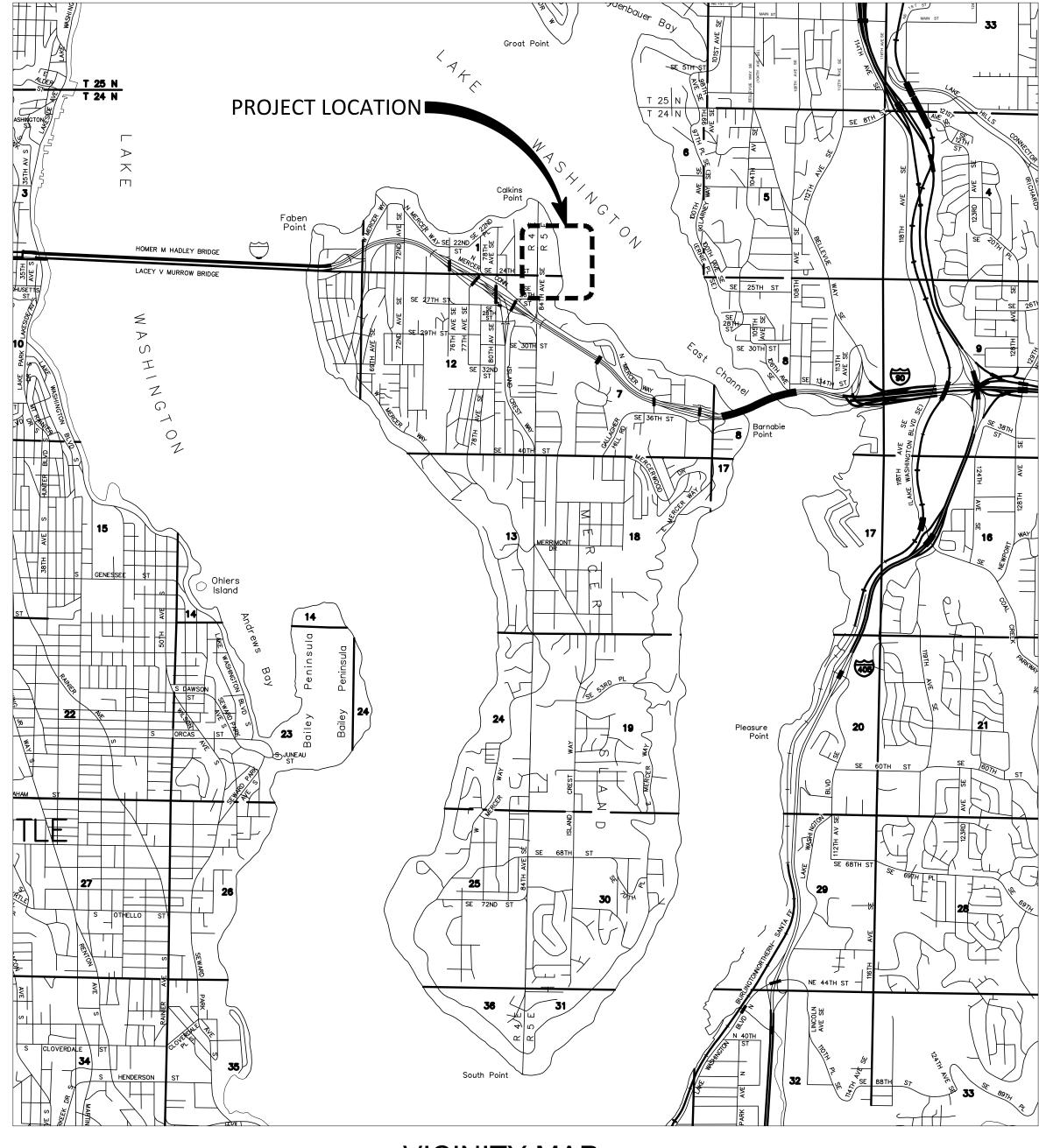
### LUTHER BURBANK PARK COMPREHENSIVE WATERFRONT IMPROVEMENTS CITY OF MERCER ISLAND PUBLIC WORKS DEPARTMENT



VICINITY MAP

OWNER

CITY OF MERCER ISLAND PUBLIC WORKS DEPARTMENT 9611 SE 36TH STREET MERCER ISLAND, WA 98040

PROJECT ADDRESS 2048)84TH AVE SE MERCER ISLAND, WA 98040

PAUL WEST (206) 677-1028

### PROJECT CONTACT

paul.west@mercerisland.gov





	NO.	DATE	BY	REVISION	
	1	4/14/2023	WBC	LAND USE PERMIT REVISION	
<u>.</u>	1A	4/14/2023		MIFD COMMENT REVISION	
<u> </u>					
-					



### LUTHER BURBANK PARK

WATERFRONT IMPROVEMENTS

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DEMO AND TESC SITE PLAN - 2

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SILVA CELL DETAIL

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INTAKE AND PUMPING FACILITIES - EQUIPMENT PLAN AND SECTIONS (1 OF 2)

INTAKE AND PUMPING FACILITIES - EQUIPMENT PLAN AND SECTIONS (2 OF 2)

