY THE ARCHITECT. SINGLE SPAN BEAMS SHALL BE COMBINATION 24F-V4, Fb=2400 PSI, Fv=265 PSI, E=1,800,000 PSI; CANTILEVERED SPAN BEAMS SHALL BE COMBINATION 24F-V8, Fb=2400 PSI, Fv=265 PSI, E=1,800,000 PSI.

2. MINIMUM NAILING SHALL COMPLY WITH TABLE 2304.10.1 OF THE 2015 IBC.

3. ALL NAILS SIZES SPECIFIED ON DRAWINGS ARE BASED ON THE FOLLOWING SPECIFICATIONS:

NAIL SIZE, LENGTH, AND DIAMETER 6D 2" 0.113" 8D 2-1/2" 0.131 10D 3" 0.148 16D BOX 3" 0.131

THE FOLLOWING STAPLES MAY BE SUBSTITUTED FOR NAILING OF PLYWOOD

NAIL SIZE, EQUIVALENT STAPLE, AND MINIMUM LENGTH 6D 16GA 1-3/4" 8D 15GA 1-3/4" 10D 13GA 1-3/4"

4. GALVANIZED METAL TIMBER CONNECTORS CALLED OUT BY LETTERS AND NUMBERS SHALL BE "STRONG-TIE" BY SIMPSON COMPANY INCLUDING SIMPSON STRONG WALLS AND SIMPSON GARAGE PORTAL WALLS (WHERE OCCUR) OR OWNER APPROVED EQUAL. IF NO SPESIFIC HANGER IS CALLED OUT, ANY HANGER MADE FOR THE SPECIFIED BEAM OR JOIST CAN BE USED.

5. ALL EXTERIOR WALL STUDS ARE 2X6 DOUGLAS FIR NO.2 STUDS AT 16" ON CENTER. ALL INTERIOR BEARING AND SHEAR WALL STUDS ARE 2X4 DOUBLAS FIR NO.2 STUDS AT 16" ON CENTER. PROVIDE ONE BEARING STUD AND ONE FULL HEIGHT STUD AT EACH SIDE OF DOOR AND WINDOW OPENINGS WHEN THEIR ROUGH OPENING WIDTH IS EQUAL OR LESS THAN 3'-0". PROVIDE TWO BEARING STUDS AND TWO FULL HEIGHT STUDS AT EACH SIDE OF DOOR AND WINDOW OPENINGS WHEN THEIR ROUGH OPENING WIDTH IS GREATER THAN 3'-0" OR WALL IS FRAMED WITH (2)2X6 AT 16" ON CENTER. PROVIDE MULTIPLE STUDS UNDER ALL BEAM AND KING-TRUSS BEARING LOCATIONS WITH THEIR TOTAL WIDTH/DEPTH EQUAL OR WIDER/DEEPER THATN THE BEAM/KING-TRUSS WIDTH. THESE MULTIPLE STUDS NEED TO EXTEND DOWN TO THE TOP OF CONCRETE. PROVIDE EQUAL AMOUNT OF MULTIPLE VERTICAL BLOCKING AT JOIST SPACING TO ALLOW CONTIUNITY. THIS ALSO APPLIES TO ALL HOLDOWN STUDS FOR THE SHEAR WALLS. FACE NAIL WALL TOP DOUBLE PLATE WITH 16D @ 12" AND LAP MINIMUM 4'-0" AT JOINTS AND PROVIDE (6) 16D @ 4" ON CENTER EACH SIDE OF JOINT. FACE NAIL WALL SILL PLATE THROUGH FLOOR SHEATHING TO DOUBLE PLATES, BEAM, OR SUPPORTING MEMBER BELOW WITH 16D @ 6" ON CENTER. MULTIPLE STUD SHALL BE NAILED TOGETHER WITH 16D @ 12" ON CENTER STAGGERED EACH FACE. PROVIDE SOLID BLOCKING BETWEEN STUDS AT MID-HEIGHT FOR ALL STUD WALLS OVER 10' IN HEIGHT.

6. PROVIDE DOUBLE JOISTS UNDER ALL PARALLEL PARTITIONS THAT EXTEND OVER MORE THAN HALF THE JOIST LENGTH AND AROUND ALL OPENING IN FLOOR. FLOOR JOISTS SHALL BE BLOCKED PER THE JOIST MANUFACTURER'S INSTRUCTIONS.

7. ALL FLOOR FRAMING LUMBERS: DOUGLAS FIR NO.2.

ALL HEADERS: DOUGLAS FIR NO.2. TYPICAL HEADER 4X8 MINIMUM UNLESS OTHERWISE SHOWN ON THE PLANS.

ALL POSTS: DOUGLAS FIR NO.2 UNLESS OTHERWISE SHOWN ON THE PLANS STUDS, PLATES, AND MISCELLANEOUS LIGHT NON-STRUCTURAL FRAMING: HEM-FIR NO.2

8. METAL PLATE CONNECTED WOOD TRUSSES: WOOD TRUSSES SHALL BE DESIGNED, MANUFACTURED AND INSTALLED PER TRUSS PLATE INSTITUTE (ANSI/TPI 1) SPECIFICATIONS. TPI SPECIFICATIONS SHALL NOT REVISE TRUSS ENGINEER'S AND TRUSS MANUFACTURER'S RESPONSIBILITY NOTED BELOW. WEB AND CHORD SIZES INDICATED ON PLANS AND NOTES ARE MINIMUM ONLY. ROOF DESIGN LIVE LOAD PER DESIGN LOADING CRITERIA. ROOF DESIGN DEAD LOAD 10 PSF MINIMUM TOP CHORD AND 7 PSF MINIMUM BOTTOM CHORD WITH LIVE LOAD OF 40 PSF MINIMUM AT ATTIC FLOOR WHERE APPLICABLE. USE 2X6 MINIMUM BOTTOM CHORD FOR ATTIC FLOOR. ROOF DESIGN WIND UPLIFT 15 PSF MINIMUM TYPICAL, EXCEPT USE 30 PSF MINIMUM WITHIN 10 FEET OF ROOF EAVES OR RAKES. DESIGN TRUSSES FOR SUPPORT OF DEAD, LIVE, SNOWDRIFT, AND WIND LOADS AND MECHANICAL/ELECTRICAL EQUIPMENT, PIPING, ETC AS REQUIRED. SNOW DRIFT LOADING LOCATIONS AND VALUES TO BE DETERMINED BY TRUSS ENGINEER. SUBMIT SHOP DRAWINGS AND DESIGN CALCULATIONS SHOWING TRUSSES, TRUSS TO TRUSS AND TRUSS TO SUPPORTING STRUCTURE CONNECTIONS, ERECTION AND PERMANENT BRACING SIZES AND CONNECTIONS. PROVIDE STANDARD TRUSS CAMBER.

PROVIDE ERECTION BRACING PER MANUFACTURE'S INSTRUCTIONS. PROVIDE AND INSTALL PERMANENT BRACING FOR LATERAL SUPPORT OF INDIVIDUAL WEB AND CHORD MEMBERS AS DESIGNED BY THE TRUSS ENGINEER. PROVIDE AND INSTALL ALL TRUSS TO TRUSS AND TRUSS TO SUPPORTING STRUCTURE CONNECTIONS.

TRUSS ALTERATIONS SHALL NOT OCCUR UNLESS THE APPROVAL OF A DESIGN PROFESSIONAL.

9. VENT BLOCKINGS CALLED OUT IN THE DRAWINGS ARE 2X WOOD BLOCKING WITH (3) EQUAL SPACED 1-1/2" DIAMETER HOLES ON EACH BLOCKING WITH MASH INSTALLED.

10. ROOF SHEATHING: 15/32"(1/2") MINIMUM CDX PLYWOOD OR STRUCTURAL PANEL WITH SPAN RATING OF 32/16, UNBLOCKED, LAID UP WITH FACE GRAIN PERPENDICULAR TO FRAMING BELOW, STAGGER END JOINTS. INSTALL PLYCLIPS AS REQUIRED. NAILING IS AS FOLLOWS: 10D @ 6" DIAPHRAGM BOUNDARIES, OVER EXTERIOR WALLS, AND INTERIOR SHEAR WALLS, 10D @ 6 ALL SUPPORTED EDGES, AND 10D @ 12" FIELD.

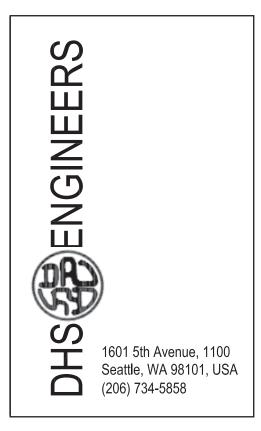
11. FLOOR AND ROOF DECK SHEATHING: 23/32"(3/4") MINIMUM CDX TONGUE AND GROOVE PLYWOOD WITH SPAN RATING OF 40/20, UNBLOCKED FOR FLOOR JOIST SPACED AT 16" ON CENTER; 7/8" MINIMUM CDX TONGUE AND GROOVE PLYWOOD WITH SPAN RATING OF 40/20 UNBLOCKED FOR FLOOR JOIST SPACED AT 24" ON CENTER; LAID UP WITH FACE GRAIN PERPENDICULAR TO FRAMING BELOW, STAGGER END JOINTS. GLUE FLOOR SHEATHING TO ALL SUPPORTS WITH A CONTINUOUS 3/16" DIAMETER BEAD MINIMUM. PROVIDE TWO BEADS AT PANEL JOINTS. NAILING IS AS FOLLOWS: 10D @ 6" DIAPHRAGM BOUNDARIES, OVER EXTERIOR WALLS, AND INTERIOR SHEAR WALLS, 10D @ 6" ALL SUPPORTED EDGES, AND 10D @ 10" FIELD.

12. EXTERIOR/INTERIOR/SHEAR WALL SHEATHING 15/32" (1/2") MINIMUM CDX PLYWOOD WITH SPAN RATING OF 24/0, EXTERIOR SIDE BLOCKED (BLOCK ALL UNSUPPORTED EDGES), NAIL WITH 10D @ 6" ALL EDGES AND 10D @ 12" FIELD. NAIL BOTTOM PLATE TO FRAMING BELOW WITH 16D @ 6".

13. WALL SILL PLATES OVER THE CONCRETE ARE TO BE 3X TREATED LUMBER WITH 1/2" DIAMETER ANCHOR BOLTS AT 4'-0" ON CENTER WITH EMBED IN CONCRETE OF 7" MINIMUM. ALL BOLTS SHALL HAVE 3X3X1/4 STEEL WASHER PLATE UNDER BOLT NUTS. THE EDGE OF A WASHER SHOULD NOT BE LOCATED MORE 1/2" AWAY FROM THE INSIDE FACE OF A SHEAR WALL SHEATHING. MINIMUM OF TWO BOLTS PER PLATE WITH BOLT END DISTANCE OF 6" MINIMUM. SHEAR WALL BOTTOM PLATE NAILING AND ALL NAILING AT PRESSURE TREATED PLATE/MEMBERS SHALL BE HOT-DIPPED ZINC-COATED GALVANIZED STEEL OR STAINLESS-STEEL NAILS.

