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ALL CONSTRUCTION SHALL CONFORM TO THE 2018 INTERNATIONAL RESIDENTIAL CODE (IRC) AS AMENDED BY THE STATE OF WASHINGTON AND BE IN ACCORDANCE WITH WASHINGTON STATE LAWS, REGULATIONS AND VARIOUS CODES IMPOSED BY LOCAL

DO NOT SCALE DRAWINGS OR DETAILS - USE GIVEN DIMENSIONS. CHECK DETAILS FOR LOCATION OF ALL ITEMS NOT DIMENSIONED ON THE PLANS. DIMENSIONS ON THE PLANS ARE TO FACE OF FRAMING OR CENTERLINE OF COLUMNS UNLESS

DOOR AND CASED OPENINGS WITHOUT DIMENSIONS ARE TO BE 4" FROM FACE OF ADJACENT WALL OR CENTERED BETWEEN

WALLS, UNLESS NOTED OTHERWISE. VERIFY FIELD CONDITIONS PRIOR TO COMMENCEMENT OF EACH PORTION OF THE WORK.

THE CONTRACTOR SHALL COORDINATE ALL PORTIONS OF THE WORK AS DESCRIBED IN THE CONTRACT DOCUMENTS. NOTIFY THE ARCHITECT FOR RESOLUTION OF ALL DISCREPANCIES PRIOR TO CONSTRUCTION.

CONTRACTOR TO VERIFY ALL DIMENSIONS AND STRUCTURAL MEMBER SIZES PRIOR TO CONSTRUCTION. CONTRACTOR TO INFORM ARCHITECT OF ANY DISCREPANCIES IN THE DRAWINGS OR FROM THE CODES. CONTRACTOR INITIATED CHANGES SHALL BE SUBMITTED IN WRITING TO THE OWNER / ARCHITECT AND STRUCTURAL ENGINEER

CONTRACTOR SHALL BE RESPONSIBLE FOR ALL REQUIRED SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES ALL STRUCTURAL SYSTEMS SUCH AS WOOD TRUSSES WHICH ARE TO BE COMPOSED OF COMPONENTS TO BE FIELD ERECTED

SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH INSTRUCTIONS PREPARED BY THE SUPPLIER.

CONTRACTOR TO COORDINATE FRAMING LAYOUT WITH ELECTRICAL AND MECHANICAL PLAN.

OTHER UTILITIES NOT SHOWN WHICH MAY BE EFFECTED BY THE WORK.

FOR APPROVAL PRIOR TO FABRICATION OR CONSTRUCTION

SOILS DESIGN CRITERIA SHALL BE IN ACCORDANCE WITH THE SOILS REPORT PREPARED BY GEOTECH CONSULTANTS, INC., DATED CLEARING AND GRADING (T.E.S.C. MEASURES):

ALL CLEARING AND GRADING MUST BE IN ACCORDANCE WITH LOCAL JURISDICTION CLEARING AND GRADING EROSION CONTROL STANDARDS, DEVELOPMENT STANDARDS, LAND USE CODE, INTERNATIONAL RESIDENTIAL CODE, PERMIT CONDITIONS, AND ALL OTHER APPLICABLE CODES, ORDINANCES AND STANDARDS. THE DESIGN ELEMENTS WITH THESE PLANS HAVE BEEN REVIEWED TO THESE REQUIREMENTS. ANY VARIANCE FROM THE ADOPTED EROSION CONTROL STANDARDS IS NOT ALLOWED UNLESS SPECIFICALLY APPROVED BY THE LOCAL JURISDICTION PRIOR TO CONSTRUCTION.

A COPY OF THE APPROVED PLANS MUST BE ON-SITE WHENEVER CONSTRUCTION IS IN PROGRESS. THE APPLICANT IS RESPONSIBLE FOR OBTAINING ANY OTHER REQUIRED OR RELATED PERMITS PRIOR TO BEGINNING CONSTRUCTION. ALL LOCATIONS OF EXISTING UTILITIES HAVE BEEN ESTABLISHED BY FIELD SURVEY OR OBTAINED FROM AVAILABLE RECORDS AND SHOULD, THEREFORE, BE CONSIDERED ONLY APPROXIMATE AND NOT NECESSARILY COMPLETE. IT IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR TO INDEPENDENTLY VERIFY THE ACCURACY OF ALL UTILITY LOCATIONS AND TO DISCOVER AND AVOID ANY

FINAL SITE DRAINAGE MUST DIRECT DRAINAGE AWAY FROM ALL BUILDING STRUCTURES AT A MINIMUM OF 6" WITHIN THE FIRST 10'. Ref IRC R401.3 CRAWL SPACE:

UNDER-FLOOR AREAS SHALL BE VENTED BY AN APPROVED MECHANICAL MEANS OR BY OPENINGS IN EXTERIOR FOUNDATION WALLS. SUCH OPENINGS SHALL HAVE A NET AREA OF NOT LESS THAN 1 SQ. FT. FOR EACH 150 SQ. FT. OF UNDER-FLOOR AREA. ONE OPENING SHALL BE WITHIN 3' OF EACH CORNER OF THE BUILDING. Ref IRC R408.2 CRAWL SPACE, UNOBSTRUCTED ACCESS, MINIMUM 18" x 24". Ref IRC R408.4

PROVIDE 18" MINIMUM CRAWL SPACE UNDER WOOD JOIST AND 12" MINIMUM CRAWL SPACE UNDER WOOD GIRDERS. Ref IRC

A GROUND COVER VAPOR BARRIER OF MIN. 6 MIL. (0.006") POLYETHYLENE (0R EQUIVALENT) SHALL BE INSTALLED IN ALL CRAWL SPACES, JOINTS LAPPED 12", EXTEND UP FOUNDATION WALL AND SECURE TO SILL PLATE WHEREVER PRACTICAL ALL WOOD IN CONTACT WITH CONCRETE, CMU OR WITHIN 8" OF SOILS SHALL BE PRESSURE TREATED WOOD. Ref IRC R317.1

OPENINGS FROM A PRIVATE GARAGE DIRECTLY INTO A ROOM USED FOR SLEEPING PURPOSES SHALL NOT BE PERMITTED. DOORS BETWEEN GARAGE AND DWELLING SHALL BE SOLID WOOD DOORS; MINIMUM 1 3/8" THICK WITH SELF CLOSING DEVICE. Ref

SEPARATION FROM DWELLING TO GARAGE, SHOP OR SIMILAR AREAS SHALL BE SEPARATED FROM RESIDENCE AND ITS ATTIC AREA BY NOT LESS THAN 1/2" GYPSUM BOARD APPLIED TO THE GARAGE SIDE. GARAGES BENEATH HABITABLE ROOMS SHALL BE SEPARATED FROM ALL HABITABLE ROOMS ABOVE BY NOT LESS THAN 5/8" TYPE X GYPSUM BOARD OR EQUIVALENT. WHERE THE SEPARATION IS A FLOOR-CEILING ASSEMBLY. THE STRUCTURE SUPPORTING THE SEPARATION SHALL ALSO BE PROTECTED BY

HEATING AND/OR COOLING EQUIPMENT LOCATED IN GARAGE SHALL BE INSTALLED WITH PILOTS AND BURNERS OR HEATING ELEMENTS AND SWITCHES AT LEAST 18" ABOVE THE FLOOR LEVEL. Ref IRC G2408.2

NOT LESS THAN 1/2" GYPSUM BOARD OR EQUIVALENT. Ref IRC R302.6 & TABLE 302.6

FACTORY-BUILT FIREPLACES SHALL BE LISTED AND LABELED AND SHALL BE INSTALLED IN ACCORDANCE WITH THE CONDITIONS OF THE LISTING. FACTORY-BUILT FIREPLACES SHALL BE TESTED IN ACCORDANCE WITH UL 127. Ref IRC R1004.1

MASONRY FIREPLACES. BARBECUES. SMOKE CHAMBERS AND FIREPLACE CHIMNEYS SHALL BE CONSTRUCTED OF MASONRY OR REINFORCED CONCRETE. FOUNDATIONS SHALL BE MIN. 12" THICK AND EXTEND MIN. 6" BEYOND MASONRY. FIREBOX WALLS MIN. 10" THICK EXCEPT MIN. 8" THICK WHERE A FIREBRICK LINING IS USED. COMBUSTIBLE MATERIALS SHALL NOT BE PLACED WITHIN 2 INCHES OF FIREPLACE, SMOKE CHAMBER OR CHIMNEY WALLS. COMBUSTIBLE MATERIAL SHALL NOT BE PLACED WITHIN 6" OF THE FIREPLACE OPENING. MIN. 4" THICK NON-COMBUSTIBLE HEARTH EXTENDING 16" IN FRONT AND 8" TO THE SIDE OF THE FIREPLACE OPENING. COMBUSTIBLE MATERIAL WITHIN 12" OF THE FIREPLACE OPENING SHALL NOT PROJECT MORE THAN 1/8" FOR EACH 1"

CEILING HEIGHTS:

GARAGES:

HABITABLE SPACE SHALL HAVE A CEILING HEIGHT OF NOT LESS THAN 7"-0". NOT MORE THAN 50% OF REQUIRED FLOOR AREA OF A SPACE IS PERMITTED TO HAVE A SLOPED CEILING LESS THAN 7'-0" IN HEIGHT WITH NO PORTION LOWER THAN 5'-0". BATHROOM SHALL HAVE A MIN CEILING HEIGHT OF 6'-8" OVER THE FIXTURE AND ITS FRONT CLEARANCE AREA. Ref IRC R:305

APPLY ROOFING IN ACCORDANCE WITH IRC R905

BALCONIES, LANDINGS, EXTERIOR STAIRWAYS, OCCUPIED ROOFS AND SIMILAR SURFACES EXPOSED TO THE WEATHER AND SEALED UNDERNEATH SHALL BE WATERPROOFED AND SLOPED A MINIMUM OF 1/4" PER 12" (2% SLOPE) FOR DRAINAGE.

PROVIDE ATTIC VENTILATION AS INDICATED ON ROOF FRAMING PLANS. THE MINIMUM NET FREE VENTILATING AREA SHALL BE 1/150 OF THE AREA OF THE VENTED SPACE. EXCEPTION: THE MINIMUM NET FREE VENTILATION AREA SHALL BE 1/300 OF THE VENTED SPACE PROVIDED NOT LESS THAN 40 PERCENT AND NOT MORE THAN 50 PERCENT OF THE REQUIRED VENTILATING AREA IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF THE ATTIC OR RAFTER SPACE. UPPER VENTILATORS SHALL BE LOCATED NOT MORE THAN 3 FEET BELOW THE RIDGE OR HIGHEST POINT OF THE SPACE, MEASURED VERTICALLY, WITH THE BALANCE OF THE REQUIRED VENTILATION PROVIDED BY EAVE OR CORNICE VENTS. Ref IRC R806.2 ATTIC ACCESS SHALL HAVE A ROUGH FRAMED OPENING NOT LESS THAN 22 INCHES BY 30 INCHES LOCATED IN A READILY

ACCESSIBLE LOCATION. THE MINIMUM UNOBSTRUCTED HEADROOM IN THE ATTIC SPACE SHALL BE 30 INCHES MEASURED /ERTICALLY FROM THE BOTTOM OF THE CEILING FRAMING MEMBERS. Ref IRC R807. FOR ACCESS REQUIREMENTS WHERE MECHANICAL EQUIPMENT IS LOCATED IN ATTICS Ref IRC M1305.1.3

GLAZING:

TO BE IN COMPLIANCE WITH IRC R308 AND WASHINGTON STATE SAFETY GLASS LAW.

GLAZING IN HAZARDOUS LOCATIONS SUCH AS GLASS ON DOORS, GLAZING WITHIN 24" ON EITHER SIDE OF A DOOR OPENING AREAS WITHIN 60" VERTICAL AND 36" HORIZONTAL OF THE BOTTOM LANDING OF A STAIRWAY, STORM DOORS, RAILINGS, SHOWER DOORS, SLIDING GLASS DOORS, AND TUB ENCLOSURES SHALL BE SAFETY GLAZING MATERIAL. Ref IRC R308.4 ALL EXTERIOR WALL GLAZING SHALL COMPLY WITH THE 2021 EDITION OF THE WASHINGTON STATE ENERGY CODE.

EGRESS IN EVERY SLEEPING ROOM SHALL HAVE AT LEAST ONE OPERABLE EMERGENCY EXIT WITH A MINIMUM NET CLEAR OPENING OF 5.7 SQ. FT. THE MINIMUM NET CLEAR OPENING HEIGHT DIMENSION SHALL BE 24" MINIMUM NET CLEAR OPENING WIDTH DIMENSION OF 20" AND A FINISHED SILL HEIGHT NOT MORE THAN 44" ABOVE THE FLOOR. IRC R310.1.

ONE EXIT DOOR CONFORMING TO IRC R311.2 IS REQUIRED. FIRE & CARBON MONOXIDE PROTECTION:

SMOKE & CARBON MONOXIDE DETECTOR POWER SOURCES TO BE INSTALLED IN ACCORDANCE WITH NFPA 72, IRC R314 & IRC R315. ALL ALARM DEVICES SHALL BE INTERCONNECTED PER IRC R314.1. FIREBLOCKING PER IRC R1003.19, R1001.12, R302.11 & R602.8. DRAFTSTOPPING PER IRC R302.12 & R502.12.

HABITABLE ROOMS NOT PROVIDED WITH AN OPENABLE EXTERIOR OPENING OF AT LEAST 4% OF THE FLOOR AREA, A MECHANICAL VENTILATION SYSTEM MUST BE PROVIDED THAT PROVIDES MIN. .35 AIR CHANGES PER HOUR. IRC R303.1. DRYER & BATH FANS TO BE 50 CFM, AND RANGE/OVEN FANS TO BE 100 CFM MIN, VENT TO THE OUTSIDE. IRC303 AND 2006 WA

NATURAL LIGHTING TO BE NOT LESS THAN 8% OF THE FLOOR AREA OR ALL HABITABLE SPACES. IRC R303.

MINIMUM HEADROOM OF 6'-8" MEASURED VERTICALLY FROM A SLOPED PLANE ADJOINING THE TREAD NOSING OR FROM THE FLOOR SURFACE OR PLATFORM. IRC R311.7.2 MINIMUM WIDTH 36", IRC 311.7.1 MINIMUM TREAD 10", MAXIMUM RISER 7 3/4", HANDRAIL MINIMUM 34" AND MAXIMUM 38" ABOVE STAIR NOSING. HANDRAIL TO BE 1 1/4" TO 2" CROSS SECTION AND 1 1/2" AWAY FROM WALL. IRC R311.7.5 & 311.7.8. INSTALL FIRE BLOCKING AT MID STRINGER SPAN AND AT WALL ALONG STRINGER. COVER WALLS AND SOFFITS OF USABLE SPACE UNDER STAIR WITH 1/2" GYPSUM BOARD. IRC

GUARDRAILS: ANY WALKING SURFACE 30" OR MORE ABOVE GRADE OR ADJACENT SURFACE SHALL HAVE MIN. 36" HIGH

BATHROOMS: ALL TUB AND SHOWER STALLS SHALL HAVE FIREBLOCKING BETWEEN STUDS.

ALL GLAZING USED FOR DOORS OR ENCLOSURES IN BATHROOMS SHALL BE SAFETY GLAZING. GLAZING IN ANY PORTION OF A BUILDING WALL ENCLOSING A SHOWER OR BATHTUB WHERE THE BOTTOM EXPOSED EDGE IS LESS THAN 60 INCHES ABOVE THE

OR SIMILAR ANCHORING METHOD.

IRC G2404.4

STANDING SURFACE AND DRAIN INLET SHALL BE SAFETY GLAZING. IRC R308.4 BATH TUB & SHOWER STALL NON-ABSORBENT WAINSCOTS SHALL BE A MINIMUM OF 72 INCHES ABOVE THE FLOOR. IRC R307.2. WATERCLOSETS SHALL HAVE MIN. 15" TO SIDE WALLS FROM CENTER OF FIXTURE, AND MIN. 21" FRONT CLEARANCE. IRC R307.1 APPLIANCES IN A FIXED POSITION SHALL BE SECURELY FASTENED IN PLACE TO STRUCTURAL MEMBERS WITH STRAP ANCHORS

> SLAB ON GRADE: INSULATE PER TABLE R402.1.1. PROVIDE EXTRUDED RIGID CLOSED CELL INSULATION. INSULATION, INSTALLED INSIDE THE FOUNDATION WALL, SHALL EXTEND DOWNWARD FROM THE TOP OF THE SLAB 24" MIN. OR DOWNWARD AND THEN HORIZONTALLY BENEATH THE SLAB FOR A COMBINED 24" MIN. INSULATION INSTALLED OUTSIDE THE FOUNDATION SHALL EXTEND VAPOR BARRIERS: VAPOR RETARDERS SHALL BE INSTALLED ON THE WARM SIDE (IN WINTER) OF INSULATION PER TABLE R402.4.1.1 FLOORS SEPARATING CONDITIONED SPACE FROM UNCONDITIONED SPACE SHALL HAVE MIN. 4 MIL POLYETHYLENE OR KRAFT

WALLS: INSULATED PER WSEC TABLE R402.1.1.

FLOORS: INSULATE PER WSEC TABLE R402.1.1

PLUMBING FIXTURES:

Skylight U-Factor ^s

Wood Frame Wall &

Below Grade Wall ^{c,h}

Slab ^{a,r} R-Value & Depth

R-10 AND UNDER ENTIRE SLAB

R402.1.1, UNLIMITED GLAZING WITH MODIFICATIONS

ENERGY CREDITS - 3 CREDITS REQUIRED, 3 CREDITS SELECTED

Glazed Fenestration SHGC 19

PSI. WAC 51-56-0400 (407.2.1.1 RESIDENTIAL LAVATORY FAUCETS)

SHOWERHEADS WAC 51-56-0400 (408.2 WATER CONSUMPTION)

AGENCY (EPA) WAC 51-56-0400 (411.2.2 PERFORMANCE)

WITH ASME A112.18.1/CSA B125.1 *WAC 51-56-0400 (420.2 WATER CONSUMPTIO*

RATE OF ALL SHOWERHEADS AND/OR OTHER SHOWER OUTLETS CONTROLLED BY A SINGLE VALVE SHALL NOT EXCEED 1.8

OPERATION AT A TIME WAC 51-56-0400 (408.2.1 MULTIPLE SHOWERHEADS SERVING ONE SHOWER)

ACCORDANCE WITH ASME A112.19.2/CSA B45.1 WAC 51-56-0400 (411.2 WATER CONSUMPTION)

GALLONS (6.81 L) PER MINUTE AT 80 PSI, OR THE SHOWER SHALL BE DESIGNED TO ALLOW ONLY ONE SHOWER OUTLET TO BE IN

THE EFFECTIVE FLUSH VOLUME OF ALL WATER CLOSETS SHALL NOT EXCEED 1.28 GALLONS (4.8 L) PER FLUSH WHEN TESTED IN

DUAL FLUSH WATER CLOSETS SHALL COMPLY WITH ASME A112.19.14. THE EFFECTIVE FLUSH VOLUME FOR DUAL FLUSH WATER

CLOSETS SHALL BE DEFINED AS THE COMPOSITE, AVERAGE FLUSH VOLUME OF TWO REDUCED FLUSHES AND ONE FULL FLUSH.

WATER CLOSETS INSTALLED SHALL MEET OR EXCEED THE MINIMUM PERFORMANCE CRITERIA DEVELOPED FOR CERTIFICATION

All Climate Zones (Table R402.1.

10/15/21 int + TB

METHOD OF COMPLIANCE - PRESCRIPTIVE METHOD FOR GROUP R OCCUPANCY, CLIMATE ZONE PER TABLE R301.1, TABLE

0.5 CREDITS - OPTION 1.3 - EFFICIENT BUILDING ENVELOPE: BASED ON TABLE 402.1.1 WITH THE FOLLOWING MODIFICATIONS

ERTICAL FENESTRATION U=0.28. FLOOR R-38. SLAB ON GRADE R-10 PERIMETER AND UNDER ENTIRE SLAB - BELOW GRADE SLAB

0.5 CREDITS - OPTION 2.1 - AIR LEAKAGE CONTROL AND EFFICIENT VENTILATION: COMPLIANCE BASED ON R402.4.1.2: REDUCEDTHE

ALL WHOLE HOUSE VENTILATION REQUIREMENTS AS DETERMINED BY SECTION M1507.3 OF THE INTERNATIONAL RESIDENTIAL

CODE OR SECTION 403.8 OF THE INTERNATIONAL MECHANICAL CODE SHALL BE MET WITH A HIGH EFFICIENCY FAN(S) (MAXIMUM 0.35 WATTS/CFM), NOT INTERLOCKED WITH THE FURNACE FAN (IF PRESENT). VENTILATION SYSTEMS USING A FURNANCE

1.0 CREDITS - OPTION 3.1 - HIGH EFFICIENCY HVAC: ENERGY STAR RATED (U.S. NORTH) GAS OR PROPANE FURNANCE WITH

0.5 CREDITS - OPTION 4.1 - HIGH EFFICIENCY HVAC DISTRIBUTION SYSTEM: ALL SUPPLY AND RETURN DUCTS LOCATED IN AN

FOR MECHANICAL EQUIPMENT LOCATED OUTSIDE THE CONDITIONED SPACE, A MAXIMUM OF 10 LINEAR FEET OF RETURN DUCT AND 5 LINEAR FEET OF SUPPLY DUCT CONNECTIONS TO THE EQUIPMENT MAY BE OUTSIDE THE DEEPLY BURIED INSULATION. ALL

METALLIC DUCTS LOCATED OUTSIDE THE CONDITIONED SPACE MUST HAVE BOTH TRANSVERSE AND LONGITUDINAL JOINTS

0.5 CREDITS - OPTION 5.2 - EFFICIENT WATER HEATING: WATER HEATING SYSTEM SHALL INCLUDE ONE OF THE FOLLOWING:

ROOF AND CEILING: INSULATED PER WSEC TABLE R402.1.1. PROVIDE INSULATION IN CEILING WHERE POSSIBLE AND IN 2x12

RAFTERS IF VAULTED CEILING CONDITION EXISTS. MAINTAIN A MINIMUM OF 2" CLEAR BETWEEN TOP OF INSULATION AND BOTTOM OF SHEATHING FOR VENTING. VENTING MUST OCCUR IN EACH JOIST SPACE. WHERE CONTINUOUS VENTING WITHIN A JOIST SPACE IS INTERRUPTED BY A HEADER (I.E., SKYLIGHT OR AT HIP END), PROVIDE (2) 1 1/2" VENTING HOLES AT THE TOP OF THE

UNCONDITIONED ATTIC SHALL BE DEEPLY BURIED IN CEILING INSULATION IN ACCORDANCE WITH SECTION R403.3.7.

NCLUDING AN ECM MOTOR ARE ALLOWED, PROVIDED THAT THEY ARE CONTROLLED TO OPERATE AT LOW SPEED IN VENTILATION

0.0 CREDIT - OPTION 1 - COMBUSTION HEATING MINIMUM NAECA, PER TABLE C403.3.2(4) OR C403.3.2(5)

TESTED AIR LEAKAGE TO 3.0 AIR CHANGES PER HOUR MAXIMUM AT 50 PASCALS AND

SEALED WITH MASTIC. IF FLEX DUCTS ARE USED, THEY CANNOT CONTAIN SPLICES.

ENERGY STAR RATED GAS OR PROPANE WATER HEATER WITH A MINIMUM UEF OF 0.80.

AIR HANDLER(S) SHALL BE LOCATED WITHIN THE CONDITIONED SPACE.

DUCT LEAKAGE SHALL BE LIMITED TO 3 CFM PER 100 SQUARE FEET OF CONDITIONED FLOOR AREA.

RAFTER AT THE HEADER TO ALLOW FOR CONTINUAL THROUGH-VENTING INTO THE NEXT JOIST SPACE.

OF HIGH-EFFICIENCY TOILETS UNDER THE WATERSENSE PROGRAM SPONSERED BY THE U.S. ENVIRONMENTAL PROTECTION

FACED MATERIAL. ROOF/CEILING ASSEMBLIES WHERE THE VENTILATION SPACE ABOVE THE INSULATION IS LESS THAN AN AVERAGE OF 12 INCHES SHALL BE PROVIDED WITH A VAPOR RETARDER. WALLS SEPARATING CONDITIONED SPACE FROM UNCONDITIONED SPACE SHALL HAVE A VAPOR RETARDER INSTALLED. FACED BATT INSULATION SHALL BE FACE STAPLED. A GROUND COVER OF MIN. 6 MIL BLACK POLYETHYLENE SHALL BE LAID OVER THE GROUND WITHIN CRAWL SPACES W/ JOINTS

GLAZING AND DOORS: GLAZING AND DOOR U-FACTORS SHALL BE DETERMINED IN ACCOURDANCE WITH WSEC SECTIONS R402.1.1

SEIFERT REMODEL

3261 67TH AVE SE MERCER ISLAND, WA 98040



PROJECT DIRECTORY

CODE CITED CURRENT AS OF SEPTEMBER 2022. CONTRACTOR AND THEIR CONSULTANTS TO CONFIRM LISTED CODE IS CURRENT Lochwood - Lozier Custom Homes THE MAXIMUM FLOW RATE OF RESIDENTIAL LAVATORYFAUCETS SHALL NOT EXCEED 1.2 GALLONS (4.54 L) PER MINUTE AT 60 PSI.

THE MINIMUM FLOW RATE OF RESIDENTIAL LAVATORY FAUCETS SHALL NOT BE LESS THAN 0.8 GALLONS (3.03 L) PER MINUTE AT 20 8708 152nd Ave NE SHOWERHEADS SHALL MEET THE MAXIMUM FLOW RATE OF 1.8 GALLONS (6.81 L) PER MINUTE MEASURED AT 80 PSI. SHOWERHEADS SHALL BE CERTIFIED TO THE PERFORMANCE CRITERIA OF THE U.S. EPAWATERSENSE SPECIFICATIONS FOR Redmond, WA 98052 (425) 576-9200 t.lozier@lochwoodlozier.com WHEN A SHOWER IS SERVED BY MORE THAN ONE SHOWERHEAD. INCLUDING HANDHELD SHOWERHEADS. THE COMBINED FLOW

STRUCTURAL ENGINEER

Dibble Engineers Attn: Travis Colliander 1029 Market Street

Kirkland WA 98033 (425) 828-4200 ext 224 SINK FAUCETS SHALL HAVE A MAXIMUM FLOW RATE OF NOT MORE THAN 2.2 GPM AT 60 PSI (8.3 L/M AT 414 KPA) IN ACCORDANCE travisc@dibbleengineers.com KITCHEN FAUCETS SHALL HAVE A MAXIMUM FLOW RATE OF NOT MORE THAN 1.8 GALLONS (6.81 L) PER MINUTE AT 60 PSI. KITCHEN FAUCETS MAY TEMPORARILY INCREASE THE FLOW ABOVE MAXIMUM RATE, BUT NOT TO EXCEED 2.2 GALLONS (8.3 L) PER MINUTE AT 60 PSI. AND MUST DEFAULT TO A MAXIMUM FLOW RATE OF 1.8 GALLONS (6.81 L) PER MINUTE AT 60 PSI. WAC 51-56-0400 (420.2.1)

ENVIRONMENTAL CONSULTANT

Altmann Oliver Associates, LLC Attn: John Altmann

PO Box 578 Carnation, WA 98014 425.333.4535 john@altoliver.com

GEOTECHNICAL ENGINEER

2401 10th Avenue East Seattle, WA 98102 425.260.1116

marcm@geotechnw.com

GENERAL CONTRACTOR

Gelotte Hommas Drivdahl Architecture Attn: Todd Lozier Attn: David Grubb

2340 130th Ave. NE, Suite 100 Bellevue, WA 98005 425.828.3081 davidg@ghdarch.com

INTERIOR DESIGNER

<u>ARCHITECT</u>

NB Design Group Interiors Attn: Whitney Maehara 1932 First Avenue, Suite 826 Seattle, WA 98101 206.441.7754 whitney@nbdesigngroup.net

LANDSCAPE ARCHITECT

Altman Oliver Associates, LLC Attn: Simone Oliver PO Box 578 Carnation WA 98014 (425) 333-4535

simone@altoliver.com

Geotech Consultants, INC Attn: Marc McGinnis

VICINITY MAP



BUILDING AREA CALCULATIONS

PERMIT IS REQUIRED."

PROJECT ADDRESS

MERCER ISLAND, WA 98040

LEGAL DESCRIPTION & TAX PARCEL NUMBER

2018 IFGC - International Fuel Gas Code, 2014 NFPA 58 & 2014 NFPA 54

CITY OF MERCER ISLAND

Unlimited

"A NFPA 13D FIRE SPRINKLER SYSTEM IN COMPLIANCE WITH NFPA BD AND CoMI

STANDARDS SHALL BE INSTALLED THROUGHOUT THE RESIDENCE. A SEPARATE FIRE 🛴

R-3 (SINGLE FAMILY RESIDENCE)

3 STORIES ABOVE GRADE PLANE (4 Stories

THAT PORTION OF TRACT 7, JERSEY WATER FRONT ADDITION TO EAST

SEATTLE, ACCORDING TO THE PLAT THEREOF RECORDED IN VOLUME 8 OF

COMMENCING AT THE POINT ON THE WEST LINE OF HOOD AVENUE IN THE

PLATS, PAGE 64, RECORDS OF KING COUNTY, WASHINGTON, DESCRIBED AS

3261 67TH AVE SE

REFER TO SHEET A1.03 FOR GROSS FLOOR AREA CALCS

BUILDING HEIGHT CALCULATIONS

REFER TO SHEET A1.04 FOR HEIGHT CALCULATIONS

DRAWING INDEX

COVER SHEET

DEMOLITION SITE PLAN PROPOSED ARCHITECTURAL SITE PLAN

GROSS FLOOR AREA CALCS

BUILDING HEIGHT CALCS

WALL RETAINAGE CALCS

LOWER FLOOR PLAN DEMO MAIN FLOOR PLAN DEMO

UPPER FLOOR PLAN DEMO

ROOF PLAN DEMO

EXTERIOR ELEVATIONS DEMO EXTERIOR ELEVATIONS DEMO

LOWER FLOOR PLAN PROPOSED

MAIN FLOOR PLAN PROPOSED UPPER FLOOR PLAN PROPOSED

ROOF PLAN PROPOSED

EXTERIOR ELEVATIONS PROPOSED EXTERIOR ELEVATIONS PROPOSED

BUILDING SECTIONS

BUILDING SECTIONS **BUILDING SECTIONS**

DOOR AND WINDOW SCHEDULES INTERIOR ELEVATIONS LOWER FLOOR

GENERAL NOTES, ABBREVIATIONS, INDEX FOUNDATION PLAN, SW KEY PLAN

MAIN FLOOR FRAMING PLAN, SW KEY PLAN UPPER FLOOR FRAMING PLAN

SECTIONS & DETAILS

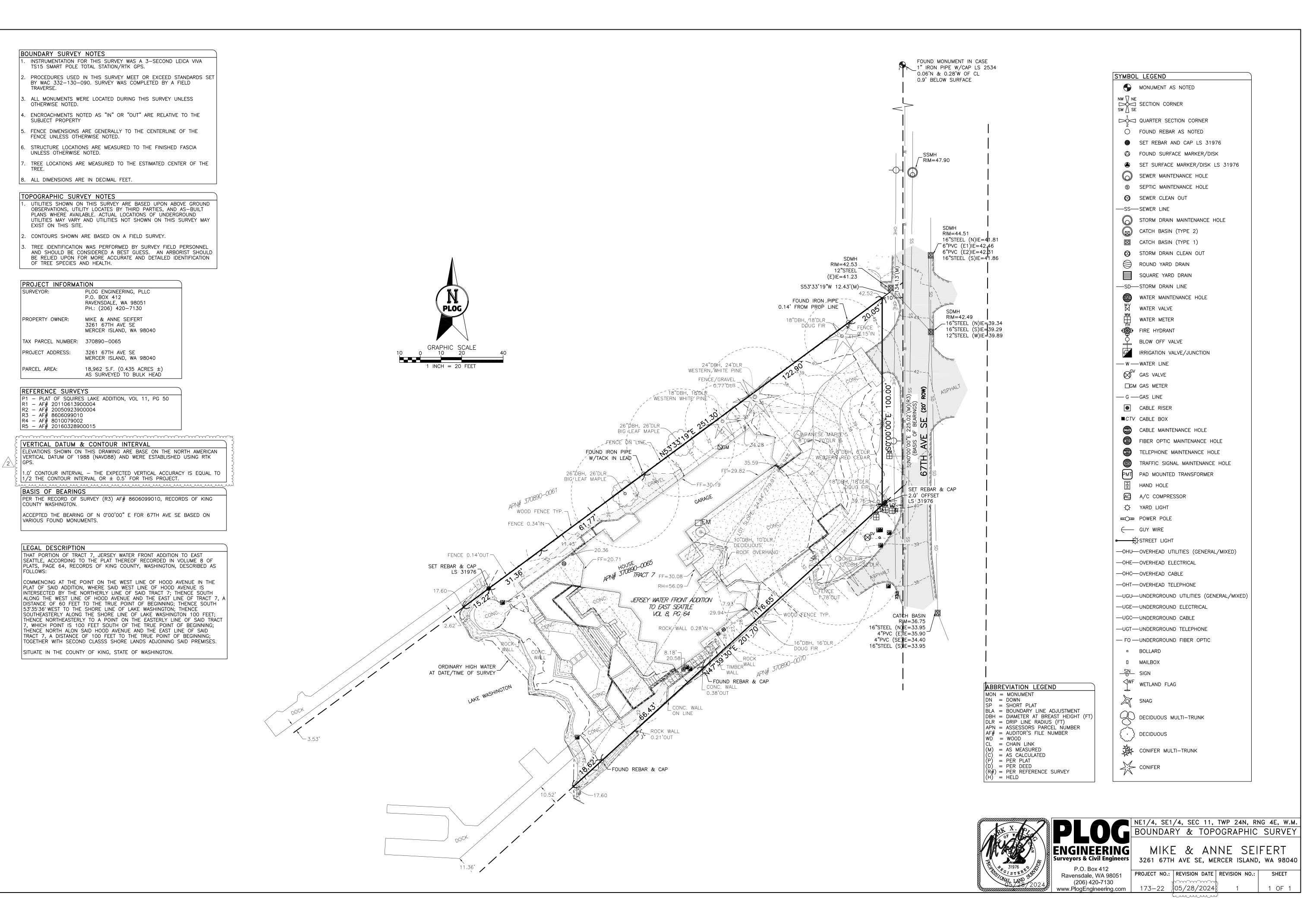
ROOF FRAMING PLAN SECTIONS & DETAILS

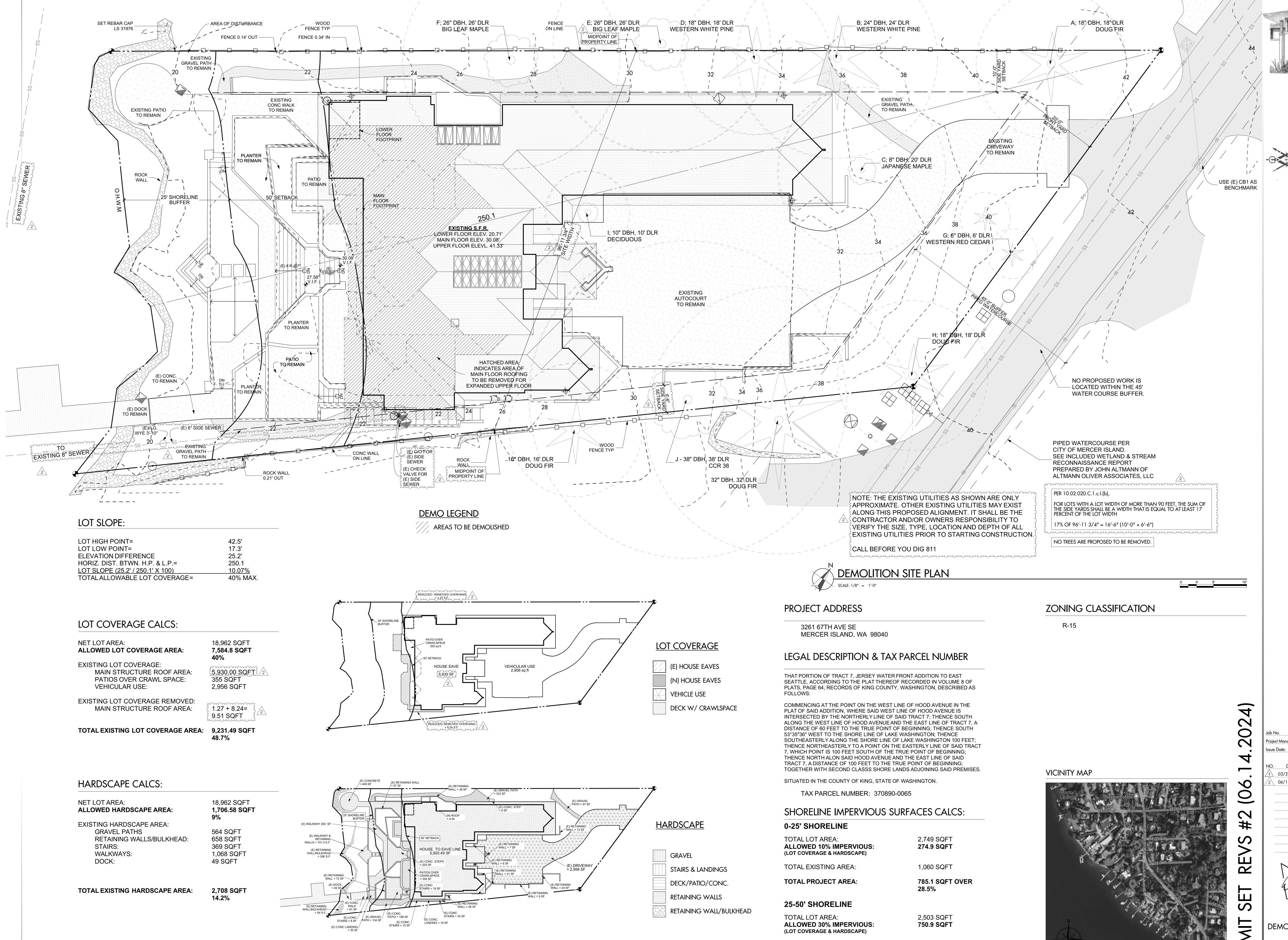
SECTIONS & DETAILS

SECTIONS & DETAILS

SECTIONS & DETAILS

03/29/2024 03/29/2024 OWNER REVISIONS 06/14/2024 PERMIT REVISION #2





TOTAL EXISTING AREA:

TOTAL PROJECT AREA:

1,676 SQFT

66.9%

924.1 SQFT OVER

REA SEIFE

03/29/2024

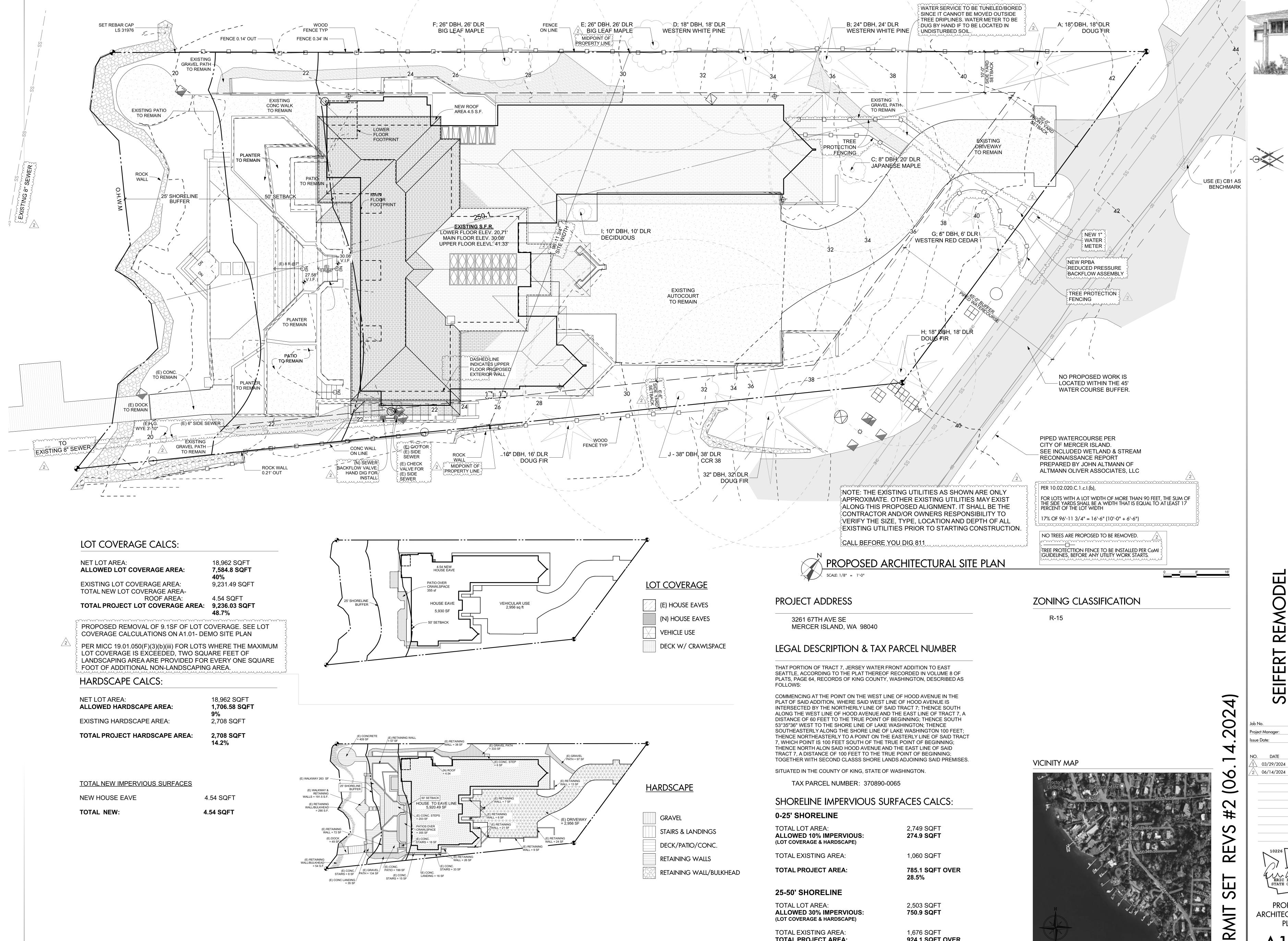
03/29/2024 OWNER REVISIONS 06/14/2024 PERMIT REVISION #2

ERIC L. DRIVDAHL STATE OF WASHINGTON

DEMOLITION SITE PLAN

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PER



TOTAL PROJECT AREA:

924.1 SQFT OVER

66.9%

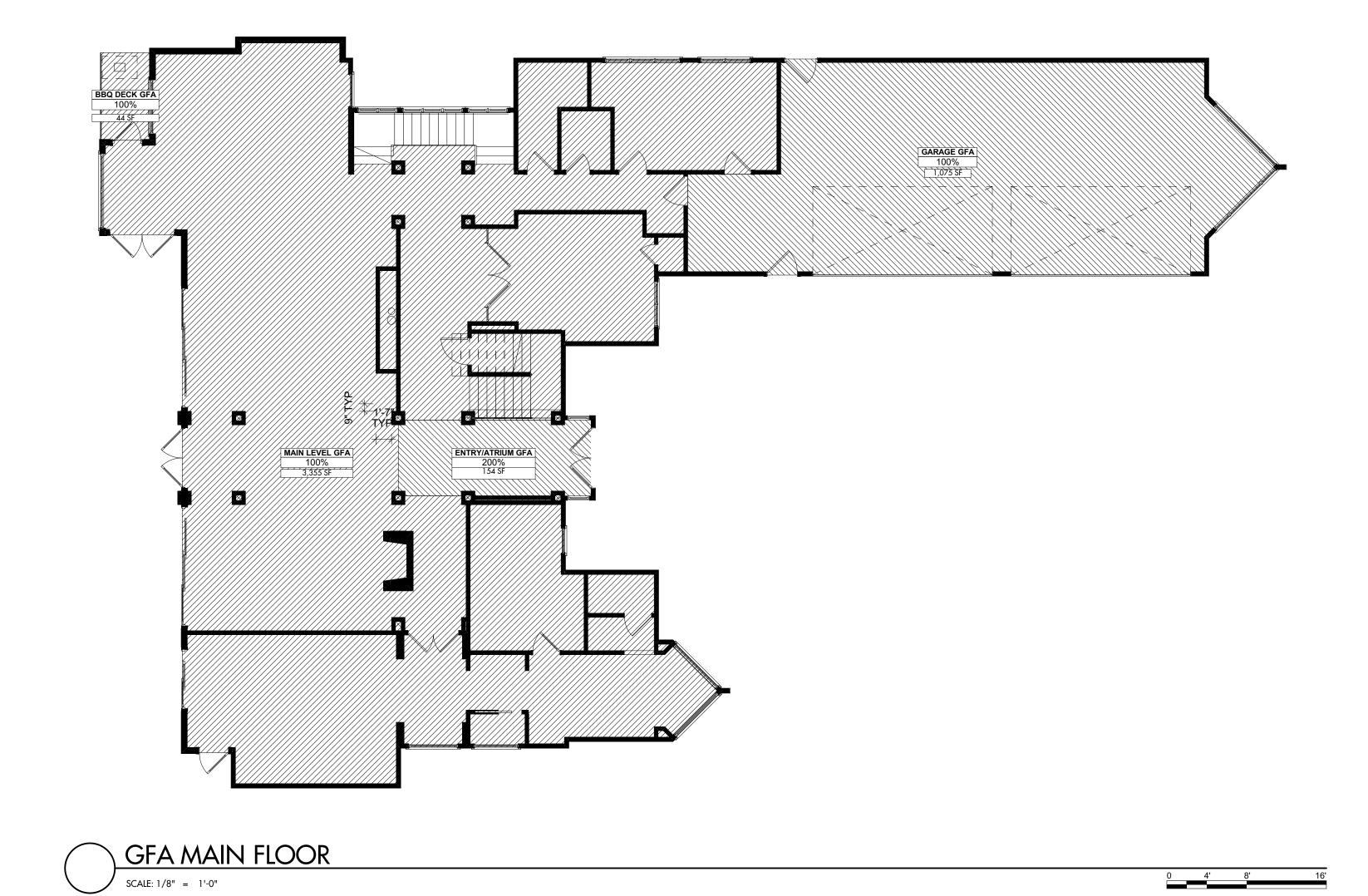
03/29/2024

03/29/2024 OWNER REVISIONS 06/14/2024 PERMIT REVISION #2

ERIC L. DRIVDAHL STATE OF WASHINGTON **PROPOSED** ARCHITECTURAL SITE

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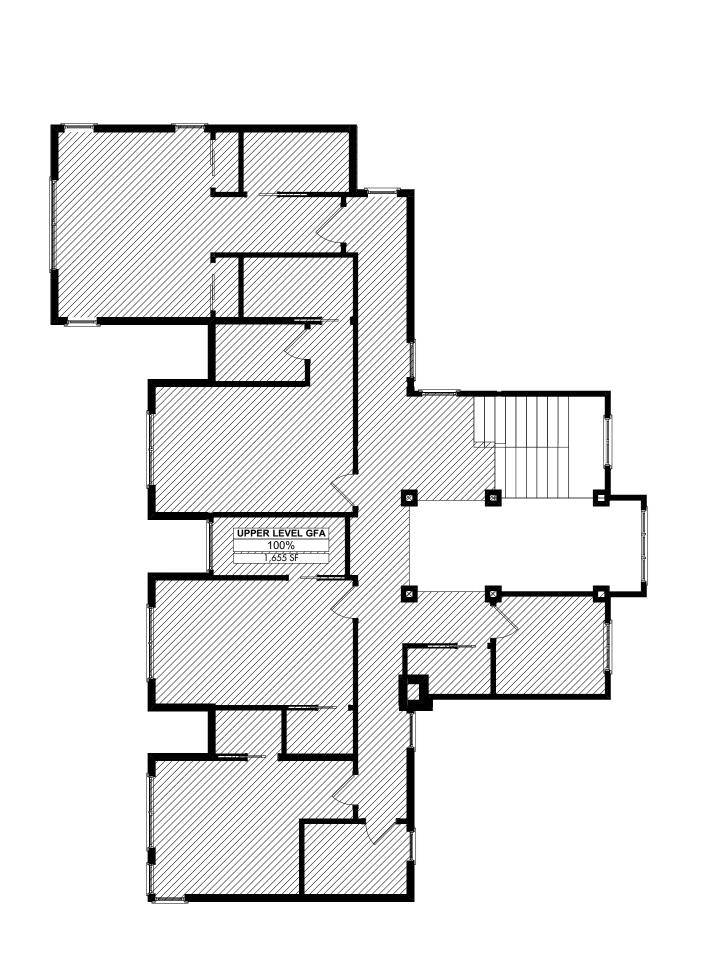


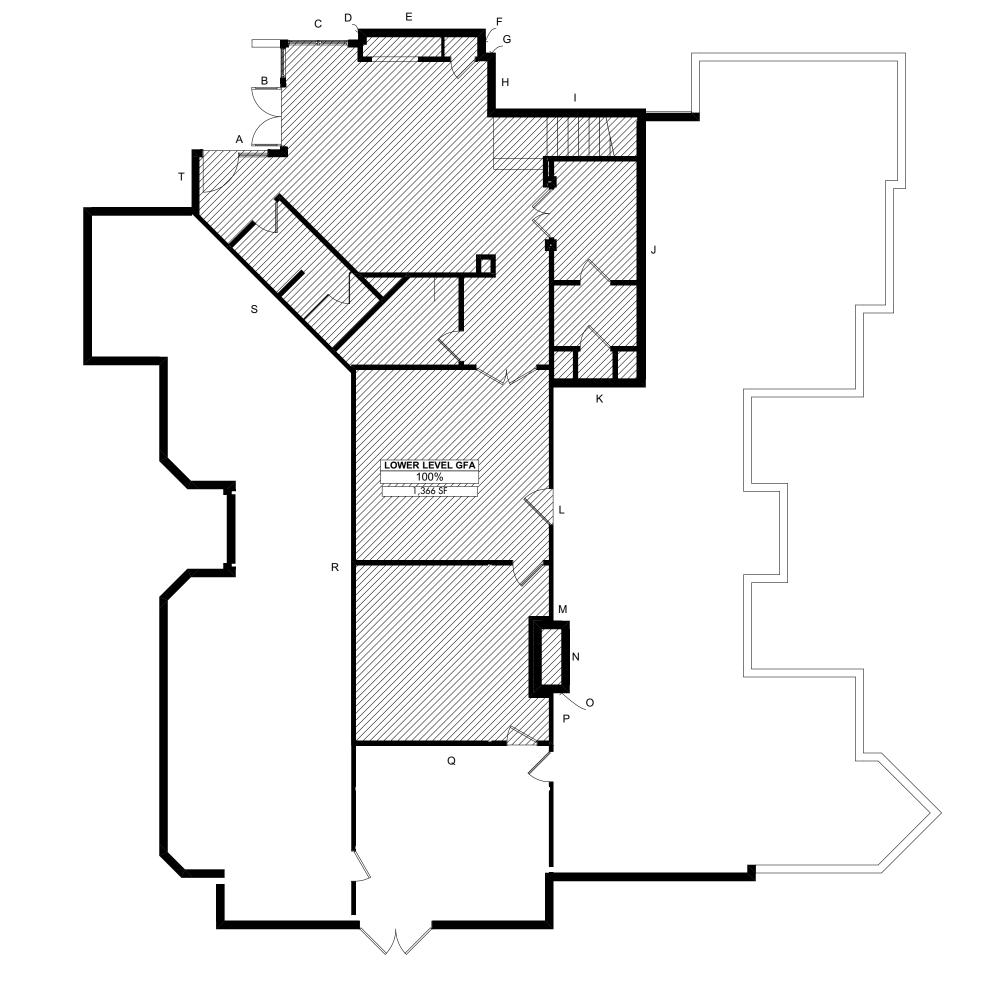
	Lot Area	Code factor		
Allowable Area:	18,962	0.40	7,585	
Proposed Areas:				
Lower Floor:			880	
Main Floor:			3,355	
Interior Entry 200%			154	
BBQ Deck:			44	
Upper Floor:			1,655	
Attached Garage:			1,075	
Total Area:			7,162	-423
Proposed % of Lot Area:			38%	

Lower Floor Area exclusion
Seifert Residence
D - 1 - 14 /00 /0004

evised 1/0: Point T	•	I Wall ht.	Exist. grade midpoint ht.	Coverage	Result	Dorcontago
	Length			Coverage		Percentage
A	7.33	8.38	0.00	0.00	0.00	0%
В	6.83	8.38	0.00	0.00	0.00	0%
С	6.42	8.38	1.79	0.21	1.37	1%
D	1.00	8.38	2.29	0.27	0.27	0%
E	10.58	8.38	3.04	0.36	3.84	4%
F	2.00	8.38	4.00	0.48	0.96	1%
G	0.83	8.38	4.13	0.49	0.41	0%
Н	4.66	8.38	4.29	0.51	2.39	2%
ı	12.50	8.38	5.79	0.69	8.64	9%
J	23.19	8.38	6.00	0.72	16.61	17%
K	7.69	8.38	5.13	0.61	4.70	5%
L	19.48	8.38	4.16	0.50	9.68	10%
М	1.35	8.38	4.00	0.48	0.65	1%
N	6.00	8.38	3.79	0.45	2.72	3%
0	1.35	8.38	3.68	0.44	0.59	1%
Р	4.33	8.38	3.54	0.42	0.57	1%
Q	16.83	8.38	2.83	0.34	1.46	1%
R	31.00	8.38	2.29	0.27	8.48	8%
S	18.66	8.38	1.50	0.18	3.34	3%
Т	5.33	8.38	0.00	0.00	0.00	0%
	187.37	1	1	<u> </u>		67%

Total floor area to outside of exterior wall:	1366
Total % / Total length:	0.36
Total area excluded from Gross Floor area:	486.15
Total area remaining that counts toward Gross Floor Area	879.85





GFA UPPER FLOOR

SCALE: 1/8" = 1'-0"

GFA LOWER FLOOR

SEIFERT 4.2024) 1 03/29/2024 OWNER REVISIONS 2 (06. 06/14/2024 PERMIT REVISION #2 R

SET

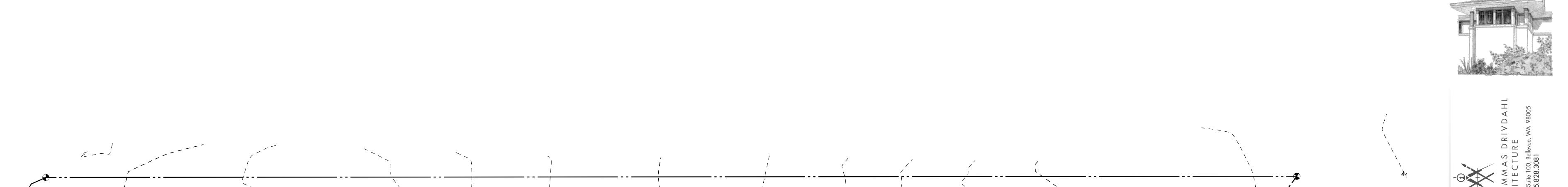
PERMIT

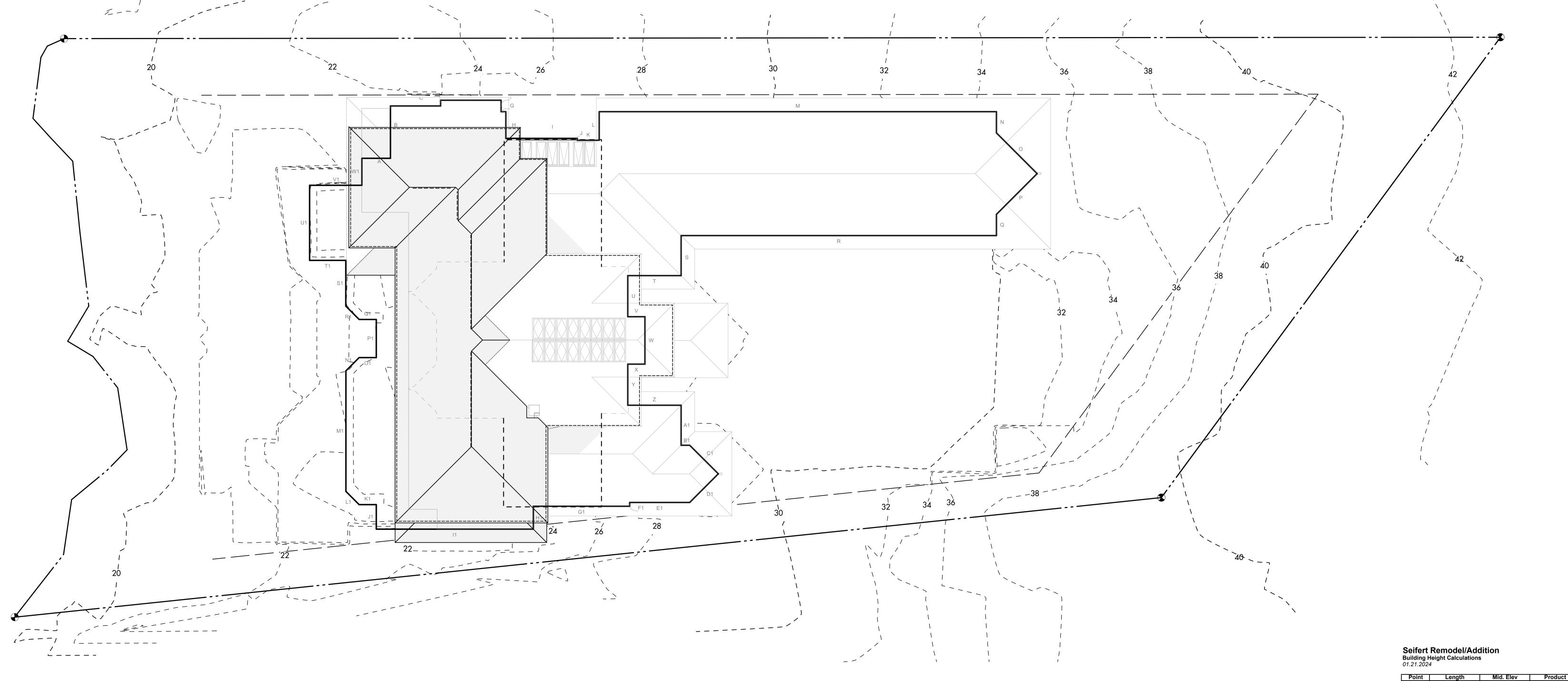
REMODEL

3261 67TH AVE SE MERCER ISLAND, WA

03/29/2024

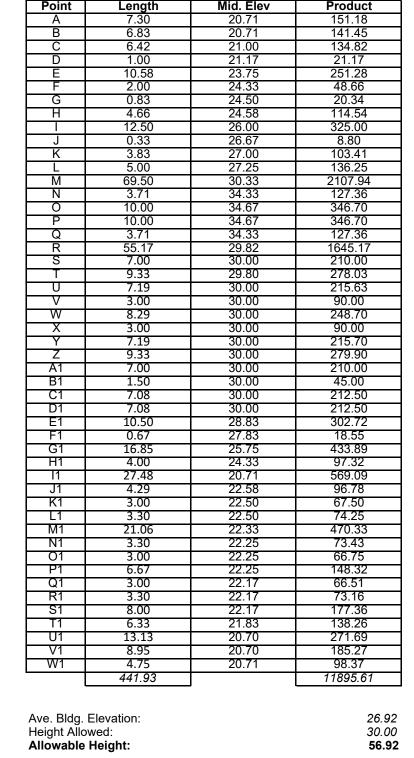
GROSS FLOOR AREA





BUILDING HEIGHTS SITE PLAN

SCALE: 1/8" = 1'-0"



REVS SET **PERMIT**

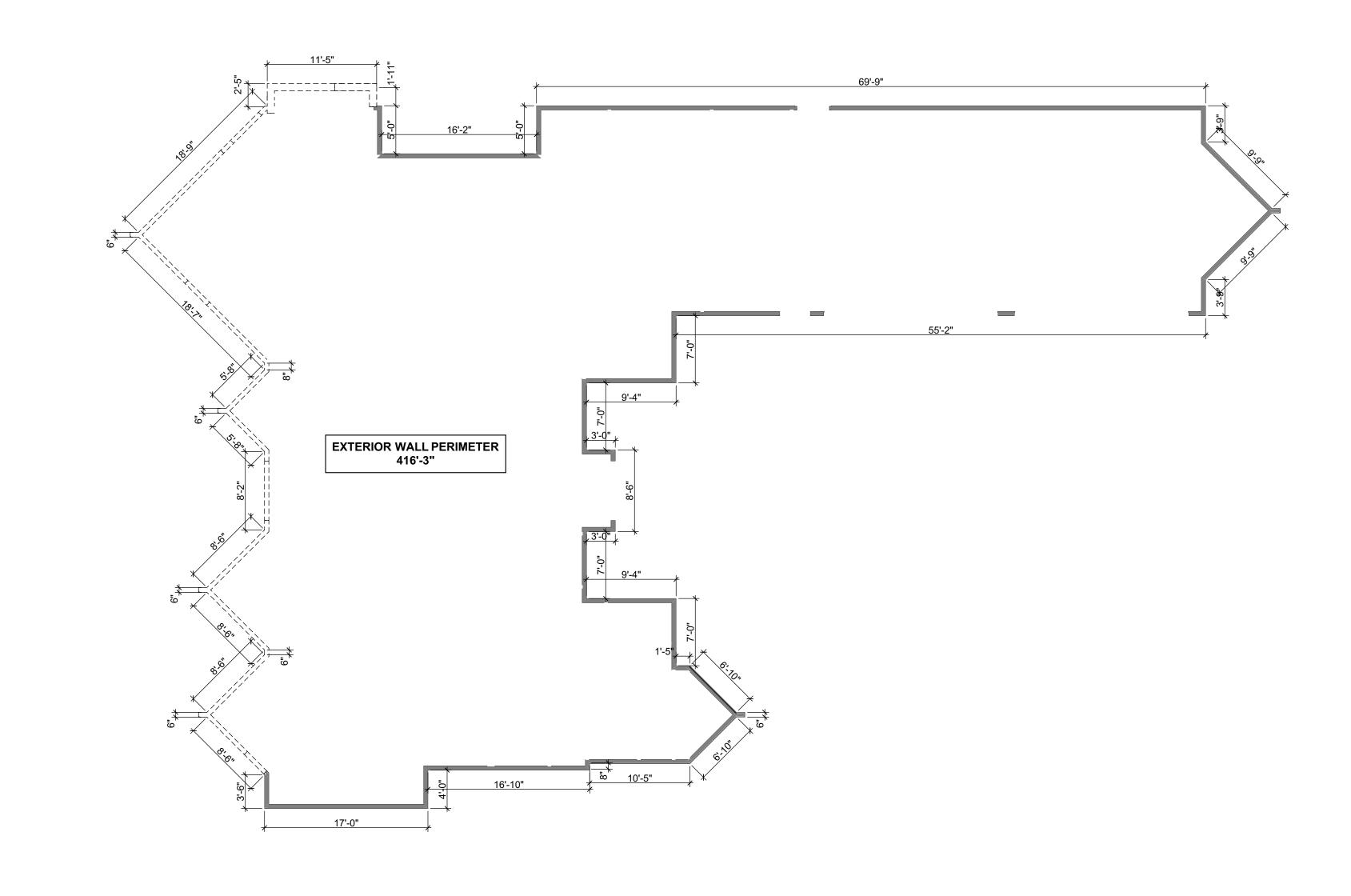
Issue Date:

REGISTERED ARCHITECT ERIC L. DRÍVDAHL STATE OF WASHINGTON BUILDING HEIGHT

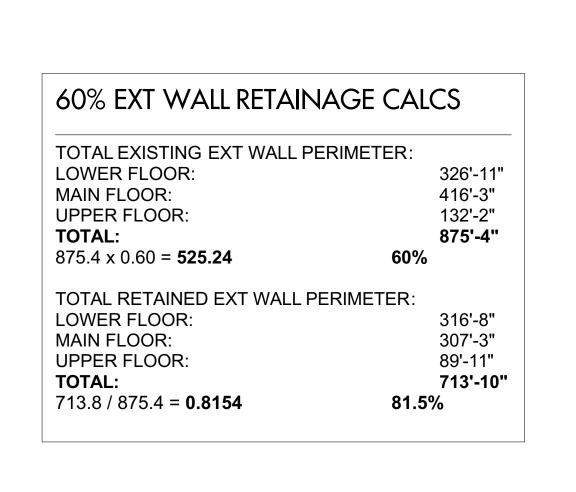
06/14/2024 PERMIT REVISION #2

03/29/2024

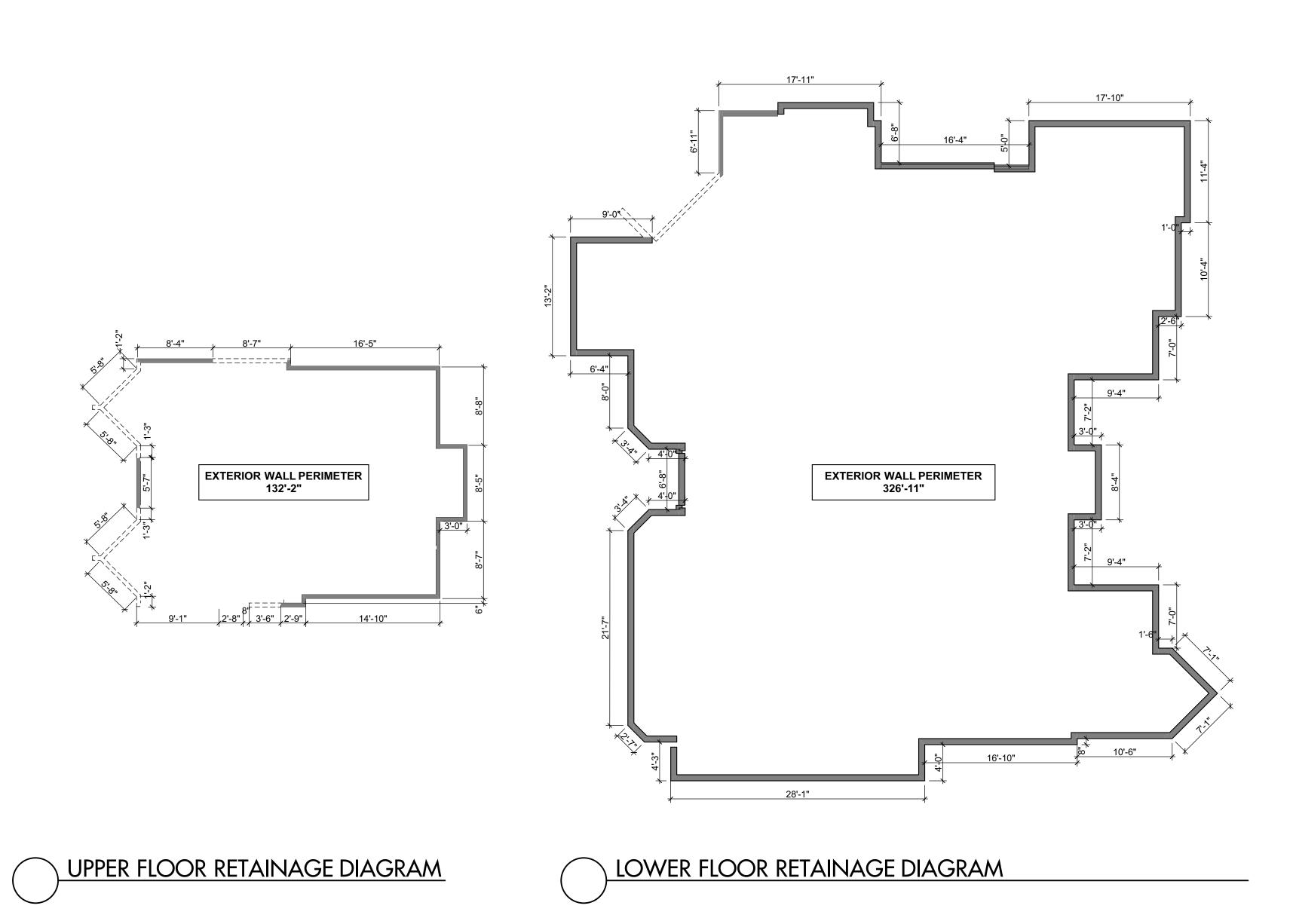
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MAIN FLOOR RETAINAGE DIAGRAM