ENLARGED PLAN - STORM DRAIN

OVERALL SITE IMPROVEMENT AND SURFACING PLAN

RAWING NO.	G-001
HECKED: AKB	DATE: 10/7/2022
ESIGN: WBC	SCALE: AS SHOWN
RAWN: RRT	PROJECT NO.: 2200248

**COVER SHEET** 

### PROJECT GENERAL NOTES

- 1. UNLESS NOTED OTHERWISE CONTRACTOR SHALL FOLLOW THE WSDOT SPECIFICATIONS FOR ROAD, BRIDGE AND MUNICIPAL CONSTRUCTION (2018).
- 2. <u>DATUM:</u>
  - A. VERTICAL DATUM: NAVD 88, OLWM: 16.75, OHWM: 18.67 B. HORIZONTAL DATUM: NAD83
- 3. BOUNDARY SURVEY: TOPOGRAPHIC SURVEY AND CONTROL PROVIDED BY KPFF IN AUGUST OF 2020, ADDITIONAL SURVEY COMPLETED IN JUNE 2022 AND JANUARY 2023.
- 4. BATHYMETRY: BATHYMETRIC CONTOURS COLLECTED BY WILSON ENGINEERING.
- 5. **EXISTING STRUCTURES:** 
  - A. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS OF EXISTING STRUCTURES THAT MAY IMPACT THE WORK.
  - B. CONTRACTOR SHALL COORDINATE WITH THE ENGINEER, IF THERE ARE ANY CONFLICTS BETWEEN PROPOSED WORK AND EXISTING STRUCTURES TO REMAIN ON—SITE.
- 6. <u>UTILITIES:</u>
  - A. CONTRACTOR SHALL UTILIZE A UTILITY LOCATE SERVICE TO IDENTIFY UNDERGROUND UTILITIES, AT LEAST 48 HOURS PRIOR TO CONSTRUCTION AT THE SITE.
  - B. CONTRACTOR SHALL PROTECT—IN—PLACE ALL UTILITIES THAT ARE NOT INDICATED FOR DEMOLITION.
  - C. ANY DAMAGE TO EXISTING UTILITIES, FACILITIES OR EQUIPMENT, EXCEPT ITEMS TO BE DEMOLISHED, SHALL BE REPAIRED BY AND AT THE EXPENSE OF THE CONTRACTOR.
  - D. EXISTING BURIED WATER AND SANITARY UTILITIES CURRENTLY AT UNKNOWN LOCATIONS. CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING ALL UNKNOWN UTILITIES IN UPLAND AREA PRIOR TO ANY TEMPORARY UTILITY RE—ROUTE IMPLEMENTATION.

- E. CONTRACTOR SHALL COORDINATE WITH THE ENGINEER AND OWNER FOR ALL MAINTAINED UTILITY SERVICES INCLUDING, BUT NOT LIMITED TO EXISTING STORM DRAINAGE, WATER AND ELECTRICAL LINES.IF NEEDED CONTRACTOR SHALL COORDINATE THE ACTIVATION AND DE—ACTIVATION OF UTILITIES WITH THE ENGINEER AND OWNER. CONTRACTOR SHALL PROVIDE A MINIMUM OF 2 DAYS ADVANCE WRITTEN NOTICE TO THE ENGINEER AND OWNER.
- 7. TRAFFIC CONTROL:
  - A. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY VEHICULAR OR PEDESTRIAN TRAFFIC CONTROL REQUIRED DURING THE PROJECT. ALL TRAFFIC CONTROL SHALL BE IN ACCORDANCE TO THE MANUAL OF UNIFORM TRAFFIC CONTROL DEVICES (MUTCD).
- 8. <u>IN WATER WORK:</u>
  - A. CONTRACTOR SHALL LOCATE ALL EQUIPMENT WITHIN PROJECT WORK AREA LIMITS.
  - B. IN WATER WORK SHALL BE COMPLETED IN ACCORDANCE WITH THE PROJECT PERMITS.
- 9. STORMWATER POLLUTION PREVENTION PLAN (SWPPP):
  - A. CONTRACTOR SHALL PROVIDE THE SWPPP.

#### **CODES AND DESIGN CRITERIA:**

- STORM DRAINAGE:
- A. STORM DRAINAGE SYSTEM IS DESIGNED USING AND SHALL CONFORM TO THE 2014 DEPARTMENT OF ECOLOGY STORM WATER MANUAL.
- B. CONTRACTOR SHALL BE RESPONSIBLE FOR RE-ROUTING STORMWATER DURING CONSTRUCTION.
- C. CONTRACTOR SHALL PROVIDE ALL PUMPS, TANKS, PIPES, HOSES, APPURTENANCES AND POWER TO COLLECT AND DISCHARGE THE STORMWATER DURING CONSTRUCTION ACTIVITIES.
- 2. FIRE PROTECTION SYSTEM:
  - A. THE FIRE PROTECTION SYSTEM INCLUDING PIPES AND APPURTENANCES SHALL CONFORM TO THE 2018 VERSION OF THE INTERNATIONAL FIRE CODE, PARTICULARLY CHAPTER 36 MARINAS AND BOATYARDS. THE FIRE SYSTEM SHALL ALSO CONFORM TO THE 2019 EDITION OF THE NFPA 14 FOR THE INSTALLATION OF STANDPIPES AND HOSE SYSTEMS. WHERE THERE IS CONFLICT THE SEATTLE FIRE CODE SHALL BE ADHERED TO.
  - B. PORTABLE FIRE EXTINGUISHERS WITH A MINIMUM RATING OF 2A 20-BC SHALL BE PLACED ON THE DOCK AND FLOAT AT EACH STANDPIPE LOCATION AND ADDITIONALLY SUCH THAT NO PORTION OF THE FLOAT MORE THAN 75 FEET FROM AN EXTINGUISHER.
  - C. A SIGN AT DOCK STATING THE FOLLOWING IS REQUIRED: LOCATIONS OF FIRST AID FACILITIES, TELEPHONES, FIRE FIGHTING EQUIPMENT, EMERGENCY EQUIPMENT, AND FIRE EXITS. A SIGN SHOULD ALSO INCLUDE TELEPHONE NUMBERS OF CLOSE AMBULANCE SERVICE, HOSPITAL, POLICE, AND FIRE DEPARTMENT.
  - D. FOR FIRE PROTECTION ON FLOAT, ABOVE WATER LEVEL PIPING SHALL BE GALVANIZED STEEL OF THE SIZE SHOWN IN THE DRAWINGS.
  - E. FOR FIRE PROTECTION ON FLOAT, BELOW WATER LEVEL PIPING SHALL BE HDPE OF A PRESSURE CLASS TO WITHSTAND THE REQUIREMENTS OF THE FIRE CODE.
  - F. FIRE SUPPRESSION SYSTEM SHALL CONSIST OF 6"Ø PIPING IN A DRY SYSTEM UNLESS NOTED OTHERWISE IN PLAN.