14. WOOD IN CONTACT WITH CONCRETE SHOULD BE PRESERVATIVE-TREATED WOOD IN ACCORDNACE WITH AWPA U1 AND M4 STANDARDS.

15. FASTENERS INSTALLED IN PRESERVATIVE-TREADED OR IN FIRE-RETARDANT-TREATED WOOD SHALL BE HOT-DIPPED ZINE-COATED GALVANIZED WITH A MINIMUM COATING WEIGHT COMPLYING WITH ASTM A153. THIS INCLUDES NUTS AND WASHERS. FASTENERS OTHER THAN NAILS AND TEMBER RIVETS ARE PERMITTED TO BE MACHANICALLY DEPOSITED ZINC-COATED WITH COATING WEIGHTS COMPLYING WITH ASTM B 695, CLASS 55 MINIMUM.

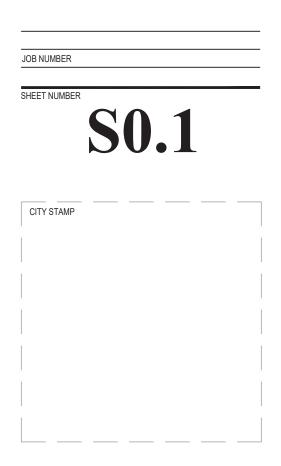


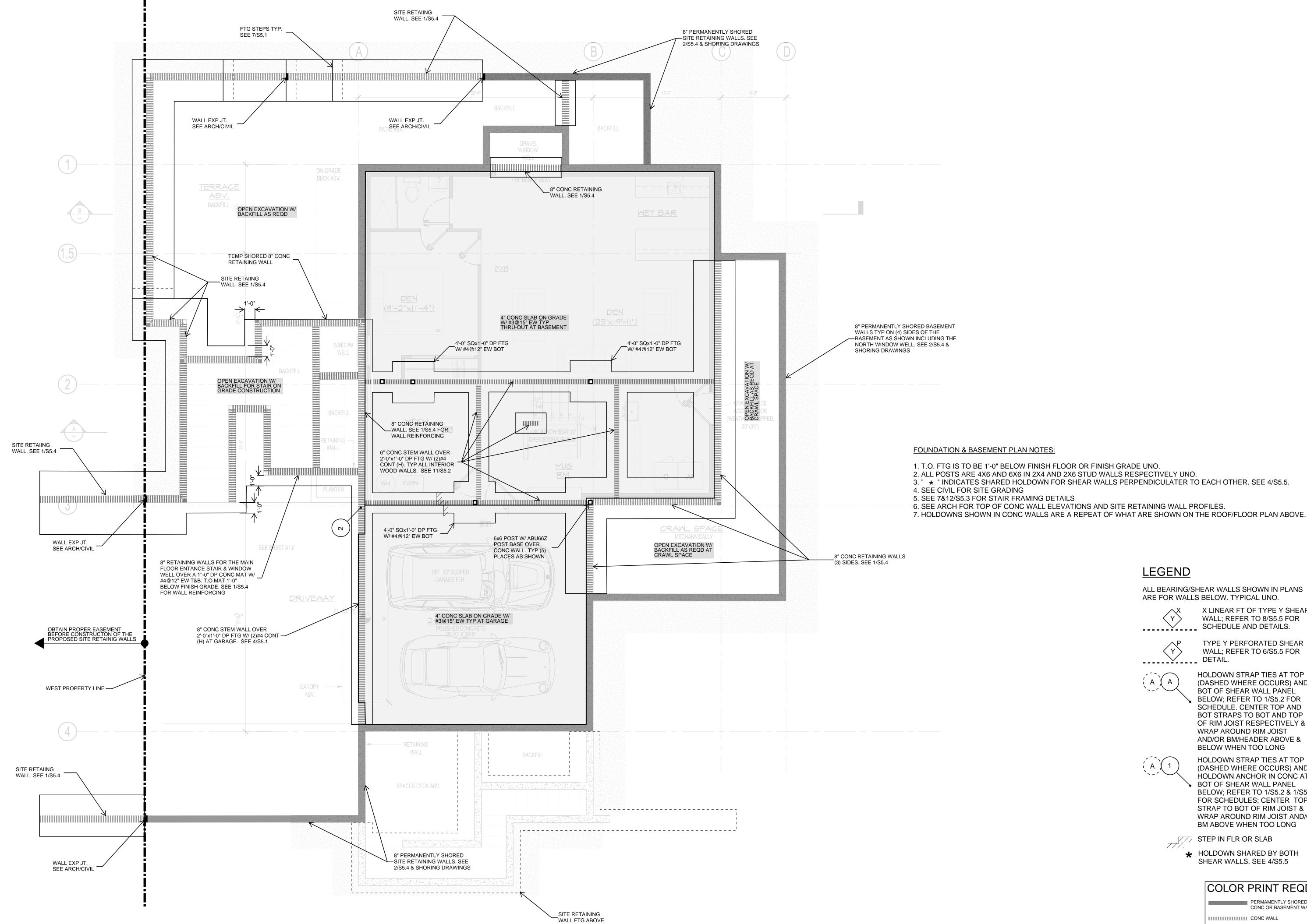


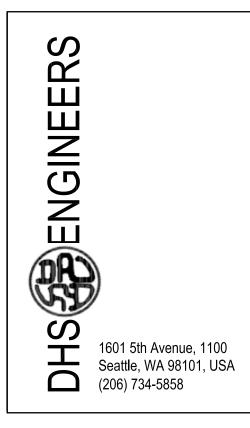


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GENERAL NOTE









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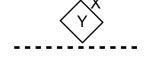
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SHEET TITLE **FOUNDATION & BASEMENT** FLOOR PLAN

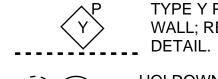


LEGEND

ALL BEARING/SHEAR WALLS SHOWN IN PLANS ARE FOR WALLS BELOW. TYPICAL UNO.



X LINEAR FT OF TYPE Y SHEAR WALL; REFER TO 8/S5.5 FOR SCHEDULE AND DETAILS.



TYPE Y PERFORATED SHEAR WALL; REFER TO 6/S5.5 FOR

HOLDOWN STRAP TIES AT TOP ίΑ (Α) `~__`

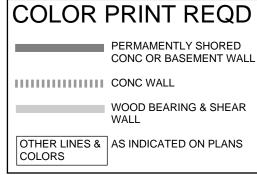
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(DASHED WHERE OCCURS) AND BOT OF SHEAR WALL PANEL BELOW; REFER TO 1/S5.2 FOR SCHEDULE. CENTER TOP AND BOT STRAPS TO BOT AND TOP OF RIM JOIST RESPECTIVELY & WRAP AROUND RIM JOIST AND/OR BM/HEADER ABOVE & BELOW WHEN TOO LONG

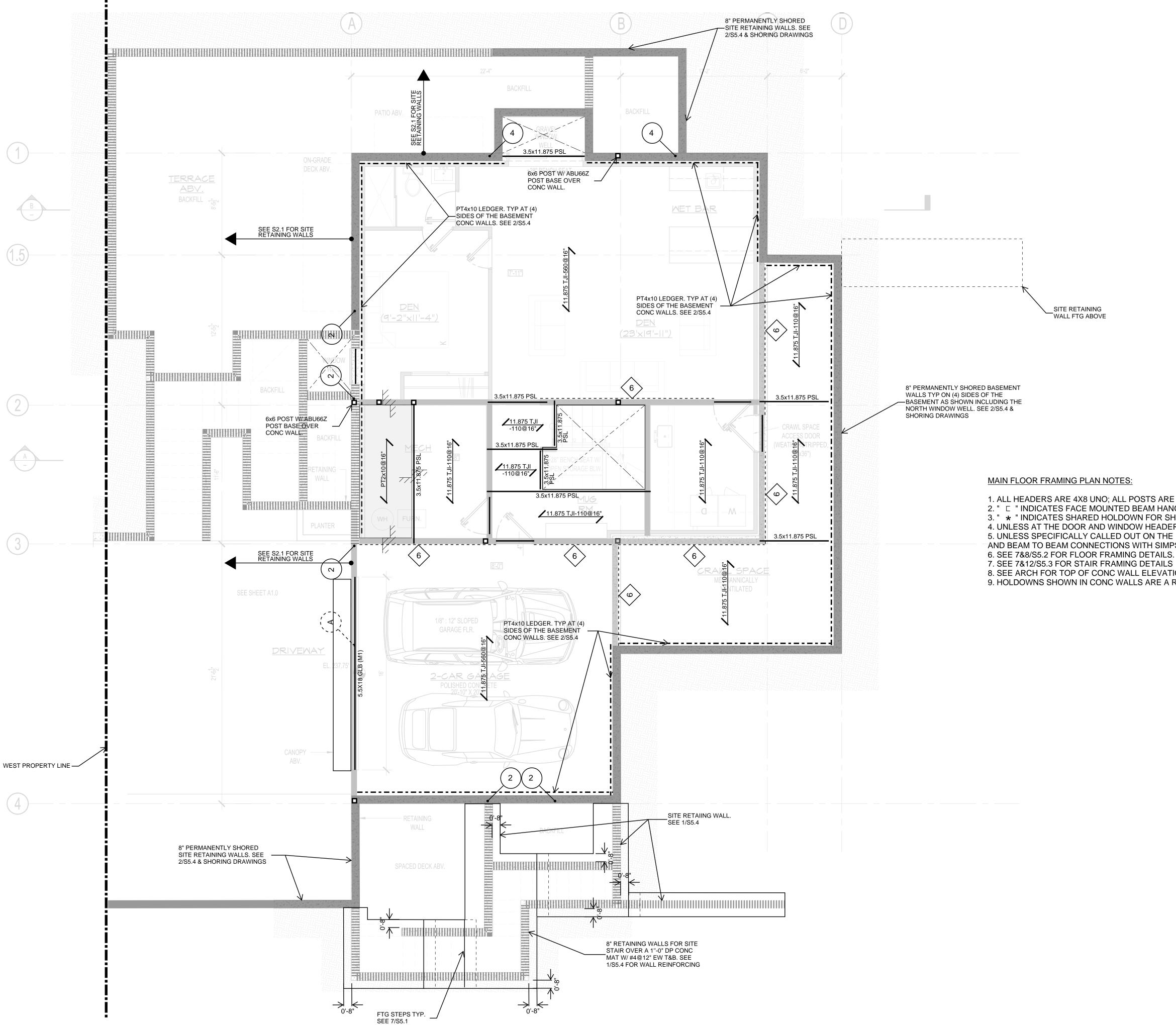
HOLDOWN STRAP TIES AT TOP (DASHED WHERE OCCURS) AND HOLDOWN ANCHOR IN CONC AT BOT OF SHEAR WALL PANEL BELOW; REFER TO 1/S5.2 & 1/S5.5 FOR SCHEDULES; CENTER TOP STRAP TO BOT OF RIM JOIST & WRAP AROUND RIM JOIST AND/OR BM ABOVE WHEN TOO LONG

STEP IN FLR OR SLAB

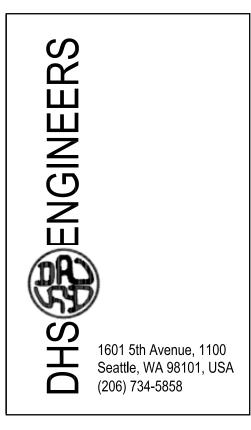
★ HOLDOWN SHARED BY BOTH SHEAR WALLS. SEE 4/S5.5







MAIN FLOOR FRAMING PLAN 1/4"=1'-0"



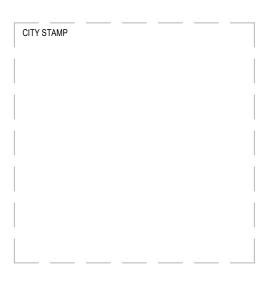




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SHEET TITLE MAIN FLOOR FRAMING PLAN

JOB NUMBER SHEET NUMBER **S2.2**



1. ALL HEADERS ARE 4X8 UNO; ALL POSTS ARE 4X6 AND 6X6 IN 2X4 AND 2X6 STUD WALLS RESPECTIVELY UNO. 2. " □ " INDICATES FACE MOUNTED BEAM HANGER WITH SDS FASTENERS UNO.