WALLS TO BE DEMOLISHED WALLS TO BE RETAINED



202 **REVS** SET **PERMIT**

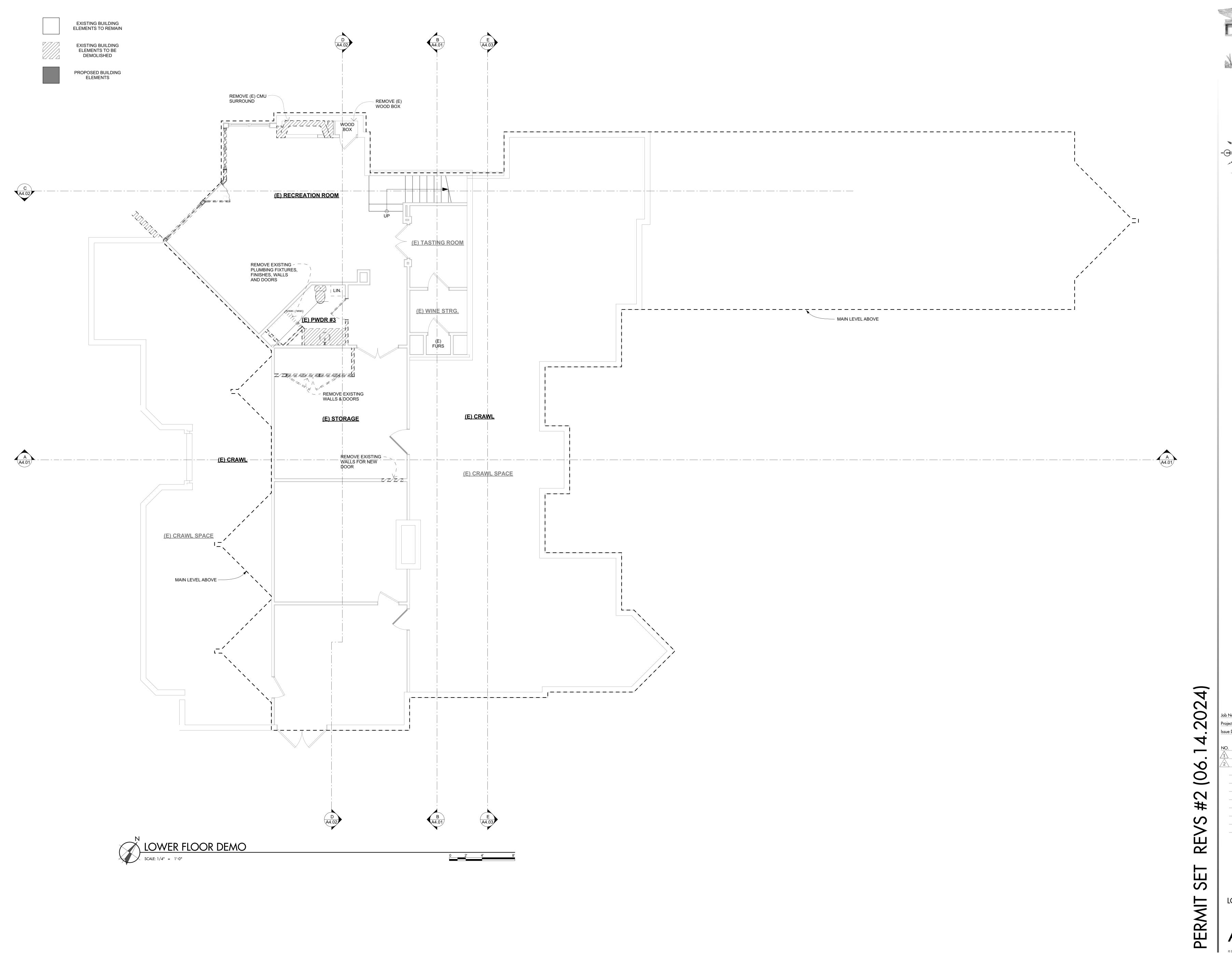
WALL RETAINAGE

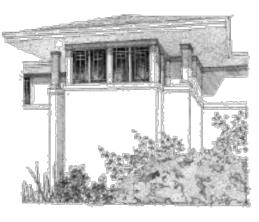
REMODE

03/29/2024

03/29/2024 OWNER REVISIONS

06/14/2024 PERMIT REVISION #2





GELOTTE HOMMAS DRI ARCHITECTURE

FERT REMODEL

 Job No.
 221

 Project Manager:
 D

 Issue Date:
 03/29/202

 NO.
 DATE
 REVISION

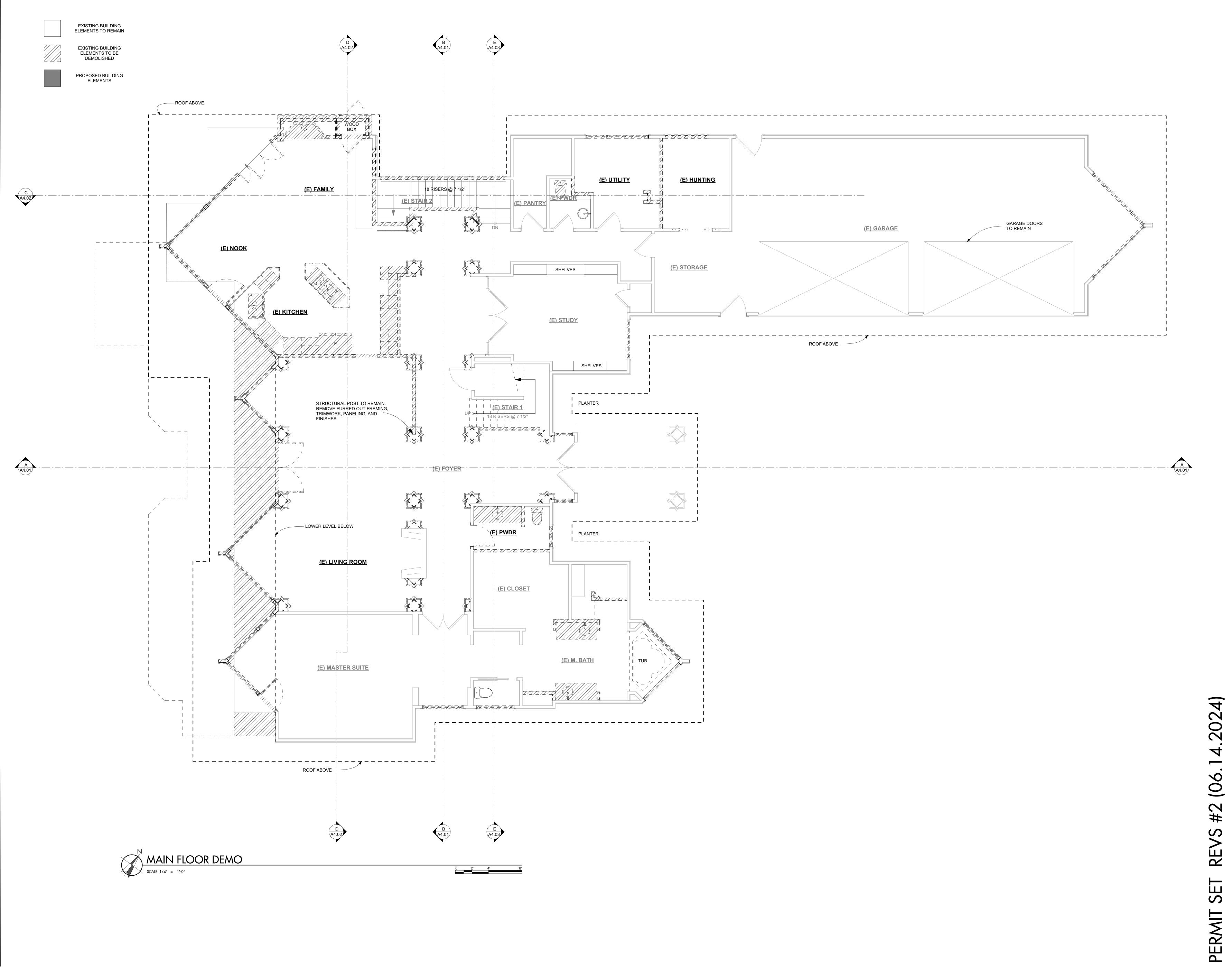
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 03/29/2024
 OWNER REVISIONS

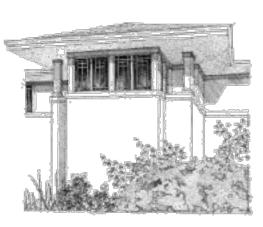
 2
 06/14/2024
 PERMIT REVISION #2

10226 REGISTERED ARCHITECT



\$ | A2.01D







Issue Date: 03/29/2024 OWNER REVISIONS 06/14/2024 PERMIT REVISION #2

03/29/2024

MAIN FLOOR PLAN DEMO

A2.02D

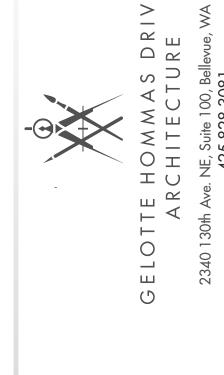


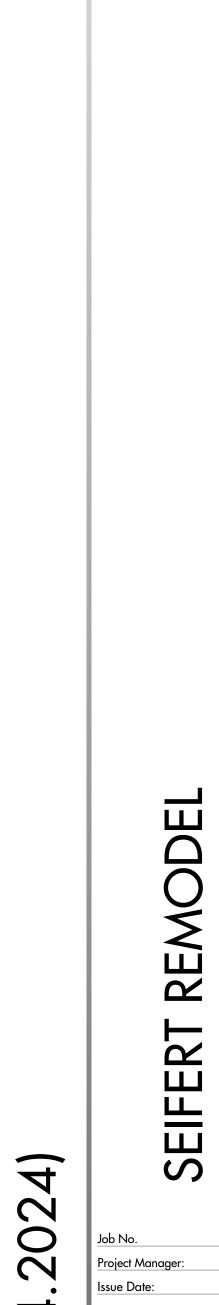


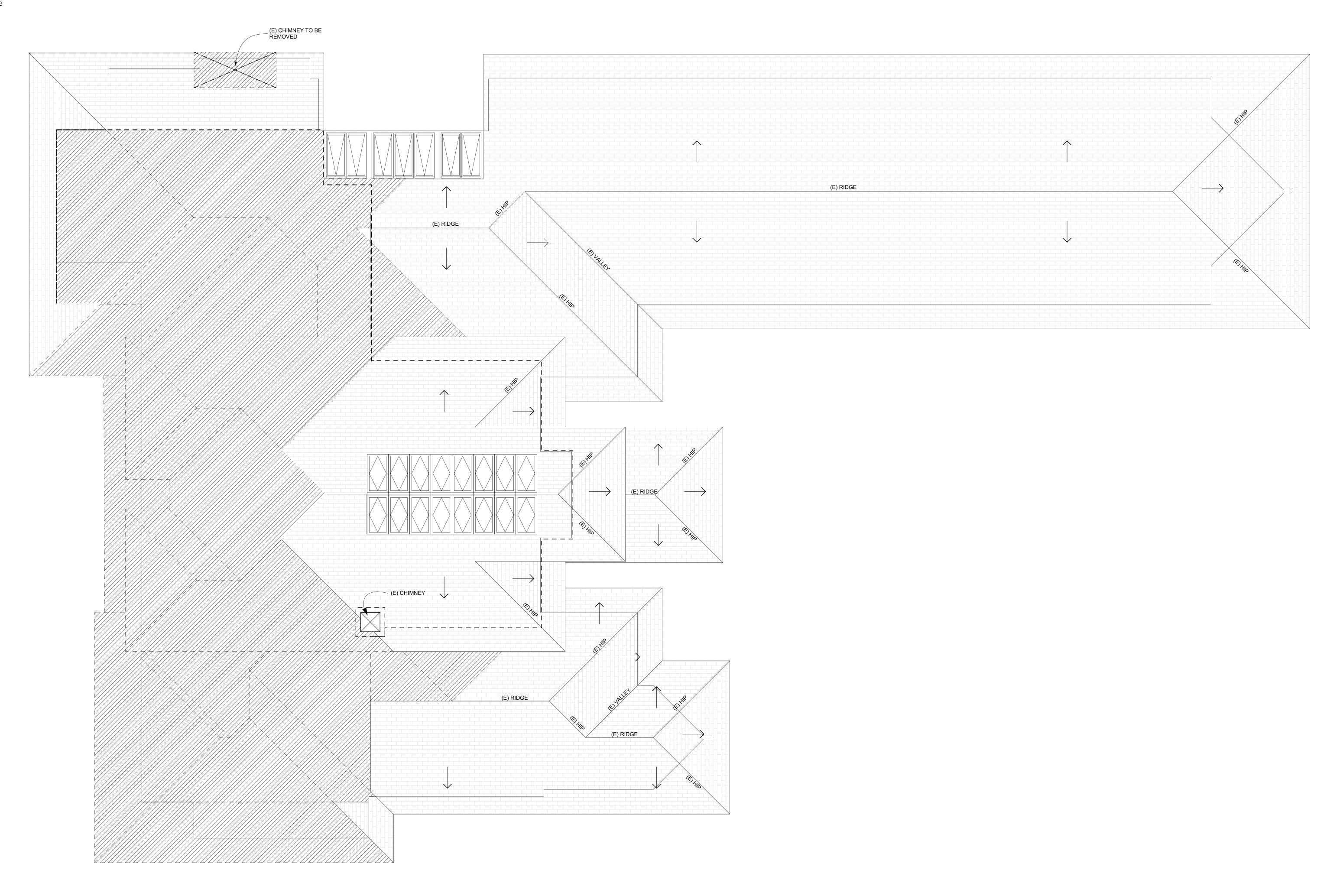
03/29/2024 OWNER REVISIONS 06/14/2024 PERMIT REVISION #2

03/29/2024

UPPER FLOOR PLAN DEMO A2.03D





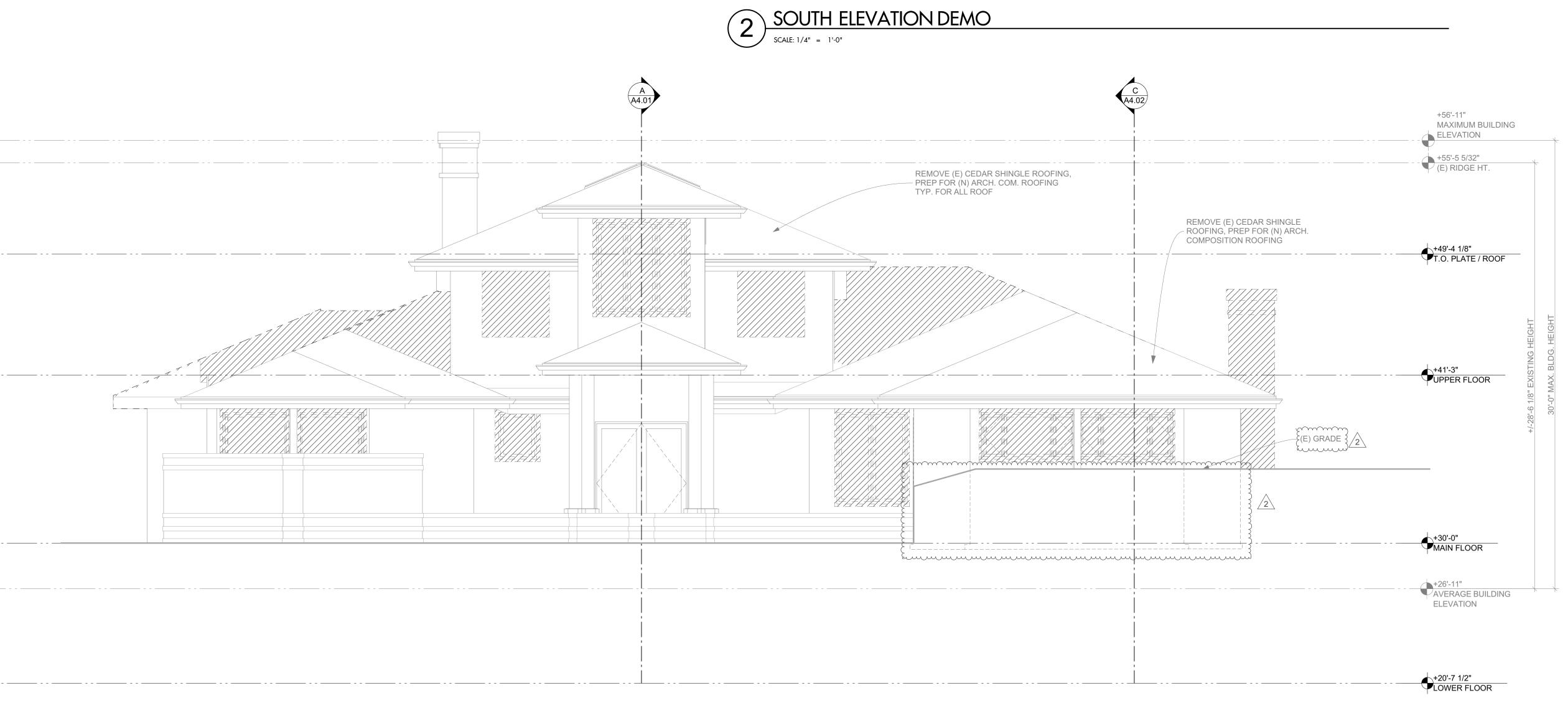




#2 (06. **REVS** SET PERMIT

ROOF PLAN DEMO A2.04D

3261 67TH AVE SE MERCER ISLAND, WA



SCALE: 1/4" = 1'-0"

.2024) ~ # RE SET **PERMIT**

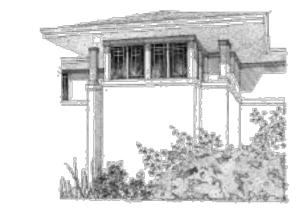
REMODE SEIFERT Issue Date: 03/29/2024 OWNER REVISIONS 06/14/2024 PERMIT REVISION #2

3261 67TH AVE SE MERCER ISLAND, WA

DG

03/29/2024

EXTERIOR ELEVATIONS



SELOTTE HOMMAS DRIVDAH ARCHITECTURE 2340 130th Ave. NE, Suite 100, Bellevue, WA 98005 425.828.3081

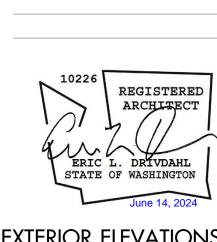
SEIFERT REMODEL
3261 67TH AVE SE
MERCER ISLAND, WA 98040

Project Manager: DG

Issue Date: 03/29/2024

NO. DATE REVISION

1 03/29/2024 OWNER REVISIONS
2 06/14/2024 PERMIT REVISION #2



SET

PERMIT

EXTERIOR ELEVATIONS DEMO

3 WEST ELEVATION DEMO

SCALE: 1/4" = 1'-0"

BLDG AREA - ALL STORIES

(E) FINISHED AREAS

(N) FINISHED AREAS

(E) FINISHED AREAS

(N) FINISHED AREAS

(E) FINISHED AREAS

(N) FINISHED AREAS

(E) GARAGE

272

1,075

7,640 ft²

LOWER FLOOR

MAIN FLOOR

UPPER FLOOR

SE

R

PERMIT

STATE OF WASHINGTON

03/29/2024 OWNER REVISIONS

06/14/2024 PERMIT REVISION #2

SEIFI

03/29/2024

Issue Date:

LOWER FLOOR PLAN PROPOSED

(N) FINISHED AREAS

7,640 ft²



GELOTTE HOMMAS DRIVDA ARCHITECTURE 2340 130th Ave. NE, Suite 100, Bellevue, WA 980

PERMIT SET REVS #2 (06.14.2024)

REGISTERED ARCHITECT

ERIC L. DRIVDAHL STATE OF WASHINGTON

June 14, 2024

MAIN FLOOR PLAN PROPOSED

03/29/2024 OWNER REVISIONS

06/14/2024 PERMIT REVISION #2

SEI

03/29/2024

Issue Date:

A2.02

MAIN FLOOR

UPPER FLOOR

(E) FINISHED AREAS

(N) FINISHED AREAS

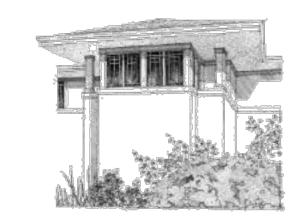
(E) FINISHED AREAS

(N) FINISHED AREAS

(E) GARAGE

1,075

7,640 ft²



SEIFI Issue Date: 03/29/2024 OWNER REVISIONS

R

SE

PERMIT



06/14/2024 PERMIT REVISION #2

03/29/2024

UPPER FLOOR PLAN PROPOSED





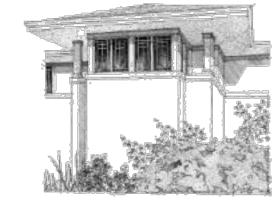
RE/ SEIFEI Issue Date: 03/29/2024 OWNER REVISIONS 06/14/2024 PERMIT REVISION #2 **REVS**

SET

PERMIT

03/29/2024

ROOF PLAN PROPOSED



GELOTTE HOMMAS DRIVDAHI ARCHITECTURE 2340 130th Ave. NE, Suite 100, Bellevue, WA 98005

SEIFERT REMODEL
3261 67TH AVE SE
MERCER ISLAND, WA 98040

 Job No.
 2219

 Project Manager:
 DG

 Issue Date:
 03/29/2024

 NO.
 DATE
 REVISION

 1
 03/29/2024
 OWNER REVISIONS

 2
 06/14/2024
 PERMIT REVISION #2

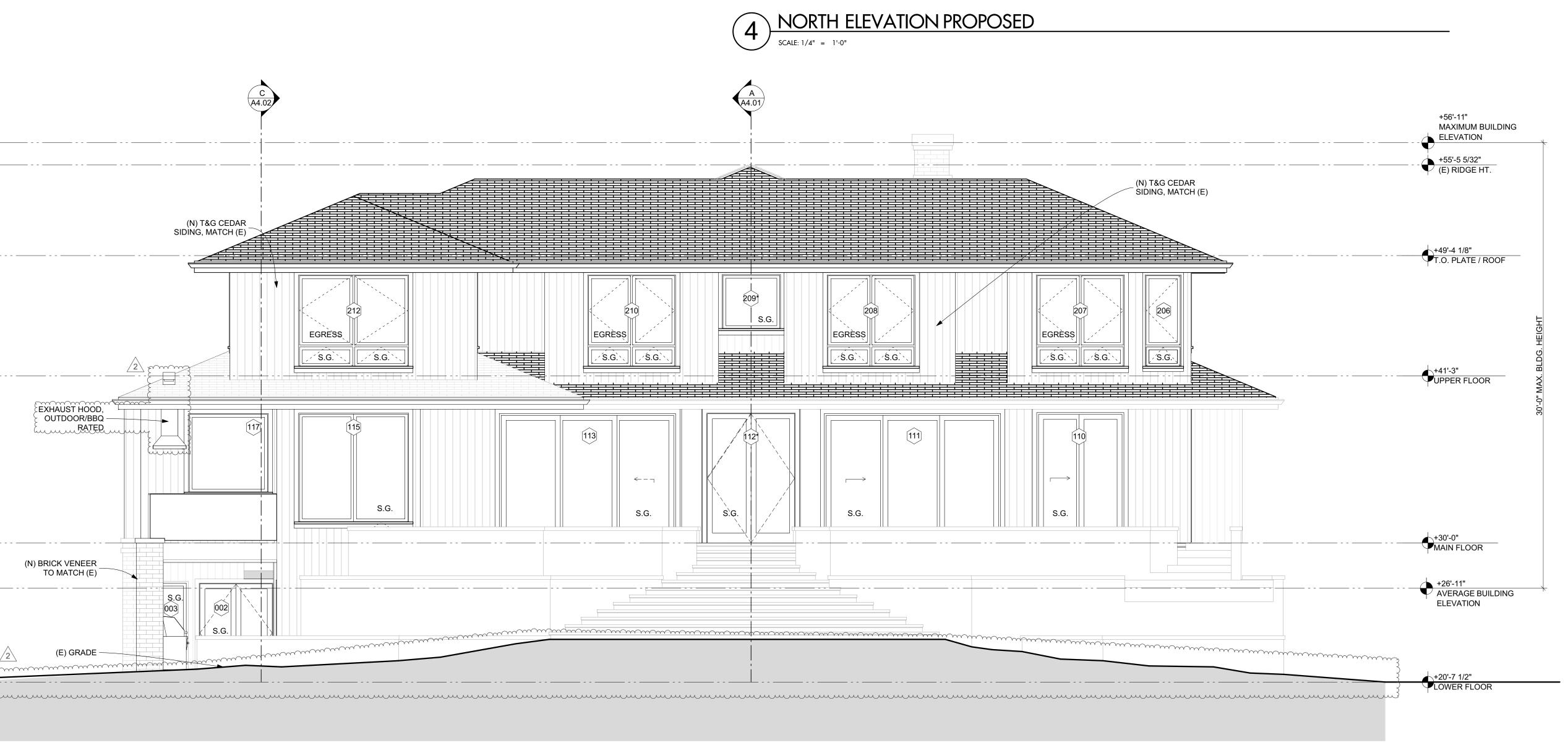
REGISTERED ARCHITECT

ERIC L. DRIVDAHL STATE OF WASHINGTON

June 14, 2024

EXTERIOR ELEVATIONS PROPOSED

A3.01



202 ~ # R SET PERMIT

REA SEIFERT Issue Date: ○ 03/29/2024 OWNER REVISIONS

3261 67TH AVE SE MERCER ISLAND, WA

DG

03/29/2024

06/14/2024 PERMIT REVISION #2

EXTERIOR ELEVATIONS PROPOSED

3 WEST ELEVATION PROPOSED

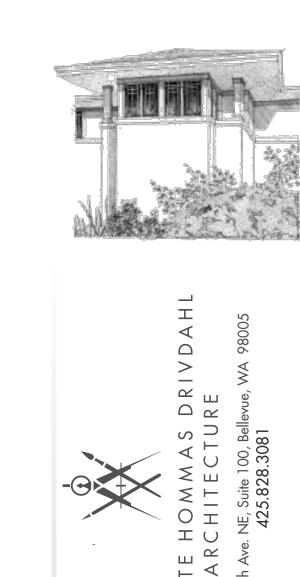
SCALE: 1/4" = 1'-0"

(E) RAILING

PÓSITION

(N) BONUS

(E) HALL



FLOOR SPACE) * (N) FLOORING PER I.D. * (E) SUBFLOOR * (E) FLOOR JOIST * (N) SOUND INSULATION AS NEEDED * (N) 5/8" G.W.B. + PVA PRIMER FLOOR ASSEMBLY #2 (LOCATED AT NEW UPPER FLOOR SPACE) MAXIMUM BUILDING * (N) FLOORING PER I.D. * (N) SUBFLOOR * (N) FLOOR JOIST * (N) SOUND INSULATION AS NEEDED * (N) 5/8" G.W.B. + PVA PRIMER FLOOR ASSEMBLY #3 (LOCATED AT LOWER FLOOR AT NEW CONDITIONED SPACES) * (N) FLOORING PER I.D. * (E) & (N) CONCRETE SLAB +49'-4 1/8" 4 T.O. PLATE / ROOF * (N) R-10 RIDGE INSULATION AT NEW LOCATIONS, WHERE POSSIBLE. * (N) VAPOR BARRIER 10 MIL. * (N) 5/8" G.W.B. + PVA PRIMER ROOF ASSEMBLY #1 (LOCATED AT EXISTING ROOFS) * (E) CEDAR SHAKE SHINGLES * (E) ROOF UNDERLAYMENT * (E) ROOF SHEATHING * (E) TRUSSES OR RAFTERS * (E) INSULATION (IF CEILING EXPOSED, NÈW INSULATION TO COMPLY WITH CURRENT WSEC) * (E) G.W.B. (NEW G.W.B. IF CEILING EXPOSED. MATCH EXISTING THICKNESS.) ROOF ASSEMBLY #2 (LOCATED AT NEW UPPER ROOFS) * (N) CEDAR SHAKE SHINGLES * (N) ROOF UNDERLAYMENT
* (N) ROOF SHEATHING, PER STRUCTURAL
* (N) TRUSSES OR RAFTERS, PER STRUCTURAL * (N) BATT INSULATION PER CURRENT * (N) 5/8" G.W.B. + PVA PRIMER <u>WALLS</u> WALL ASSEMBLY #1 (LOCATED AT EXISTING WALLS) * (E) VERTICAL CEDAR T&G SIDING * (E) BUILDING PAPER * (E) WALL SHEATHING * (E) 2X4 STUD WALL @ 16" O.C. * (E) INSULATION (IF WALL EXPOSED, NEW INSULATION TO COMPLY WITH CURRENT WSEC) * (E) G.W.B. (NEW G.W.B. IF WALL EXPOSED. MATCH EXISTING THICKNESS.) WALL ASSEMBLY #2 (LOCATED AT NEW TYPICAL WALLS) * (N) VERTICAL CEDAR T&G SIDING, MATCH EXISTING * (E) BUILDING PAPER * (E) WALL SHEATHING * (E) 2X4 STUD WALL @ 16" O.C. * (E) INSULATION (IF WALL EXPOSED, NÈW INSULATION TO COMPLY WITH CURRENT WSEC) * (E) G.W.B. (NEW G.W.B. IF WALL EXPOSED. MATCH EXISTING THICKNESS.)

FLOOR ASSEMBLY #1

+56'-11"

ELEVATION

+55'-5 5/32" (E) RIDGE HT.

/-----

(LOCATED AT EXISTING MAIN AND UPPER

REA 3261 67TH AVE SE MERCER ISLAND, WA SEIFERT

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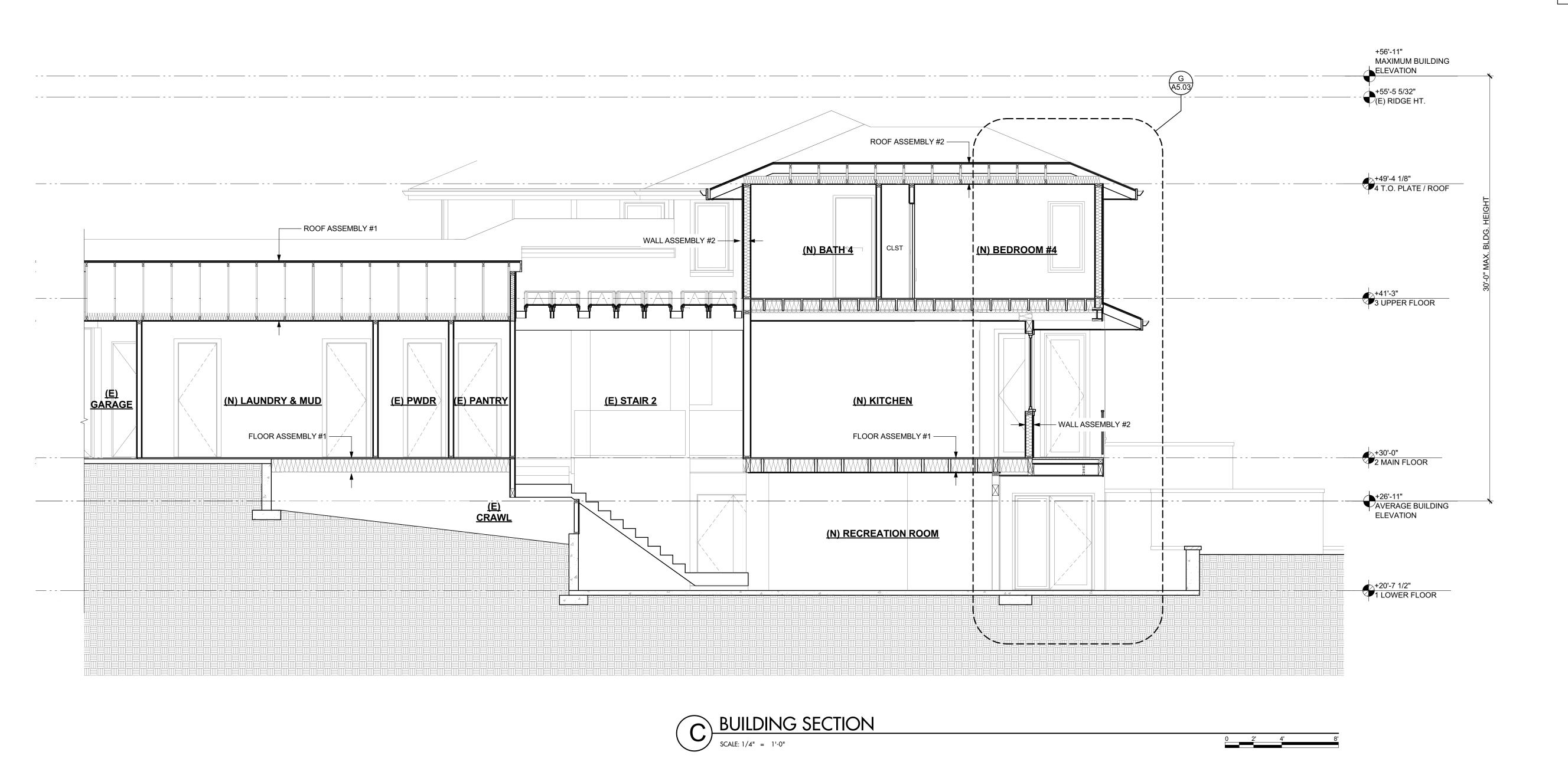
REVS

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PERMIT

Issue Date: 03/29/2024 03/29/2024 OWNER REVISIONS 06/14/2024 PERMIT REVISION #2

BUILDING SECTIONS



FLOOR ASSEMBLY #1 (LOCATED AT EXISTING MAIN AND UPPER FLOOR SPACE)

* (N) FLOORING PER I.D.

* (E) SUBFLOOR * (E) FLOOR JOIST * (N) SOUND INSULATION AS NEEDED * (N) 5/8" G.W.B. + PVA PRIMER FLOOR ASSEMBLY #2

(LOCATED AT NEW UPPER FLOOR SPACE) * (N) FLOORING PER I.D. * (N) SUBFLOOR * (N) FLOOR JOIST * (N) SOUND INSULATION AS NEEDED * (N) 5/8" G.W.B. + PVA PRIMER

FLOOR ASSEMBLY #3 (LOCATED AT LOWER FLOOR AT NEW CONDITIONED SPACES) * (N) FLOORING PER I.D. * (E) & (N) CONCRETE SLAB

* (N) R-10 RIDGE INSULATION AT NEW LOCATIONS, WHERE POSSIBLE. * (N) VAPOR BARRIER 10 MIL. * (N) 5/8" G.W.B. + PVA PRIMER

ROOF ASSEMBLY #1 (LOCATED AT EXISTING ROOFS)

* (E) CEDAR SHAKE SHINGLES * (E) ROOF UNDERLAYMENT * (E) ROOF SHEATHING * (E) TRUSSES OR RAFTERS * (E) INSULATION (IF CEILING EXPOSED, NÈW INSULATION TO COMPLY WITH CURRENT WSEC) * (E) G.W.B. (NEW G.W.B. IF CEILING EXPOSED. MATCH EXISTING THICKNESS.)

ROOF ASSEMBLY #2 (LOCATED AT NEW UPPER ROOFS)

* (N) CEDAR SHAKE SHINGLES * (N) ROOF UNDERLAYMENT * (N) ROOF SHEATHING, PER STRUCTURAL * (N) TRUSSES OR RAFTERS, PER STRUCTURAL * (N) BATT INSULATION PER CURRENT * (N) 5/8" G.W.B. + PVA PRIMER

<u>WALLS</u>

WALL ASSEMBLY #1 (LOCATED AT EXISTING WALLS)

* (E) VERTICAL CEDAR T&G SIDING * (E) BUILDING PAPER * (E) WALL SHEATHING * (E) 2X4 STUD WALL @ 16" O.C. * (E) INSULATION (IF WALL EXPOSED, NEW INSULATION TO COMPLY WITH CURRENT WSEC)

* (E) G.W.B. (NEW G.W.B. IF WALL
EXPOSED. MATCH EXISTING THICKNESS.)

WALL ASSEMBLY #2 (LOCATED AT NEW TYPICAL WALLS)

* (N) VERTICAL CEDAR T&G SIDING, MATCH EXISTING * (E) BUILDING PAPER * (E) WALL SHEATHING * (E) 2X4 STUD WALL @ 16" O.C. * (E) INSULATION (IF WALL EXPOSED, NÈW INSULATION TO COMPLY WITH CURRENT WSEC) * (E) G.W.B. (NEW G.W.B. IF WALL EXPOSED. MATCH EXISTING THICKNESS.)

3261 67TH AVE SE MERCER ISLAND, WA REA SEIFERT

Issue Date: 03/29/2024 03/29/2024 OWNER REVISIONS 06/14/2024 PERMIT REVISION #2

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BUILDING SECTIONS

FLOOR ASSEMBLY #1 (LOCATED AT EXISTING MAIN AND UPPER FLOOR SPACE)

* (N) FLOORING PER I.D. * (E) SUBFLOOR * (E) FLOOR JOIST * (N) SOUND INSULATION AS NEEDED

* (N) 5/8" G.W.B. + PVA PRIMER FLOOR ASSEMBLY #2

(LOCATED AT NEW UPPER FLOOR SPACE) * (N) FLOORING PER I.D. * (N) SUBFLOOR * (N) FLOOR JOIST * (N) SOUND INSULATION AS NEEDED

* (N) 5/8" G.W.B. + PVA PRIMER FLOOR ASSEMBLY #3 (LOCATED AT LOWER FLOOR AT NEW CONDITIONED SPACES)

ш

* (N) FLOORING PER I.D. * (E) & (N) CONCRETE SLAB * (N) R-10 RIDGE INSULATION AT NEW LOCATIONS, WHERE POSSIBLE. * (N) VAPOR BARRIER 10 MIL. * (N) 5/8" G.W.B. + PVA PRIMER

ROOF ASSEMBLY #1 (LOCATED AT EXISTING ROOFS)

* (E) CEDAR SHAKE SHINGLES * (E) ROOF UNDERLAYMENT * (E) ROOF SHEATHING * (E) TRUSSES OR RAFTERS * (E) INSULATION (IF CEILING EXPOSED, NEW INSULATION TO COMPLY WITH CURRENT WSEC) * (E) G.W.B. (NEW G.W.B. IF CEILING EXPOSED. MATCH EXISTING THICKNESS.)