	NO.	DATE	BY	REVISION
	1	4/14/2023	ROL	LAND USE PERMIT REVISION
	1A	4/14/2023	ROL	MIFD COMMENT REVISION
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LUTHER BURBANK PARK
WATERFRONT IMPROVEMENTS

PROJECT GENERAL NOTES

DRAWING NO.	G-002	(
CHECKED: NAW	<b>DATE:</b> 10/07/2022	
DESIGN: SS	SCALE: AS SHOWN	
DRAWN: AS	PROJECT NO.: 2200248	[   •

SHEET NO. 02 OF 56



G-003

TEMPORARY ACCESS PLAN

CAD USER: dgilbert PLOT DATE: Apr 13, 2023-02

1601 5th Avenue, Suite 1600 Seattle, WA 98101

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1	NO.	DATE	BY	REVISION
	1	4/14/2023	ROL	LAND USE PERMIT REVISION
	1A	4/14/2023	ROL	MIFD COMMENT REVISION
5				

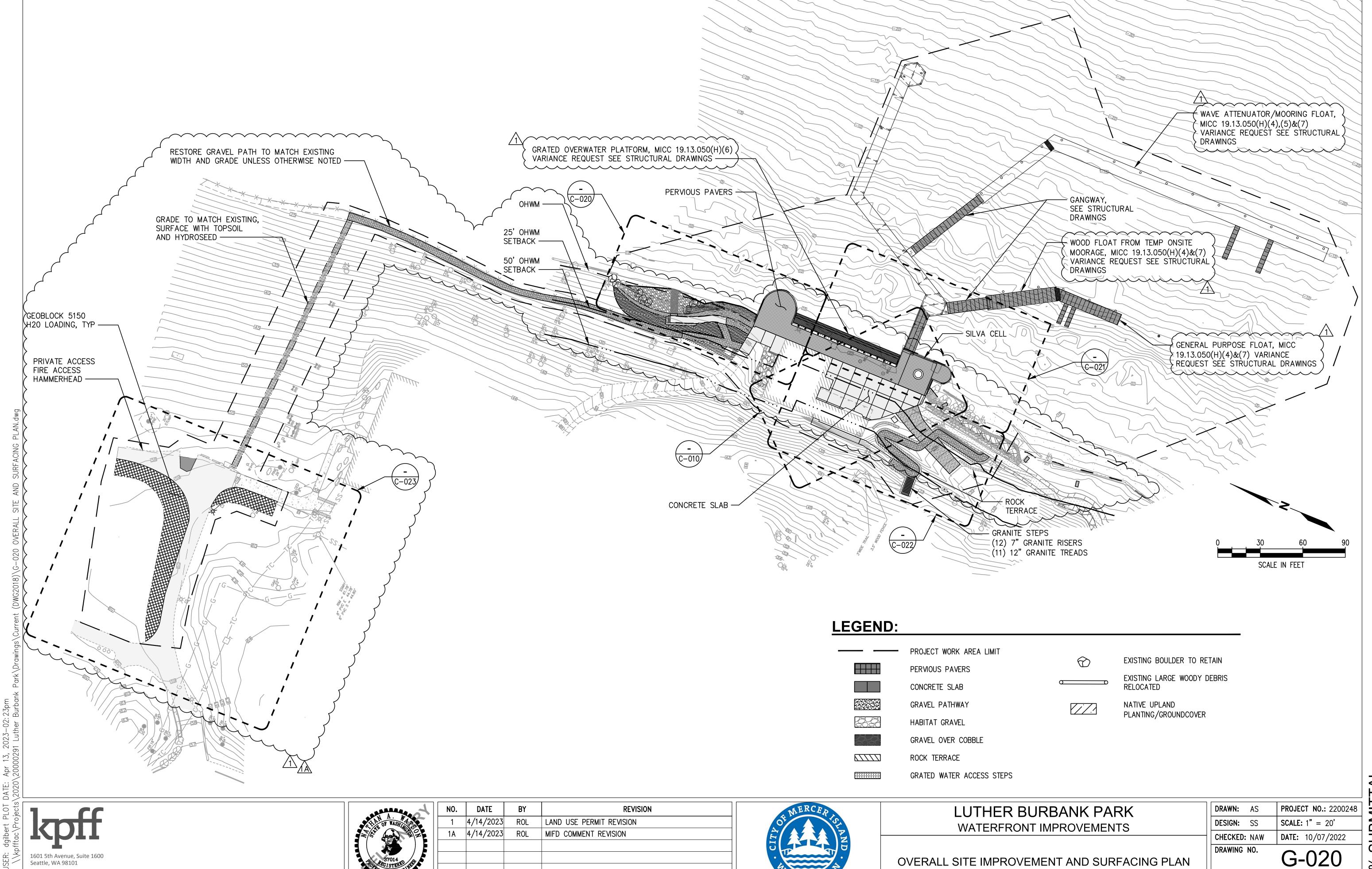


**EXISTING CONDITIONS SITE PLAN** 

	DRAWING NO.	G-010
	CHECKED: NAW	<b>DATE:</b> 10/07/2022
$\Big\} \Big[$	DESIGN: SS	<b>SCALE:</b> 1" = 30'
	DRAWN: AS	<b>PROJECT NO.:</b> 2200248
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SHEET NO.