3. * * INDICATES SHARED HOLDOWN FOR SHEAR WALLS PERPENDICULATER TO EACH OTHER. SEE 4/S5.5. 4. UNLESS AT THE DOOR AND WINDOW HEADERS, ALL BEAMS ARE TOP FLUSH.

5. UNLESS SPECIFICALLY CALLED OUT ON THE PLAN, PROVIDE FACE MOUNTED HANGERS FOR THE JOIST TO BEAM AND BEAM TO BEAM CONNECTIONS WITH SIMPSON "IUS" AND "HU" SERIES HANGERS RESPECTIVELY.

8. SEE ARCH FOR TOP OF CONC WALL ELEVATIONS AND SITE RETAINING WALL PROFPILES.

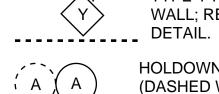
9. HOLDOWNS SHOWN IN CONC WALLS ARE A REPEAT OF WHAT ARE SHOWN ON THE ROOF/FLOOR PLAN ABOVE.

LEGEND

ALL BEARING/SHEAR WALLS SHOWN IN PLANS ARE FOR WALLS BELOW. TYPICAL UNO.

 $\langle Y \rangle$ ----**`**----

X LINEAR FT OF TYPE Y SHEAR WALL; REFER TO 8/S5.5 FOR SCHEDULE AND DETAILS.



TYPE Y PERFORATED SHEAR WALL; REFER TO 6/S5.5 FOR

`~__`

(DASHED WHERE OCCURS) AND BOT OF SHEAR WALL PANEL BELOW; REFER TO 1/S5.2 FOR SCHEDULE. CENTER TOP AND BOT STRAPS TO BOT AND TOP OF RIM JOIST RESPECTIVELY & WRAP AROUND RIM JOIST AND/OR BM/HEADER ABOVE & BELOW WHEN TOO LONG

HOLDOWN STRAP TIES AT TOP

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HOLDOWN STRAP TIES AT TOP (DASHED WHERE OCCURS) AND HOLDOWN ANCHOR IN CONC AT BOT OF SHEAR WALL PANEL

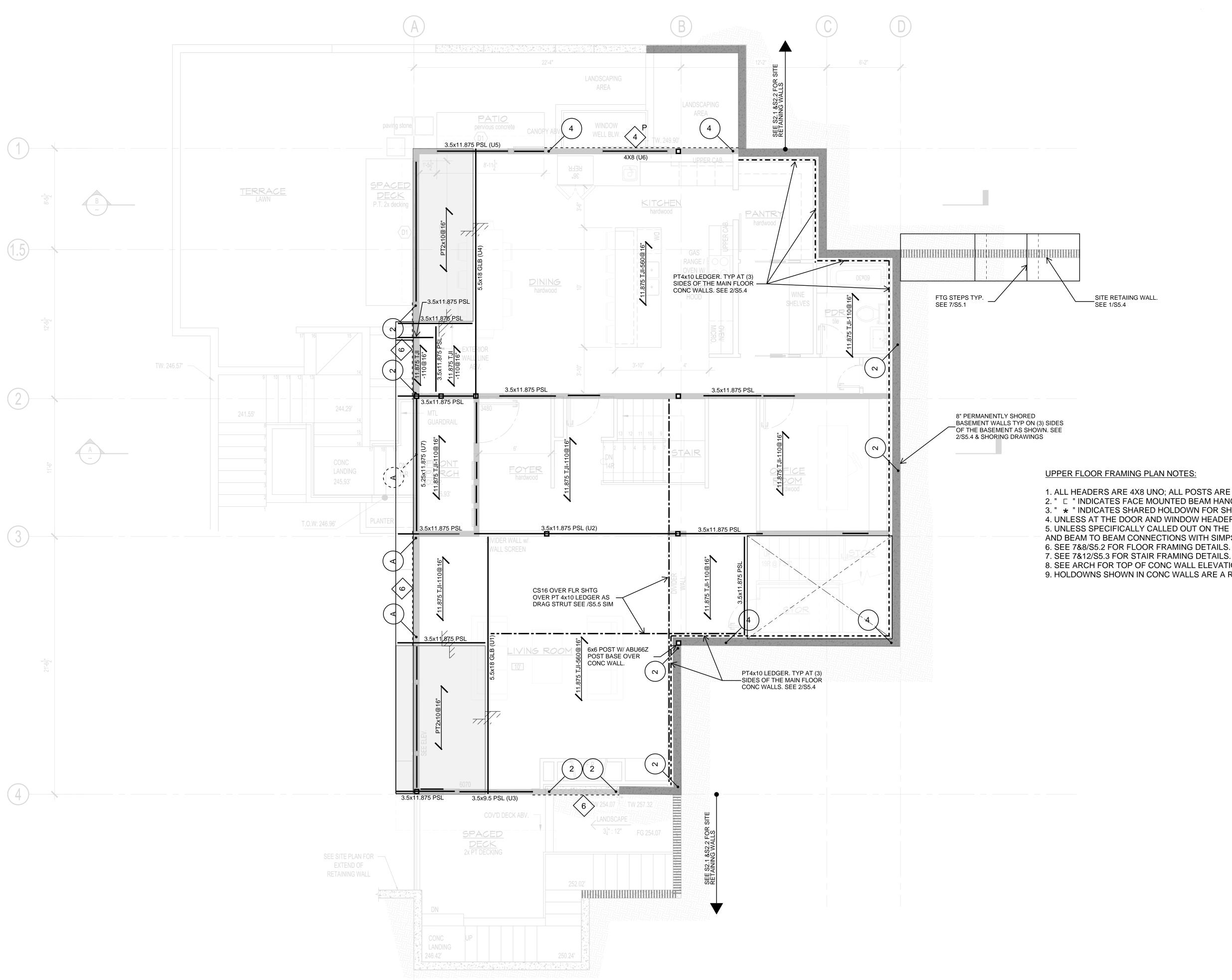
BELOW; REFER TO 1/S5.2 & 1/S5.5 FOR SCHEDULES; CENTER TOP STRAP TO BOT OF RIM JOIST & WRAP AROUND RIM JOIST AND/OR BM ABOVE WHEN TOO LONG

STEP IN FLR OR SLAB

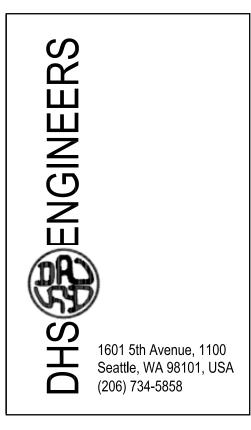
★ HOLDOWN SHARED BY BOTH SHEAR WALLS. SEE 4/S5.5

COLOR PRINT REQD PERMAMENTLY SHORED CONC OR BASEMENT WAL CONC WALL WOOD BEARING & SHEAR WALL OTHER LINES & AS INDICATED ON PLANS COLORS





UPPER FLOOR FRAMING PLAN 1/4"=1'-0"

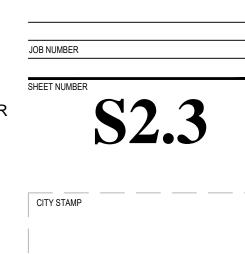






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SHEET TITLE **UPPER FLOOR FRAMING** PLAN



OF RIM JOIST RESPECTIVELY & WRAP AROUND RIM JOIST AND/OR BM/HEADER ABOVE &
BELOW WHEN TOO LONG
HOLDOWN STRAP TIES AT TOP
(DASHED WHERE OCCURS) AND
HOLDOWN ANCHOR IN CONC AT
BOT OF SHEAR WALL PANEL
BELOW; REFER TO 1/S5.2 & 1/S5.5
FOR SCHEDULES; CENTER TOP

S5.5 OP STRAP TO BOT OF RIM JOIST & WRAP AROUND RIM JOIST AND/OR BM ABOVE WHEN TOO LONG

STEP IN FLR OR SLAB

★ HOLDOWN SHARED BY BOTH SHEAR WALLS. SEE 4/S5.5

COLOR PRINT REQD PERMAMENTLY SHORED CONC OR BASEMENT WALL CONC WALL WOOD BEARING & SHEAR WALL OTHER LINES & AS INDICATED ON PLANS COLORS

1. ALL HEADERS ARE 4X8 UNO; ALL POSTS ARE 4X6 AND 6X6 IN 2X4 AND 2X6 STUD WALLS RESPECTIVELY UNO. 2. " C "INDICATES FACE MOUNTED BEAM HANGER WITH SDS FASTENERS UNO.

3. * * INDICATES SHARED HOLDOWN FOR SHEAR WALLS PERPENDICULATER TO EACH OTHER. SEE 4/S5.5. 4. UNLESS AT THE DOOR AND WINDOW HEADERS, ALL BEAMS ARE TOP FLUSH.

5. UNLESS SPECIFICALLY CALLED OUT ON THE PLAN, PROVIDE FACE MOUNTED HANGERS FOR THE JOIST TO BEAM AND BEAM TO BEAM CONNECTIONS WITH SIMPSON "IUS" AND "HU" SERIES HANGERS RESPECTIVELY.

8. SEE ARCH FOR TOP OF CONC WALL ELEVATIONS AND SITE RETAINING WALL PROFILES.

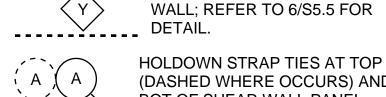
9. HOLDOWNS SHOWN IN CONC WALLS ARE A REPEAT OF WHAT ARE SHOWN ON THE ROOF/FLOOR PLAN ABOVE.