ROOF ASSEMBLY #2 (LOCATED AT NEW UPPER ROOFS)

* (N) CEDAR SHAKE SHINGLES * (N) ROOF UNDERLAYMENT * (N) ROOF SHEATHING, PER STRUCTURAL * (N) TRUSSES OR RAFTERS, PER STRUCTURAL * (N) BATT INSULATION PER CURRENT * (N) 5/8" G.W.B. + PVA PRIMER

WALL ASSEMBLY #1

(LOCATED AT EXISTING WALLS) * (E) VERTICAL CEDAR T&G SIDING * (E) BUILDING PAPER * (E) WALL SHEATHING * (E) 2X4 STUD WALL @ 16" O.C. * (E) INSULATION (IF WALL EXPOSED, NEW INSULATION TO COMPLY WITH CURRENT WSEC)

* (E) G.W.B. (NEW G.W.B. IF WALL
EXPOSED. MATCH EXISTING THICKNESS.)

WALL ASSEMBLY #2

(LOCATED AT NEW TYPICAL WALLS) * (N) VERTICAL CEDAR T&G SIDING, MATCH EXISTING * (E) BUILDING PAPER * (E) WALL SHEATHING * (E) 2X4 STUD WALL @ 16" O.C.

* (E) INSULATION (IF WALL EXPOSED,
NEW INSULATION TO COMPLY WITH CURRENT WSEC) * (E) G.W.B. (NEW G.W.B. IF WALL EXPOSED. MATCH EXISTING THICKNESS.)

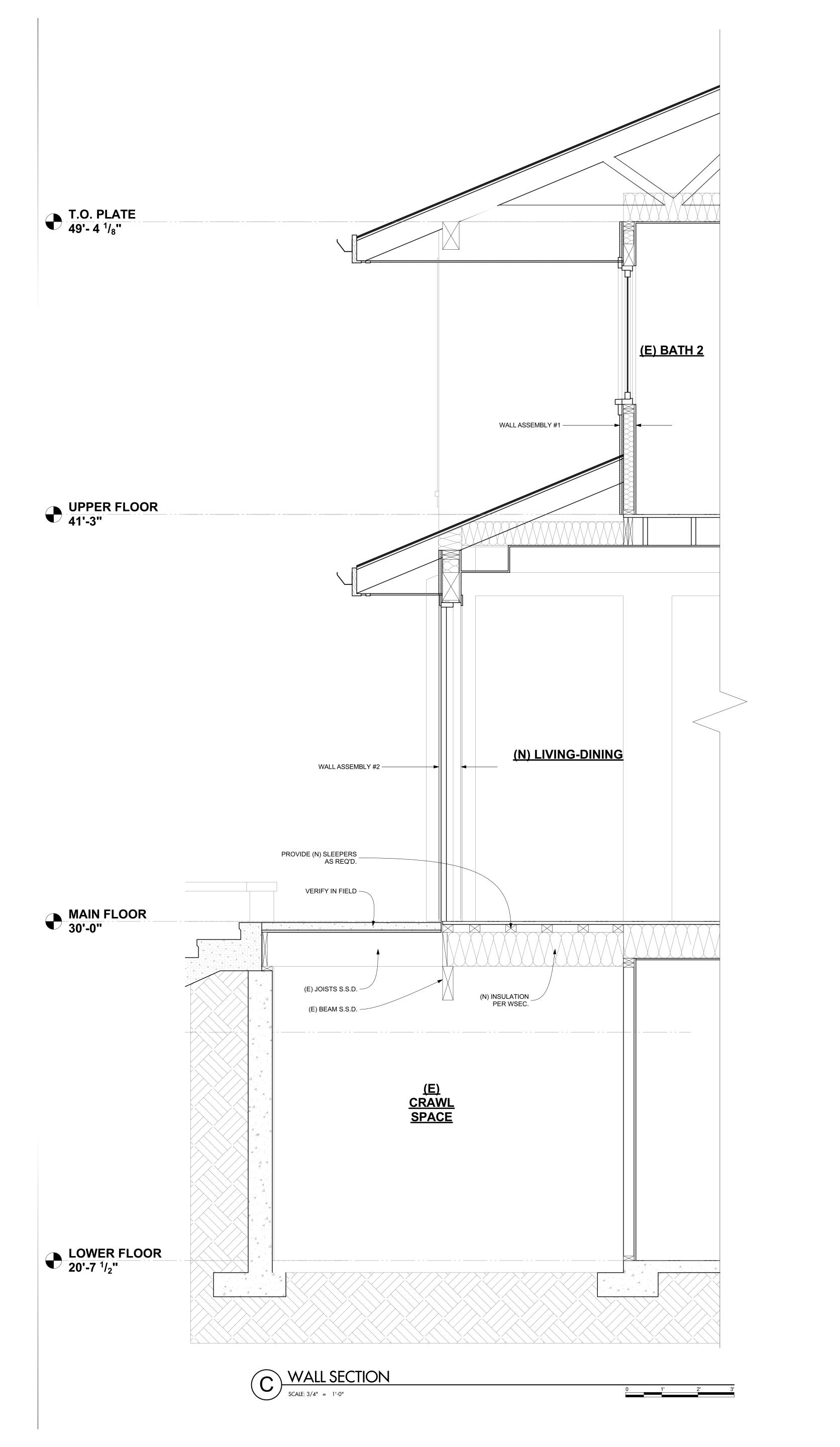
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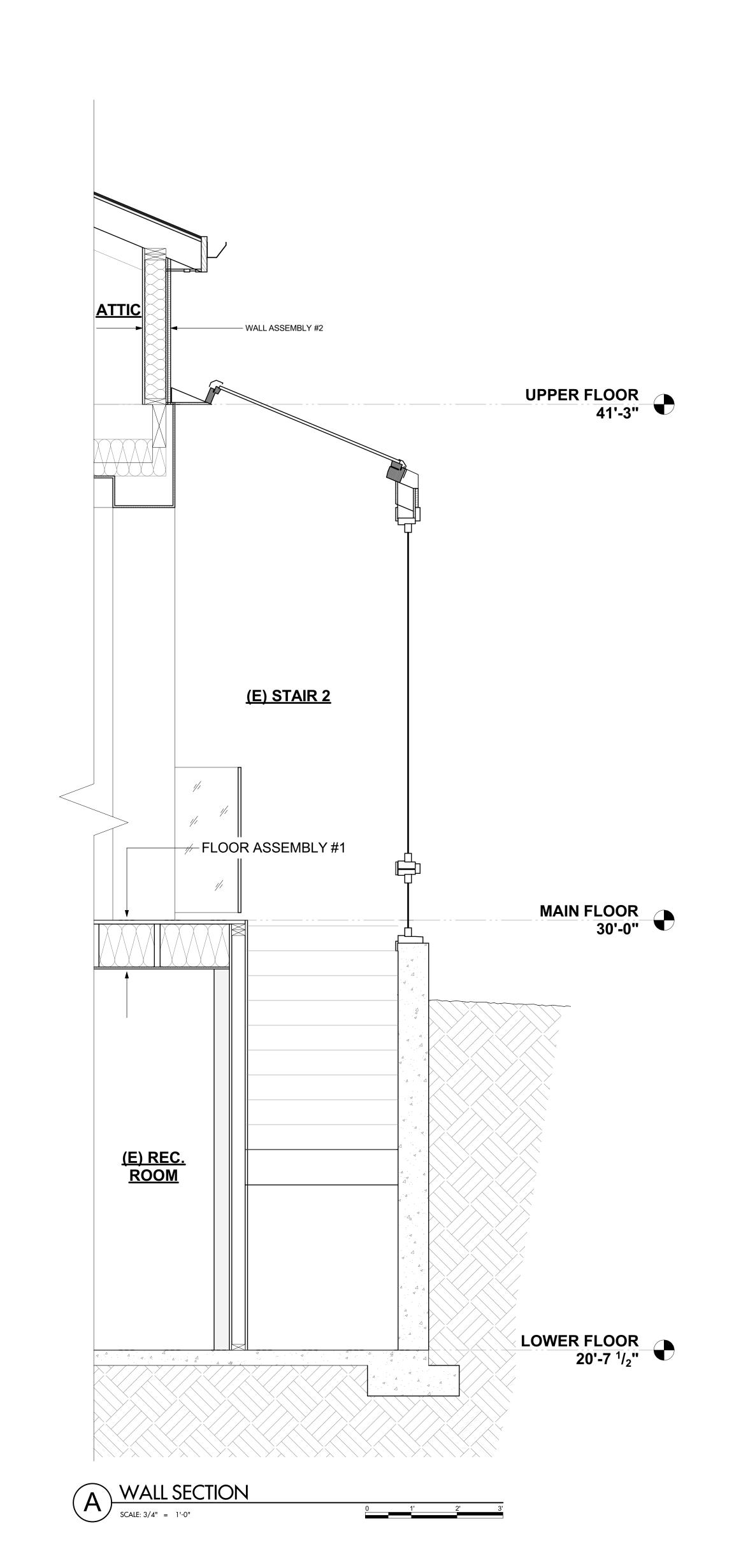
03/29/2024 03/29/2024 OWNER REVISIONS 06/14/2024 PERMIT REVISION #2

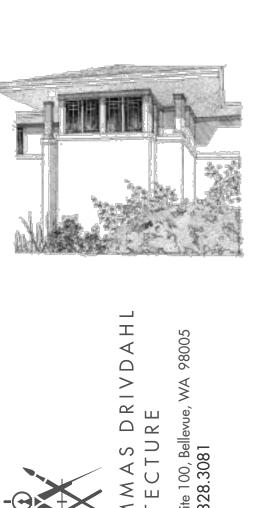


BUILDING SECTIONS

2 **REVS** SET **PERMIT**







PERMIT SET REVS #2 (06.14.2024)

NO. DATE REVISION

03/29/2024 OWNER REVISIONS

06/14/2024 PERMIT REVISION #2

10226

REGISTERED ARCHITECT

ERIC L. DRIVDAHL STATE OF WASHINGTON

June 14, 2024

REMODE

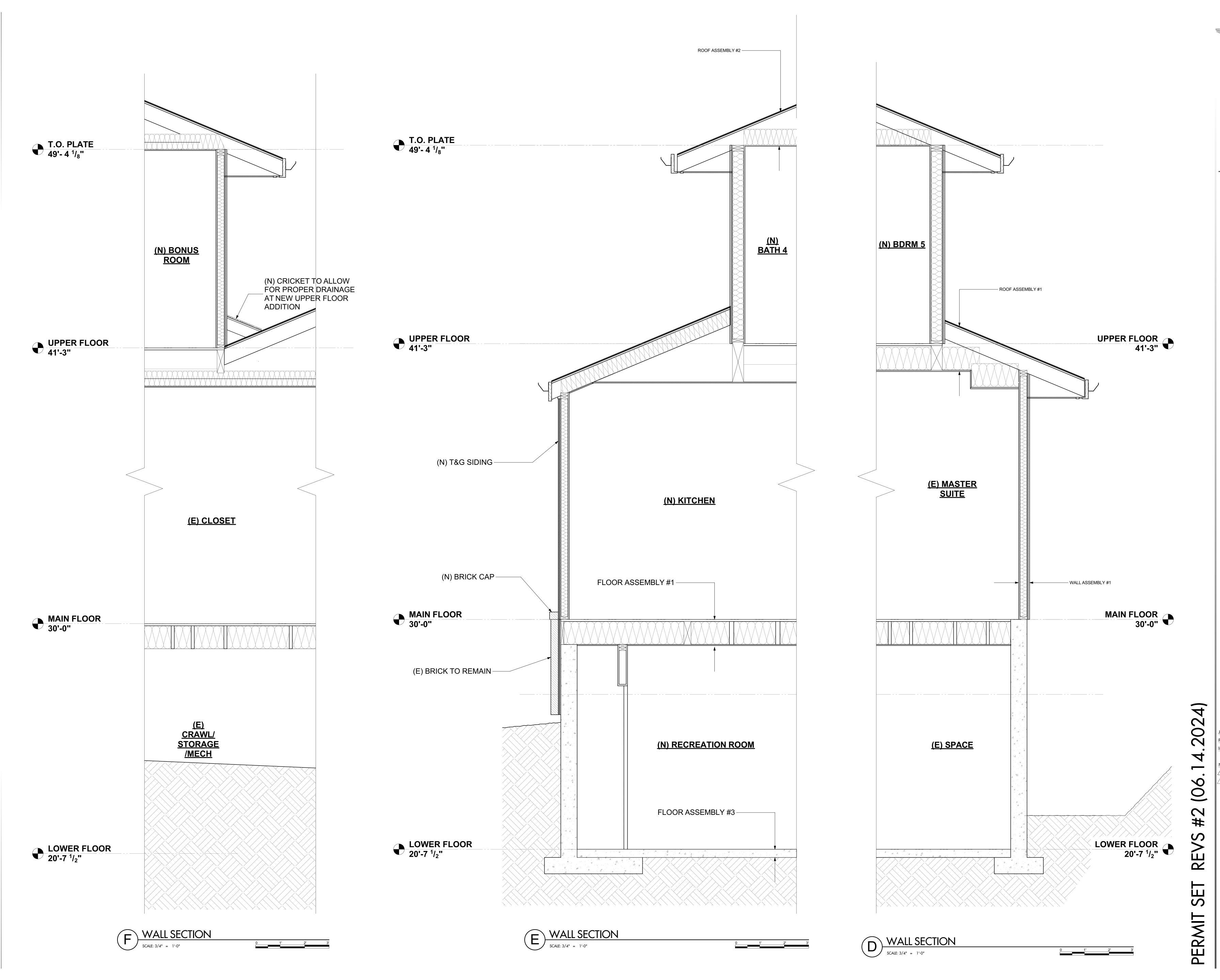
SEIFERT

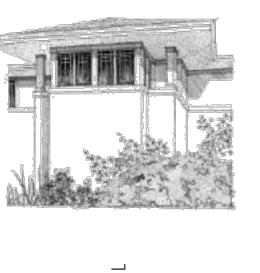
3261 67TH AVE SE MERCER ISLAND, WA

03/29/2024

WALL SECTIONS

A5.01





ARCHITECTURE
ARCHITECTURE
340 130th Ave. NE, Suite 100, Bellevue, WA 98005
425.828.3081

REMODEL

SEIFERT

\ 03/29/2024 OWNER REVISIONS

06/14/2024 PERMIT REVISION #2

ERIC L. DRIVDAHL STATE OF WASHINGTON

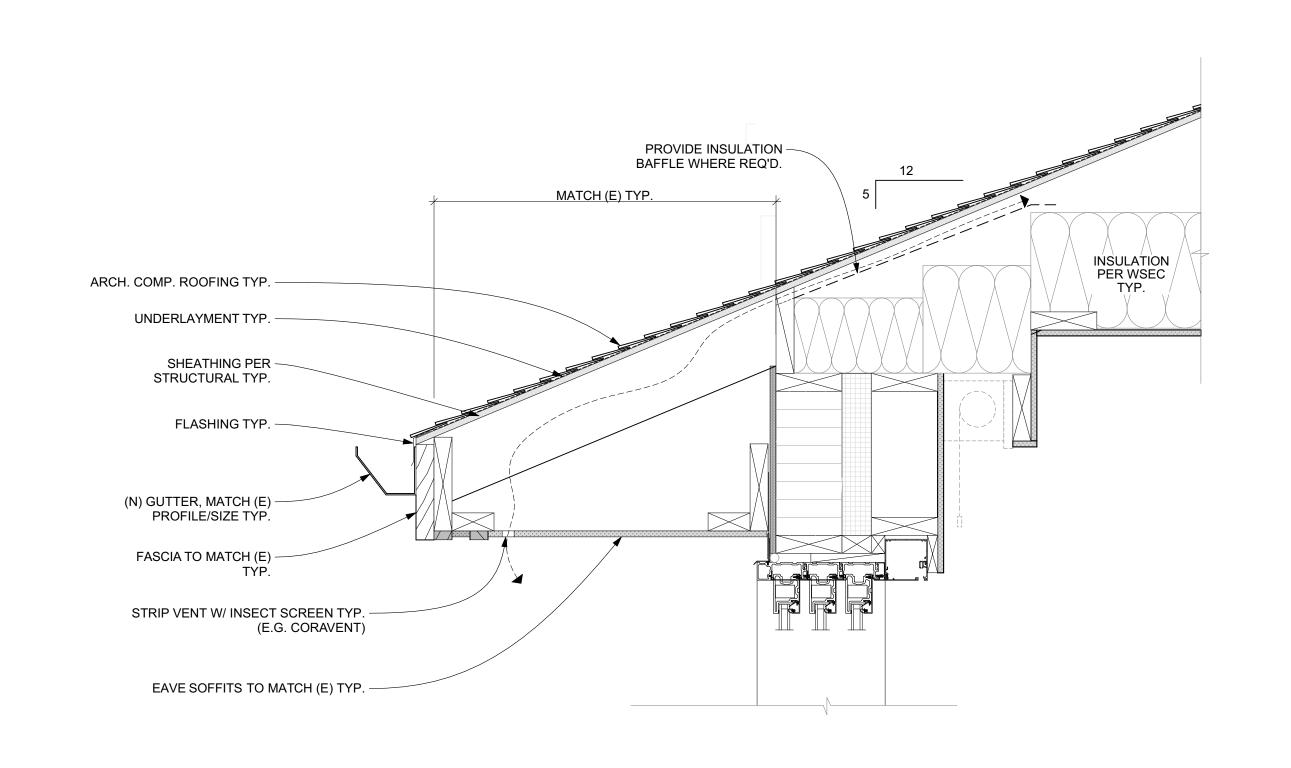
WALL SECTIONS

A5.02

3261 67TH AVE SE MERCER ISLAND, WA

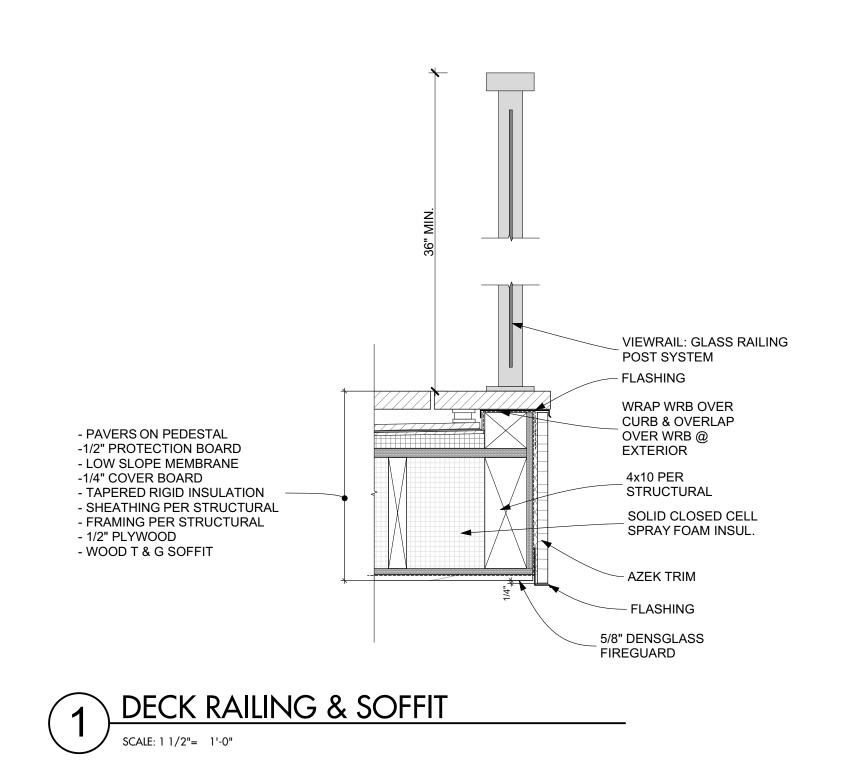
DG

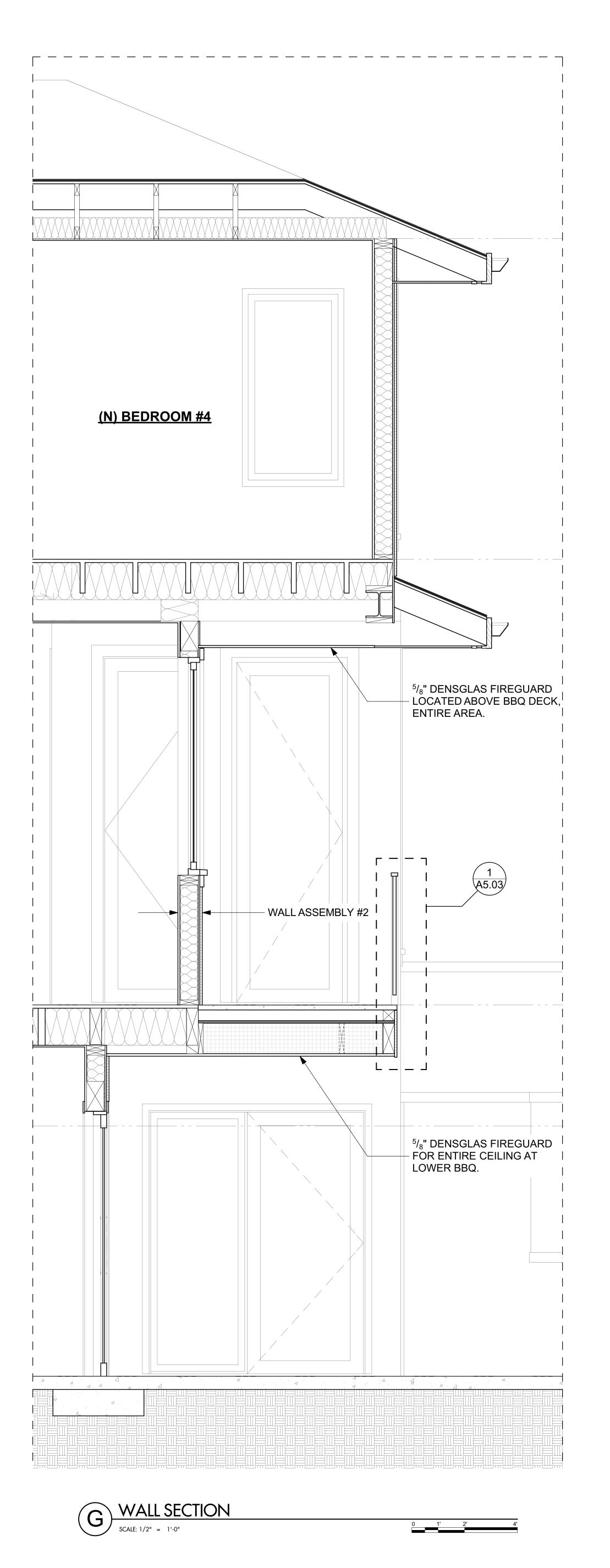
03/29/2024



2 TYP. EAVE DETAIL

SCALE: 1 1/2"= 1'-0"









REMODE SEIFERT Issue Date: 03/29/2024 OWNER REVISIONS 06/14/2024 PERMIT REVISION #2

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202

REV

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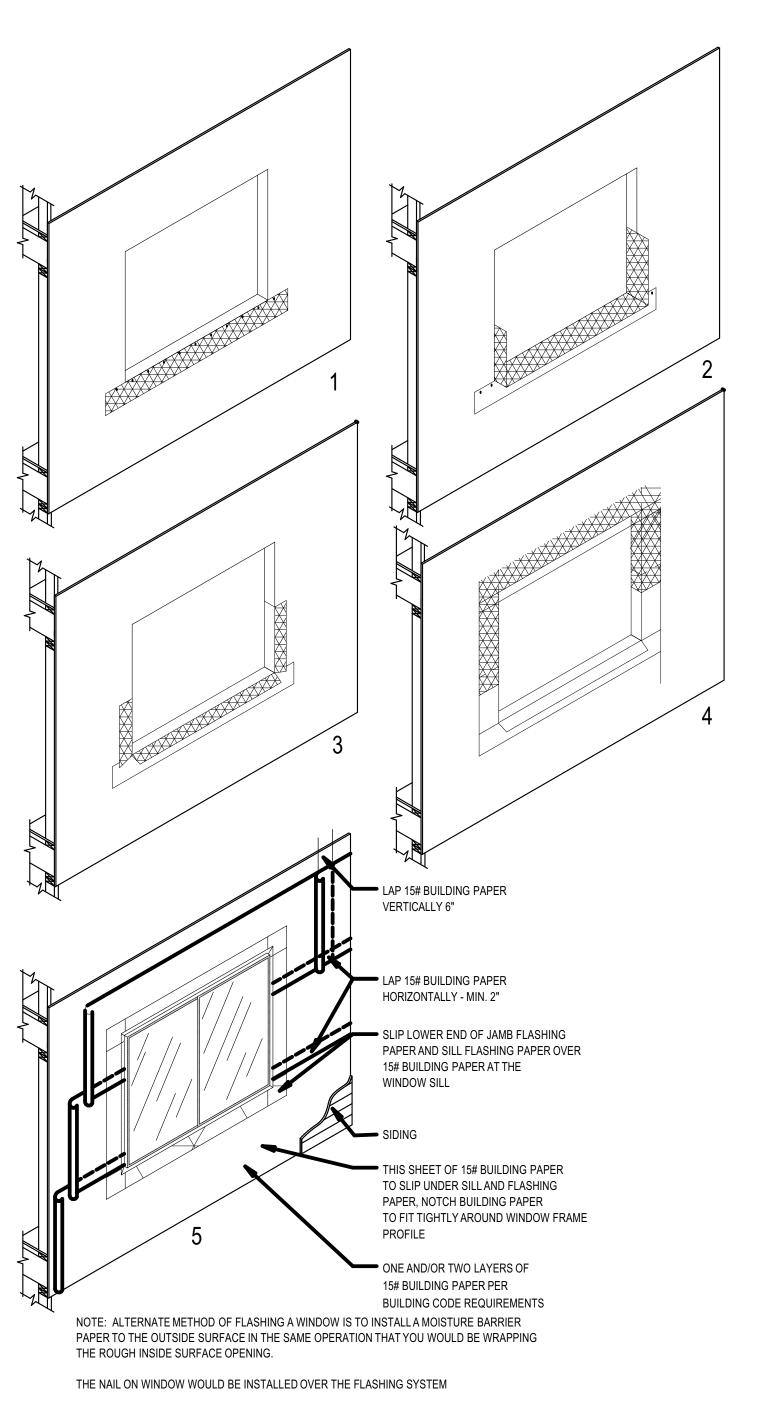
PERMIT

3261 67TH AVE SE MERCER ISLAND, WA

DG

03/29/2024

A5.03

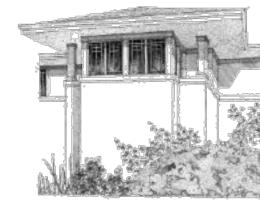


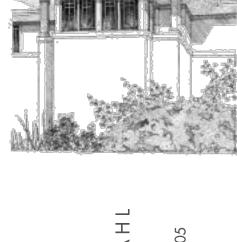


LOCATION	NO.	R.O. WIDTH	R.O. HEIGHT	HEAD HEIGHT	TYPE	EGRESS	SAFETY GLASS	NOTES
LOWER FLOOR								
LOWERTEOOR	003	2'-6"	6'-0 3/8"	6'-9 1/2"	W2		YES	
	004	5'-0 3/4"	6'-0 3/8"	6'-9 1/2"	W3		YES	
MAIN FLOOR								
	101*	2'-2 3/4"	6'-7 3/8"	8'-9 1/4"	W2		YES	
	103*	2'-2 3/4"	6'-7 3/8"	8'-9 1/4"	W2		YES	
	104*	2'-6 3/4"	3'-0 3/8"	8'-9 1/4"	W2			
	105*	6'-0 3/4"	7'-0 3/8"	8'-9 1/4"	W2		YES	
	106*	6'-0 3/4"	7'-0 3/8"	8'-9 1/4"	W2		YES	
	107*	4'-6"	2'-0"	8'-9 1/4"	W2			
	108*	5'-0"	6'-0"	8'-9 1/4"	W2		YES	
	115	7'-6"	7'-6"	8'-9 1/4"	W2		YES	
	117	5'-6"	5'-6"	8'-9 1/4"	W2			
	118	3'-3 3/4"	5'-6"	8'-9 1/4"	W2			
	125*	7'-6 3/4"	4'-0 3/8"	8'-9 1/4"	W8			
	126	5'-0 3/4"	4'-0 3/8"	8'-9 1/4"	W7			
	128*	8'-6 3/4"	3'-0 3/8"	8'-9 1/4"	W8			
	129*	8'-6 3/4"	3'-0 3/8"	8'-9 1/4"	W8			
	133*	4'-6 3/4"	6'-0 3/8"	8'-9 1/4"	W7			
	134*	4'-0 1/2"	7'-9"	10'-4 3/4"	W2		YES	MULLED WITH 134A
	134A*	4'-0 1/2"	1'-7 1/2"	2'-7 1/4"	W2		YES	
	135*	6'-0 1/2"	7'-9"	10'-4 3/4"	W9		YES	MULLED WITH 135A
	135A*	6'-1"	1'-7 1/2"	2'-7 1/4"	W9		YES	
	136*	4'-0 1/2"	7'-9"	10'-4 3/4"	W2		YES	MULLED WITH 136A
	136A*	4'-0 1/2"	1'-7 1/2"	2'-7 1/4"	W2		YES	
UPPER FLOOR				T T		1	Γ	1
	201*	6'-0 3/4"	6'-0 3/8"	10'-2 1/2"	W10			
	202	4'-0"	4'-0"	6'-10"	W8			_
	203	3'-0"	4'-0"	6'-10"	W2			
	204	2'-6 3/4"	4'-0"	6'-10"	W2			
	205	2'-6 3/4"	5'-0"	6'-10"	W5			
	206	2'-6"	6'-4"	6'-10"	W6	VEC	VEC	
	207	6'-0" 6'-0"	6'-4" 6'-4"	6'-10" 6'-10"	W7 W7	YES YES	YES YES	
	209*	4'-0 3/4"	3'-10"	6'-10 1/2"	W2	163	150	
	210	6'-0"	6'-4"	6'-10"	W7	YES	YES	+
	211	2'-6 3/4"	5'-0"	6'-10"	W5	TES	163	+
	212	7'-6"	6'-4"	6'-10"	W7	YES	YES	
	213	2'-6 3/4"	5'-0"	7'-10"	W5	123	120	
	214	2'-6 3/4"	5'-0"	6'-10"	W5	+		
	215	2'-6 3/4"	5'-0"	6'-10"	W5	+		
	227	3'-0"	4'-0"	6'-10"	W2			
	228	3'-0"	4'-0"	6'-10"	W2			
	229	4'-0"	4'-0"	6'-10"	W8	+		
		1 0	-T U	3 10	***			

			D 🔿	DOLIEAD			C A FFTY		
LOCATION	NO.	R.O. WIDTH	R.O. Height	R.O.HEAD HEIGHT	TYPE	EGRESS	SAFETY GLASS	U-VALUE	NOTES:
LOWER FLOOR									
	001	5'-6 1/2"	6'-8 1/4"	6'-8 1/4"	D4		YES		
	002	5'-0 1/2"	6'-8 1/4"	6'-8 1/4"	D3		YES		
MAIN FLOOR	•					•		•	
	(E)101	6'-0"	8'-0"	8'-0 3/4"	D4				EXISTING ENTRY DOOR
	(E)112	3'-0"	8'-0"	8'-0 3/4"	D1				EXISTING DOOR TO GAR
	109	3'-0 1/2"	8'-9 1/4"	8'-9 1/4"	D2	YES	YES		
	110	6'-0"	8'-9 1/4"	8'-9 1/4"	D6		YES		SLIDER
	111	12'-0"	8'-9 1/4"	8'-9 1/4"	D5		YES		SLIDER
	112*	5'-10 1/2"	8'-8 1/4"	8'-9 1/4"	D4		YES		
	113	12'-0"	8'-9 1/4"	8'-9 1/4"	D5		YES		SLIDER
	114	6'-6 1/2"	8'-9 1/4"	8'-9 1/4"	D4		YES		
	116	3'-0 1/2"	8'-9 1/4"	8'-9 1/4"	D2		YES		
	127*	3'-0"	8'-2 1/4"	8'-2 1/4"	D1				
	130*	18'-0"	9'-0"		D7				GARAGE
	131*	18'-0"	9'-0"		D7				GARAGE

		NOMINAL	NOMINAL	TYPE	HDWR	SAFETY	NOTES:
LOCATION	NO.	WIDTH	HEIGHT		IID VVIK	GLASS	110120.
LOWER FLOOR		ı					I
	005	2'-6"	6'-8"	D1	PRIVACY		
	006	2'-6"	6'-8"	D1			
	007	2'-6"	6'-8"	D1			
MAIN FLOOR	1	•	-		1	1	•
	102	2'-8"	8'-0"	D8			
	103	2'-10 1/2"	8'-0"	D1			EXISTING DOOR, REVERSE SWING
	104	2'-8"	8'-0"	D1			
	105	2'-8"	8'-0"	D1			
UPPER FLOOR							
	201	2'-8"	7'-0"	D1			
	202	2'-6"	7'-0"	D9			
	203	2'-6"	7'-0"	D1	PRIVACY		
	204	2'-6"	7'-0"	D1	PRIVACY		
	208	2'-6"	7'-0"	D9			
	209	2'-6"	7'-0"	D9			
	212	2'-4"	6'-8"	D9			
	216	2'-6"	7'-0"	D1			
	217	2'-4"	6'-8"	D9	PRIVACY		





REMODE SEIFERT

202 DG Issue Date: 03/29/2024 ○ 03/29/2024 OWNER REVISIONS 90) 06/14/2024 PERMIT REVISION #2

2219



SCHEDULES

PERMIT

STRUCTURAL NOTES

GENERAL REQUIREMENTS

BUILDING CODE & REFERENCE STANDARDS: The "International Building Code" (IBC), 2018 Edition, as adopted and modified by the City of Mercer Island, governs the design and construction of this project. Reference to a specific section in the Code does not relieve the contractor from compliance with the entire materials reference standards noted below. The latest edition of the materials reference standards shall be used.

SCOPE OF STRUCTURAL WORK: Structural engineering of an expanded second story and remodel of existing first story of a single-family residence.

DEFINITIONS: The following definitions apply to these general notes "Structural Engineer of Record" (EOR) - The Structural Engineer who is legally responsible for stamping & signing the structural documents for the project. The EOR is responsible for the design of the Primary Structural System.

"Specialty Structural Engineer" (SSE) - A licensed professional Engineer, not the EOR, who performs specialty structural engineering

services necessary to complete the structure, who has experience and training in the specific specialty. The General Contractor, subcontractor, or supplier who is responsible for the design, fabrication and installation of specialty-engineered elements shall retain the SSE. Submittals shall be stamped and signed by the SSE. Documents stamped and signed by the SSE shall be completed by or under the direct supervision of the SSE with a PE or SE license issued by the State of Washington.

NOTE PRIORITIES: Notes on the individual drawings shall govern over these general notes.

"Deferred Submittals - Deferred Submittal is engineering work to be designed-by-others or bidder-designed.