04 of 56



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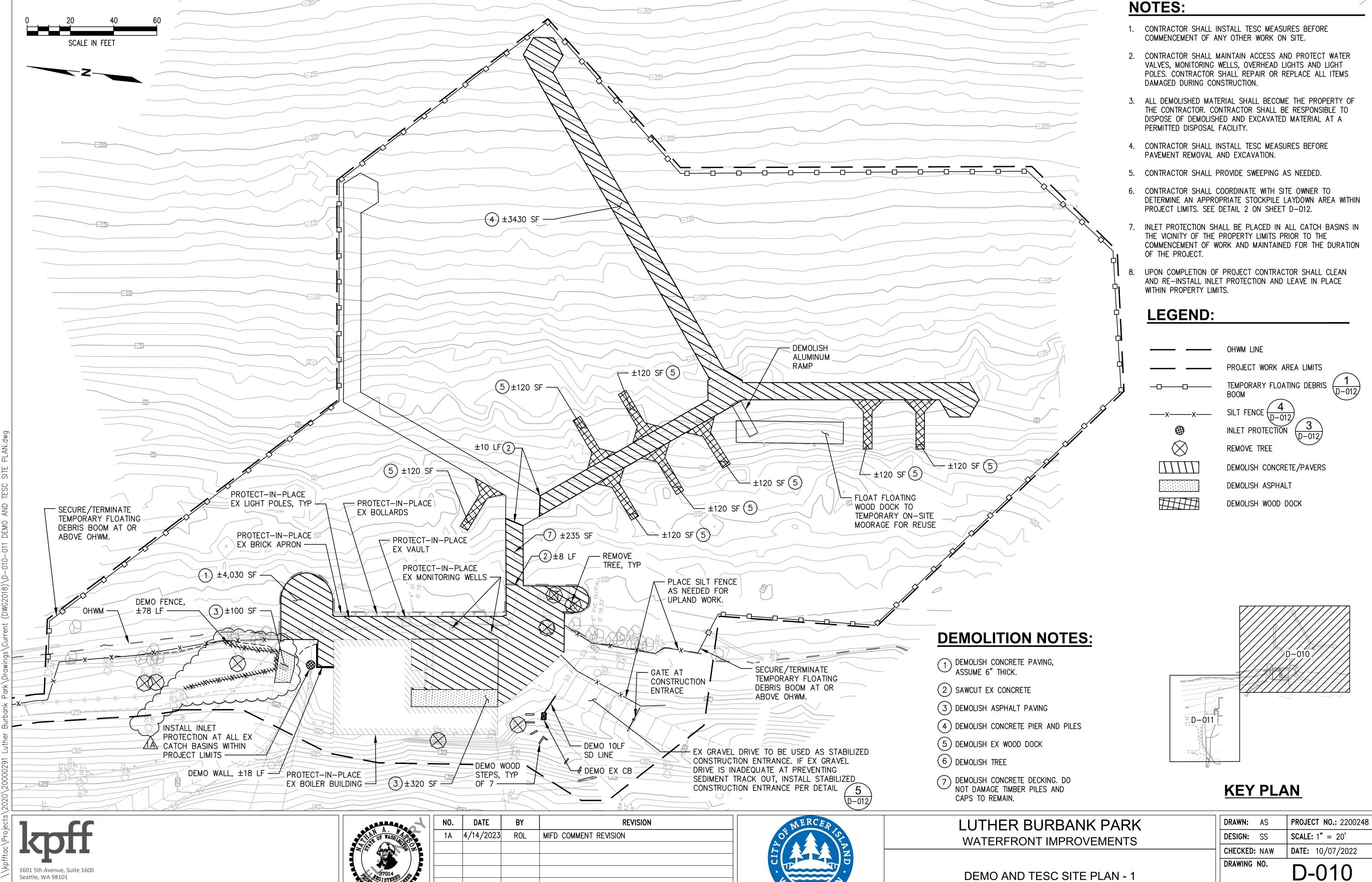


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	1	4/14/2023	ROL	LAND USE PERMIT REVISION
	1A	4/14/2023	ROL	MIFD COMMENT REVISION
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OVERALL SITE IMPROVEMENT AND SURFACING PLAN

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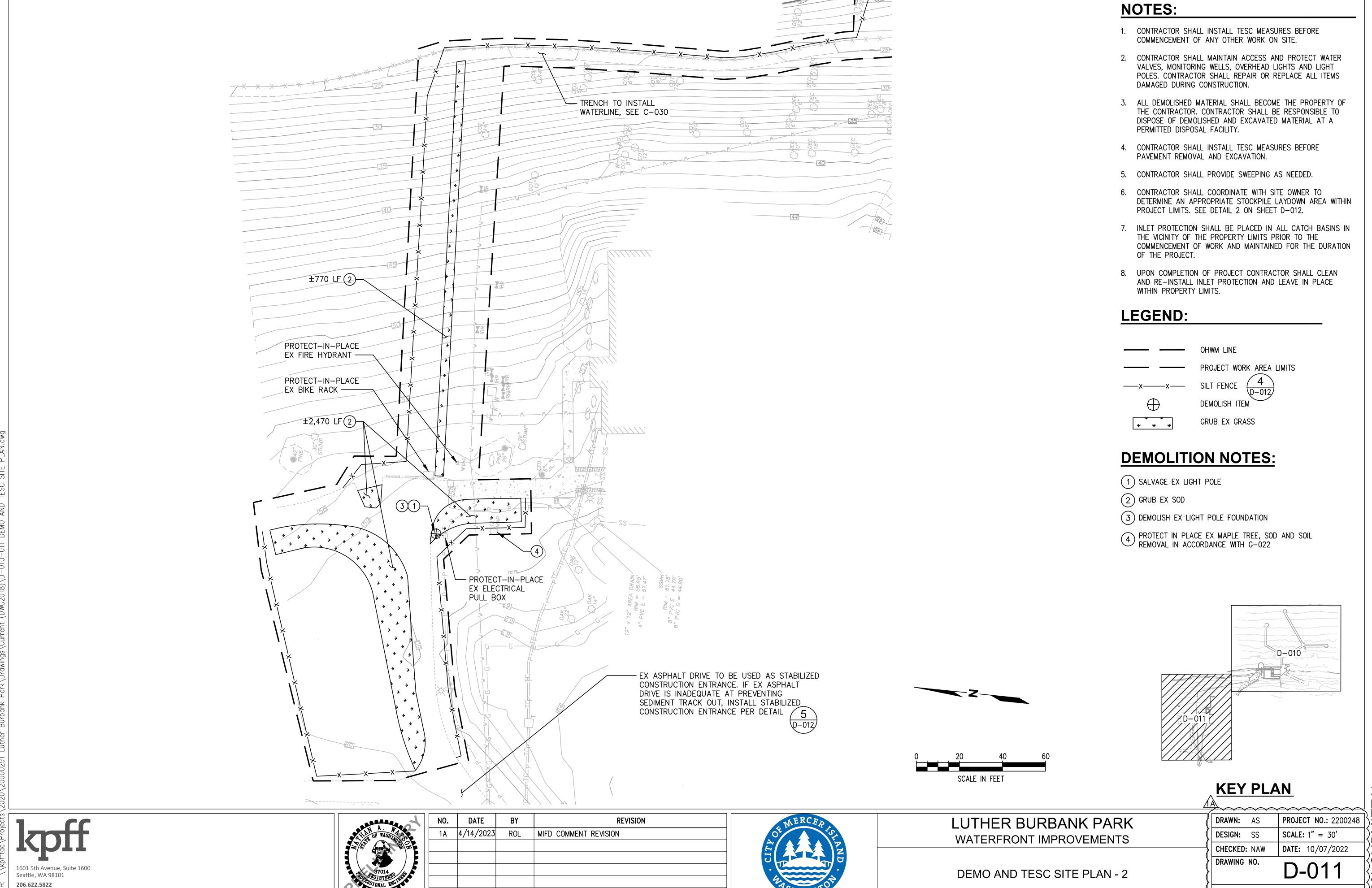