LEGEND

ALL BEARING/SHEAR WALLS SHOWN IN PLANS ARE FOR WALLS BELOW. TYPICAL UNO.

 $\langle Y \rangle$ ----**`**----

X LINEAR FT OF TYPE Y SHEAR WALL; REFER TO 8/S5.5 FOR SCHEDULE AND DETAILS.



TYPE Y PERFORATED SHEAR WALL; REFER TO 6/S5.5 FOR

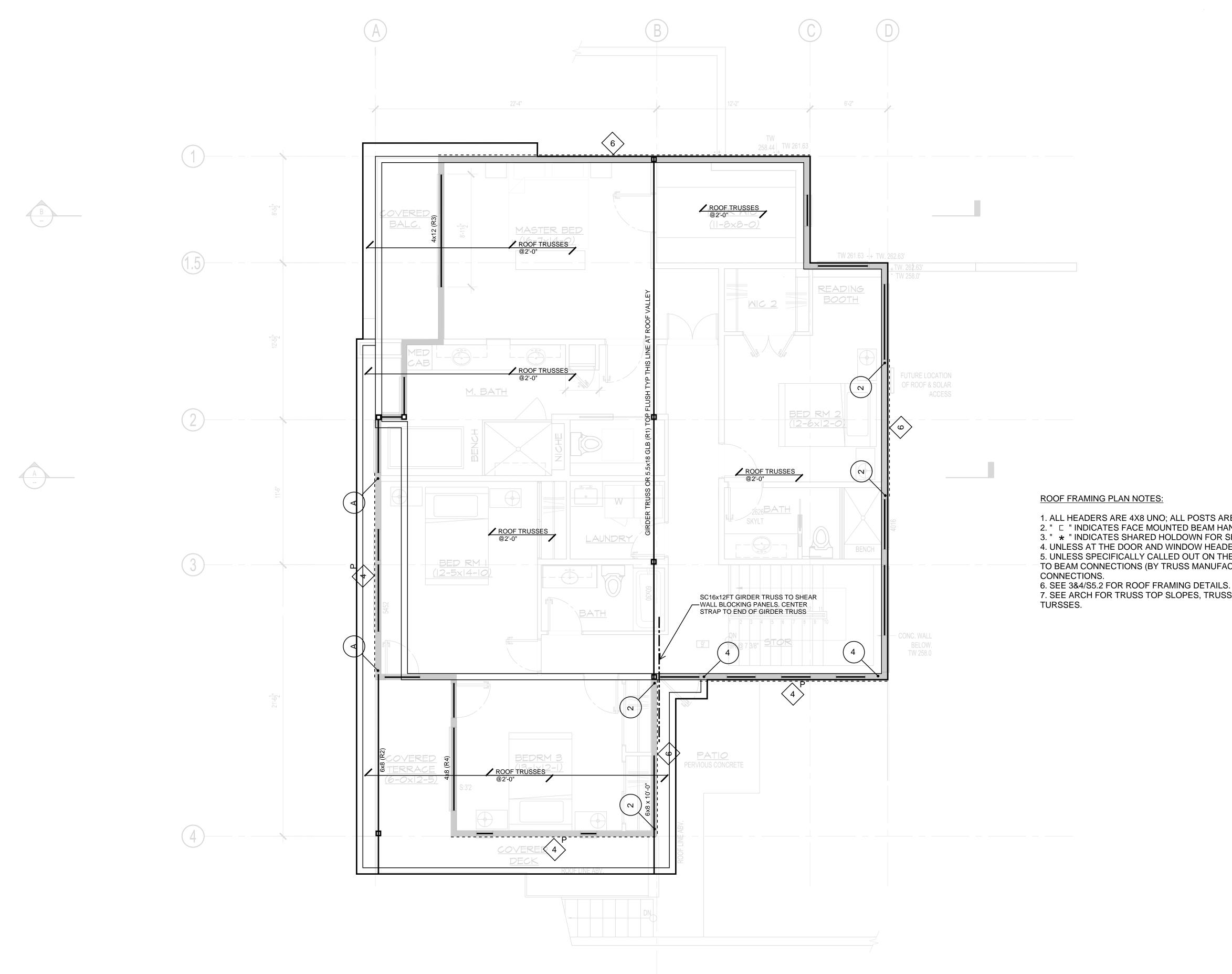
(DASHED WHERE OCCURS) AND BOT OF SHEAR WALL PANEL `~ _ · BELOW; REFER TO 1/S5.2 FOR SCHEDULE. CENTER TOP AND BOT STRAPS TO BOT AND TOP

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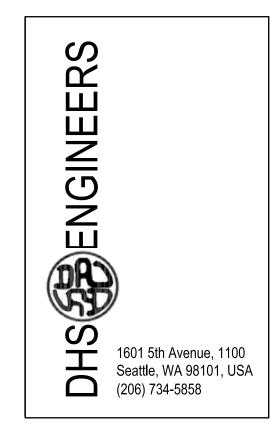




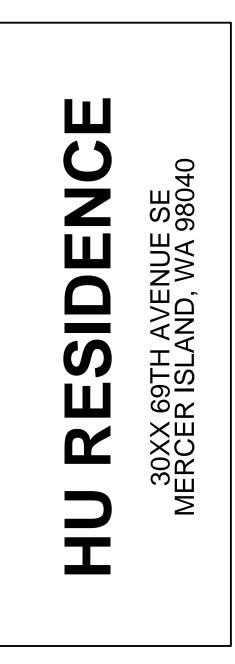




ROOF FRAMING PLAN 1/4"=1'-0"

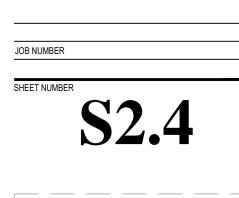


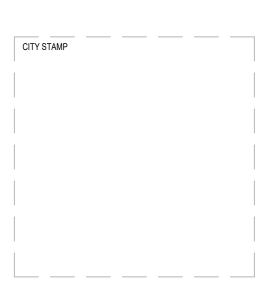




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SHEET TITLE **ROOF FRAMING PLAN**





1. ALL HEADERS ARE 4X8 UNO; ALL POSTS ARE 4X6 AND 6X6 IN 2X4 AND 2X6 STUD WALLS RESPECTIVELY UNO. 2. " □ "INDICATES FACE MOUNTED BEAM HANGER WITH SDS FASTENERS UNO.

3. * * INDICATES SHARED HOLDOWN FOR SHEAR WALLS PERPENDICULATER TO EACH OTHER. SEE 4/S5.5. 4. UNLESS AT THE DOOR AND WINDOW HEADERS, ALL BEAMS ARE TOP FLUSH. 5. UNLESS SPECIFICALLY CALLED OUT ON THE PLAN, PROVIDE FACE MOUNTED HANGERS FOR THE ROOF TRUSS

TO BEAM CONNECTIONS (BY TRUSS MANUFACTURER) AND "HU" SERIES HANGERS FOR BEAM TO BEAM

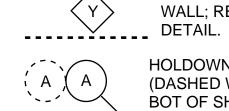
7. SEE ARCH FOR TRUSS TOP SLOPES, TRUSS PROFILES, AND REQUIRED SHORT PARAPETS AS PART OF THE

LEGEND

ALL BEARING/SHEAR WALLS SHOWN IN PLANS ARE FOR WALLS BELOW. TYPICAL UNO.

 $\langle Y \rangle$ ----**`**----

X LINEAR FT OF TYPE Y SHEAR WALL; REFER TO 8/S5.5 FOR SCHEDULE AND DETAILS.



TYPE Y PERFORATED SHEAR WALL; REFER TO 6/S5.5 FOR

HOLDOWN STRAP TIES AT TOP (DASHED WHERE OCCURS) AND BOT OF SHEAR WALL PANEL BELOW; REFER TO 1/S5.2 FOR SCHEDULE. CENTER TOP AND BOT STRAPS TO BOT AND TOP OF RIM JOIST RESPECTIVELY & WRAP AROUND RIM JOIST AND/OR BM/HEADER ABOVE & BELOW WHEN TOO LONG

ίΑ (1)

HOLDOWN STRAP TIES AT TOP (DASHED WHERE OCCURS) AND HOLDOWN ANCHOR IN CONC AT BOT OF SHEAR WALL PANEL

BELOW; REFER TO 1/S5.2 & 1/S5.5 FOR SCHEDULES; CENTER TOP STRAP TO BOT OF RIM JOIST & WRAP AROUND RIM JOIST AND/OR BM ABOVE WHEN TOO LONG

TT STEP IN FLR OR SLAB

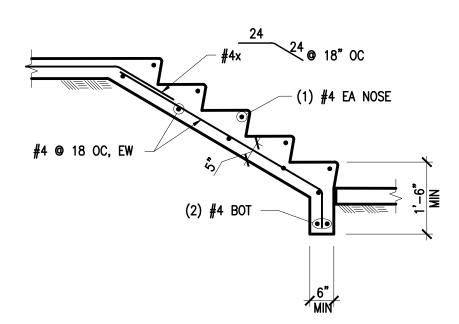
★ HOLDOWN SHARED BY BOTH SHEAR WALLS. SEE 4/S5.5

COLOR PRINT REQD PERMAMENTLY SHORED CONC OR BASEMENT WALL CONC WALL WOOD BEARING & SHEAR WALL OTHER LINES & AS INDICATED ON PLANS COLORS

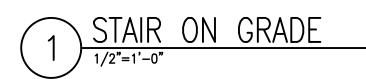


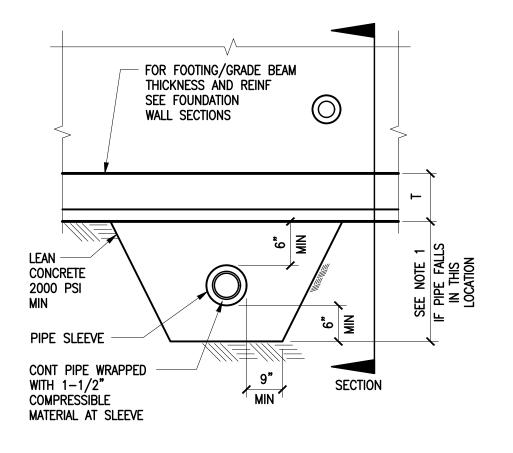
0' 1' 2' 3' 4'





NOTES: 1. SEE ARCH FOR STAIR DIMENSIONS AND CONFIGURATION.