SPECIFICATIONS: Refer to the contract specifications for information in addition to that contained in these notes and the structural drawings. Refer to these notes, structural drawings, and architectural drawings which serve as specifications for this project.

STRUCTURAL DETAILS: The structural drawings are intended to show the general character and extent of the project and are not intended to show all

ARCHITECTURAL DRAWINGS: Refer to the Architectural drawings for information including, but not limited to: dimensions, elevations, slopes, door and where satisfactory data from adjacent area is available that demonstrates an investigation is not necessary for any of the conditions in IBC Sections window openings, non-bearing walls, curtain walls, stairs, elevators, curbs, drains, depressions, railings, waterproofing, finishes and other nonstructural 1803.5.1 - 1803.5.6 and IBC Sections 1803.5.10 - 1803.5.11.

STRUCTURAL RESPONSIBILITIES: The EOR is responsible for the strength and stability of the Primary Structure in its completed state.

CONTRACTOR RESPONSIBILITIES: The contractor is responsible for the means and methods of construction and all job-related safety standards such as OSHA and WISHA. The contractor is responsible for the strength and stability of the structure during construction and shall provide temporary shoring, bracing and other elements required to maintain stability until the structure is completed. It is the contractor's responsibility to be familiar with the work required in the construction documents and the requirements for executing it properly.

DISCREPANCIES: In case of discrepancies between these general notes, the contract drawings, and specifications, and/or reference standards, the EOR shall determine which shall govern. Discrepancies shall be brought to the attention of the EOR before proceeding with the work. Accordingly, any

Coefficient of Sliding Friction conflict in or between the Contract Documents shall not be a basis for adjustment in the Contract Price.

SITE VERIFICATION: The contractor shall verify all dimensions and conditions at the site prior to fabrication and/or construction. Conflicts between the drawings and actual site conditions shall be brought to the attention of the EOR before proceeding with the work. All underground utilities shall be determined by the Contractor prior to excavation.

ADJACENT UTILITIES: The contractor shall determine the locations of all adjacent underground utilities prior to excavation. Any utility information shown on the drawings and details is approximate and not necessarily complete.

DESIGN CRITERIA

details of the work.

CONSTRUCTION LOADS: Loads on the structure during construction shall not exceed the design loads or the capacity of the partially completed

Wood Deck with concrete topping = 36 psf Wood Floor = 15 psf

SNOW LOAD: The roof snow load is determined by using Chapter 7 of ASCE 7-16 in accordance with IBC Section 1608 and with the following factors Ground Snow Load, Pg = 10 psf

WIND DESIGN: Wind load is determined using Chapter 26 to 30 of ASCE 7-16 in accordance with IBC Section 1609 with the following factors: Basic Wind Speed (3-Second Gust) V = 97 MPH (Ultimate) / 75 MPH (ASD) Wind Importance Factor Iw = 1.0 Risk-Category = II $GCpi = \pm 0.18$ Exposure Category = C Components & Cladding Pressure = 25.7 PSF (Ultimate) Components & Cladding End Zone Pressure = 31.7 PSF (Ultimate)

Analysis Procedure - Directional Procedure per ASCE 7, Table 27.2-1

SEISMIC DESIGN: Earthquake design is determined using Chapter 12 ASCE 7-16 in accordance with IBC Chapter 16 with the following factors: Importance Factor le = 1.0 Risk Category= II Ss = 1.415 g Sds = 1.132 g

S1 = 0.492 gSd1 = 0.593 gRedundancy Factor, $\rho = 1.3$

Basic Seismic Force Resisting System: A-15 (Bearing Wall Systems) Light-framed walls with wood structural panels rated for shear

 Analysis Procedure: Equivalent lateral force procedure, per ASCE 7-16, Section 12.8 R = 6.5

• Cs = 0.15 Cd = 4 \bullet Ω = 2.5

Seismic demands on nonstructural components, structural components engineered as part of deferred submittals, and connections of those components to the primary structure shall be designed in accordance with the aforementioned building code, the general seismic criteria listed

DESIGN BASE SHEAR: Design Base Shear (Seismic Governed) (ASD), V = 30.47 K

Floor Total Load Deflection Limit: Floor Live load Deflection Limit: L/480 L/240 Roof Total Load Deflection Limit: L/360 Roof Live load Deflection Limit: **Operable Partition Support Members:** L/600 or 1/4" (whichever is less)

Roof (Live) 20 PSF Roof (Snow) 25 PSF Balconies and Decks 1.5 X occupancy served ≤ 100 psf Residential Floor

DEFERRED SUBMITTAL LOADS: All pre-engineered, pre-fabricated, pre-manufactured, or other products designed by others shall be designed for the tributary dead and live loads plus wind, earthquake, and component, and cladding loads when applicable. Design shall conform to the project drawings

CONCRETE REINFORCEMENT and specifications, reference standards, and governing code.

Roof Dead Load Top Chord Dead Load 12 PSF 8 PSF Bottom Chord Dead Load Attic Bottom Chord Dead Load 18 PSF 20 PSF Roof Live Load Top Chord Live Load 20 PSF 10 PSF Bottom Chord Live Load Total Deflection Limit L/240 Live Load Deflection Limit L/360 Truss Uplift Load (Gross) 10 PSF

SUBMITTALS

SUBMITTALS: Shop Drawings shall be submitted to the Architect/EOR prior to any fabrication or construction for all structural items as noted below. The contractor shall review and place a shop drawings stamp on the submittal before forwarding to the EOR. Submittals shall be made in time to provide a minimum of one week for review by the EOR. Additional submittals required for this project are specified in the specific sections below. Reference the individual material section for specific information to be included in the submittal.

If the shop drawings differ from or add to the design of the Structural drawings, they shall bear the seal and signature of the Washington State Registered Professional Engineer who is responsible for the design...

ALTERNATES: Product or manufacturer components specified in these drawings are used as the basis of design for this project. Alternates for specified items may be submitted to the EOR for review. However, contractor shall submit a current ICC-ESR/IAPMO-ER report identifying that an alternative component has the same or greater load capacity than the specified item.

SHOP DRAWING REVIEW: Review by the Architect/EOR is for general compliance with the design concept and the contract documents. Dimensions and quantities are not reviewed by the EOR, and therefore, must be verified by the General Contractor. Markings or comments shall not be construed as relieving the contractor from compliance with the project plans and specifications, nor departures therefrom. The contractor remains responsible for details and accuracy; for confirming and correlating all quantities and dimensions; for selecting fabrication processes; for techniques of assembly; and for performing work in a secure manner. When shop drawings (component design drawings) differ from or add to the requirements of the Structural drawings they shall be designed and stamped by the responsible SSE. Allow one week for EOR review time.

DEFERRED SUBMITTALS: Per IBC Section 107.3.4.1, drawings, calculations, and product data for the design and fabrication of items that are designed-by-others shall bear the seal and signature of the Washington State Registered Professional Engineer (SSE) who is responsible for the design minimum 8" on all sides and edges. and shall be submitted to the Architect/EOR and the building department for review prior to fabrication. Allow one week for EOR review time.

The SSE shall submit stamped and signed calculations and shop drawings to the EOR for review. Review of the SSE's shop drawings is for general compliance with design criteria and compatibility with the design of the primary structure and does not relieve the SSE of responsibility for that design. All necessary bracing, ties, anchorage, and proprietary products shall be furnished and installed per manufacturer's instructions or the SSE's design drawings and calculations. Submitted drawings shall indicate all reaction forces imparted to the primary structure. The design of the connection to the primary structure is the responsibility of the supplier and SSE. Submitted calculations are for cursory review only and will generally not be returned. Deferred submittals include but are not limited to the following: Prefabricated Wood Roof Trusses/Joists (RT/RJ)

NON-STRUCTURAL COMPONENTS: Design, detailing and anchorage of all nonstructural components shall be in accordance with ASCE 7-16, Chapter 13 and the project specifications. Nonstructural components designed by others shall not induce torsional loading into supporting steel structural members without additional bracing of those members to eliminate torsional forces. Torsional bracing shall be designed by the nonstructural component designer and approved by the EOR. Anchorage to the primary structure is per the bidder-design contractor or supplier.

TESTS & INSPECTIONS

INSPECTIONS: All construction is subject to inspection by the Building Official in accordance with IBC Sec 110. The contractor shall coordinate all required inspections with the Building Official. Submit copies of all inspection reports to the Architect/EOR for review. The Building Official may accept inspection of and reports by approved inspection agencies in lieu of Building Official's inspections. The contractor shall obtain approval of Building Official to use the third-party inspection agency and contractor shall alert the Architect/EOR as such.

SPECIAL INSPECTIONS: In addition to the inspections required by IBC Sec 110, a Special Inspector shall be hired by the Owner as an independent third-party inspector to perform the special inspections per IBC Ch. 17. Special inspections shall be performed by an approved testing agency as outlined in the Special Inspection Schedule, the contract documents, and/or the project specification. Special Inspections shall meet the requirements outlines in the specific materials sections of IBC Sec 1705. The contractor is responsible for scheduling the inspections, per the city/Building Official requirements. The EOR shall be independent of the special inspection process. All questions regarding Special Inspections shall be directed to the Building Department or an approved special inspection agency.

Special Inspections shall be performed for the following:

Periodic inspection of reinforcing steel and cast-in-place anchors Periodic verification of the use of the required design mix.

Periodic inspection of 2x and 3x bottom plates and plate washers.

Periodic inspection of steel, bolts, nuts and washers' identification marks conform to ASTM standard and weld filler material conforms to AWS. Periodic inspection of anchor bolts, hold-downs, drag strut connections, nailing size & spacing. Periodic verification of moisture content of wood studs, plates, beams, and joists.

PREFABRICATED CONSTRUCTION: All prefabricated construction shall conform to the inspection requirements of the same material or construction type used for this project.

SOILS AND FOUNDATIONS

REFERENCE STANDARDS: Conform to IBC Chapter 18 "Soils and Foundations."

GEOTECHNICAL REPORT: Recommendations contained in "Foundation and Critical Area Considerations" by Geotech Consultants, Inc, dated October 5, 2023, and were used for design.

GEOTECHNICAL INSPECTION: The Geotechnical Engineer or third-party inspector shall inspect all prepared soil bearing surfaces prior to placement of concrete and reinforcing steel and provide a letter to the Owner stating that soils are adequate to support the "Allowable Foundation Pressure" shown below. Soil compaction shall be supervised by an approved testing agency or Geotechnical Engineer. Site soil conditions, fill placement, and load-bearing requirements shall be as required by IBC Section 1705.6 and Table 1705.6. Assumed values shall be field verified by the Building Official or the Geotechnical Engineer prior to placing concrete. The Building Official shall be permitted to waive the requirement for a geotechnical investigation

DESIGN SOIL VALUES: Allowable Soil Bearing Pressure 2500 PSF DL + LL 3332 PSF DL + LL + Seismic/wind Retaining Walls Passive Lateral Pressure Active Lateral Pressure (unrestrained) 30 PSF/FT Active Lateral Pressure (restrained) 60 PSF/FT Uniform Seismic

SLABS-ON-GRADE & FOUNDATIONS: All slabs-on-grade and foundations shall bear on structural compacted fill or competent native soil per the Geotechnical report or as noted in these documents. Exterior perimeter footings shall bear not less than 18 inches below finish grade, or as required by (3) ANSI/TPI 1 "National Design Standard for Metal-Plate-Connected Wood Truss Construction." the Geotechnical Engineer and the Building Official. Interior footings shall bear not less than 12 inches below finish floor.

FOUNDATION STEM WALLS: Unless otherwise noted on the drawings, the maximum unbalanced soil condition for all foundation stem walls (difference in elevation between interior and exterior soil grades) shall be 2'-6". Maintain a minimum 8" separation between finish grade and untreated wood

BACKFILLING: Backfill behind retaining and foundation walls shall be of free-draining material placed in maximum loose lifts of 12" or as directed by the Geotechnical Report. Backfill behind walls shall not be placed before the wall is properly supported by the floor slab or temporary bracing. Backfill shall be compacted using hand-operated equipment only. The contractor shall refrain from operating heavy equipment behind retaining and foundation walls within a distance equal to or greater than the height of the wall, unless otherwise approved by the EOR. All topsoil organics and loose surface soil shall be removed from beneath fill supporting concrete slab or paving.

COMPACTION: Unless otherwise specified by a Geotechnical Engineer, footings shall be placed on compacted material and shall be well-graded granular material with no more than 5% passing a #200 sieve. Fills placed shall be in maximum 8" lifts and all bearing soils shall be compacted to 95% maximum density at optimum moisture content using the Modified Proctor Test.

CAST-IN-PLACE CONCRETE

REFERENCE STANDARDS: Conforms to the latest editions of the following: ACI 318 "Building Code Requirements for Structural Concrete and Commentary". (2) IBC Chapter 19.

FIELD REFERENCE: The contractor shall keep a copy of ACI Field Reference manual, SP-15, "Standard Specifications for Structural Concrete (ACI 301) with Selected ACI and ASTM References."

CONCRETE MIXTURES: Conform to ACI 318 Chapter 19 "Concrete: Design and Durability Requirements."

MATERIALS: Conform to ACI 318 Chapters 19 & 20.

SUBMITTALS: Provide all submittals required by ACI 301 Sec 4.1.2. Submit mix designs for each mix in the table below.

TABLE OF MIX DESIGN REQUIREMENTS Strength Test Age Maximum Exposure Max Minimum (days) Aggregate Classification W/C Ratio Air Content & concrete retaining walls

MIX DESIGN NOTES:

(1) W/C Ratio: Water-cementitious material ratios shall be based on the total weight of cementitious materials. Ratios not shown in the table above are controlled by strength requirements. (2) Cementitious Content: The use of fly ash, other pozzolans, silica fume, or slag shall conform to ACI 301 Sec 4.2.1 "Materials". Maximum amount of fly ash shall be 20% of total cementitious content unless reviewed and approved otherwise by EOR.

Air Content: Conform to ACI 301 Sec 4.2.2.4. Horizontal exterior surfaces in contact with the soil require entrained air. Use Exposure Category F0, S0, W0, and C0 unless noted otherwise. Tolerance is +/- 1.5%. Air content shall be measured at point of placement.

Exposure Classification: The mix design provided shall meet the requirements of ACI 318 Chapter 19, based on the exposure classification indicated in the table above.

(5) Slump: Unless otherwise specified or permitted, concrete shall have at the point of delivery, a slump of 4" +/- 1". For additional criteria, reference ACI 301 Sec 4.2.2.2.

(6) Shrinkage Limit: Concrete used in elevated slabs and beams shall have a shrinkage limit of 0.045% at 28 days measured in accordance with (7) Non-chloride accelerator: Non-chloride accelerating admixture may be used in concrete slabs placed at ambient temperatures below 50F at the

FORMWORK: Conform to ACI 301 Sec 2 "Formwork and Form Accessories." Removal of Forms shall conform to Sec 2.3.2 except strength indicated in

Sec 2.3.2.5 shall be 0.75 f'c. Re-shoring shall conform to Sec 2.3.3. MEASURING, MIXING, AND DELIVERY: Conform to ACI 301 Sec 4.3.

HANDLING, PLACING, CONSTRUCTING, AND CURING: Conform to ACI 301 Sec 5.

EMBEDDED ITEMS: Position and secure in place expansion joint material, anchors and other structural and non-structural embedded items before placing concrete. Contractor shall refer to mechanical, electrical, plumbing, and architectural drawings and coordinate all other embedded items

ACI 301 "Standard Specifications for Structural Concrete, Sec 3 "Reinforcement, and Reinforcement Supports." IBC Chapter 19, Concrete.

 ACI 318 and ACI 318R. (4) ACI SP-66 "ACI Detailing Manual" including ACI 315 "Details and Detailing of Concrete Reinforcement."

(5) CRSI MSP-2 "Manual of Standard Practice."

(6) ANSI/AWS D1.4 "Structural Welding Code - Reinforcing Steel." SUBMITTALS: Conform to ACI 301 Sec 3.1.1 "Submittals, data, and drawings." Submit placing drawings showing fabrication dimensions and locations

for placement of reinforcement and reinforcement supports. MATERIALS: Reinforcing Bars ASTM A615, Grade 60, deformed bars. ASTM A706, Grade 60, deformed bars. Weldable Reinforcing Bars Smooth Welded Wire Fabric ASTM A185 CRSI MSP-2, Chapter 3 "Bar Supports." Bar Supports 16.5 gage or heavier, black annealed.

FABRICATION: Conform to ACI 301, Sec 3.2.2 "Fabrication," and ACI SP-66 "ACI Detailing Manual." WELDING: Bars shall not be welded unless authorized. When authorized, conform to ACI 301, Sec 3.2.2.2. "Welding" and provide ASTM A706, Grade

PLACING: Conform to ACI 301, Sec 3.3.2 "Placement." Placing tolerances shall conform to Sec 3.3.2.1 "Tolerances."

CONCRETE COVER: Conform to the following cover requirements from ACI 301, Table 3.3.2.3. Concrete cast against earth Concrete exposed to earth or weather (#5 & smaller)

SPLICES & DEVELOPMENT LENGTH: Conform to ACI 301, Sec 3.3.2.7. Refer to Lap Splice & Development Schedule for 2500 psi concrete below. Lap all continuous reinforcement and corner bars per Schedule. The splices and development lengths indicated on individual sheets control over the schedule. Use Class B splices unless otherwise noted. Mechanical connections may be used when approved by the EOR. WWF to be lapped a

LAP & DEVELOPMENT SCHEDULE (Concrete strength f'c = 2500)

Bar Designation 8" on all sides and edges

FIELD BENDING: Conform to ACI 301 Sec 3.3.2.8. "Field Bending or Straightening." Bar sizes #3 through #5 may be field bent cold the first time. Other bars require preheating. Do not twist bars.

CORNERS BARS: Provide matching-sized "L" corner bars for all horizontal wall and footing bars with the appropriate splice length, UNO.

confirm minimum reinforcement of walls with EOR prior to rebar fabrication. Vertical Bars #4 @ 12" OC @ CL of Wall #5 @ 12" OC @ CL of Wall

STRUCTURAL STEEL

Bars in slabs and walls

DESIGN STANDARDS: Structural steel for this project is designed in accordance with the latest edition of the AISC Steel Construction Manual.

REFERENCE STANDARDS: Conform to: ANSI/AISC 360 "Specification for Structural Steel Buildings" - Referred to as "AISC Specification"

AISC 303 "Code of Standard Practice for Steel Buildings & Bridges" RCSC "Specification for Structural Joints using ASTM A325 or A490 Bolts."

AWS D1.1 "Structural Welding Code - Steel." AISC 341 "Seismic Provisions for Structural Steel Buildings." (1) Submit shop drawings in accordance with AISC Specification Sec M1 "Shop and Erection Drawings."

Structural WF Shapes ASTM A992, Fy = 50 ksi ASTM A36, Fy = 36 ksi Bars & Plates Anchor Bolts & Bolts in Wood ASTM A307 ASTM A563 or ASTM A194, Grade 2H ASTM F436 Washers (flat or beveled) ASTM F1554, Grade 36 Anchor Rods (hooked, headed, threaded/nutted) ASTM A36, Fy = 36 ksi Threaded Rods Welded Headed/Threaded Studs (WHS, WTS) ASTM A108 E70XX, 70 ksi, low hydrogen, typical Welding Electrodes **Expansion Anchors** Per Drawings Simpson Strong-Tie Adhesive Anchors Per Drawings Simpson SET-3G Simpson TITEN HD Concrete Screws

WELDING: Conform to AWS D1.1, D1.3 & D1.8. Welders shall be certified in accordance with AWS and WABO requirements. Use E70 electrodes of type required for materials to be welded.

FABRICATION/ERECTION: Conform to AISC Specification Sec M2 "Fabrication," AISC Code Sec 6 "Fabrication and Delivery" and AISC Code Sec 8 "Quality Control." The fabricator and erector shall maintain a quality control program to the extent deemed necessary so that all of the work is performed in accordance with this Code, the AISC Specification, contract documents, and project specifications.

SHOP PAINTING: Conform to AISC 360, AISC Specification Sec M3, and AISC Code Sec 6.5. Do not paint steel to be embedded in concrete, fireproofed, or concealed by the interior building finish. Do not paint surfaces to be field welded or where slip-critical bolts are specified. All other interior steel shall be painted with one coat of grey shop primer. All exposed exterior steel shall be painted with an exterior multi-coat system as per the Architect or project specifications or galvanized per section below. Field touch-up painting shall be with primer for exposed interior surfaces and as per the

GALVANIZING: Where required, all exposed steel outside the building envelope shall be hot-dipped galvanized. Apply field touch-ups per project

ERECTION: Conform to AISC Specification Sec M4 "Erection" and AISC Code Sec 7 "Erection." Steel work shall be carried up true and plumb within the limits defined in AISC 303-16 Sec 7.11.

WOOD FRAMING

Member Use

REFERENCE STANDARDS: Conform to: IBC Chapter 23 "WOOD."

Architect or project specifications for exposed exterior surfaces.

(2) NDS and NDS Supplement - "National Design Specification for Wood Construction."

(4) BCSI 2013 "Building Component Safety Information." ALTERNATES: Alternates for specified item may be submitted to the EOR for review. Contractor shall submit a current ICC-ESR/IAPMO-ER report

identifying that an alternative component has the same or greater load capacity than the specified item. IDENTIFICATION: All sawn lumber and pre-manufactured wood products shall be identified by the grade mark or a certificate of inspection issued by the

Sawn Lumber: Conform to grading rules of WWPA, WCLIB, or NLGA. Finger jointed studs acceptable at interior non-structural walls only.

Size	Species	Grade
2x, 3x	HF	No. 2
4x	HF	No. 2
2x	HF	No. 2
4x	HF	No. 2
6x	DF	No. 1
6x	DF	No. 1
	2x, 3x 4x 2x 4x 6x	2x, 3x HF 4x HF 2x HF 4x HF 6x DF

Glued Laminated Timber: Conform to AITC 117 "Standard Specifications for Structural Glued Laminated Timber of Softwood Species, Manufacturing and Design" and ANSI/AITC A190.1 "Structural Glued Laminated Timber." Glued laminated member beams shall not be cambered other than the stock camber of 5000', unless shown otherwise on the plans or specifications.

Sizes	Species	Stress Class	Uses
All	DF/DF	24F-V4	Simple Spans
All	DF/DF	24F-V8	Cantilever Spans

Metal Plate Connected Wood Roof Trusses: Reference DEFERRED SUBMITTAL section above. Conform to IBC Sec 2303.4 "Trusses."

Wood Structural Sheathing (Plywood): Wood APA-rated structural sheathing includes: all veneer plywood, oriented strand board, waferboard, particleboard, T1-11 siding, and composites of veneer and wood-based material. Conform to Product Standards PS-1 and PS-2 of the U.S. Dept. of

	Minimum APA Rating						
ocation	Thickness	Span Rating	Plywood Grade	Exposure	_		
oof	15/32"	24/16	C-D	1			
oor	23/32" T&G	24 OC	Sturd-I-Floor	1			
/alls	15/32"	32/16	C-D	1			

loist Hangers and Connectors: Simpson Strong-Tie Company Inc. as specified in their latest catalogs was used as the basis of design for this project. Alternate connectors by other manufacturers may be substituted provided they have current ICC-ESR/IAPMO-ER approval for equivalent or greater load capacities and are reviewed and approved by the EOR prior to ordering. Connectors shall be installed per the manufacturer's instructions. Where connector straps connect two members, place 1/2 of the nails or bolts in each member. Unless noted otherwise all nails shall be full length common. Nail straps to wood framing as late as possible in the framing process to allow the wood to shrink and the building to settle.

Nails and Staples: Conform to IBC Sec 2303.6 "Nails and Staples." Unless noted on plans, nail per IBC Table 2304.10.1. Unless noted otherwise all nails shall be common. Nail sizes specified on the drawings are based on the following specifications:

COMMON NAILS		
Size	Length	Diame
8d	2-1/2"	0.131'
10d	3"	0.148'
16d	3-1/2"	0.162"
40-10:-1	0.4/41	0.440

Lag Bolts/Thru-Bolts/Anchor Bolts: Conform to ASTM A307. Provide plate washers/BPS washers under the heads and nuts of all bolts and lag screws

Wood Holdowns: Holdowns specified are as manufactured by Simpson Strong-Tie Company Inc. Additional framing members shall be provided per the manufacturer's requirements. Acceptable equivalent product substitutions are available from other manufacturers with EOR approval. Do not countersink

Engineered Wood Products (EWP): The following materials are based on lumber manufactured by TrusJoist by Weyerhaeuser. Trus-Joist by Weyerhaeuser was used as the basis of design for this project. Alternate products by other manufacturers may be substituted provided they have current

ICC-ESR/IAPMO-ER approval for equivalent or greater load and stiffness properties and are reviewed and approved by the EOR. b) Parallel Strand Lumber (PSL): Conform to ICC-ES Report No. ESR-1387, CCMC Report No. 11161-R, or NES Report No. NER-481. Use

2.0E unless noted otherwise c) Laminated Strand Lumber (LSL): Conform to ICC-ES Report No. ESR-1387, CCMC Report No. 12627-R, or NES Report No. NER-481.

NAILING REQUIREMENTS: Provide minimum nailing in accordance with IBC Table 2304.10.1 "Fastening Schedule" except as noted on the drawings. Nailing for roof/floor diaphragms/shear walls shall be per drawings. Nails shall be driven flush and shall not fracture the surface of sheathing.

STANDARD LIGHT-FRAME CONSTRUCTION: Unless noted on the drawings, construction shall conform to IBC Sec 2308 "Conventional Light-Frame

Construction" and IBC Sec 2304 "General Construction Requirements." (1) Wall Framing (Unless noted otherwise on plans and details) All interior walls shall be 2x4 @ 16"OC and all exterior walls shall be 2x6 @ 16"OC. Provide (2) bundled studs min at wall ends and each side of all openings. All solid sawn lumber beams and headers shall be supported by a minimum of (1) trim and (1) king stud and all glulam or engineered wood beams and headers by (2) trim and (2) king studs. Provide minimum (2) 2x10 headers at all interior and exterior wall openings. Stitch-nail bundled studs with (2) 10d @ 12"OC. Provide solid blocking thru floors to supports below for bearing walls and posts. Attach bottom plates of stud walls to wood framing below with 16d @ 12"OC or to concrete with 5/8"-dia. anchor bolts x 7" embedment at 48"OC. Refer to shear wall schedule for specific sheathing, stud, and nailing requirements at shear walls.

(2) Roof/Floor Framing: (Unless noted otherwise on plans and details) Provide double joists/rafters under all parallel bearing partitions and solid blocking at all bearing points. Provide double joists around all roof/floor openings. Multi-joists/rafters shall be stitch-nailed together with (2)10d @ 12"OC. Provide roof sheathing edge clips centered between framing at unblocked plywood edges. All floor sheathing shall have tongue and groove joints or be supported by solid blocking. Allow 1/8" spacing at all panel edges and ends of roof/floor sheathing. Roof/floor sheathing shall be laid face grain perpendicular to framing members.

Provide gypsum sheathing on interior surfaces and plywood sheathing on exterior surfaces.

(3) Blocking: (Unless noted otherwise on plans and details) All blocking shall be full-height 2x at solid sawn framing systems or a full-height I-joist or

MOISTURE CONTENT: Wood material used for this project shall have maximum moisture content of 19% except for the pressure-treated wood sill plate. PRESERVATIVE TREATMENT: Wood materials are required to be "treated wood" under certain conditions in accordance with IBC Sec 2304.12

"Protection against decay and termites." Conform to the appropriate standards of the American Wood-Preservers Association (AWPA) for sawn lumber,

glued laminated timber, round poles, wood piles, and marine piles. Follow American Lumber Standards Committee (ALSC) quality assurance

procedures. Products shall bear the appropriate mark. Coat all ends of cut pressure treated framing with treatment complying with AWPA U1. METAL CONNECTORS/PT WOOD: All metal hardware and fasteners in contact with pressure treated lumber shall be stainless steel Type 316L. At the Owner's risk and discretion, hot-dipped galvanized metal hardware and fasteners may be investigated for use in lieu of stainless steel provided that the finish has a minimum zinc content of at least 1.85 oz./SF and its use is coordinated by the Contractor and Wood Supplier for the expected environment

NAILERS ON STEEL COLUMNS AND BEAMS. Wood 3x nailers are required at all steel columns and steel beams abutting or embedded within wood framing. Unless noted otherwise, attach the wood to the steel with 5/8" diameter bolts or welded threaded studs at 16" oc. Wood nailers on beams that support joist hangers shall not overhang the beam flange by more than 1/4".

<u>POST-INSTALLED ANCHORS:</u> Provide post-installed anchors as specified in these drawings.

and moisture exposure for appropriate use based on the method of preservative treatment of the wood.

TYPICAL CONCRETE REINFORCEMENT: Unless noted on the plans, concrete walls shall have the following minimum reinforcement. Contractor shall

Use of alternate products, or of post-installed anchors at locations not shown in these drawings, is subject to the approval of the Architect/EOR. Submit proposed anchors to the Architect/EOR with an ICC-ESR/IAPMO-ER report valid for the 2018 IBC or municipality where the building is to be constructed. Submitted ICC/IAPMO reports shall demonstrate that the anchors are suitable for use in cracked concrete. Use acrylic anchors of equivalent strength when base material falls below 40F. Install anchors in strict accordance with ICC-ESR/IAPMO-ER report and manufacturer's instructions. Where anchors resist seismic loads, submitted ICC-ESR/IAPMO-ER reports shall demonstrate that the anchors are suitable for the resistance of seismic loads.

> CONCRETE SCREWS: Concrete screws shall be SIMPSON Titen HD or EOR approved equal with current ICC-ESR/IAPMO-ER report. Install screws in accordance with manufacturer's instructions. Embedment lengths shall be as shown on the drawings.

RENOVATION

DEMOLITION: Contractor shall verify all existing conditions before commencing any demolition. Shoring shall be installed to support existing construction as required and, in a manner, suitable to the work sequences. Existing reinforcing shall be saved where and as noted on the plans. Saw cutting, if and where used, shall not cut existing reinforcing that is to be saved. Demolition debris shall not be allowed to damage or overload the existing structure. Limit construction loading (including demolition debris) on existing floor systems to 40 psf.

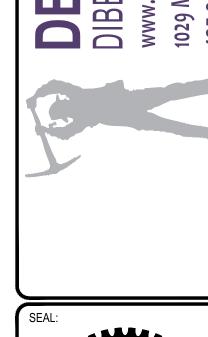
(1) All new openings through existing walls, slabs, and beams shall be accomplished by saw cutting wherever possible. Contractor shall verify all existing conditions and location of members prior to cutting any openings. (3) Small round openings shall be accomplished by core drilling, if possible.

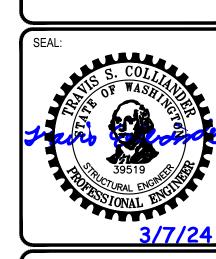
(4) Where new reinforcing terminates at existing concrete, threaded bars into threaded expansion inserts in existing concrete shall be provided to match horizontal or vertical reinforcing, unless otherwise noted on plans.