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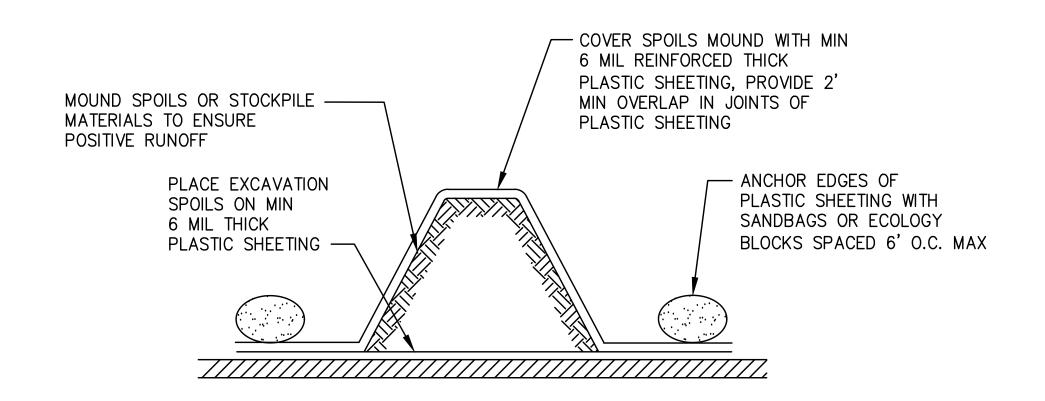


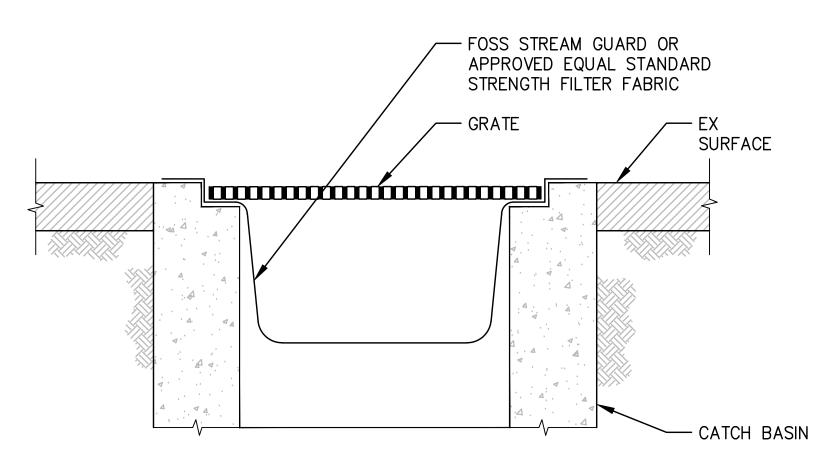
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SHEET NO.

CAD USER: dgilbert PLOT DATE: Apr 13, 2023-02:23pm

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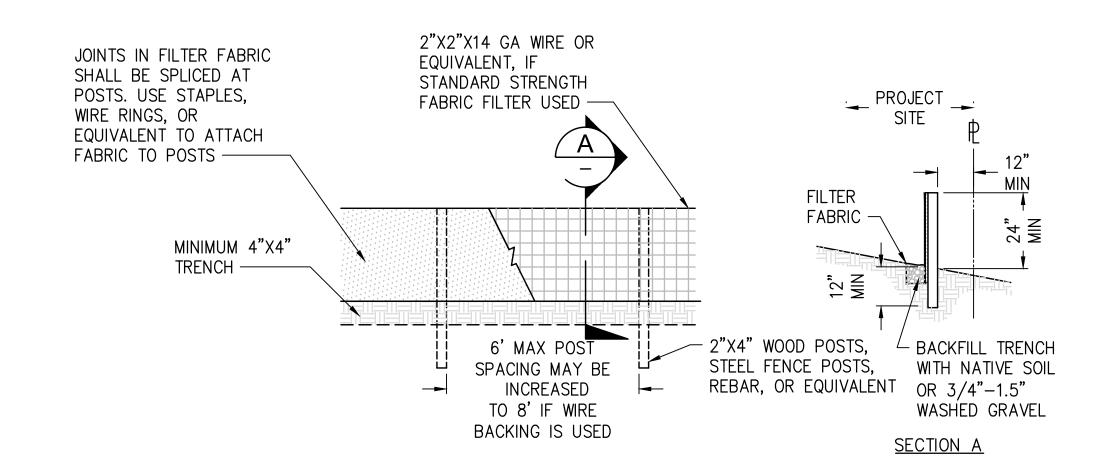


#### MAINTENANCE STANDARDS:

- 1. EXISTING CATCH BASINS SHALL BE FITTED WITH FILTER FABRIC AS SHOWN.
- 2. ANY ACCUMULATED SEDIMENT ON OR AROUND THE FILTER FABRIC SHALL BE REMOVED IMMEDIATELY. SEDIMENT SHALL NOT BE REMOVED BY FLUSHING WITH WATER. ALL SEDIMENT MUST BE DISPOSED OF OFF—SITE.
- 3 INLET PROTECTION DETAIL
  NTS