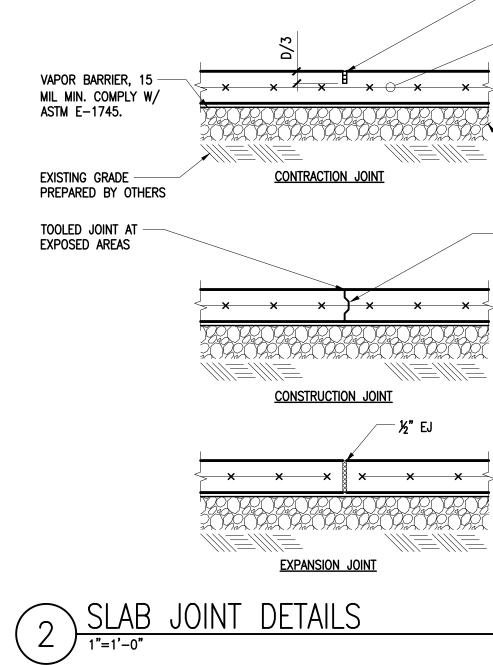


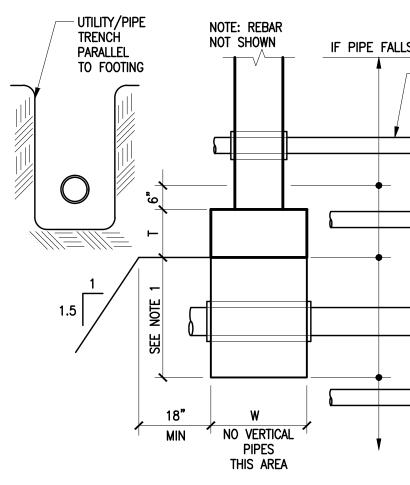


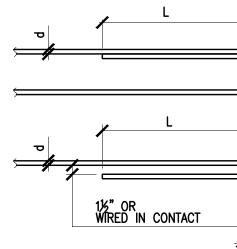
NOTES: 1. SINGLE PIPES 8" OR LESS PERPENDICULAR TO AND GREATER THAN 24" CLEAR BELOW FOOTINGS DO NOT REQUIRE CONCRETE ENCASEMENT. (PIPE GROUPINGS BELOW 24" SHALL BE REVIEWED BY THE STRUCTURAL ENGINEER).

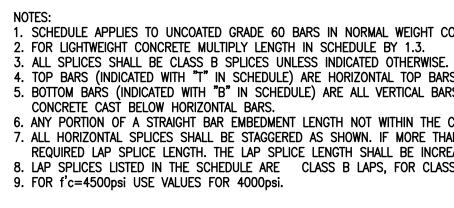
2. PIPES LARGER THAN 8" SHALL BE REVIEWED BY THE STRUCTURAL ENGINEER.

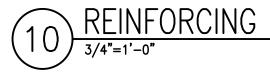












REINFORCING SPLICE SCHEDULE

6. ANY PORTION OF A STRAIGHT BAR EMBEDMENT LENGTH NOT WITHIN THE CONFINED CORE SHALL BE INCREASED BY A FACTOR OF 1.6. 7. ALL HORIZONTAL SPLICES SHALL BE STAGGERED AS SHOWN. IF MORE THAN 50% OF VERTICAL REINFORCING IS LAP SPLICED WITHIN THE 8. LAP SPLICES LISTED IN THE SCHEDULE ARE CLASS B LAPS, FOR CLASS A LAPS REDUCE LENGTH BY 25%.

MIN A

6" 2%

4. TOP BARS (INDICATED WITH "T" IN SCHEDULE) ARE HORIZONTAL TOP BARS WITH MORE THAN 12" OF CONC CAST BLW THE BARS. 5. BOTTOM BARS (INDICATED WITH "B" IN SCHEDULE) ARE ALL VERTICAL BARS AND HORIZONTAL BARS WITH LESS THAN 12" OF REQUIRED LAP SPLICE LENGTH. THE LAP SPLICE LENGTH SHALL BE INCREASED BY 33%.

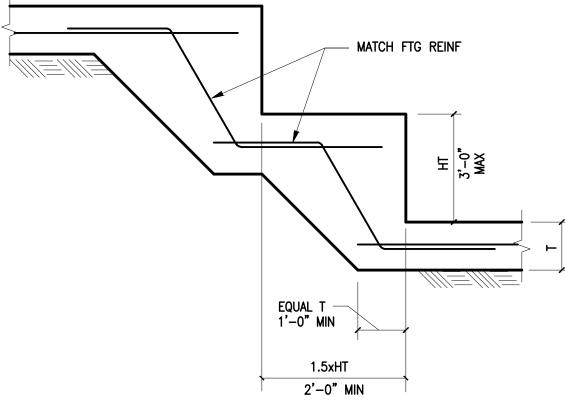
1. SCHEDULE APPLIES TO UNCOATED GRADE 60 BARS IN NORMAL WEIGHT CONCRETE.

- TOP REINF — SLOPE 1:12 MAX, TYP BOT REINF L

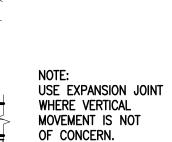
1/2"=1'-0"

STEPPED FOOTING 7

IF PIPE FALLS IN THESE LOCATIONS CONT PIPE, TYP PROVIDE PIPE SLEEVE AT FOUNDATION AND WRAP THE CONT PIPE WITH 1-1/2" COMPRESSIBLE MATERIAL, TYP NO PIPE THIS AREA STEP FOOTING & SLEEVE PIPE THRU WALL SLEEVE PIPE & CAST FOOTING Y TO BOTTOM OF TRENCH BACKFILL AND COMPACT TRENCH







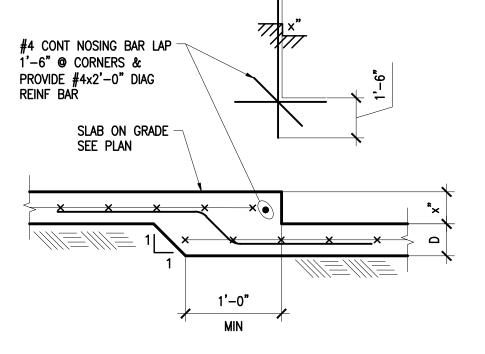
SEE SOILS REPORT

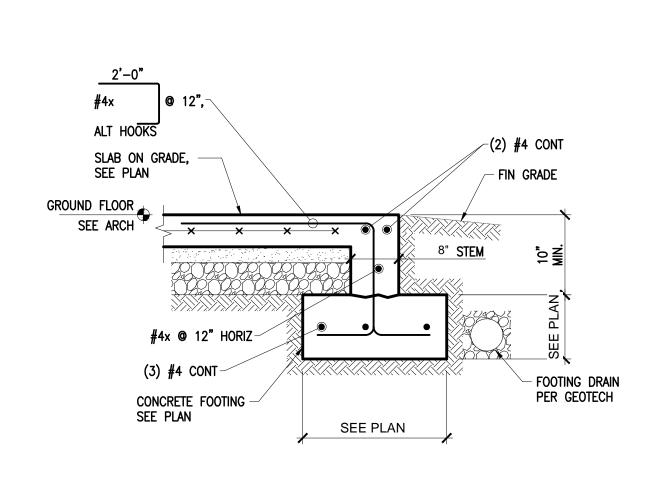
PRE-MFR JOINT W/ REINF THROUGH

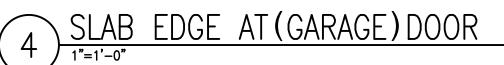
— 6" GRAVEL, MIN. CLASSIFIED AS GW OR GP PER UNITED SOIL CLASSIFICATION SYSTEM.

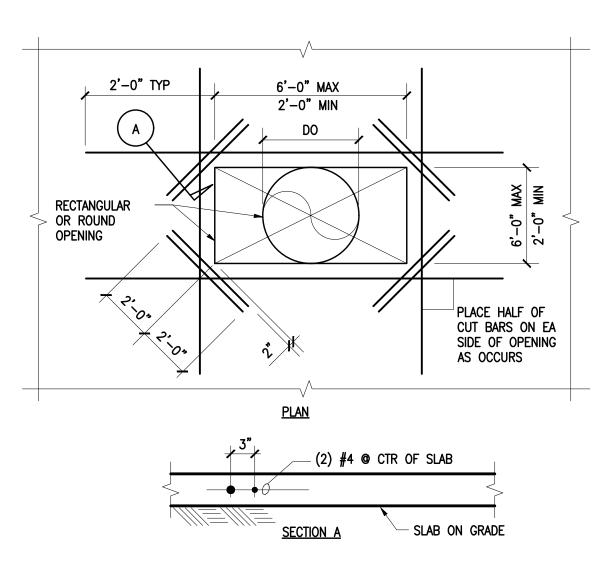
Δ

SAWCUT OR USE 1/2" PREMOLDED JOINT - REINF THROUGH











SPLICE OR DEVELOF

0.625"

#5

0.500"

#4

T | B | T | B | T | B | T

28 22 38 29 47 36 56

25 | 19 | 33 | 25 | 41 | 31 | 49

22 17 29 23 36 28 44

20 | 16 | 27 | 21 | 33 | 26 | 40

22 | 17 | 29 | 22 | 36 | 28 | 43

19 | 15 | 25 | 19 | 31 | 24 | 37

17 | 13 | 23 | 17 | 28 | 22 | 34

16 12 21 16 26 20 31

0.375"

#3

NORMAL WT. CONCRETE

3000

4000

5000

6000

_

3000

4000

5000

6000

f'c (psi)

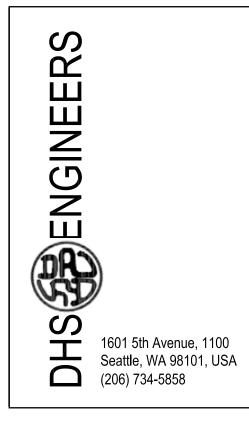
"CLASS E

0.750"

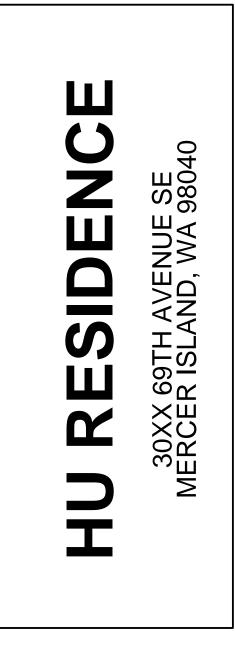
#6

DEVEL

PMENT LENGTH (INCHES)										
B" TENSION LAP SPLICE SCHEDULE										
99	0.875"		1.0	1.000" 1.128"		28"	1.270"		1.410"	
	# 7		#	8	#9		# 10		# 11	
В	Т	В	Т	В	Т	В	Т	В	Т	В
43	81	63	93	72	105	81	116	90	128	98
37	71	54	81	62	91	70	101	78	111	85
34	63	49	72	56	81	63	90	69	99	76
31	58	45	66	51	74	57	82	63	90	70
LOPMENT LENGTH "Ld" SCHEDULE										
33	63	48	72	55	81	62	90	69	98	76
29	54	42	62	48	70	54	78	60	85	66
26	49	38	56	43	63	48	69	54	76	59
24	45	34	51	39	57	44	63	49	70	54