STRUCTURAL ABBREVIATIONS

AB	ANCHOR BOLT	LLH	LONG LEG HORIZONTAL
ADD'L	ADDITIONAL	LLV	LONG LEG VERTICAL
ALT	ALTERNATE	LONGIT	LONGITUDINAL
CH	ARCHITECT(URAL)	Ls	SPLICE LENGTH
R	ALL-THREADED ROD	LSL	LAMINATED STRAND LUMBER
/	BOTTOM OF	LVL	
VI.	DOLINDADY MAILING	MAT'L	MATERIAL
i DG	BUILDING	MAX	MAXIMUM
LDG LKG	BLOCKING	MECH	MECHANICAL
BOTT	BOTTOM	MFR	MANUFACTURER
RG	BEARING	MIN	MINIMUM
BTWN		MISC	MISCELLANEOUS
) W N <u>'</u>		(N)	NEW
	CENTERLINE	NIC	NOT IN CONTRACT
	CONSTRUCTION or CONTROL JOINT	NOM	NOMINAL
LR	CLEAR(ANCE)	NTS	NOT TO SCALE
CONC		OC	ON CENTER
CONN	CONNECTION		
CONST	CONSTRUCTION	OPNG	OPENING
CONT	CONSTRUCTION CONTINUOUS	OPP	OPPOSITE OFFICIAL POARS
TRD	CENTERED	OSB	ORIENTED STRAND BOARD
CTSK	COUNTERSINK	OWWJ	OPEN WEB WOOD JOIST
RI	CENTERED COUNTERSINK DOUBLE(R)	P.	PLATE
)ET	DETAIL	PC	PRECAST
)F	DOUGLAS FIR	PERM	PERIMETER
)IAG	DIAGONAL	PERM PERP PLY	PERPENDICULAR
)N	DOWN	PLY	PLYWOOD
)P	DEPTH	PRE-MFR	PLYWOOD PRE-MANUFACTURED PARALLEL STRAND LUMBER
)WL	DOWEL	PSL	PARALLEL STRAND LUMBER
)WG		P-T	POST-TENSIONED
	DRAWING	PT	
A F	EACH	REF	
	EACH FACE	REINF	REINFORCING
EN EL	EDGE NAILING	REQ'D	REQUIRED
	ELEVATION	RET	RETAINING
	EMBEDMENT	KEI	
	ENGINEER	SCHED	SCHEDULE
Q	EQUAL	SECT	SECTION
S	EACH SIDE	SHTG	SHEATHING
	EACH WAY	SIM	SIMILAR
E)	EXISTING	SMS	SHEET METAL SCREW
EXP	EXPANSION	SOG	SLAB-ON-GRADE
EXT	EXTERIOR	SPEC	SPECIFICATION
-/	FACE OF	SQ	SQUARE
, LR	FLOOR	SS	STAINLESS STEEL
NDN	FOUNDATION	STAGG	STAGGERED
RT	FIRE RETARDANT TREATED	STD	STANDARD
TAO	FORCED TRANSFER AROUND OPENING	STIFF	STIFFENER
TG	FOOTING	STRUCT	STRUCTURAL
GA .	GAUGE	SUPPL	SUPPLEMENT
GALV	GALVANIZED	SW	SHEAR WALL
GEN	GENERAL	SYM	SYMMETRICAL
GLB	GLUE LAMINATED BEAM	T/	TOP OF
GR	GRADE	T&B	TOP & BOTTOM
GT	GIRDER TRUSS	T&G	TONGUE & GROOVE
GWB	GYPSUM WALL BOARD	THK, THK'N	
		THRU	THROUGH
HD LDD	HOLDOWN	TRANSV	TRANSVERSE
HDR	HEADER	TYP	
HF	HEM-FIR		TYPICAL
HGR	HANGER	UNO	UNLESS NOTED OTHERWISE
HORIZ	HORIZONTAL	VERT	VERTICAL
HSS	HOLLOW STRUCTURAL SECTION	VIF	VERIFY IN FIELD
HT.	HEIGHT	WHS	WELDED HEADED STUD
INT	INTERIOR	WTS	WELDED THREADED STUD
Ld	DEVELOPMENT LENGTH	l wwF	WELDED WIRE FABRIC

S $\Box\Box$ Z $\mathbf{\Omega}$





& SHEET						
	ET INDEX	SHE	STRUCTURAL			
SHEET NUMBER:	SECTIONS & DETAILS	S3.0	GENERAL NOTES, ABBREVIATIONS, INDEX	S1.0		
	SECTIONS & DETAILS	S3.1	FOUNDATION PLAN, SW KEY PLAN	S2.0		
S 1	SECTIONS & DETAILS	S3.2	MAIN FLOOR FRAMING PLAN, SW KEY PLAN	S2.1		
				I		

S3.4 SECTIONS & DETAILS

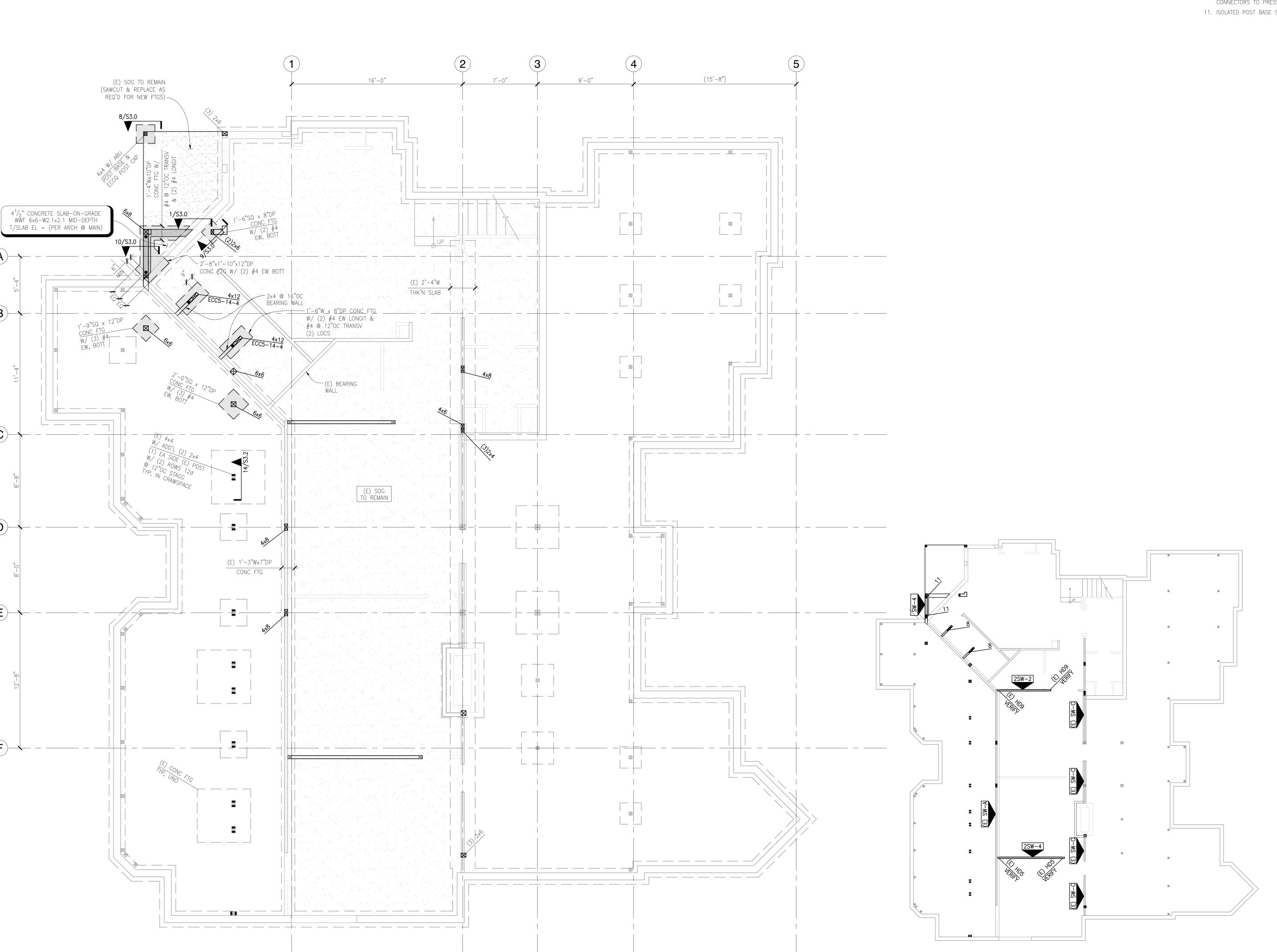
S2.2 UPPER FLOOR FRAMING PLAN, SW KEY PLAN S3.3 SECTIONS & DETAILS

S2.3 ROOF FRAMING PLAN

SHEET TITLE:

STRUCTURAL

GENERAL NOTES

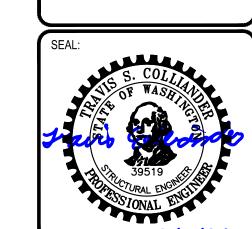


FOUNDATION PLAN AT BASEMENT

SCALE: $\frac{1}{4}$ " = 1'-0"

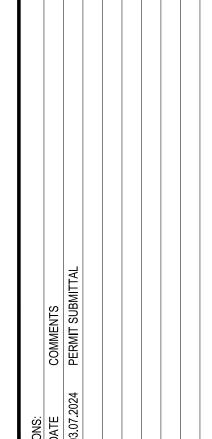
FOUNDATION PLAN NOTES

- 1. REFERENCE S1.0 FOR STRUCTURAL GENERAL NOTES, DRAWING LIST, & ABBREVIATIONS...
- 2. DIMENSIONS: VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. 3. CONCRETE DIMENSIONS: CONTRACTOR SHALL LOCATE ALL DOOR OPENINGS IN FOUNDATION WALLS. VERIFY
- LOCATION AND ELEVATIONS PER ARCHITECT'S PLANS PRIOR TO POURING CONCRETE.
- 4. FOR ALL DUCTS, CHASES AND PIPES, REFERENCE MECHANICAL, PLUMBING, ELECTRICAL AND SPRINKLER DRAWINGS, FOR STAIR DETAILS AND GUARDRAILS REFERENCE ARCHITECTURAL DRAWINGS.
- 5. MOISTURE PROOF ALL WALLS BELOW GRADE PER ARCHITECT.
- 6. PROVIDE PANEL EDGE NAILING AT ALL HOLDOWNS, POSTS/BUNDLED STUDS. 7. THE BOTTOM OF ALL FOOTINGS SHALL BE 18" MINIMUM BELOW GRADE AND BEAR UPON FIRM, UNDISTURBED
- SOIL OR ENGINEERED COMPACTED BACK-FILL. 8. FULLY BLOCK FLOOR CAVITY AT ALL POINT LOADS. POINT LOADS SHALL BE SUPPORTED CONTINUOUSLY THROUGH FLOORS TO THE FOUNDATION.
- 9. ALL WOOD IN CONTACT WITH WEATHER-EXPOSED CONCRETE OR WITHIN 8" OF FINISHED GRADE SHALL BE
- PRESSURE-TREATED. 10. USE HOT DIPPED GALVANIZED FASTENERS AND EITHER HOT DIPPED GALVANIZED OR ZMAX COATED HANGERS AT CONNECTORS TO PRESSURE TREATED LUMBER.
- 11. ISOLATED POST BASE SHALL BE A SIMPSON 'ABA' TO MATCH POST SIZE, UNO PER PLAN.



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SHEET TITLE:

STRUCTURAL FOUNDATION PLAN

SHEET NUMBER:

S 2.0

SHEAR WALL KEY PLAN NOTES

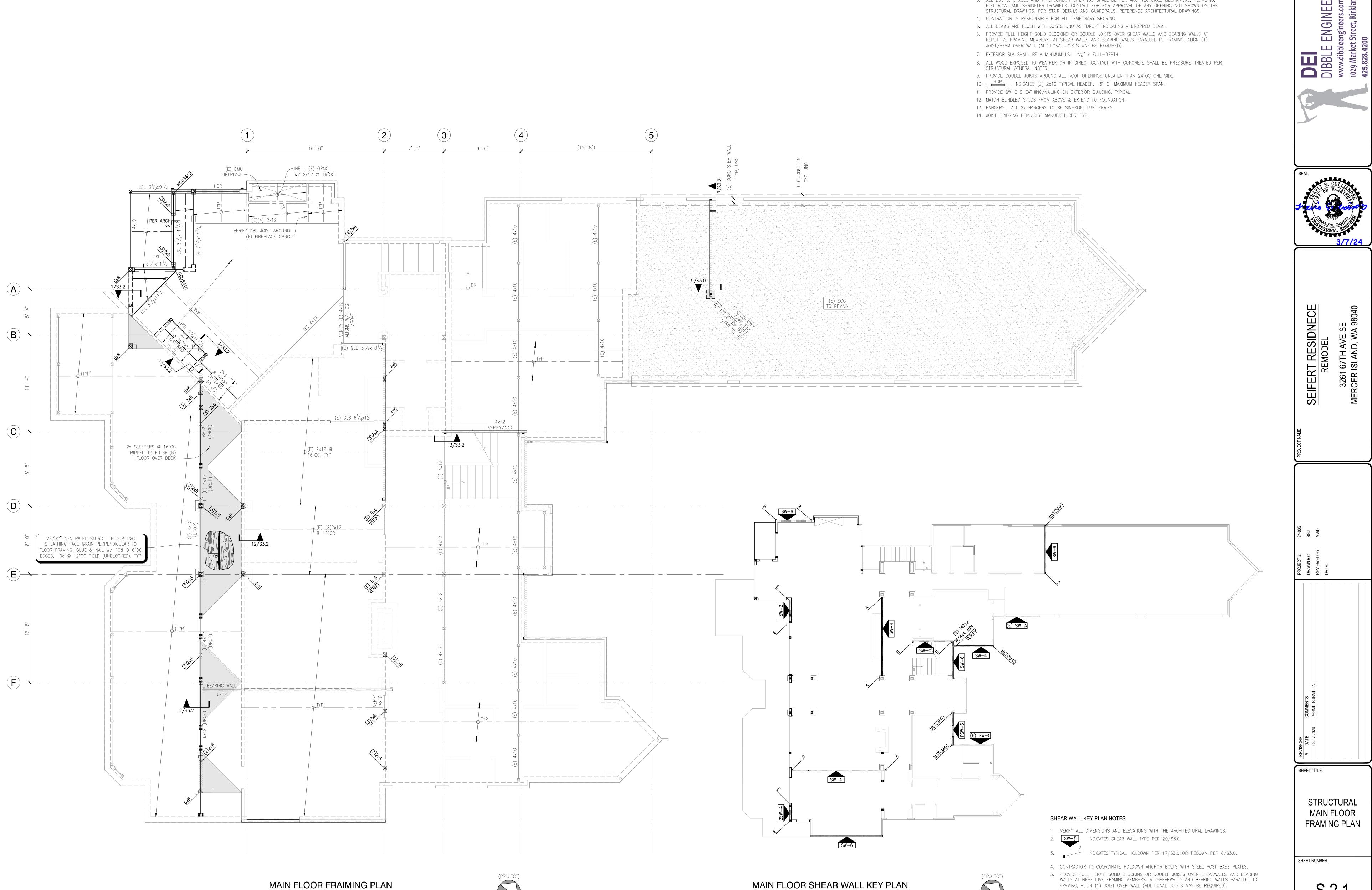
BASEMENT SHEAR WALL KEY PLAN

SCALE: NTS

1. VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. 2. SW-# INDICATES SHEAR WALL TYPE PER 20/S3.0.

INDICATES TYPICAL HOLDOWN PER 17/S3.0 OR TIEDOWN PER 6/S3.0.

4. CONTRACTOR TO COORDINATE HOLDOWN ANCHOR BOLTS WITH STEEL POST BASE PLATES. 5. PROVIDE FULL HEIGHT SOLID BLOCKING OR DOUBLE JOISTS OVER SHEARWALLS AND BEARING WALLS AT REPETITIVE FRAMING MEMBERS. AT SHEARWALLS AND BEARING WALLS PARALLEL TO FRAMING, ALIGN (1) JOIST OVER WALL (ADDITIONAL JOISTS MAY BE REQUIRED).



SCALE: NTS

SCALE: $\frac{1}{4}$ " = 1'-0"

FLOOR FRAMING PLAN NOTES

1. REFERENCE S1.0 FOR STRUCTURAL GENERAL NOTES, DRAWING LIST & ABBREVIATIONS.

2. DIMENSIONS: VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. 3. ALL DUCTS, CHASES AND PIPE/CONDUIT OPENINGS SHALL BE PER ARCHITECTURAL, MECHANICAL, PLUMBING,

6. AT MSTC40 CALLOUT REFERENCE 7/S3.2.

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STRUCTURAL MAIN FLOOR FRAMING PLAN

SHEET NUMBER:

FLOOR FRAMING PLAN NOTES

- 1. REFERENCE S1.0 FOR STRUCTURAL GENERAL NOTES, DRAWING LIST & ABBREVIATIONS.
- 2. DIMENSIONS: VERIFY ALL DIMENSIONS AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS. 3. ALL DUCTS, CHASES AND PIPE/CONDUIT OPENINGS SHALL BE PER ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL AND SPRINKLER DRAWINGS. CONTACT EOR FOR APPROVAL OF ANY OPENING NOT SHOWN ON THE
- STRUCTURAL DRAWINGS. FOR STAIR DETAILS AND GUARDRAILS, REFERENCE ARCHITECTURAL DRAWINGS. 4. CONTRACTOR IS RESPONSIBLE FOR ALL TEMPORARY SHORING.
- 5. ALL BEAMS ARE FLUSH WITH JOISTS UNO AS "DROP" INDICATING A DROPPED BEAM.
- 6. PROVIDE FULL HEIGHT SOLID BLOCKING OR DOUBLE JOISTS OVER SHEAR WALLS AND BEARING WALLS AT REPETITIVE FRAMING MEMBERS. AT SHEAR WALLS AND BEARING WALLS PARALLEL TO FRAMING, ALIGN (1) JOIST/BEAM OVER WALL (ADDITIONAL JOISTS MAY BE REQUIRED).

(15'-8")

- 7. EXTERIOR RIM SHALL BE A MINIMUM LSL $1\frac{3}{4}$ " x FULL-DEPTH.
- 8. ALL WOOD EXPOSED TO WEATHER OR IN DIRECT CONTACT WITH CONCRETE SHALL BE PRESSURE-TREATED PER STRUCTURAL GENERAL NOTES.
- 9. PROVIDE DOUBLE JOISTS AROUND ALL ROOF OPENINGS GREATER THAN 24"OC ONE SIDE.
- 10. == HDR = INDICATES (2) 2×10 TYPICAL HEADER. 6'-0" MAXIMUM HEADER SPAN.
- 11. PROVIDE SW-6 SHEATHING/NAILING ON EXTERIOR BUILDING, TYPICAL.
- 12. MATCH BUNDLED STUDS FROM ABOVE & EXTEND TO FOUNDATION.
- 13. HANGERS: ALL 2x HANGERS TO BE SIMPSON 'LUS' SERIES.
- 14. JOIST BRIDGING PER JOIST MANUFACTURER, TYP.

16'-0"

UPPER FLOOR FRAIMING PLAN

SCALE: $\frac{1}{4}$ " = 1'-0"

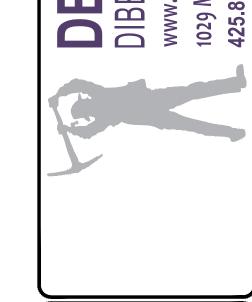
ROOF FRAMING PLAN NOTES

- 1. REFERENCE S1.0 FOR STRUCTURAL GENERAL NOTES, ABBREVIATIONS & SHEET INDEX. 2. VERIFY ALL DIMENSION AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS.
- 3. ALL DUCTS, CHASES AND PIPE/CONDUIT OPENINGS SHALL BE PER ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL AND SPRINKLER DRAWINGS. CONTACT EOR FOR APPROVAL OF ANY OPENING NOT SHOWN ON THE
- STRUCTURAL DRAWINGS. 4. REFERENCE ARCHITECTURAL PLANS FOR ROOF SLOPES, SCUPPERS AND DRAIN LOCATIONS.
- 5. CONTRACTOR RESPONSIBLE FOR ALL TEMPORARY SHORING.
- 6. ALL BEAMS ARE FLUSH WITH JOISTS UNO AS "DROP" INDICATING A DROPPED BEAM.
- 7. PROVIDE FULL HEIGHT SOLID BLOCKING OR DOUBLE JOISTS OVER SHEAR WALLS AND BEARING WALLS AT REPETITIVE FRAMING MEMBERS. AT SHEAR WALLS AND BEARING WALLS PARALLEL TO FRAMING, ALIGN (1) JOIST OVER WALL (ADDITIONAL JOISTS MAY BE REQUIRED).
- 8. PROVIDE SW-6 SHEATHING & NAILING ON EXTERIOR BUILDING, TYPICAL.
- 9. PROVIDE DOUBLE JOISTS AROUND ALL ROOF OPENINGS GREATER THAN 24"OC ONE SIDE.
- 10. REFERENCE 8/S3.1 FOR HEADER SUPPORT JAMBS. PROVIDE MINIMUM (2) 2x STUDS UNO.
- 11. "HDR" INDICATES TYPICAL (2) 2x10 DROPPED HEADER WITH 6'-0" MAXIMUM SPAN. 12. BALLOON FRAME ALL WALLS GREATER THAN ONE LEVEL WITHOUT FLOOR OR ROOF SUPPORT. ALL EXTERIOR
- WALL STUDS 12'-0" HIGH OR GREATER, USE (2) SISTERED STUDS AT 16"OC, UNO. 13. ALL WOOD EXPOSED TO WEATHER SHALL BE PRESSURE—TREATED PER STRUCTURAL GENERAL NOTES.
- 14. HORIZONTL STRAP TIES INDICATED ON THE SHEAR WALL PLANS ARE TO BE CENTERED OVER WALL TOP PLATE AND/OR HEADER, BLOCKING OR BEAM. CONTRACTOR SHALL COORDINATE ADDTIONAL WALL FURRING REQUIRED
- AT BEAMS AND POSTS WITH CONNECTIONS OR HOLDOWNS THAT EXCEED THE NOMINAL WALL THICKNESS. 15. TYPICAL TOP PLATE SPLICE: PROVIDE A MINIMUM 48" LAP W/ 16d @ 6"OC STAGGERED. REFERENCE DETAIL
- 10/S3.1. 16. TRUSS BRACING PER TRUSS MANUFACTURER, TYP.

UPPER FLOOR SHEAR WALL KEY PLAN

SCALE: NTS

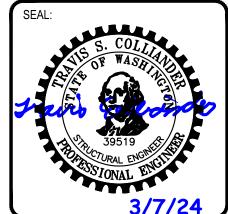
- 17. ROOF TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING CRITERIA:
- REFER TO THE STRUCTURAL GENERAL NOTES FOR STANDARD DEAD AND LIVE LOADS AND SUBMITTAL
- TRUSS SUPPLIER IS BIDDER DESIGNED AND RESPONSIBLE FOR FINAL TRUSS LAYOUT AND CONFIGURATION. TRUSS LAYOUT SHOWN IS A SUGGESTED LAYOUT. CHANGES MUST BE SUBMITTED TO THE ENGINEER-OF-RECORD THRU THE ARCHITECT WITH BEARING POINTS AND REACTIONS TO STRUCTURE. • SHADED REGION INDICATES APPROXIMATE AREA OF OVER FRAMING. TRUSS MANUFACTURER IS RESPONSIBLE
- FOR DESIGNING THE OVER FRAMING REQUIRED. TRUSSES SHALL BE DESIGNED TO SUPPORT OVER FRAMING IN ADDITION TO THE STANDARD DESIGN LOADS. • ALL GIRDER TRUSSES SHALL BE SUPPORTED BY A MINIMUM OF (3) STUDS. TRUSS MANUFACTURER TO
- SUBMIT TO ENGINEER ALL LOCATIONS WHERE REACTIONS FROM GIRDER TRUSSES EXCEED 10,000 LBS. FOR REVIEW OF COLUMN SUPPORT CAPACITY.
- ALL MULTIPLE STUDS UNDER HIP MASTER AND GIRDER TRUSS ENDS TO CONTINUE TO FOUNDATION. • PROVIDE SIMPSON H2.5A HURRICANE TIES AT ALL ROOF TRUSSES AND ROOF JOISTS, TYP.

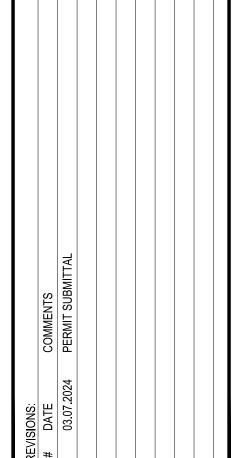


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SHEET TITLE:

STRUCTURAL **UPPER FLOOR** FRAMING PLAN

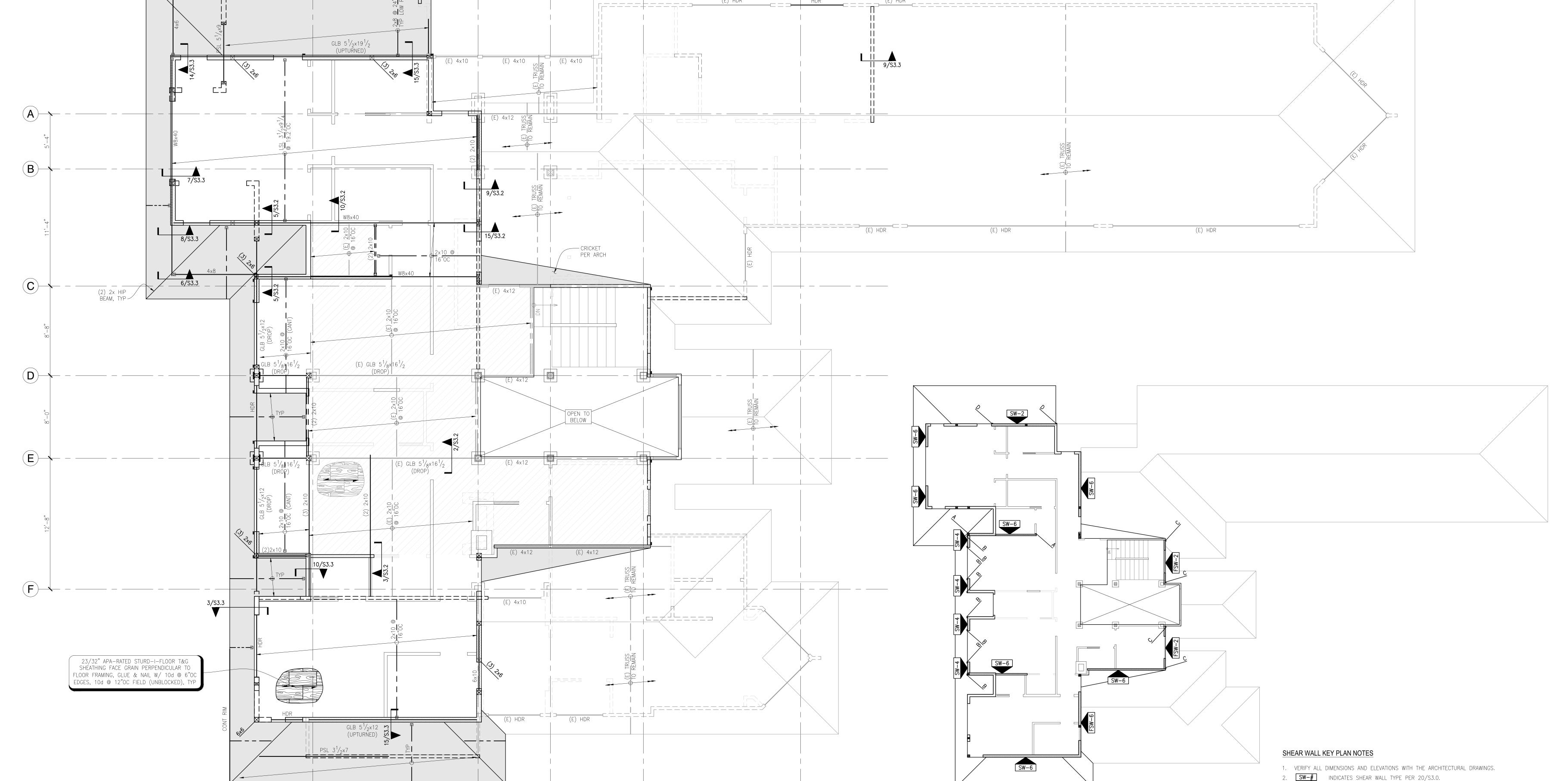
SHEET NUMBER:

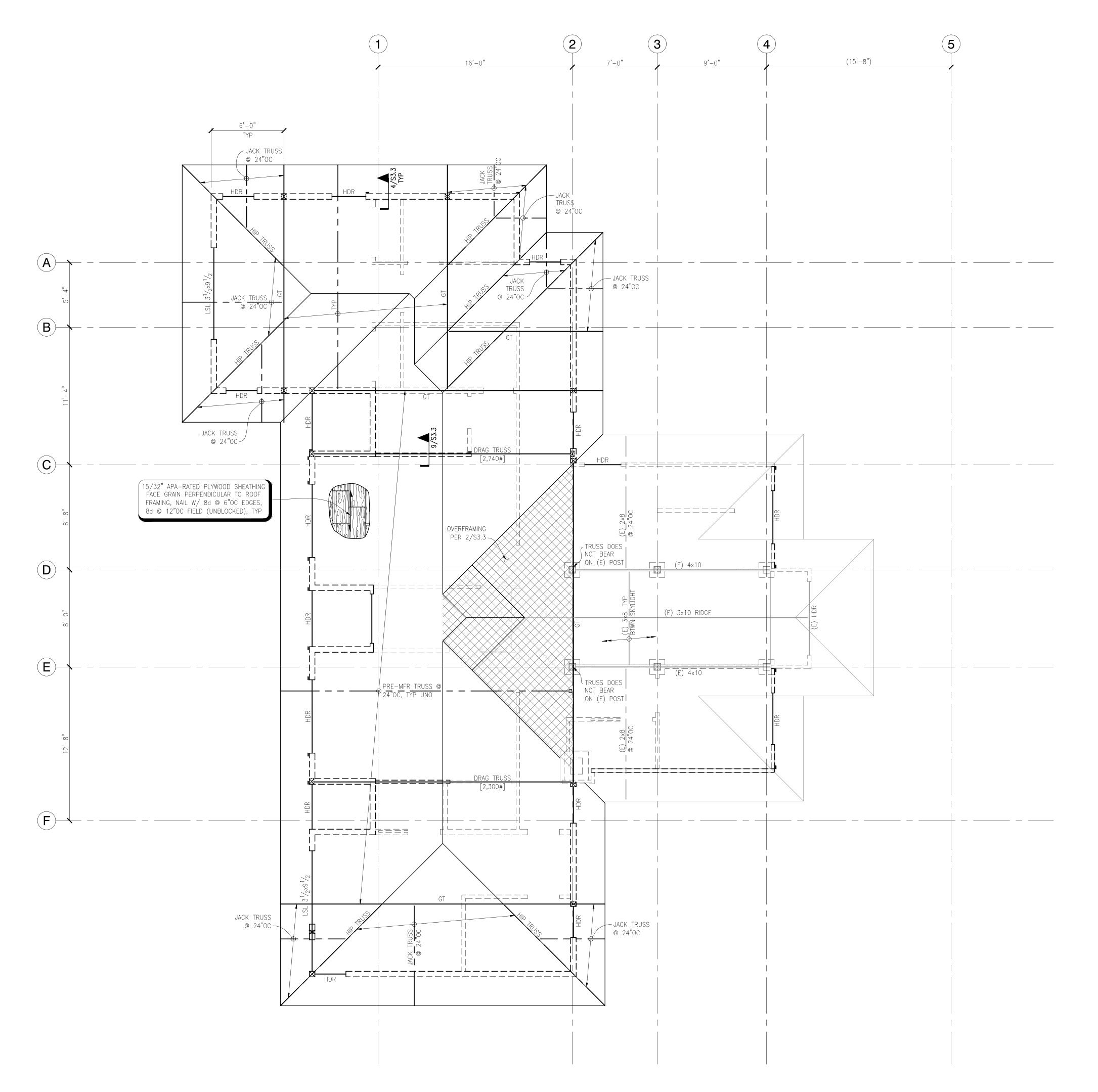
INDICATES TYPICAL HOLDOWN PER 17/S3.0 OR TIEDOWN PER 6/S3.0.

4. CONTRACTOR TO COORDINATE HOLDOWN ANCHOR BOLTS WITH STEEL POST BASE PLATES. 5. PROVIDE FULL HEIGHT SOLID BLOCKING OR DOUBLE JOISTS OVER SHEARWALLS AND BEARING WALLS AT REPETITIVE FRAMING MEMBERS. AT SHEARWALLS AND BEARING WALLS PARALLEL TO

FRAMING, ALIGN (1) JOIST OVER WALL (ADDITIONAL JOISTS MAY BE REQUIRED).

S 2.2





ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

HILLION (PROJECT OF TAXABLE)

O' 2' 4' 8'

ROOF FRAMING PLAN NOTES

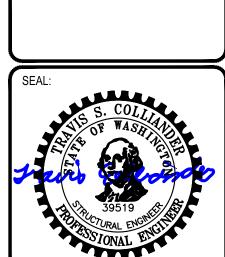
- 1. REFERENCE S1.0 FOR STRUCTURAL GENERAL NOTES, ABBREVIATIONS & SHEET INDEX.
- 2. VERIFY ALL DIMENSION AND ELEVATIONS WITH THE ARCHITECTURAL DRAWINGS.
- 3. ALL DUCTS, CHASES AND PIPE/CONDUIT OPENINGS SHALL BE PER ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL AND SPRINKLER DRAWINGS. CONTACT EOR FOR APPROVAL OF ANY OPENING NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- 4. REFERENCE ARCHITECTURAL PLANS FOR ROOF SLOPES, SCUPPERS AND DRAIN LOCATIONS.
- 5. CONTRACTOR RESPONSIBLE FOR ALL TEMPORARY SHORING.
- ALL BEAMS ARE FLUSH WITH JOISTS UNO AS "DROP" INDICATING A DROPPED BEAM.
 PROVIDE FULL HEIGHT SOLID BLOCKING OR DOUBLE JOISTS OVER SHEAR WALLS AND BEARING WALLS AT REPETITIVE FRAMING MEMBERS. AT SHEAR WALLS AND BEARING WALLS PARALLEL TO FRAMING, ALIGN (1) JOIST OVER WALL (ADDITIONAL JOISTS MAY BE REQUIRED).
- 8. PROVIDE SW-6 SHEATHING & NAILING ON EXTERIOR BUILDING, TYPICAL.
- 9. PROVIDE DOUBLE JOISTS AROUND ALL ROOF OPENINGS GREATER THAN 24"OC ONE SIDE.
- 10. REFERENCE 8/S3.1 FOR HEADER SUPPORT JAMBS. PROVIDE MINIMUM (2) 2x STUDS UNO.

 11. "HDR" INDICATES TYPICAL (2) 2x10 DROPPED HEADER WITH 6'-0" MAXIMUM SPAN.
- 12. BALLOON FRAME ALL WALLS GREATER THAN ONE LEVEL WITHOUT FLOOR OR ROOF SUPPORT. ALL EXTERIOR
- WALL STUDS 12'-0" HIGH OR GREATER, USE (2) SISTERED STUDS AT 16"OC, UNO.

 13. ALL WOOD EXPOSED TO WEATHER SHALL BE PRESSURE-TREATED PER STRUCTURAL GENERAL NOTES.
- 14. HORIZONTL STRAP TIES INDICATED ON THE SHEAR WALL PLANS ARE TO BE CENTERED OVER WALL TOP PLATE AND/OR HEADER, BLOCKING OR BEAM. CONTRACTOR SHALL COORDINATE ADDTIONAL WALL FURRING REQUIRED AT BEAMS AND POSTS WITH CONNECTIONS OR HOLDOWNS THAT EXCEED THE NOMINAL WALL THICKNESS.
- 15. TYPICAL TOP PLATE SPLICE: PROVIDE A MINIMUM 48" LAP W/ 16d @ 6"OC STAGGERED. REFERENCE DETAIL 10/S3.1.
- 16. TRUSS BRACING PER TRUSS MANUFACTURER, TYP.
- 17. ROOF TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING CRITERIA:
- REFER TO THE STRUCTURAL GENERAL NOTES FOR STANDARD DEAD AND LIVE LOADS AND SUBMITTAL INFORMATION.
- TRUSS SUPPLIER IS BIDDER DESIGNED AND RESPONSIBLE FOR FINAL TRUSS LAYOUT AND CONFIGURATION.
 TRUSS LAYOUT SHOWN IS A SUGGESTED LAYOUT. CHANGES MUST BE SUBMITTED TO THE
- ENGINEER-OF-RECORD THRU THE ARCHITECT WITH BEARING POINTS AND REACTIONS TO STRUCTURE.
 SHADED REGION INDICATES APPROXIMATE AREA OF OVER FRAMING. TRUSS MANUFACTURER IS RESPONSIBLE FOR DESIGNING THE OVER FRAMING REQUIRED. TRUSSES SHALL BE DESIGNED TO SUPPORT OVER FRAMING IN ADDITION TO THE STANDARD DESIGN LOADS.
- ALL GIRDER TRUSSES SHALL BE SUPPORTED BY A MINIMUM OF (3) STUDS. TRUSS MANUFACTURER TO SUBMIT TO ENGINEER ALL LOCATIONS WHERE REACTIONS FROM GIRDER TRUSSES EXCEED 10,000 LBS. FOR REVIEW OF COLUMN SUPPORT CAPACITY.
- ALL MULTIPLE STUDS UNDER HIP MASTER AND GIRDER TRUSS ENDS TO CONTINUE TO FOUNDATION.
- PROVIDE SIMPSON H2.5A HURRICANE TIES AT ALL ROOF TRUSSES AND ROOF JOISTS, TYP.
- [XXXX#] INDICATES SHEAR TRANSFER LOAD IN ROOF TRUSS TO BE LOCATED ABOVE SHEAR WALLS TRUSS MANUFACTURER SHALL DESIGN THESE TRUSSES FOR THE LATERAL LOAD SPECIFIED ON PLAN, IN BRACKETS, IN ADDITION TO THE DESIGN DEAD AND LIVE LOADS.

E ENGINEERS INC





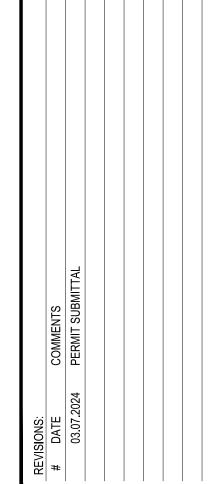
SE A 98040

REMODEL 3261 67TH AVE SE MERCER ISLAND, WA 98

PROJECT #: 24-00

DRAWN BY: BGJ

REVIEWED BY: MWE



SHEET TI

STRUCTURAL ROOF FRAMING PLAN

SHEET NUMBER:

S 2.3

NOTE: VERTICAL STEM WALL REINFORCING NOT SHOWN FOR CLARITY.