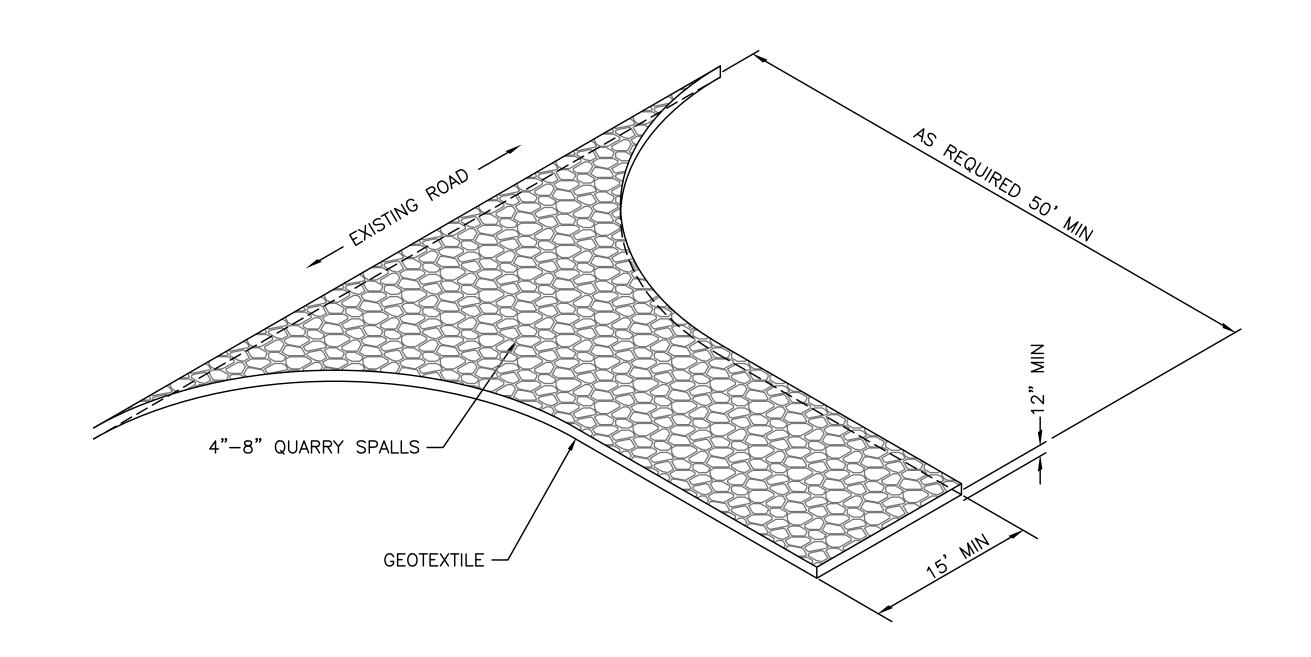






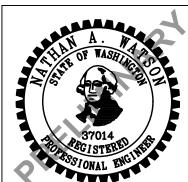
### MAINTENANCE NOTES:

- 1. REPAIR ANY DAMAGE IMMEDIATELY.
- 2. IF CONCENTRATED FLOWS ARE EVIDENT UPHILL OF THE FENCE, INTERCEPT AND CONVEY THEM TO A SEDIMENT POND.
- 3. IT IS IMPORTANT TO CHECK THE UPHILL SIDE OF THE FENCE FOR SIGNS OF THE FENCE CLOGGING, ACTING AS A BARRIER TO FLOW, AND THEN CAUSING CHANNELIZATION OF FLOWS PARALLEL TO THE FENCE. IF THIS OCCURS, REPLACE THE FENCE OR REMOVE THE TRAPPED SEDIMENT.
- 4. REMOVE SEDIMENT DEPOSITS WHEN THE DEPOSIT REACHES APPROXIMATELY ONE—THIRD THE HEIGHT OF THE SILT FENCE, OR INSTALL A SECOND SILT FENCE.
- 5. IF THE FILTER FABRIC (GEOTEXTILE)
  HAS DETERIORATED DUE TO
  ULTRAVIOLET BREAKDOWN, REPLACE IT.