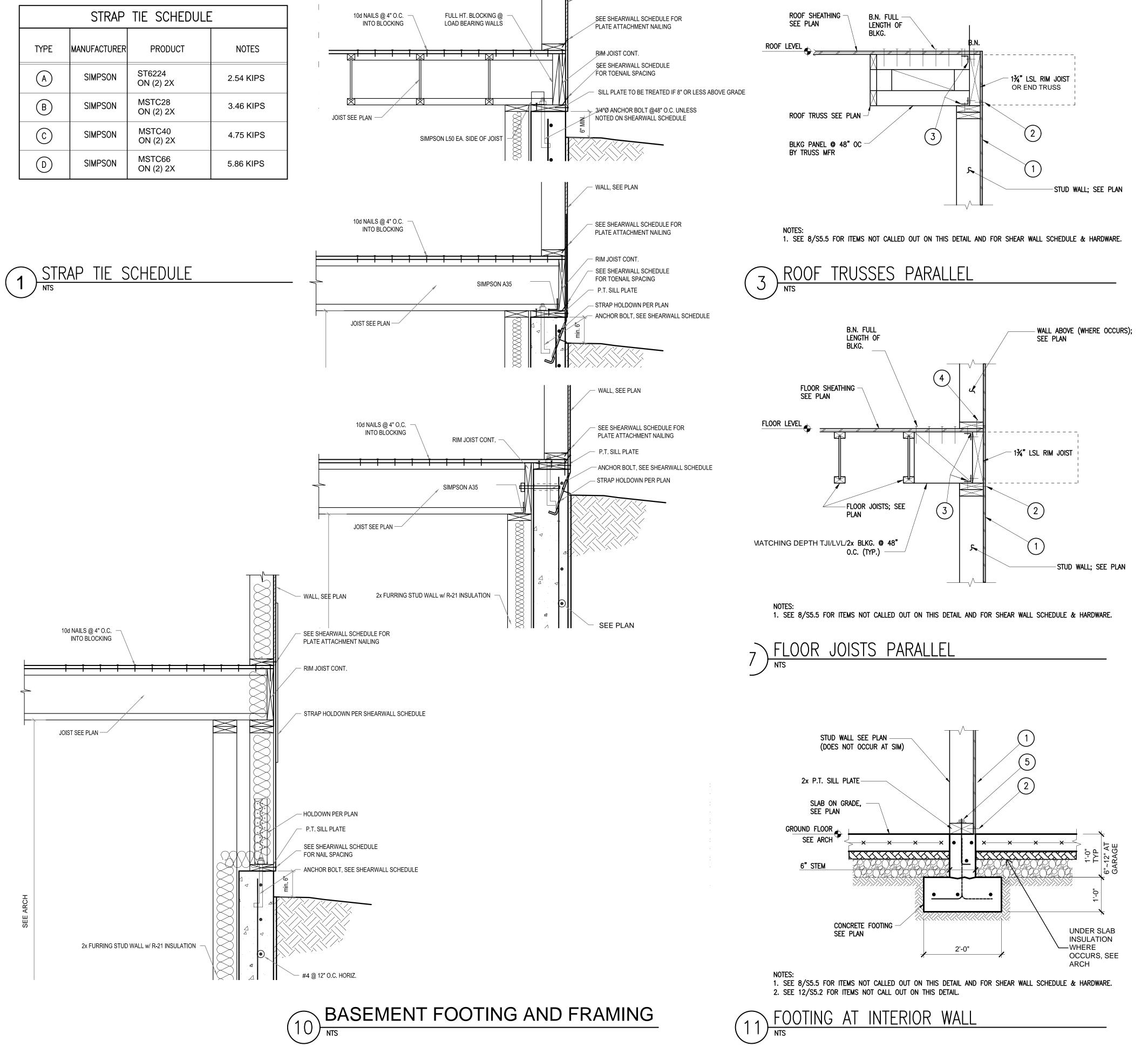




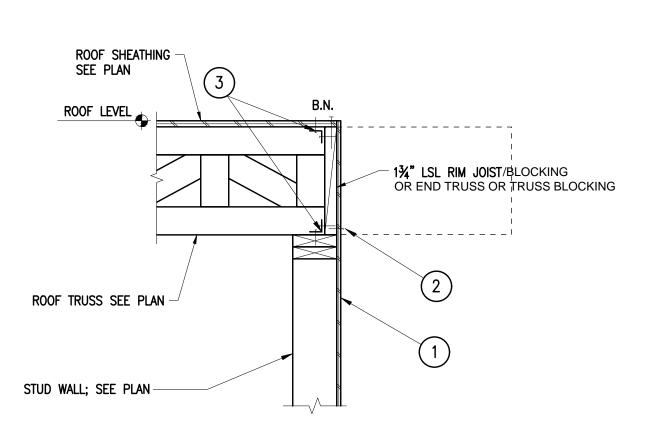
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	SHEET TITLE				

TYPICAL CONCRETE DETAIL





WALL, SEE PLAN



NOTES: 1. SEE 8/S5.5 FOR ITEMS NOT CALLED OUT ON THIS DETAIL AND FOR SHEAR WALL SCHEDULE & HARDWARE. 2. WHERE THERE IS NO WALL ABOVE, USE $\#6 \times \frac{3}{4}$ " LONG SCREWS TO SHEATHING ABOVE.

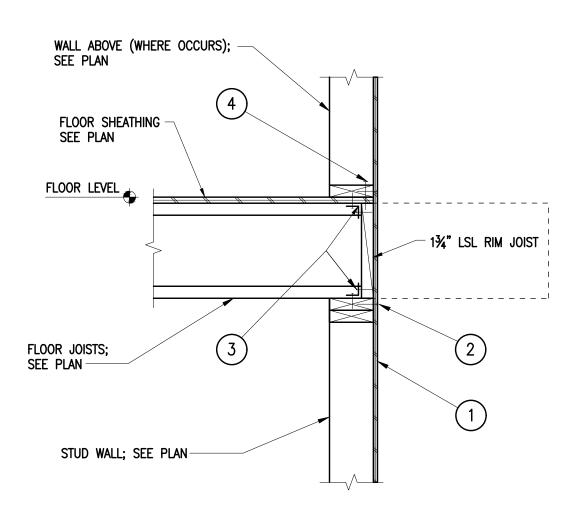
TRUSSES PERPENDICULAR ROOF NTS

4

NOTES:

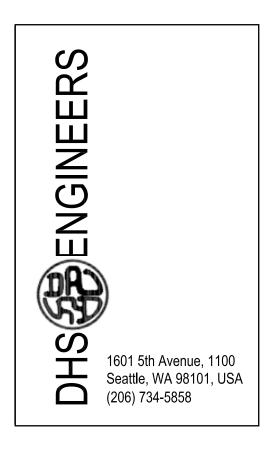
/ NTS

8



1. SEE 8/S5.5 FOR ITEMS NOT CALLED OUT ON THIS DETAIL AND FOR SHEAR WALL SCHEDULE & HARDWARE.

FLOOR JOISTS PERPENDICULAR







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SHEET TITLE **TYPICAL FRAMING DETAIL**



ROOF & FLOOR DIAPHRAGM

3. PLYWOOD SHALL BE GLUED (SUB-FLOOR ADHESIVE) FOR FLOORS.

4. USE ${}^{2}_{32}$ " PLYWOOD T & G AT FLAT ROOF.

5. MIN EDGE DISTANCE FOR NAILS SHALL BE 3/8".

6. MIN SHEATHING SHEET SIZE SHALL BE 2'-0"x4'-0". 7. NAILS SHALL NOT BE OVERDRIVEN. OPERATOR TO ADJUST AIR PRESSURE OF

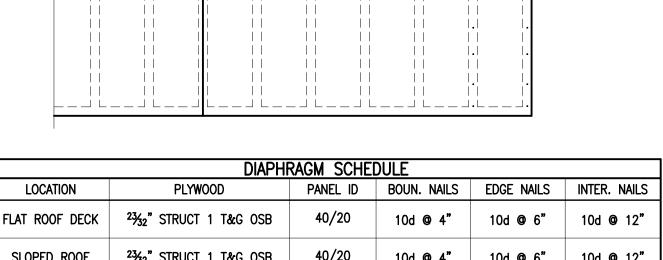
PNEUMATIC NAILER AS REQUIRED TO AVOID HEAD OF NAIL PENETRATING SKIN OF

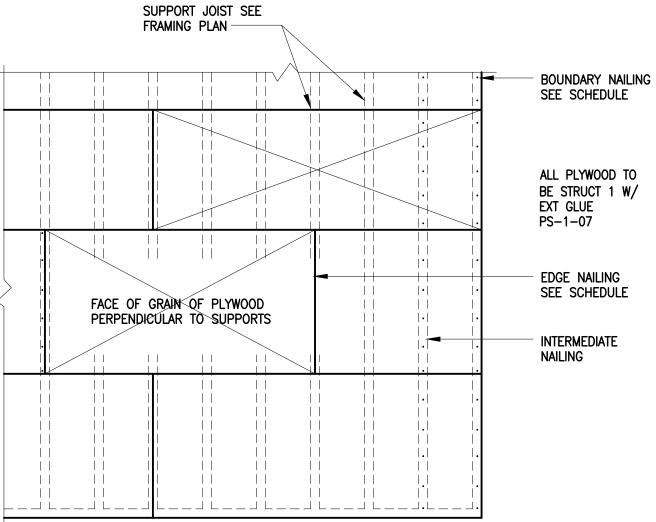
PLYWOOD SHEATHING.

8. NAILS SHALL BE COMMON WIRE TYPE.

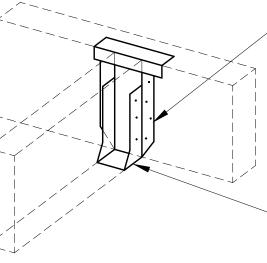
9. SEE PLANS FOR AREAS OF BLOCKED DIAPHRAGMS. 10. USE BOUNDARY NAILING AT ALL CONNECTIONS TO SHEAR WALLS.

SLOPED ROOF ²3/₃₂" STRUCT 1 T&G OSB 40/20 10d @ 6" 10d @ 12" 10d @ 4" FLOOR ²3/₃₂" STRUCT 1 T&G OSB 40/20 10d @ 6" 10d @ 12" 10d @ 4" NOTES: 1. USE BOUNDARY NAILING AT ALL HIPS, RIDGES, VALLEYS AND OPENINGS. 2. USE COMMON NAIL FOR ALL DIAPHRAGM NAILING.