NEW FOUNDATION CONNECTION TO EXISTING SCALE: NTS

(29) 16d

(33) 16d

FLR-TO-FLR HOLDOWN STRAP SCHEDULE

	TIE-DOWN STRAP SCHEDULE								
		ALTERNATE							
MARK	STRAP	MINIMUM END LENGTH	NAILING REQUIRED AT EACH END LENGTH	NAIL SPACING	ALLOWABLE UPLIFT (LBS)	STRAP	CLEAR SPAN		
А	CMST14	9"	(8) 16d	13/4"	1569	CS16	13"		
В	CMST14	14"	(13) 16d	13/4"	2550	MSTC40	16"		
С	CMST14	19"	(20) 16d	13/4"	3924	MSTC52	16"		

5690

MSTC66 16"

N/A N/A

-EQUAL NUMBER OF

SPECIFIED NAILS AT

- CONTINUOUS RIM OR ADD

SOLID BLKG x 16"LONG

MIN AT STRAP; CTRD ON

SCALE: NTS

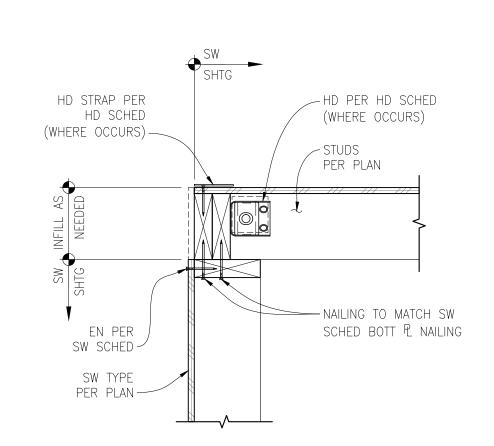
POST (SEE NOTE 4)

-BUNDLED (2) 2x MIN,

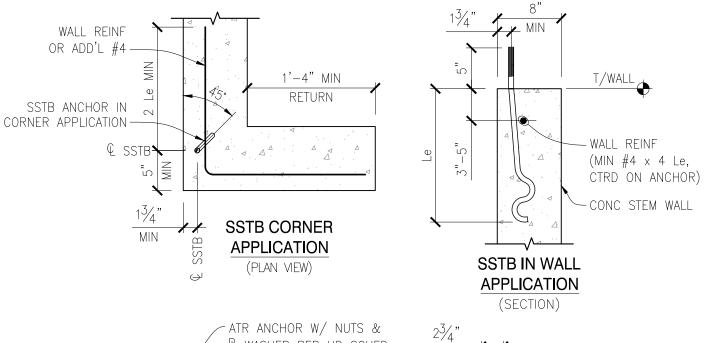
FULL HEIGHT

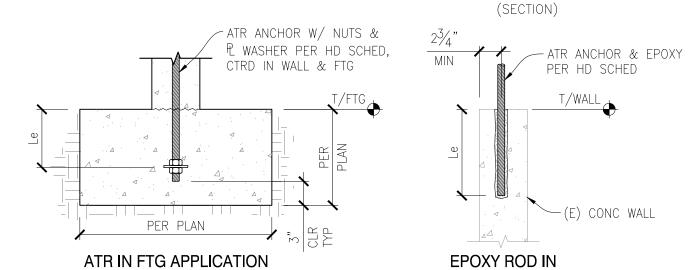
- 1. FOLLOW ALL SIMPSON STRONG-TIE GUIDELINES NECESSARY TO ACHIEVE FULL ICC DESIGN VALUES.
- 2. STRAP MAY BE INSTALLED OVER OR UNDERNEATH PLYWOOD.
- 3. EDGE NAIL PLYWOOD TO STRAPPED POST. 4. WHERE STRAPS OCCUR OVER FLOOR BEAM,
- SEE 3/S3.2. 5. ADDED BLOCKING MAY BE ELIMINATED WHERE
- FLOOR FRAMING IS DIRECTLY BETWEEN POSTS.
- 6. # INDICATES FLOOR-TO-FLOOR STRAP ON PLAN.
- 7. BASED ON SIMPSON CATALOG 2021-2023.

SCALE: NTS



PLAN VIEW - SHEAR WALL HOLDOWNS AT CORNER SCALE: NTS

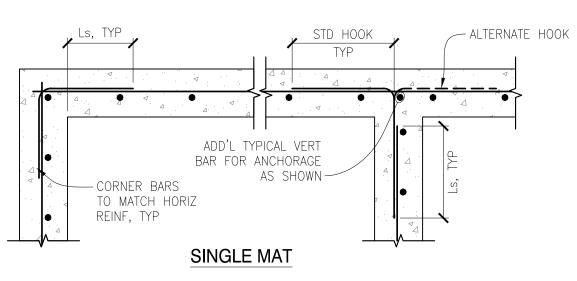


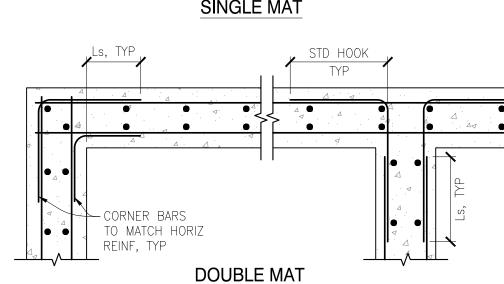


(SECTION) NOTE: Le = EMBED LENGTH PER HD SCHED

(SECTION)

TYPICAL HOLDOWN ANCHOR INSTALLATION SCALE: NTS





NOTES:

1. MEMBER SIZE & REINFORCING PER PLAN.

HD PER PLAN —

HD ANCHOR

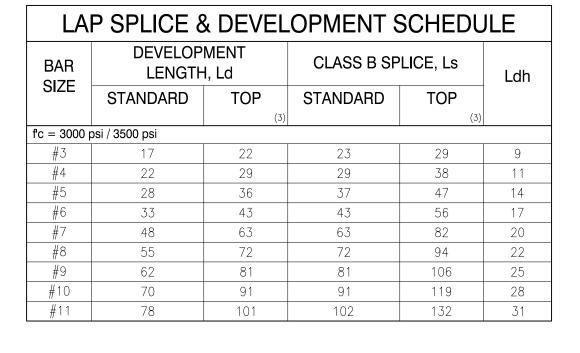
PER SCHED -

PT SILL

P/BOTT & AB

PER SW SCHED -

TYPICAL CONCRETE MEMBER INTERSECTIONS SCALE: NTS



I. VALUES FOR UNCOATED REINFORCING AND NORMAL WEIGHT CONCRETE WITH CLEAR SPACING > db, CLEAR COVER > db AND MINIMUM STIRRUPS OR TIES THROUGHOUT Ld OR CLEAR SPACING > 2db AND CLEAR COVER > db. 2. DEVELOP ALL REINFORCING IN STRUCTURAL SLABS WITH MINIMUM DEVELOPMENT LENGTH Ld.

3. TOP BAR = HORIZONTAL BAR WITH MORE THAN 12" OF FRESH CONCRETE BELOW OR AS NOTED ON DOCUMENTS AS "TOP BAR". 4. UNO, ALL LAPS SHALL BE MINIMUM CLASS B.

5. ALL TABULATED VALUES ARE IN INCHES.

6. Ldh = HOOKED BAR DEVELOPMENT LENGTH.

TYPICAL LAP SPLICE & DEVELOPMENT LENGTH SCHEDULE

⁵/₈"øx5" TITEN HD —

PLINTH

SCALE: NTS

-POST PER PLAN

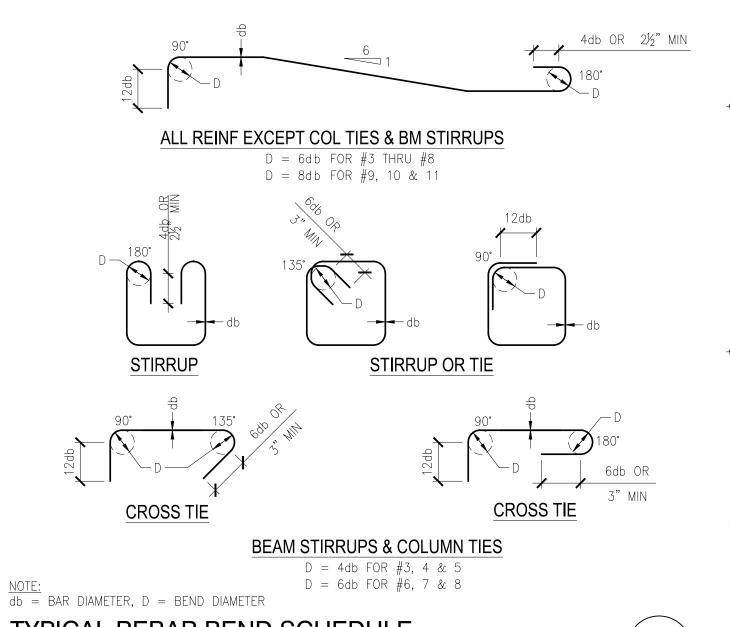
PLINTH EDGES

--#4 HAIRPINS x 18"│

-BLKG PER STRUCT DETAIL;

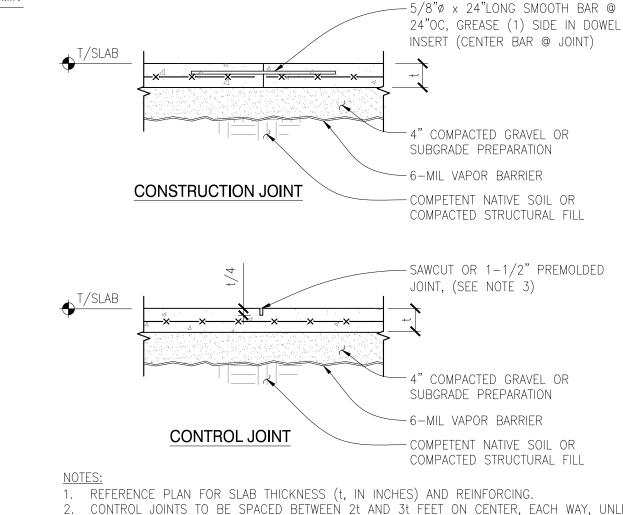
VENTING PER ARCH

(2) EACH WAY



TYPICAL REBAR BEND SCHEDULE

SCALE: NTS



2. CONTROL JOINTS TO BE SPACED BETWEEN 2t AND 3t FEET ON CENTER, EACH WAY, UNLESS NOTED OTHERWISE. RATIO OF LONG, SIDE TO SHORT SIDE OF ADJACENT JOINTS SHALL NOT EXCEED 1:1.25. 3. USE "SOFTCUT SAW" AS SOON AS POSSIBLE WITHOUT CAUSING RAVELING OF CONCRETE EDGES. SAWCUT ALONG SHORT DIRECTION OF POUR FIRST.

PT SILL PL W/ AB PER PLAN —

W/ (1) ADDT'L #4 HORIZ —

SOG & REINF PER PLAN —

#4x18" HAMMER SET

W/ 4" EMBED @ 24"OC —

SUBGRADE PER 5/S3.0 —

SCALE: NTS

CONC CURB WHERE OCCURS

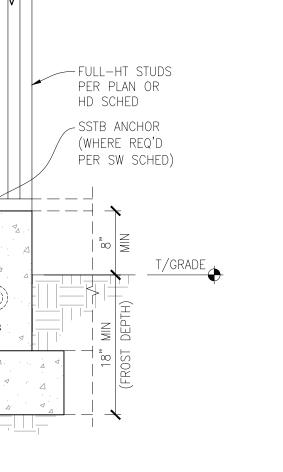
TYPICAL SLAB-ON-GRADE JOINT DETAILS SCALE: NTS

- WALL FRAMING PER PLAN

— COLD JOINT WHERE OCCURS

— STEM WALL SIZE &

REINF PER PLAN



SHEAR WALL HOLDOWN CONNECTION (NO RIM)

-SOG PER PLAN STEM WALL SIZE & REINF PER PLAN-— CONC FTG & REINF PER PLAN FTG SIZE & REINF PER PLAN -— COMPETENT NATIVE STRUCTURAL FILL

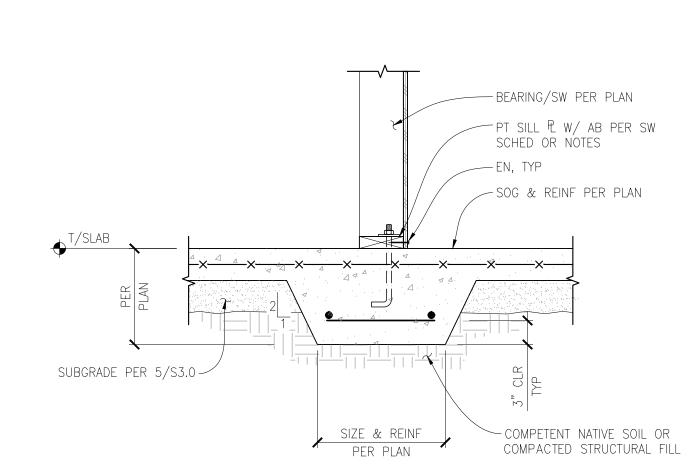
SCALE: N.T.S.

ROOF FRAMING

PER PLANS —

FASTEN ALL SIMPSON HARDWARE PER MFR SPECIFICATIONS.





TYPICAL INTERIOR THICKENED SLAB FOOTING AT BEARING / SHEAR WALL



→ STAGGER AB @ -PROVIDE (1) AB EA SIDE DBL-SIDED SW WHERE SILL PLATE IS BORED, CUT OR NOTCHED MORE THAN 1/3 THE PLATE WIDTH -SW SHTG, TYP ─ A MIN OF (2) AB SHALL BE PROVIDED IN EA SILL SECTION ∕P.T. SILL ₽, TYP --- PLATE WASHER PER SCHED, TYP PLATE WASHER SCHEDULE WALL | PLATE WASHER | ALT. SIMPSON BP SIZE (NOTE 6) 2x4 3x3x0.25 BPS 5/8-3 2x6 3x4.5x0.25 BPS 5/8-6

1. L = 6" MIN, 12" MAX 2. 5/8"ø AB W/ MIN 7" EMBED TYP, SEE STUD WALL OR SHEAR WALL SCHEDULE FOR SPACINGS & EMBED. 3. SILL PLATES TO BE PRESSURE TREATED, REFER TO GENERAL NOTES FOR GALV REQUIREMENTS FOR

4. HOLES IN SILL PLATES SHALL BE A MIN 1/32" TO MAX 1/16" LARGER THAN BOLT DIAMETER. 5. HOLES, CUTS AND NOTCHES IN TREATED SILL PLATES SHALL BE COATED W/ FIELD APPLIED P.T. LIQUID. 6. BPS BEARING PLATES W/ SLOTTED HOLES SHALL BE PLACED W/ STANDARD CUT WASHER & NUT.

PLAN VIEW -TYPICAL ANCHOR BOLT INSTALLATION SCALE: NTS

	HOLDOWN SCHEDULE (HF-WIND)							
MARK	RK MODEL #[1] ALLOWABLE UPLIFT (LBS) MIN		MIN END	STUD	CONCRETE			
MICHINIX		MID WALL	CORNER	END WALL		FASTENERS	ANCHOR [3]	
2	HDU2-SDS2.5		2215		(2) 2x	(6) ½"øx2½" SDS	5/8"ø ATR W/ 4"EMBED EPOXY @ (E) & PER NOTE 4 @ (N)	
11	HDU11-SDS2.5		8030		(4) 2x OR 6x	$(30) \frac{1}{4}$ "øx2 $\frac{1}{2}$ " SDS	1"ø ATR W/ 9" EMBED [4]	

SCALE: NTS

(E) CONC WALL

WALL APPLICATION

1. HOLDOWNS SPECIFIED ARE AS MANUFACTURED BY SIMPSON STRONG-TIE CO. INC.; ACCEPTABLE EQUIVALENT PRODUCT SUBSTITUTIONS ARE AVAILABLE FROM OTHER MANUFACTURERS WITH EOR APPROVAL. FOLLOW ALL MANUFACTURER GUIDELINES NECESSARY TO ACHIEVE FULL ICC DESIGN VALUES.

2. REFERENCE PLANS FOR ADDITIONAL STUD REQUIREMENTS WHERE OCCURS. 3. HOLDOWN SHALL BE INSTALLED TIGHT TO STUDS WITHOUT FILLERS OR NOTCHING. DO NOT BEND ANCHORS. 4. PROVIDE 1/4"x3"SQ PLATE WASHER IN BETWEEN STANDARD DOUBLE NUTS. EMBED LENGTH (Le) EQUAL TO

TOP OF CONCRETE DOWN TO TOP OF PLATE WASHER.

6. CONTRACTOR TO COORDINATE WHERE "RJ" HOLDOWNS ARE REQUIRED. 7. BASED ON SIMPSON CATALOG 2021-2023

HOLDOWN SCHEDULE (8" MIN STEM WALL)

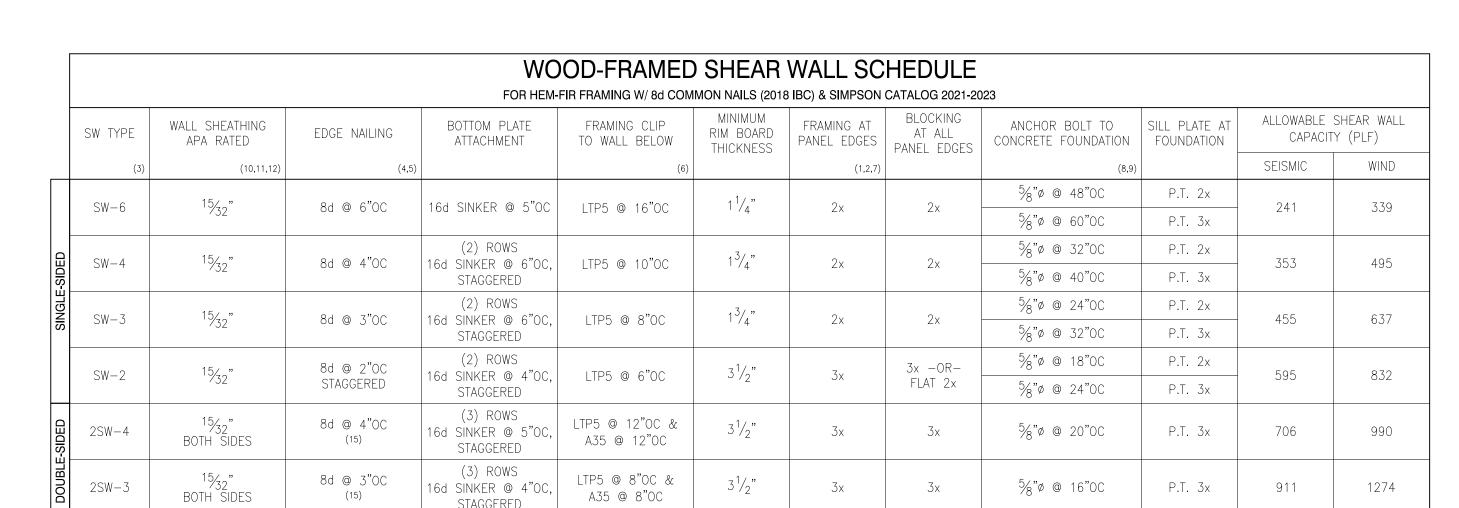
T/ROOF SHTG FULL-HT END STUDS PER HD SCHED, (2) MIN @ WALL ENDS - WALL FRAMING EN PER SW SCHED~ PER PLAN FIELD NAILING PER SW SCHED -BLKG @ PANEL EDGES -SHTG PER SW SCHED -↑ T/STEM WALL (SOG BEYOND) SOG BEYOND (WHERE OCCURS) AB PER SW SCHED — STEM WALL SIZE & (MIN $\frac{5}{8}$ "ø @ 48"OC, REINF PER PLAN EMBED 7")-- FTG SIZE & REINF PER PLAN TYPICAL SHEAR WALL ELEVATION SCALE: NTS

FLR SHTG PER PLAN ~ DBL TOP PL — HEADER , 3x BLKG ABOVE & BELOW WINDOW OPNG-HD & END STUDS PER HD

WHERE OCCURS - TIEDOWN STRAP PER PLAN & SCHED, TYP SOLID BLKG T&B; EXTEND STRAP 9" MIN PAST OPNG — EN PER SW SCHED, TYP — SHTG & NAILING SCHED, TYP — PER SW SCHED PT SILL PL - STEM WALL OR FTG PER PLAN — ANCHOR PER SW SCHED, TYP

1. STRAP ABOVE AND BELOW WINDOWS TO BE ON BOTH SIDES AT DOUBLE-SIDED SHEAR WALLS. 2. REFERENCE 20/S3.0 FOR ADDITIONAL INFO.

TYPICAL FTAO SHEAR WALL ELEVATION SCALE: NTS



1. ALL NAILS ARE COMMON, UNO. REFERENCE GENERAL STRUCTURAL NOTES FOR NAIL DIAMETER AND LENGTH.

2. REFERENCE SHEAR WALL KEY DETAIL FOR DESCRIPTION OF TERMS. 3. PROVIDE SHEAR WALL SHEATHING AND NAILING FOR ENTIRE LENGTH OF THE WALLS INDICATED ON THE PLANS. ENDS OF SHEAR

WALLS ARE TYPICALLY AT WINDOWS, DOORWAYS OR AS SHOWN ON PLAN. 4. EDGE NAILING IS REQUIRED AT ALL HOLDOWN POSTS. EDGE NAILING IS REQUIRED TO EACH STUD USED IN BUILT-UP HOLDOWN

POSTS. REFERENCE HOLDOWN SCHEDULE & DETAILS FOR ADDITIONAL INFORMATION. INTERMEDIATE FRAMING TO BE 2x MINIMUM MEMBERS UNO IN SCHEDULE. ATTACH SHEATHING TO INTERMEDIATE FRAMING WITH FIELD NAILING AT 12"OC WHERE STUDS ARE SPACED AT 16"OC AND FIELD NAILING AT 6"OC WHERE STUDS ARE SPACED AT 24"OC. 6. SIMPSON STRONG-TIE "A35" MAY BE USED IN LIEU OF "LTP5." "LTP5" CLIPS SHALL BE ORIENTED LENGTHWISE (HORIZONTAL) AT PLATE TO RIM. USE 0.131° %x1 $\frac{1}{2}$ " NAILS WHERE CLIPS ARE ATTACHED DIRECTLY TO FRAMING. USE 0.131° %x2 $\frac{1}{2}$ " WHERE CLIPS ARE INSTALLED OVER SHEATHING.

7. (2) 2x STUDS NAILED TOGETHER MAY BE USED IN PLACE OF SINGLE 3x STUD. DOUBLE 2x STUDS SHALL BE SECURED TOGETHER WITH FASTENERS OF THE SAME DIAMETER AND SPACING AS THE BOTTOM PLATE ATTACHMENT PER SCHEDULE. 8. ANCHOR BOLTS SHALL BE PROVIDED WITH HOT-DIPPED GALVANIZED STEEL PLATE WASHERS PER 12/S3.0. EMBED ANCHOR BOLTS 7" MINIMUM INTO THE CONCRETE. PROVIDE AN ANCHOR BOLT AT EACH END OF EACH PLATE AND SHALL BE AT LEAST 7 TIMES

SCREWS MAY BE USED IN LIEU OF ANCHOR BOLTS AT EXISTING CONCRETE, WITH PLATE WASHER & SPACING REQUIREMENTS PER 9. PROVIDE HOT-DIPPED GALVANIZED NAILS AND CONNECTOR PLATES (FRAMING ANGLES, ETC.) AT ALL PRESSURE TREATED LUMBER.

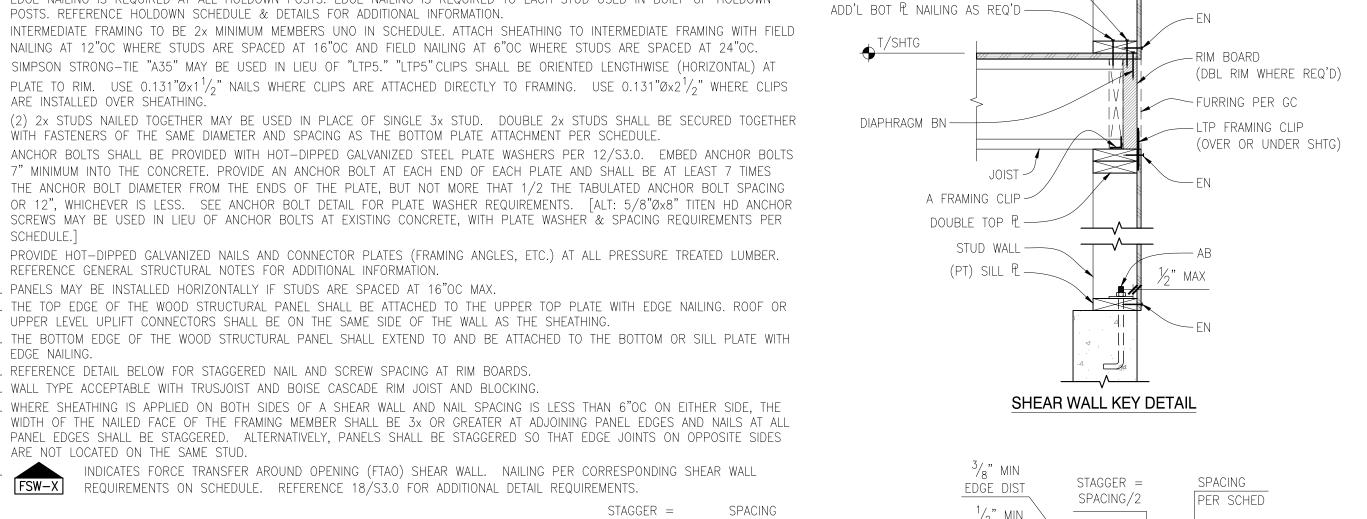
REFERENCE GENERAL STRUCTURAL NOTES FOR ADDITIONAL INFORMATION. 10. PANELS MAY BE INSTALLED HORIZONTALLY IF STUDS ARE SPACED AT 16"OC MAX.

11. THE TOP EDGE OF THE WOOD STRUCTURAL PANEL SHALL BE ATTACHED TO THE UPPER TOP PLATE WITH EDGE NAILING. ROOF OR UPPER LEVEL UPLIFT CONNECTORS SHALL BE ON THE SAME SIDE OF THE WALL AS THE SHEATHING. 12. THE BOTTOM EDGE OF THE WOOD STRUCTURAL PANEL SHALL EXTEND TO AND BE ATTACHED TO THE BOTTOM OR SILL PLATE WITH EDGE NAILING.

13. REFERENCE DETAIL BELOW FOR STAGGERED NAIL AND SCREW SPACING AT RIM BOARDS. 14. WALL TYPE ACCEPTABLE WITH TRUSJOIST AND BOISE CASCADE RIM JOIST AND BLOCKING.

15. WHERE SHEATHING IS APPLIED ON BOTH SIDES OF A SHEAR WALL AND NAIL SPACING IS LESS THAN 6"OC ON EITHER SIDE, THE WIDTH OF THE NAILED FACE OF THE FRAMING MEMBER SHALL BE 3x OR GREATER AT ADJOINING PANEL EDGES AND NAILS AT ALL PANEL EDGES SHALL BE STAGGERED. ALTERNATIVELY, PANELS SHALL BE STAGGERED SO THAT EDGE JOINTS ON OPPOSITE SIDES ARE NOT LOCATED ON THE SAME STUD.

INDICATES FORCE TRANSFER AROUND OPENING (FTAO) SHEAR WALL. NAILING PER CORRESPONDING SHEAR WALL REQUIREMENTS ON SCHEDULE. REFERENCE 18/S3.0 FOR ADDITIONAL DETAIL REQUIREMENTS. SPACING/2 PER SCHED ³/₈" MIN $\frac{3}{8}$ " MIN — P. NAILING PER SCHED

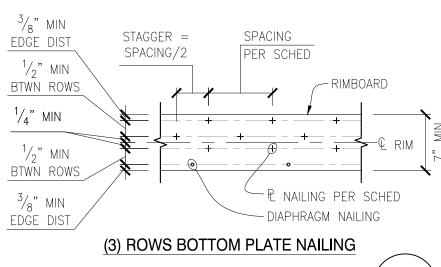


STUD WALL ——

BOTT PLATTACHMENT —

— DIAPHRAGM NAILING

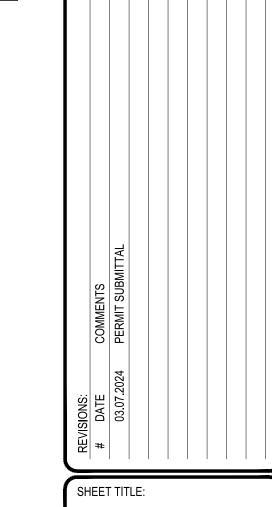
(2) ROWS BOTTOM PLATE NAILING



WALL SHTG

WOOD-FRAMED SHEAR WALL SCHEDULE

SCALE: NONE



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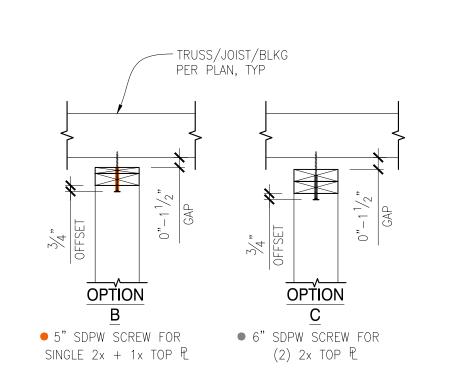
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STRUCTURAL **SECTIONS & DETAILS**

SHEET NUMBER:



<u>NOTES</u>: 1. FLOAT THE CEILING DRYWALL END AS PER

STANDARD INDUSTRY STANDARDS. 2. INSTALL SIMPSON SDPW SCREWS W/ OFFSET DRIVER BIT WITH $\frac{3}{4}$ " OFFSET TO ALLOW FOR VERTICAL MOVEMENTS UP AND DOWN.

TYPICAL INTERIOR NON-BEARING WALL TOP PLATE ANCHORAGE

SCALE: NTS

EXTERIOR/BEARING/SHEAR WALL STUDS				EXTERIOR/BEARING/SHEAR WALL STUDS			
STUD SIZE	MAX DEPTH OF EDGE CUT OR NOTCH	MIN STUD DEPTH REMAINING	STUD SIZE	MAX DIAMETER OF HOLE	MIN DEPTH REMAINING AFTER BORING		
2x4	7/8"	25/8"	2×4	13/8"	5/8" EA SIDE OF HOLE		
2x6	13/8"	41/8"	2x6	21/8"	5/8" EA SIDE OF HOLE		
2x8	13/4"	51/2"	2x8	27/8"	5/8" EA SIDE OF HOLE		

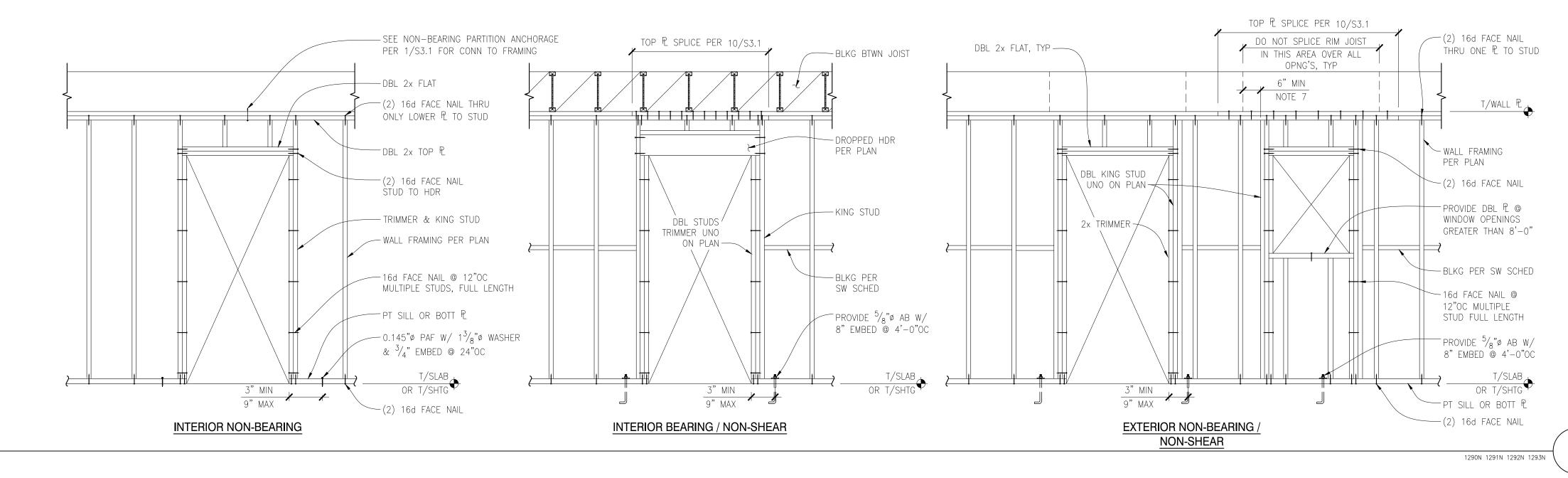
	NON-BEARING	WALL STUDS	NON-BEARING WALL STUDS			
STUD SIZE	MAX DEPTH OF EDGE CUT OR NOTCH	MIN STUD DEPTH REMAINING	STUD SIZE	MAX DIAMETER OF HOLE	MIN DEPTH REMAINING AFTER BORING	
2x4	13/8"	21/8"	2x4	2"	5/8" EA SIDE OF HOLE	
2x6	21/8"	33/8"	2x6	31/4"	5/8" EA SIDE OF HOLE	
2x8	27/8"	4 ³ / ₈ "	2x8	41/4"	5/8" EA SIDE OF HOLE	

CUTTING AND NOTCHING WOOD STUDS BORED HOLES IN WOOD STUDS

- NOTES:
 1. NO CUTTING, NOTCHING OR BORING IS ALLOWED IN SHEAR WALL HOLDOWN COMPRESSION STUDS OR PLATES.
- 2. BORINGS SHALL NOT BE MADE AT THE SAME SECTION WHERE A CUT OR NOTCH HAS BEEN MADE.
- 3. DO NOT NOTCH OR BORE MORE THAN THREE ADJACENT STUDS WITHOUT REVIEW AND APPROVAL BY EOR.

TYPICAL HOLES & NOTCHES IN WOOD STUDS

SCALE: NTS



TYPICAL WALL FRAMING DETAILS & NOTES SCALE: NTS

1. HEADERS, KING STUDS AND OTHER REFERENCES ON PLAN GOVERN OVER TYPICAL DETAILS.

6. RIM JOIST IS HEADER AT EXTERIOR AND CORRIDOR WALLS. DO NOT SPLICE OVER OPENINGS.

8. SILL PLATES TO BE PRESERVATIVE/PRESSURE TREATED WHERE IN CONTACT WITH CONCRETE

7. IF 6" OVERHANG NOT AVAILABLE, HANG RIM FROM INTERSECTING BEAM W/ SIMPSON HUC

3. REFERENCE SHEAR WALL SCHEDULE FOR CONNECTION AT TOP AND BOTTOM OF WALL.

5. ACCEPTABLE TO USE THREADED ANCHOR IN LIEU OF CAST-IN-PLACE ANCHOR BOLT.

9. REFERENCE 13/S3.0 FOR SILL PLATE CONNECTION AT PLYWOOD SHEATHING. 10. NAILING NOT SHOWN SHALL BE AS INDICATED IN TABLE 2304.10.1 OF THE IBC.

2. REFERENCE SHEAR WALL NAILING DETAIL FOR ADDITIONAL INFORMATION.

4. COORDINATE KING AND TRIM STUDS WITH HOLDOWN STUDS.

HANGER x SIZE OF RIM.

EXPOSED TO WEATHER.