# 5 CONSTRUCTION ENTRANCE DETAIL NTS





1	NO.	DATE	BY	REVISION
	1A	4/14/2023	ROL	MIFD COMMENT REVISION

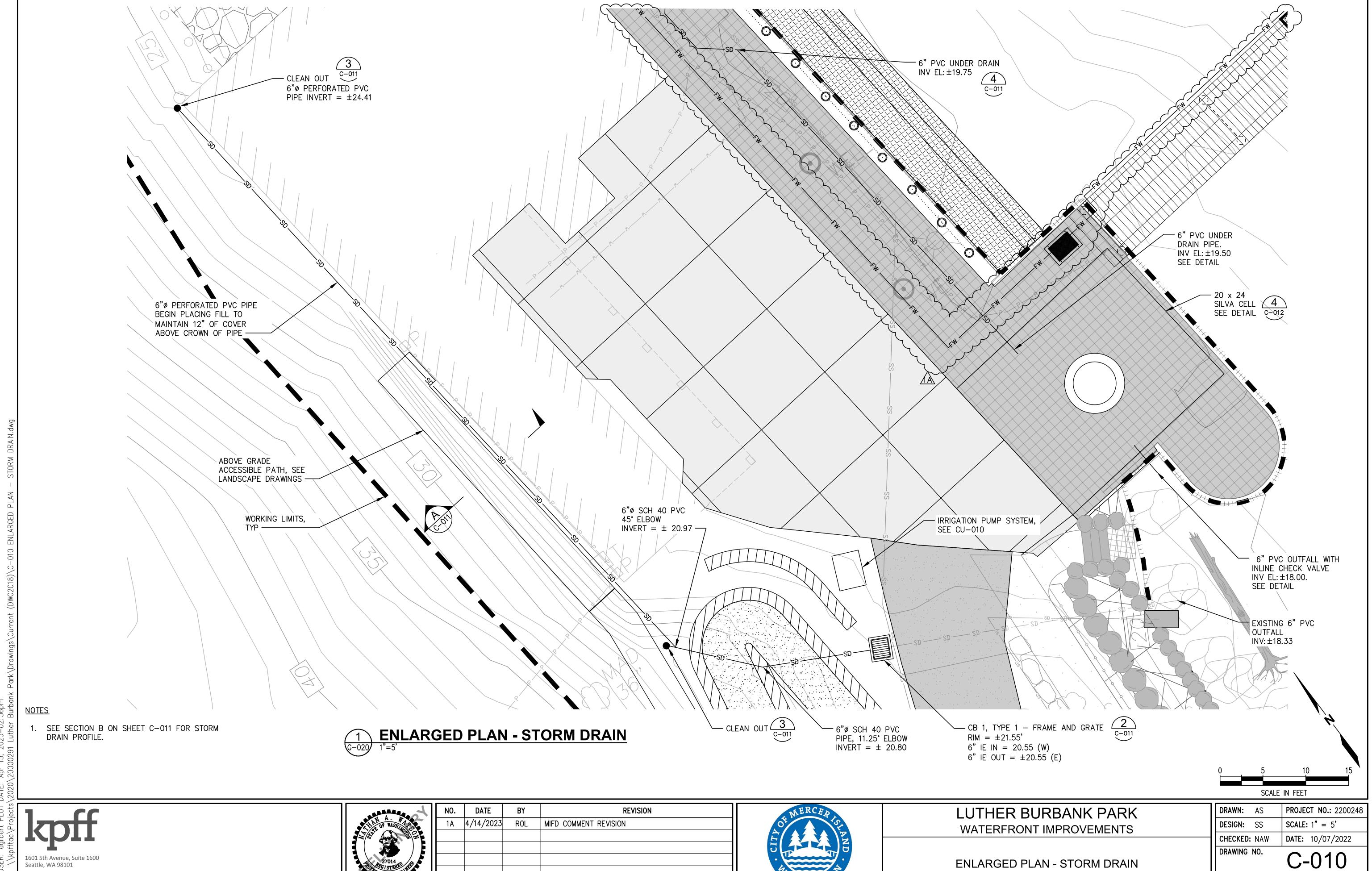


## LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

TESC DETAILS

DRAWING NO.	D-012
CHECKED: NAW	<b>DATE:</b> 10/07/2022
DESIGN: SS	SCALE: AS SHOWN
DRAWN: AS	PROJECT NO.: 2200248

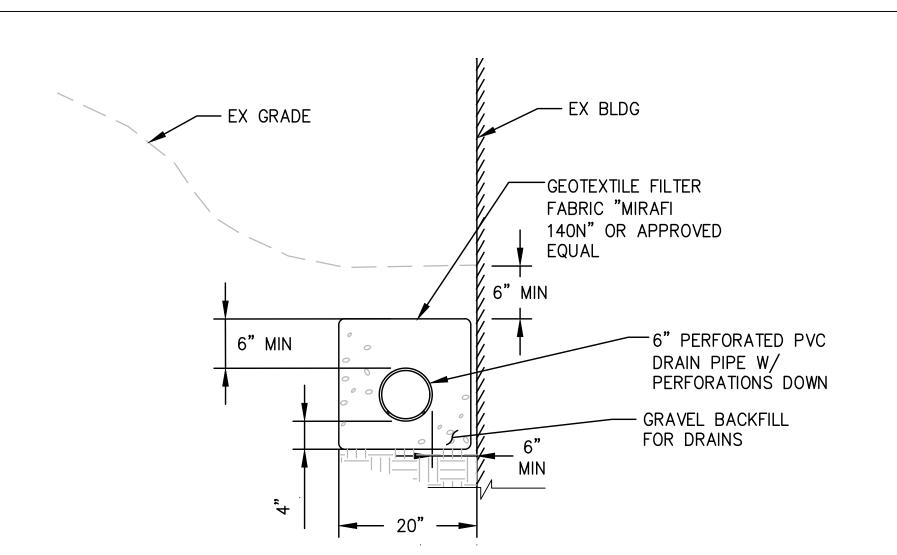
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ENLARGED PLAN - STORM DRAIN

Seattle, WA 98101

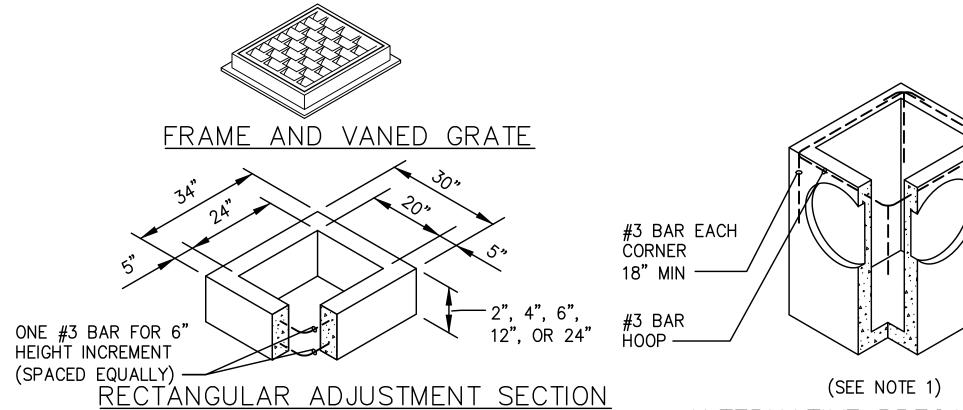
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STORM DRAINAGE WALL AND

1 FOOTING DRAIN SECTION

C-010 NTS



-22" (SEE NOTE 6)