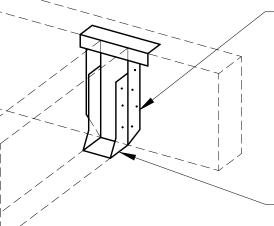




THIS TYPE OF HANGER TO BE USED TYPICAL WHERE APPLICABLE UNLESS NOTED OR SHOWN OTHERWISE ON THE PLANS AND DETAILS. USE SIZE AND TYPE OF NAILS AS REQUIRED BY MANUFACTURER AND FULLY DRIVE IN ALL NAILS. HANGER DETAIL AND SCHEDULE NTS

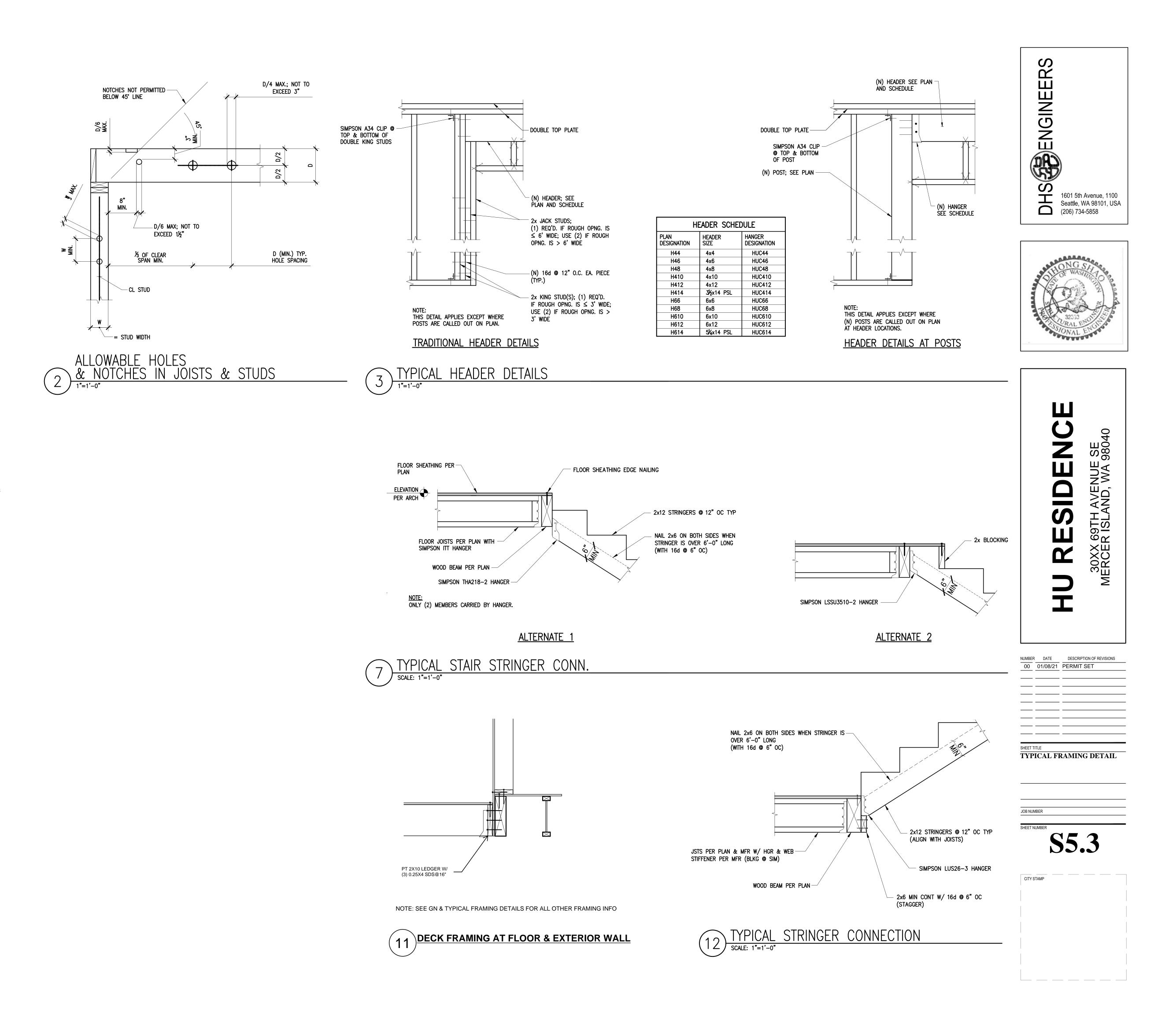


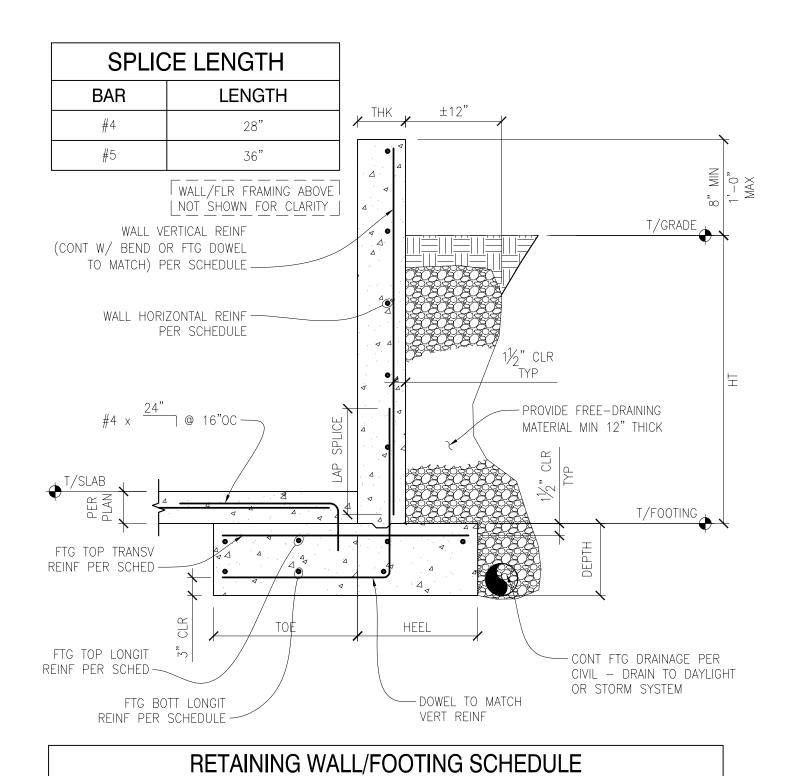
- GAGE PER MANUFACTURER'S SPECIFICATIONS



SIMPSON JOIST HANGER OR APPROVED EQUAL

HANGER SCHEDULE				
JOIST/BEAM	TYP HANGER			
2x12	JB212A			
11 %" TJI210	ITS2.06/11.88			
11 %" TJI360	ITS2.37/11.88			
(2) 11 ⁷ / ₈ " TJI210	MIT4.28/11.88			
1¾×11% LVL	BA1.81/11.88 (MIN)			
31⁄₂x117⁄8 PSL	BA3.56/11.88 (MAX)			
5¼×11% PSL	HB5.50/11.88			
51/4x16 PSL	HGLTV5.516			



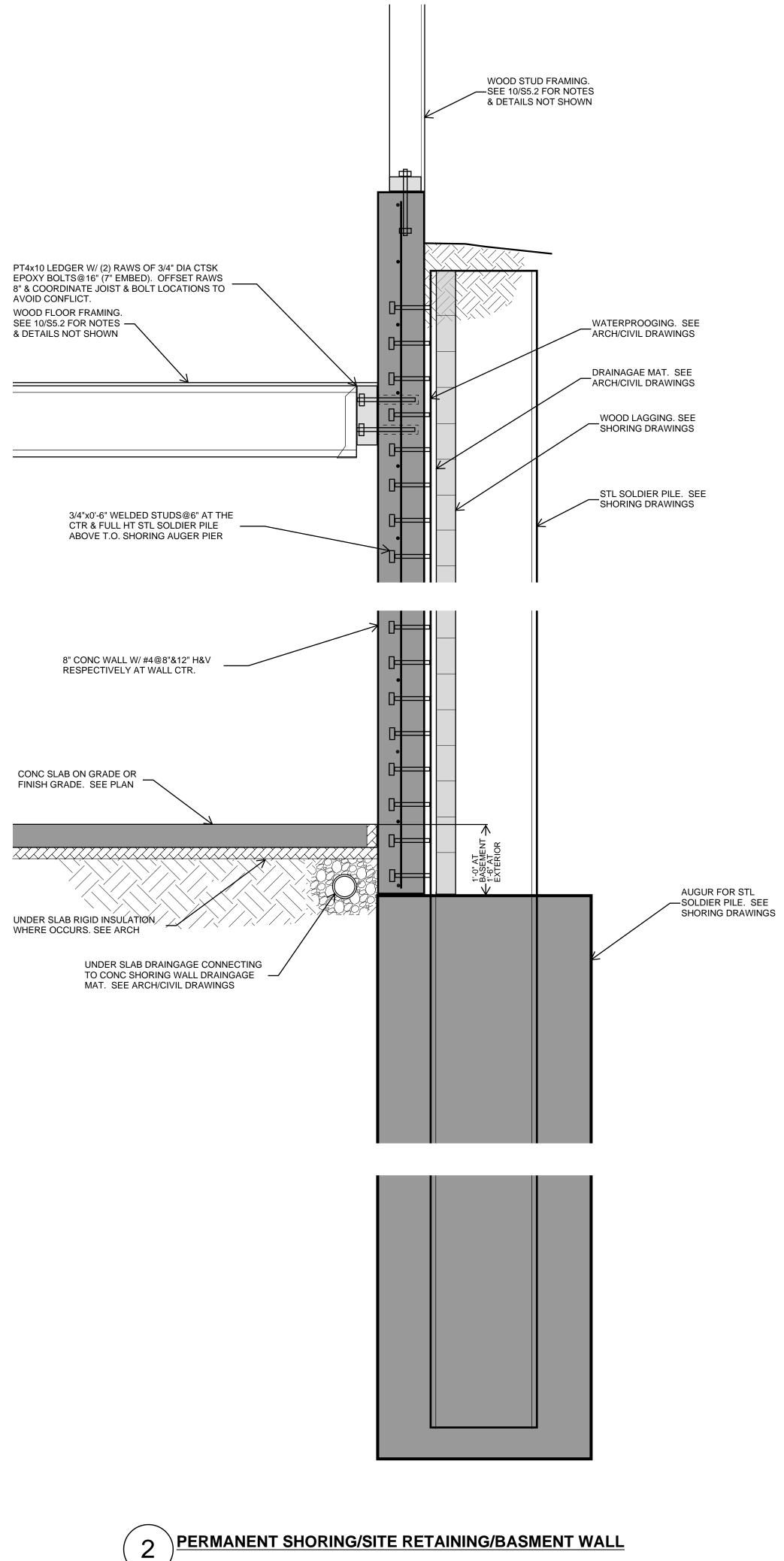


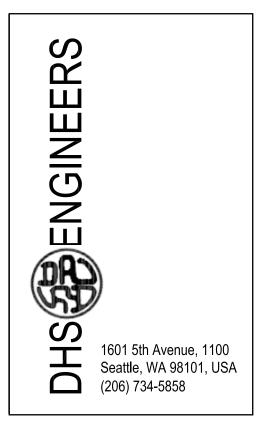
FOOTING WALL REINFORCEMENT REINFORCEMENT SIZE SIZE TOE HEEL DEPTH TOP/TRANSV TOP/LONGIT BOTTOM/LONG HT (MAX) VERTICAL HORIZONTAL 1'-0" | 10" | #4 @ 16"OC | (3) #4 4'-0" #4 @ 12"OC #4 @ 12"OC '-6" (2) #4 |#4 @ 12"OC| #4 @ 12"OC 6'-0" 2'-0''1'-6" | 10" | #4 @ 10"OC | (3) #4 (2) #4 |#5 @ 12"OC| #4 @ 12"OC 8'-0" 2'-0" | 10" | #5 @ 16"0C | (3) #4 (3) #4 #5 @ 7"OC | #4 @ 12"OC 10'-0" 3'-6" 2'-9" 14" #5 @ 12"OC (3) #4 (3) #4

RETAINING WALL AND SCHEDULE

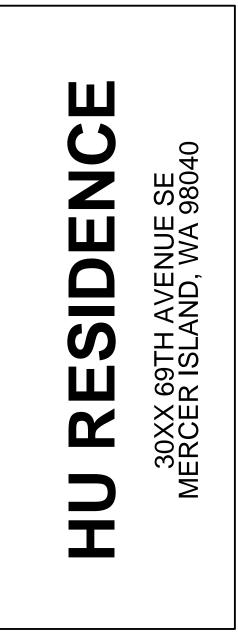
SCALE: N.T.S.