NOTES:

PER PLAN, TYP BOTT PL — RIM JOIST OR - BORED HOLES ARE TO BEAM PER PLAN BE SPACED AT LEAST A (NOTE-2) — STUD WIDTH FROM A CUT OR NOTCH DBL TOP P., SPLICE PER 10/S3.1

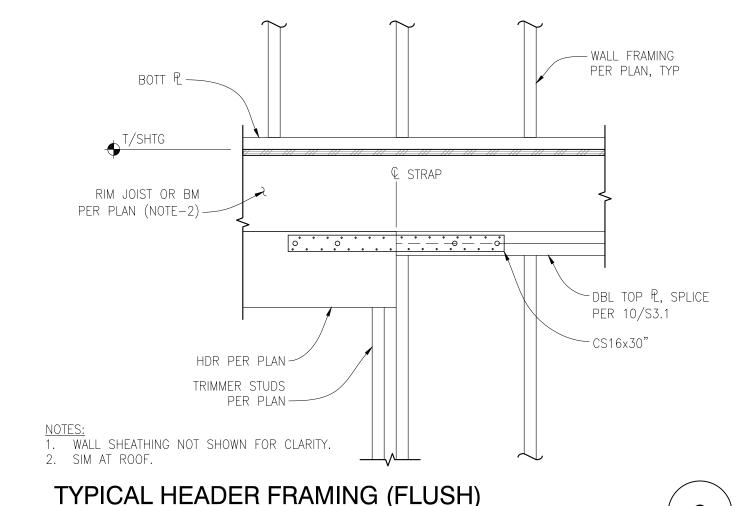
- SILL PLATES TO FOLLOW NOTES:

SAME SPACING & SIZE AS 1. WALL SHEATHING NOT SHOWN FOR CLARITY

2x BOTT PL

HDR PER PLAN ----

TYPICAL HEADER FRAMING (DROPPED)



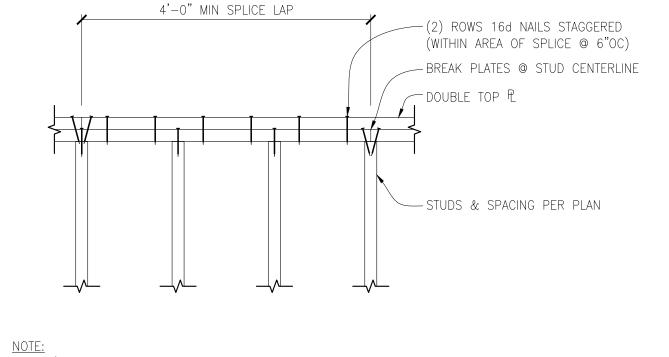
ALLOWABLE HOLES IN WOOD JOISTS & BEAMS (11 7/8") - TRUS JOIST

TYPICAL CONCRETE RETAINING WALL

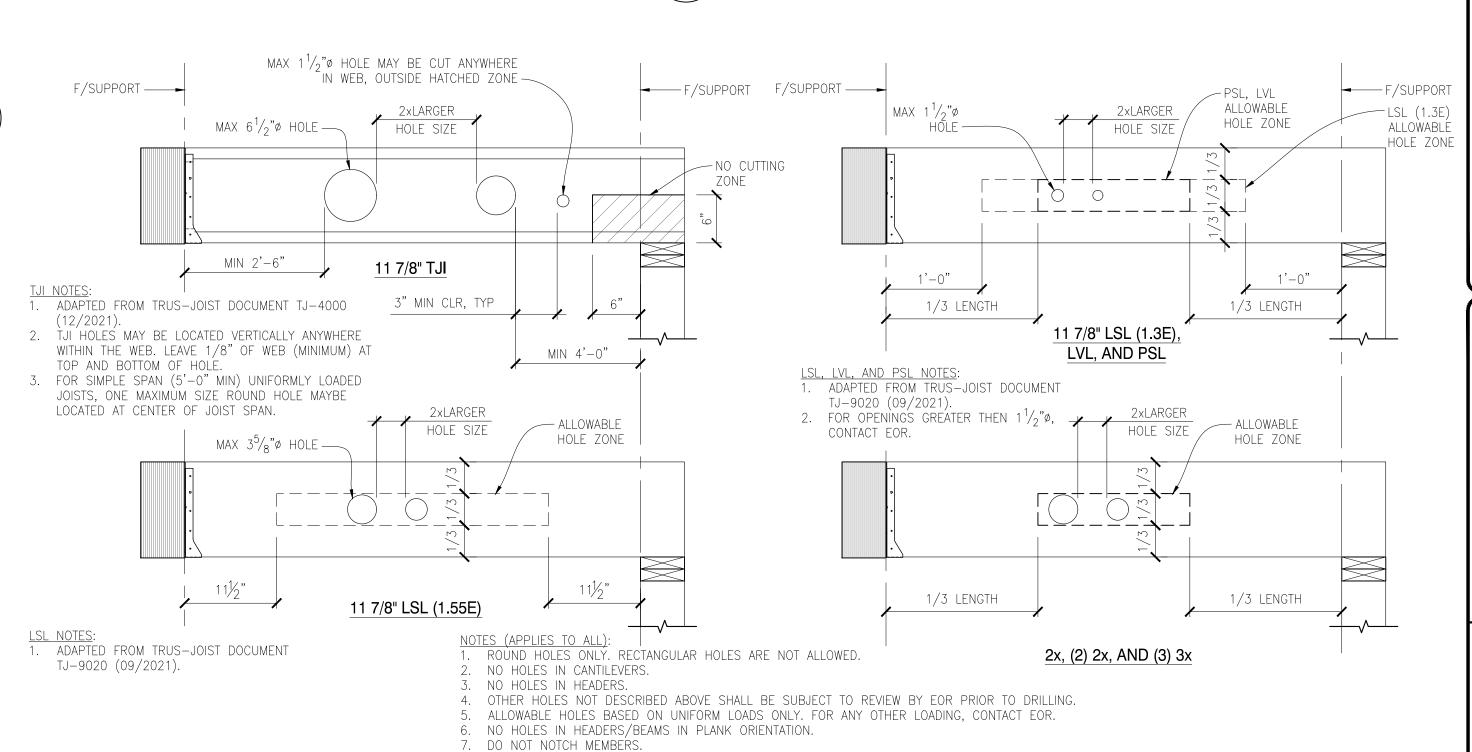
SCALE: NTS

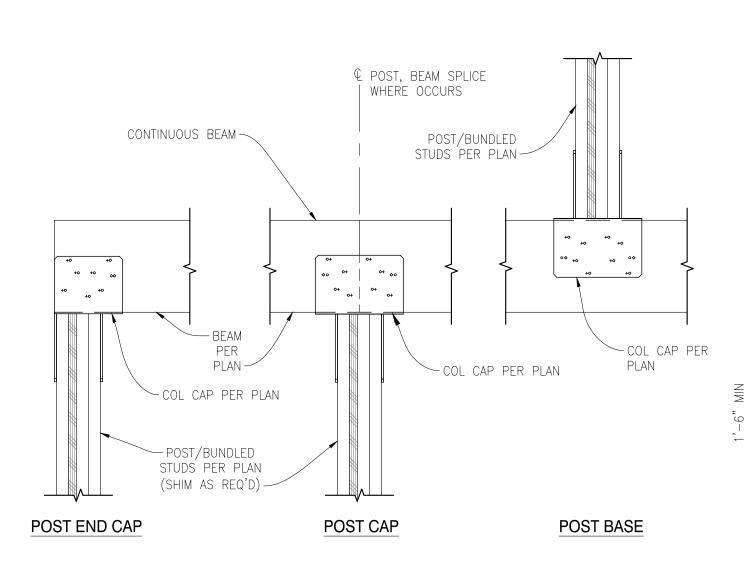
SCALE: NTS

SCALE: NTS



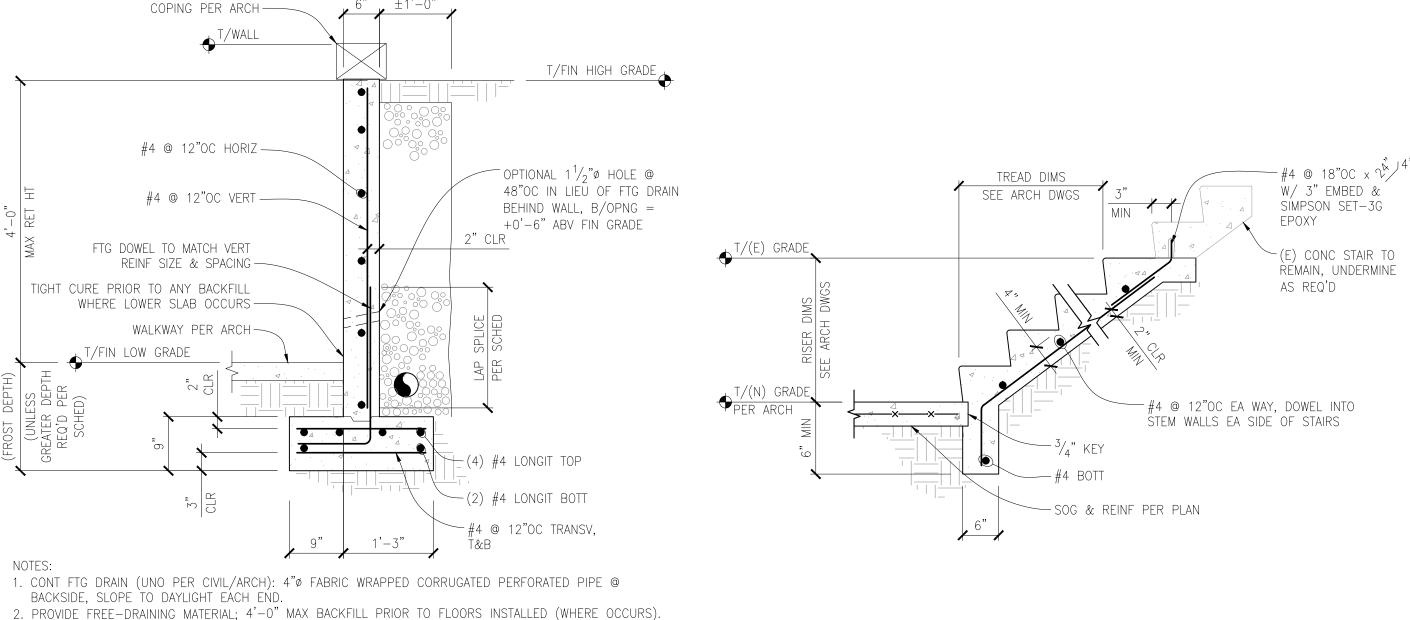
<u>NOTE:</u> FLOOR/ROOF JOISTS NOT SHOWN FOR CLARITY. TYPICAL TOP PLATE SPLICE DETAIL SCALE: NTS





TYPICAL POST TO BEAM CONNECTION SCALE: NTS

FLOOR/ROOF SHTG NOT SHOWN FOR CLARITY.



SCALE: NTS

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SHEET TITLE: STRUCTURAL **SECTIONS & DETAILS** SHEET NUMBER:

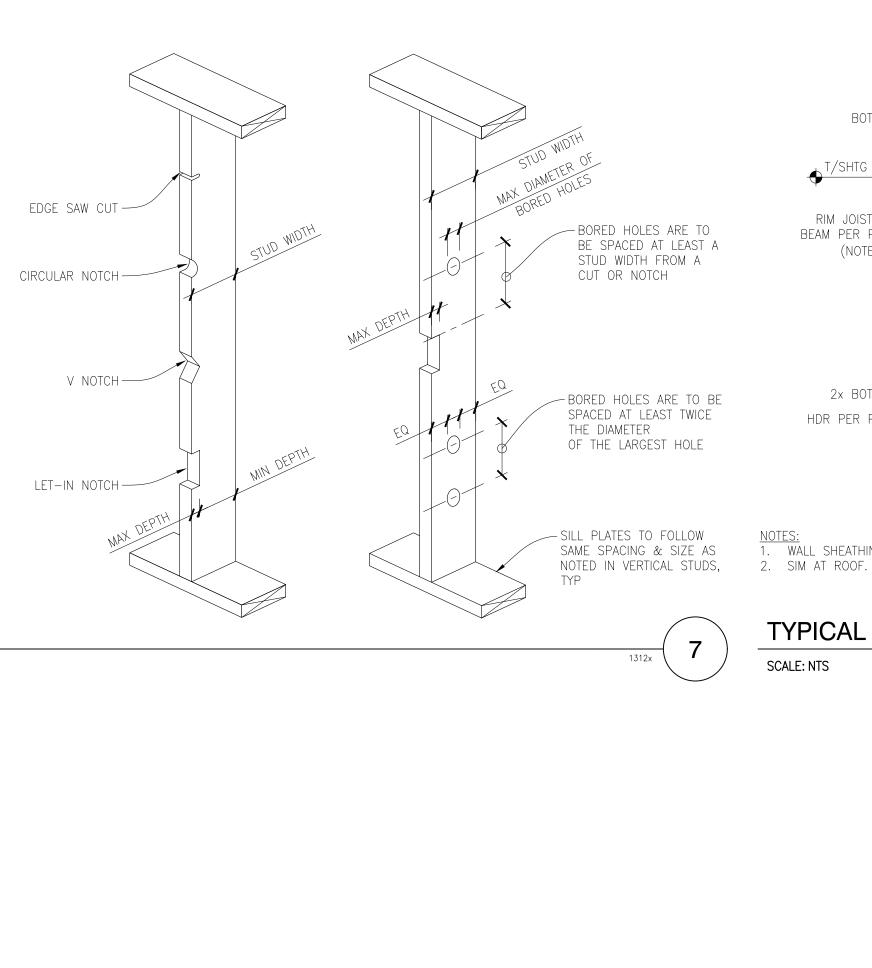
TYPICAL STAIR-ON-GRADE

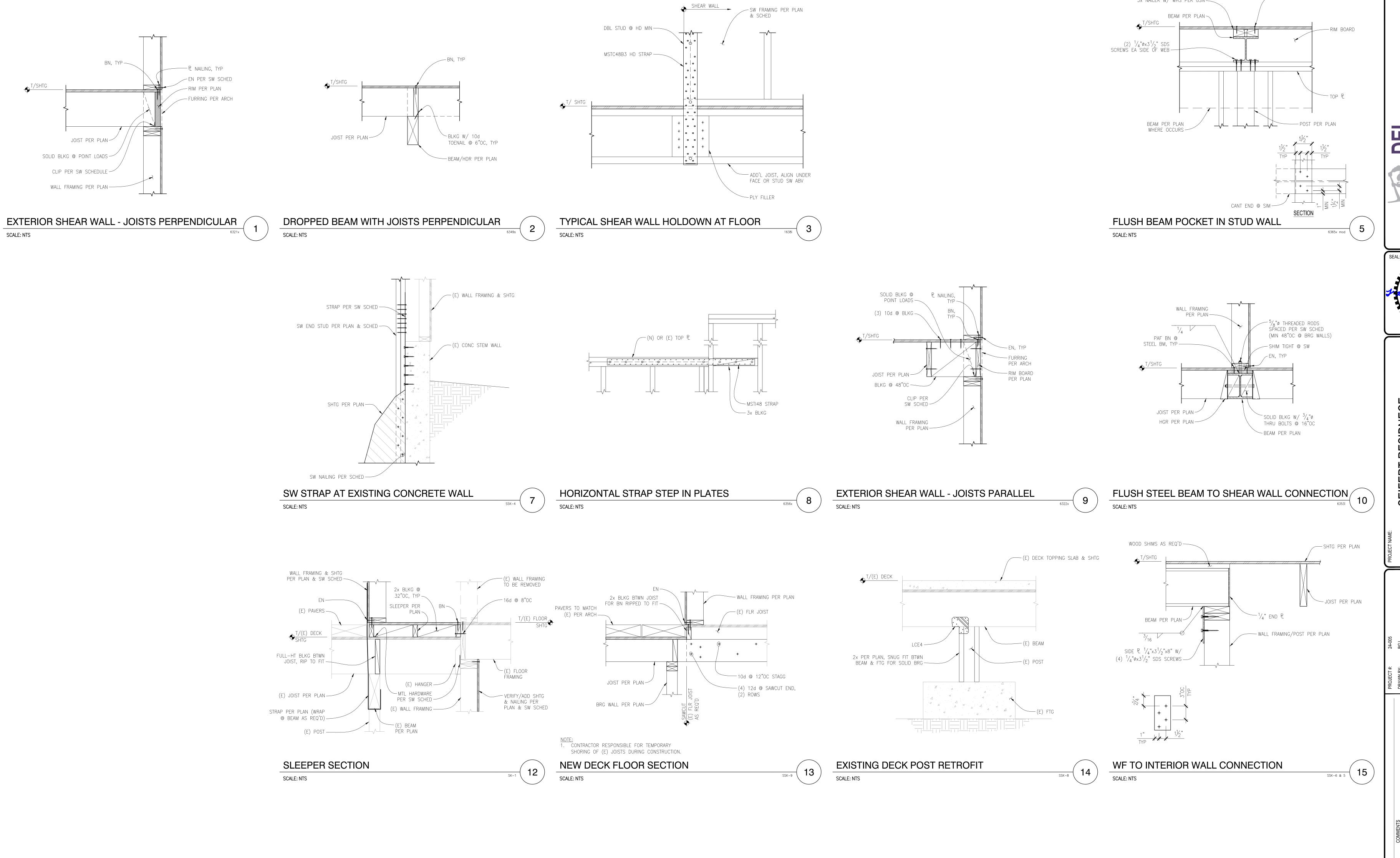
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1029 Market Street, Kirkland, WA 98033
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____BN (8d @ 6"OC MIN)

3x NAILER W/ WHS PER GSN —

SEAL:

SEAL:

WASHINGTON STORY

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SEIFERT RESIDNECE
REMODEL
3261 67TH AVE SE
MERCER ISLAND, WA 98040

FY: 24-005
BY: BGJ
ED BY: MWD

PROJECT 3
DRAWN B)
REVIEWEE

REVISIONS:

DATE COMMENTS

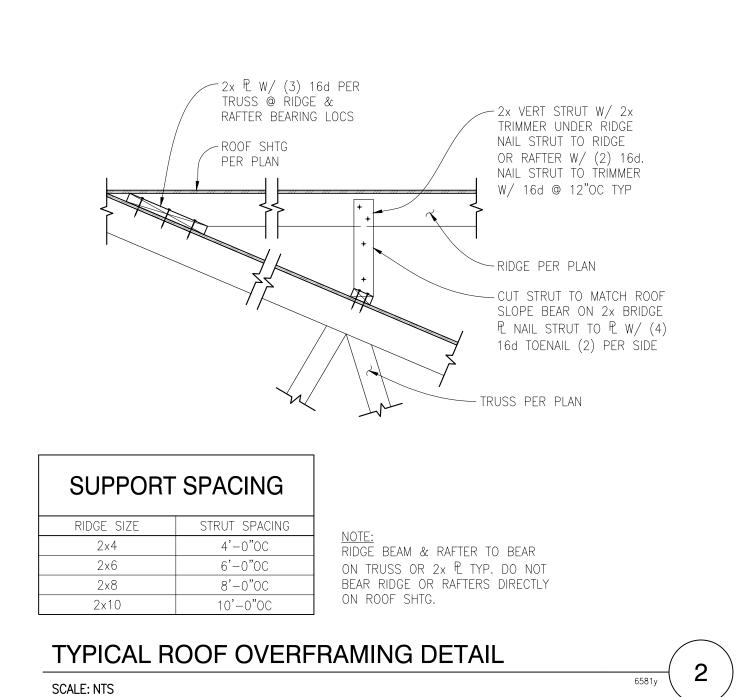
| 03.07.2024 PERMIT SUBMITTAL

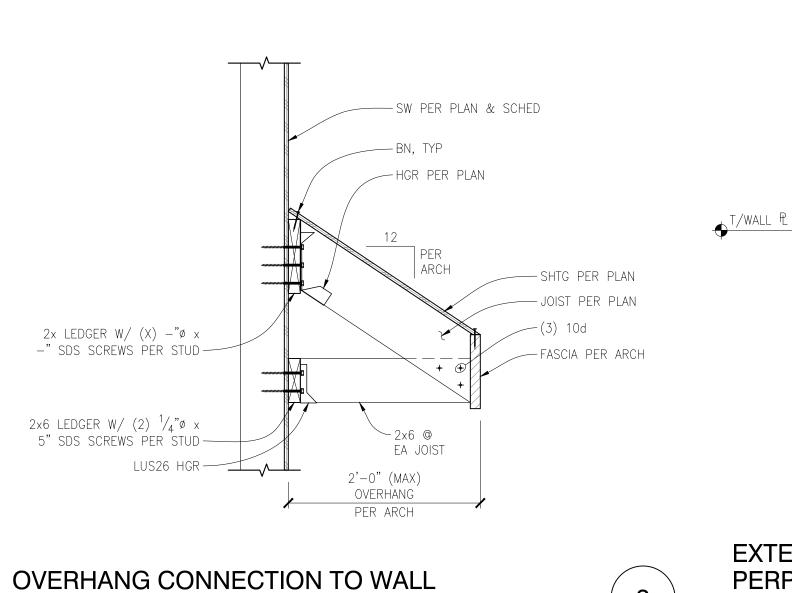
| 13.07.2024 PERMIT SUBMITTAL

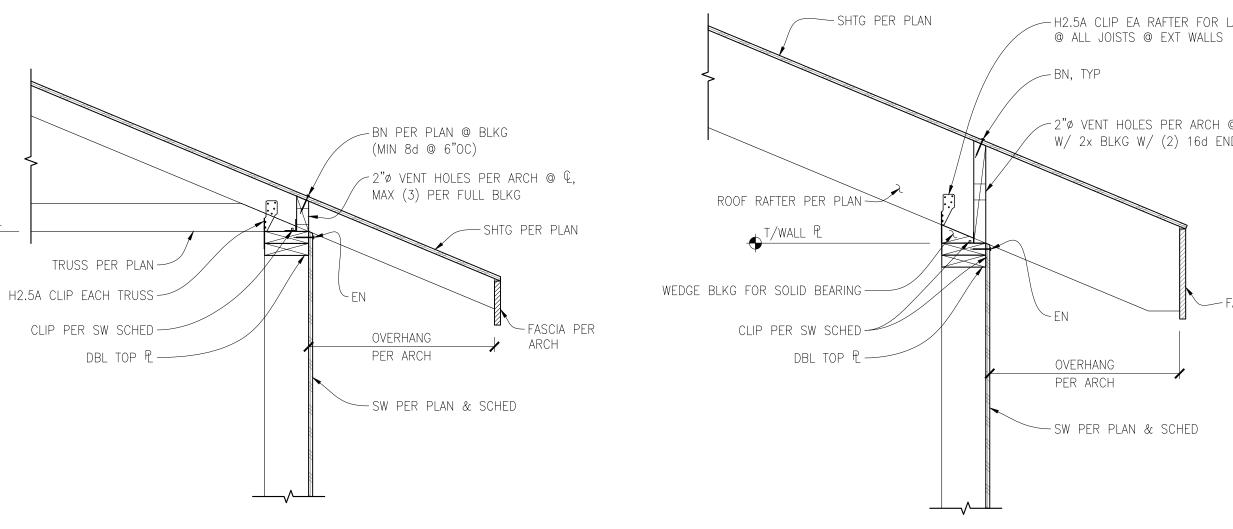
STRUCTURAL SECTIONS & DETAILS

SHEET NUMBER:

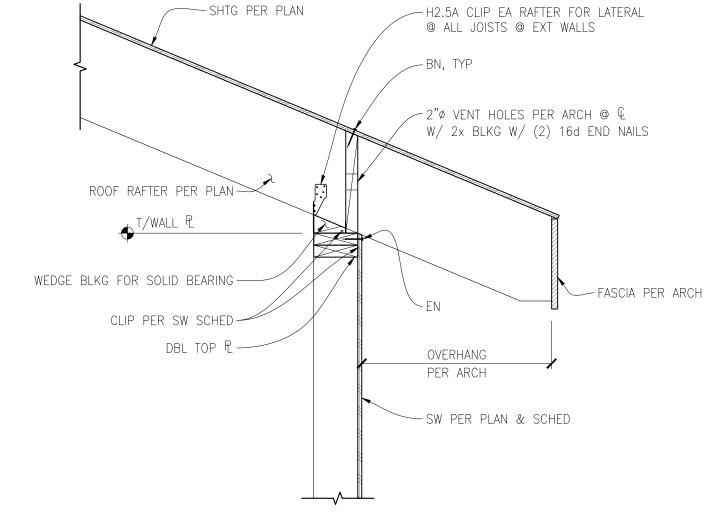
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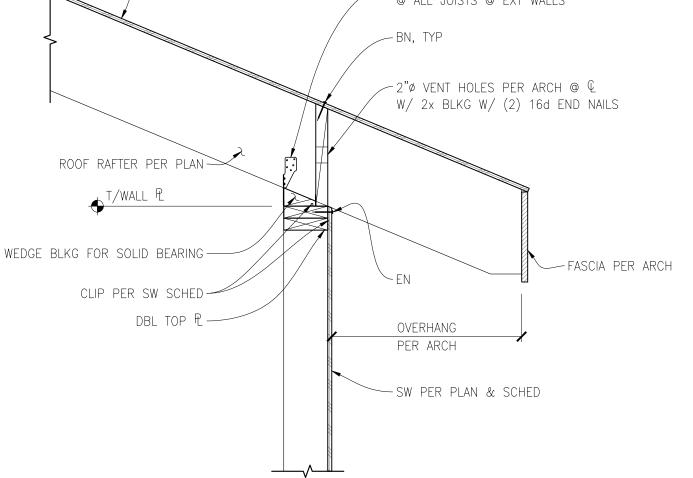


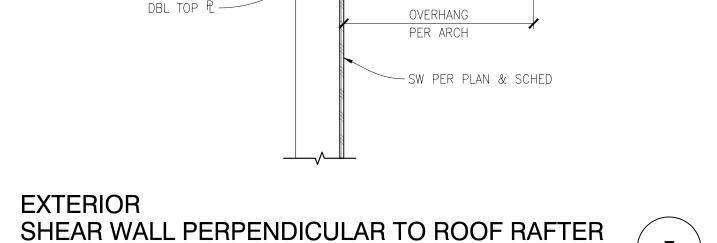


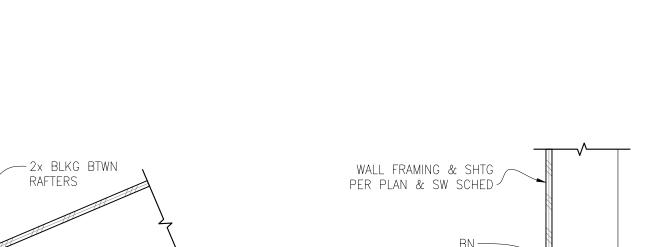


SCALE: NTS



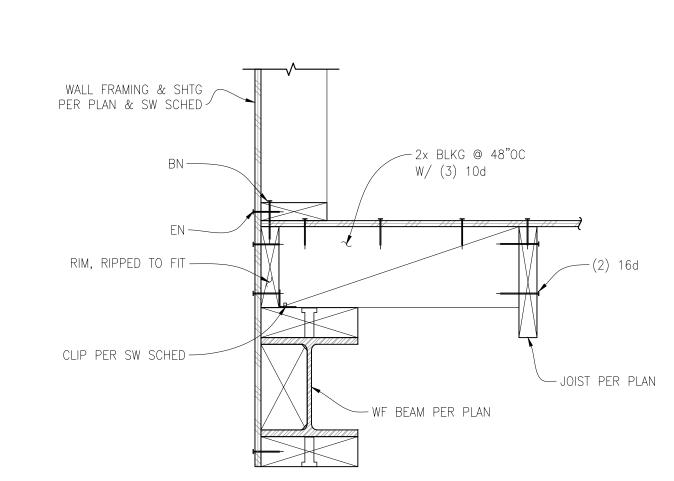


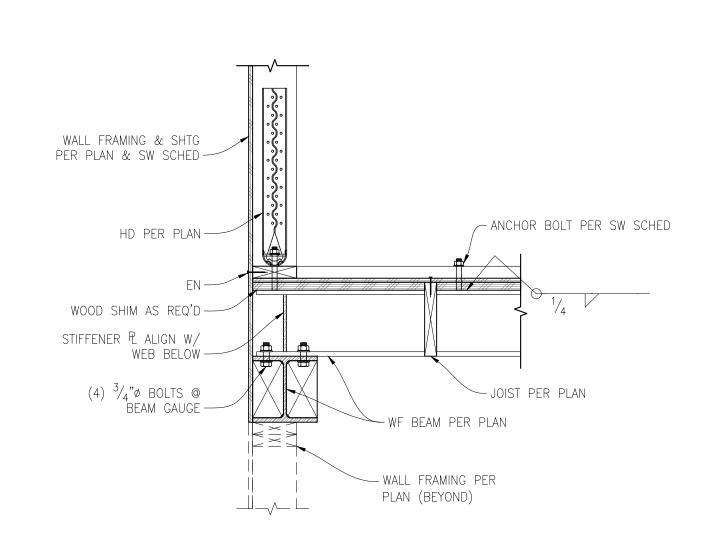




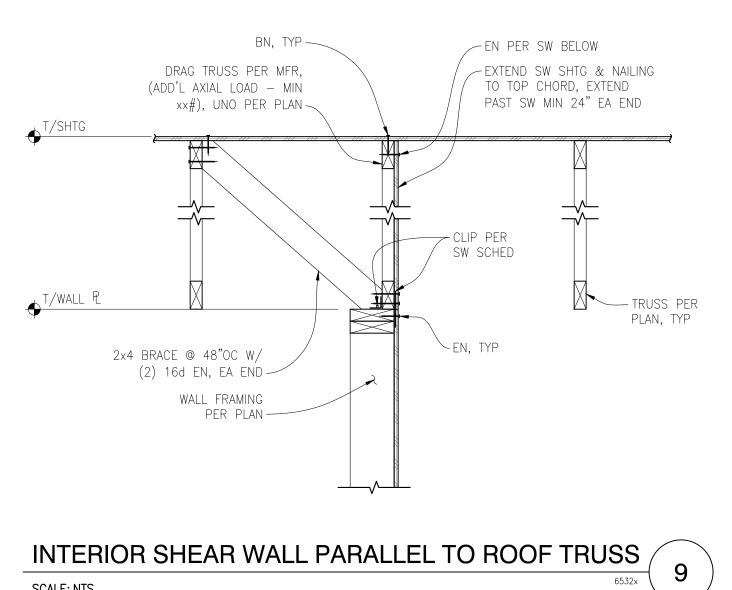
— BEAM PER PLAN (BEYOND)

HUC HGR





SCALE: NTS

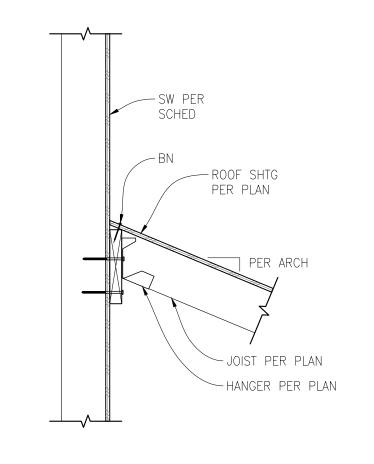


EXTERIOR SHEAR WALL

SCALE: NTS

SCALE: NTS

PERPENDICULAR TO ROOF TRUSS





WALL FRAMING & SHTG PER PLAN & SCHED



CANTILEVER (2'-6" MAX)

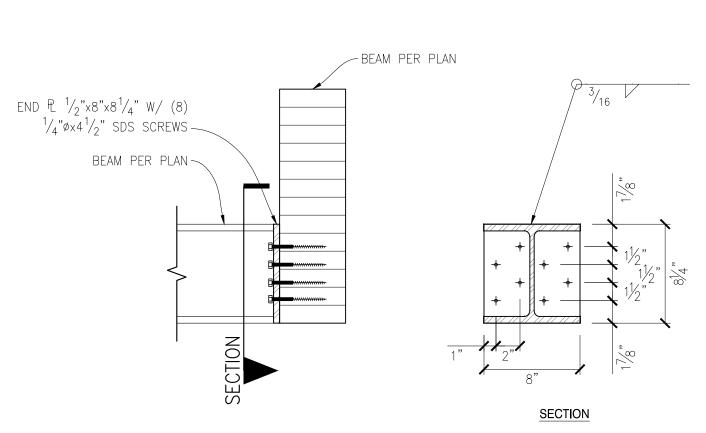
H2.5 EA RAFTER ——

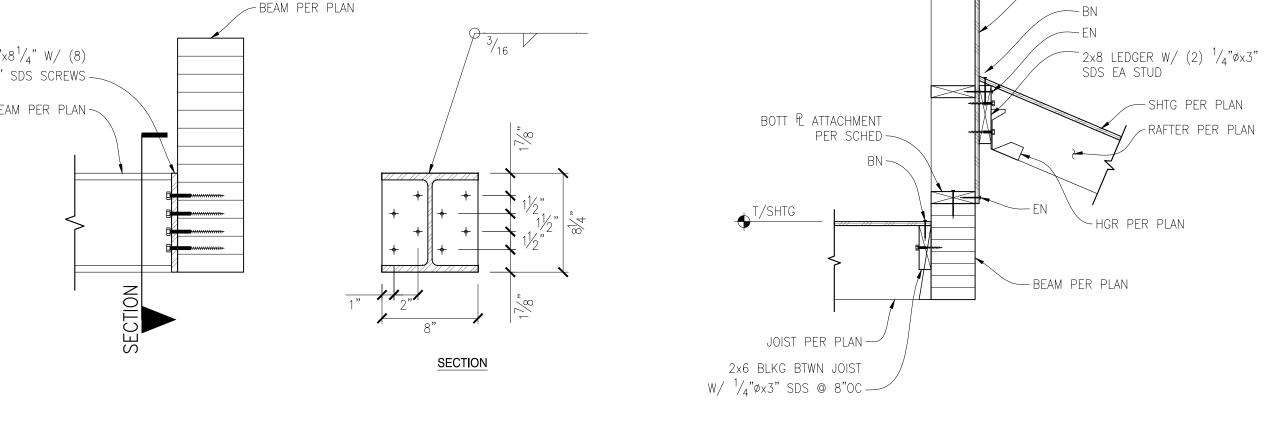
LOW ROOF FRAMING (NOTCHED AS REQ'D) & SHTG PER PLAN —

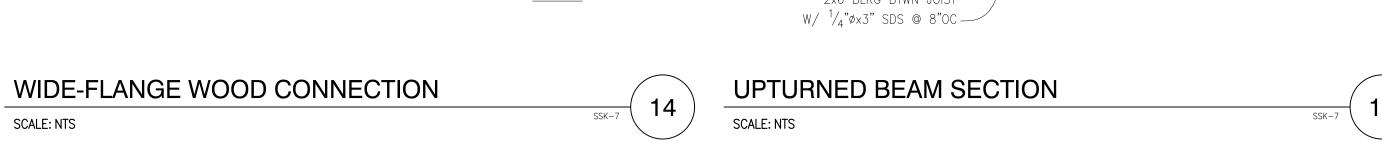
FASCIA PER

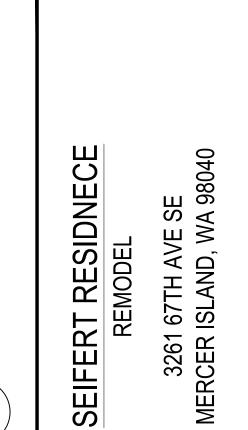
ARCH —





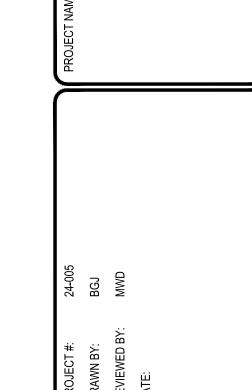


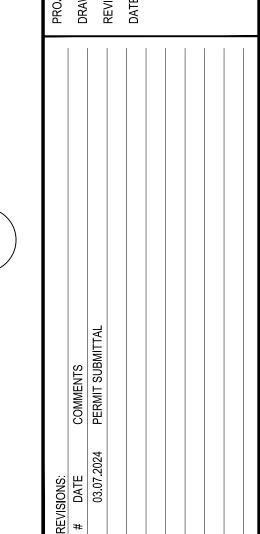




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