1



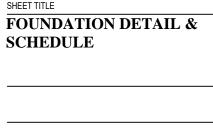






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FOUNDATION DETAIL & SCHEDULE



JOB NUMBER

SHEET NUMBER



CITY STAMP

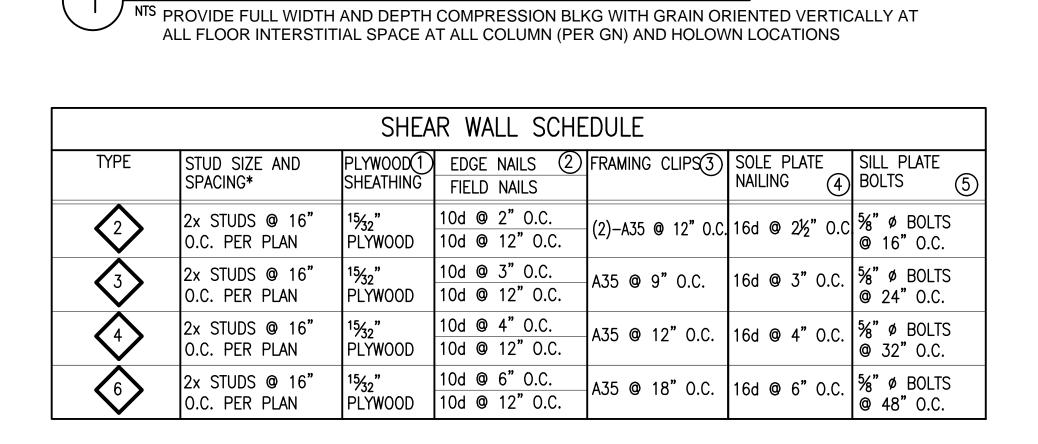
HOLDOWN SCHEDULE						
				ANCHOR ROD		
TYPE	SIMPSON HARDWARE	WALL STUD SIZE; SEE PLAN	STUD SIZE AT HOLDOWN	DIA.	EMBEDMENT (SEE 2/S5.5)	
	HDU2	4" STUDS	4x4*	57 "	~	
(2)		6" STUDS	4x6*	- 5⁄8"	24"	
	HDU4	4" STUDS	4x4*	5%"	24"	
(4)		6" STUDS	4x6*			
(F)	HDU5	4" STUDS	4x4	5%"	24"	
(5)		6" STUDS	4x6			
	HDU8	4" STUDS	4x4	- 7⁄8"	24"	
(8)		6" STUDS	4x6		27	
		4" STUDS	4x4	1"	24"	
(11)	HDU11	6" STUDS	4x6			
	HDU14	4" STUDS	4x4	1"	24"	
(14)		6" STUDS	4x6		<u>~</u> ¬	

NOTES

1. SEE PLAN FOR HODOWN TYPES AND LOCATIONS.

HOLDOWN SCHEDULE

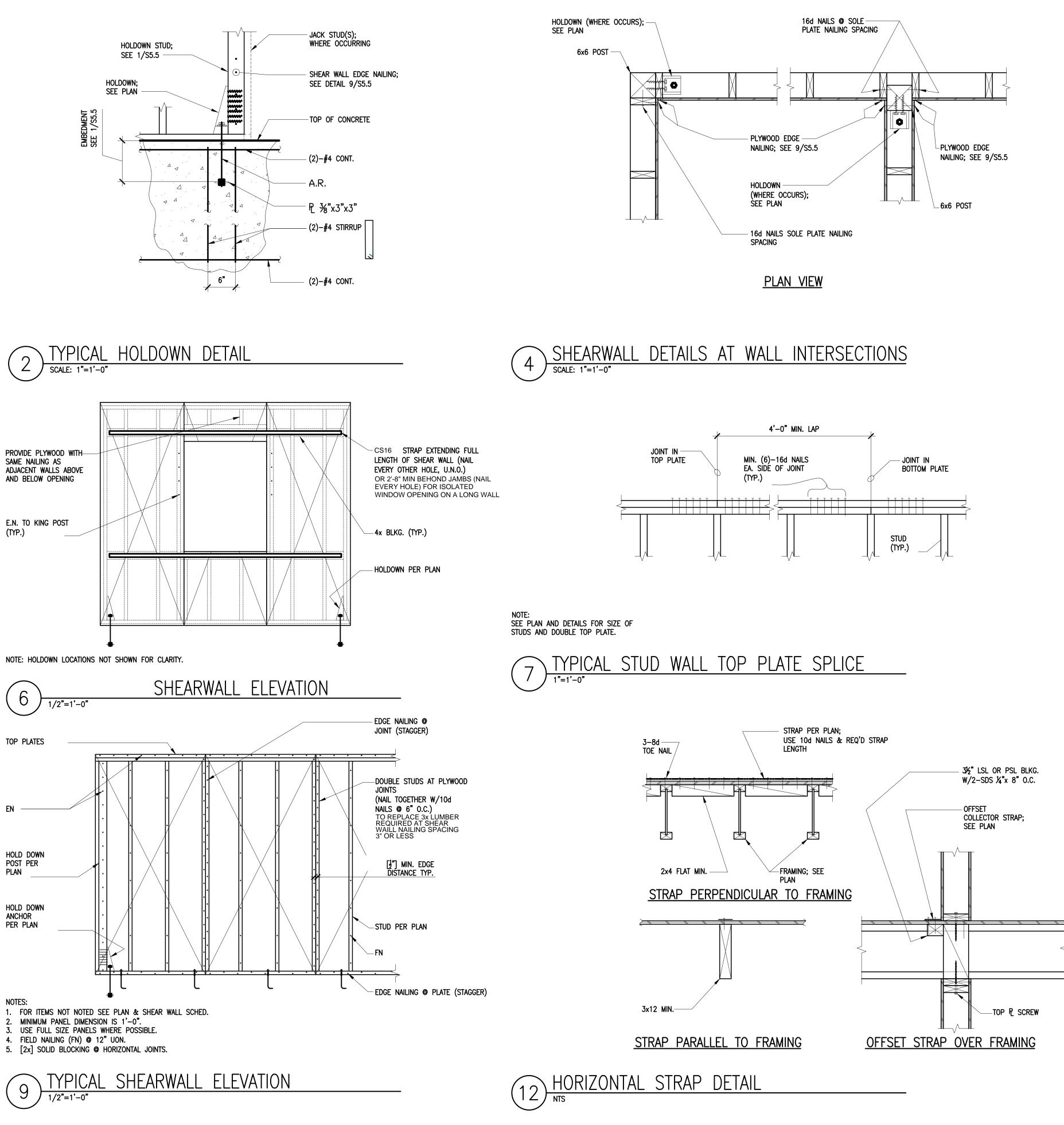
- 2. SEE PLAN FOR TYPICAL STUD SIZES IN SHEARWALLS.
- 3. REFER TO DETAIL 2/S5.5 FOR TYPICAL HOLDOWN INSTALLATION DETAILS.
- 4. ALT. USE (2) 2x STUDS.

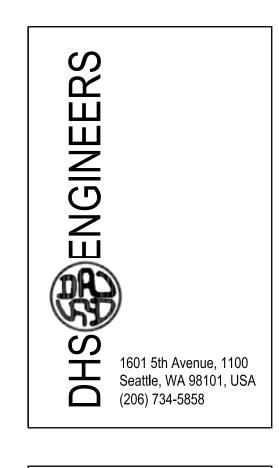


NOTES:

- 1. SEE PLANS FOR SHEAR WALL TYPE, LOCATIONS, AND HOLDOWNS.
- 2. REFER TO SHEET S5.2 FOR TYPICAL SHEAR WALL CONSTRUCTION DETAILS.
- 3. REFER TO DETAIL 9/S5.5 FOR TYPICAL SHEAR WALL ELEVATION.
- 4. REFER TO DETAIL 2/S5.5 FOR TYPICAL HOLDOWN INSTALLATION DETAILS.
- 5. PLYWOOD SHALL BE PLACED ON THE SIDE OF THE WALL WHERE THE SYMBOL () OCCURS ON THE PLAN.
- 6. ALL INFORMATION IN THE ABOVE SCHEDULE RELATES TO THE ITEMS SHOWN IN THE WALL SECTIONS ON SHEET S5.2. ALL COMPONENTS FOR EACH SHEAR WALL TYPE OCCUR IN THE WALLS BETWEEN THE LEVEL REPRESENTED BY THE FRAMING PLAN, WHERE THE SHEAR WALL TYPES AND LOCATIONS ARE SHOWN, AND THE LEVEL ABOVE.
- 7. <u>EXAMPLE:</u> A SHEAR WALL SHOWN ON THE GROUND FLOOR PLAN WITH A MARK (>> NEXT TO IT SHALL HAVE ALL REQUIRED COMPONENTS FOR TYPE 🔆 SHEAR WALL INSTALLED IN THE WALL BETWEEN THE GROUND FLOOR AND THE FIRST FLOOR.
- 8. AT CONCRETE FOOTINGS, USE 5/6" DIAMETER SILL PLATE BOLTS WITH MINIMUM 7" EMBEDMENT INTO THE CONCRETE. REFER TO THE SCHEDULE ABOVE FOR BOLT SPACING. SEE GENERAL NOTES FOR INFORMATION ABOUT ANCHOR RODS AND EPOXY BOLTS.
- 9. BLOCK ALL UNSUPPORTED PLYWOOD EDGES WITH MINIMUM 2x LAID FLAT BEHIND EDGES OF PLYWOOD.
- 10. SEE GENERAL NOTES FOR PLYWOOD GRADES AND SPECIFICATIONS.
- 11. AT WALLS WITH 2x STUDS, DOUBLE THE STUDS AT PLYWOOD JOINTS PER DETAIL 9/S5.5.

8 SHEARWALL SCHEDULE









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SHEET TITLE LATERAL DETAIL AND **SCHEDULE**