**CATCH BASIN** 

26" (SEE NOTE 6) -

#3 BAR EACH

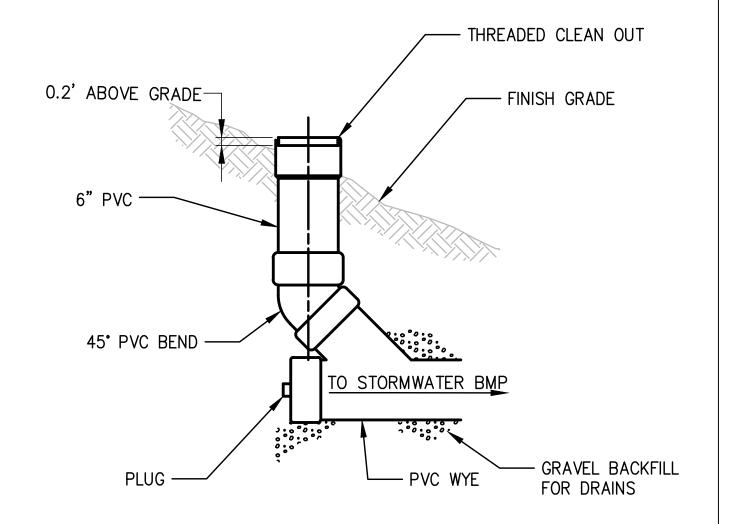
CORNER

#3 HOOP EACH -CORNER

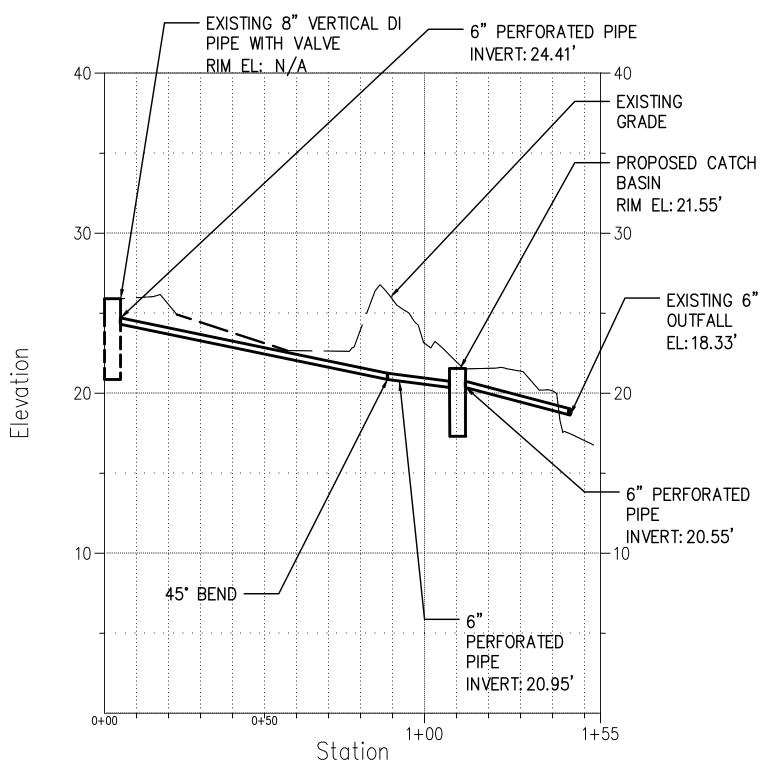
#3 BAR — EACH WAY (SEE NOTE 1)

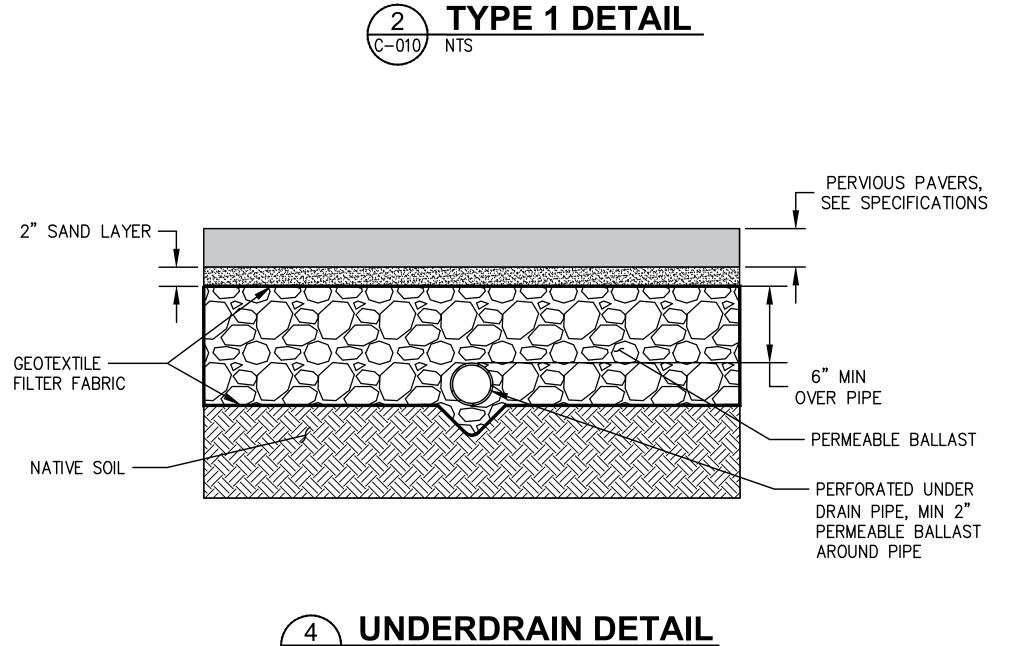
ALTERNATIVE PRECAST BASE SECTION

PIPE ALLOWANCES			
PIPE MATERIAL	MAXIMUM INSIDE DIAMETER		
REINFORCED OR PLAIN CONCRETE	12"		
ALL METAL PIPE	15"		
CPSSP * (STD.SPEC.9-05.20)	12"		
SOLID WALL PVC (STD.SPEC.9-05.12(1))	15"		
PROFILE WALL PVC (STD.SPEC.9-05.12(2))	15"		



3 CLEAN OUT DETAIL
C-010 NTS





PRECAST BASE SECTION

35

GRATING AND HANDRAILS,
SEE STRUCTURAL DRAWINGS

EXISTING BUILDING

EXISTING BUILDING

PIN PILES AND
BRACING, SEE
STRUCTURAL DRAWINGS

25

GEOTEXTILE FILTER

12" MIN
FABRIC

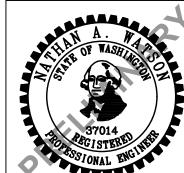
O+10

GRAVEL BACKFILL
FOR DRAINS

A STORM DRAIN PROFILE
NTS

B STORM DRAIN AND PATHWAY SECTION





4	NO.	DATE	BY	REVISION
	1	4/14/2023	ROL	LAND USE PERMIT REVISION
	1A	4/14/2023	ROL	MIFD COMMENT REVISION
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LUTHER BURBANK PARK
WATERFRONT IMPROVEMENTS

STORM DRAIN DETAILS

PRAWING NO.	C-011
CHECKED: NAW	<b>DATE:</b> 10/07/2022
DESIGN: SS	SCALE: AS SHOWN
DRAWN: AS	PROJECT NO.: 2200248
JBVMV• VC	PRO IFCT NO - 2200248

SHEET NO. 10 OF 56

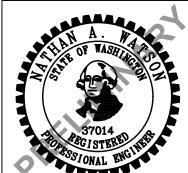
CAD USER: dgilbert PLOT DATE: Apr 13, 2023-02:24pm

**NOTES** 

1. SILVA CELL MAY CONTAIN X NUMBER OF TREES.







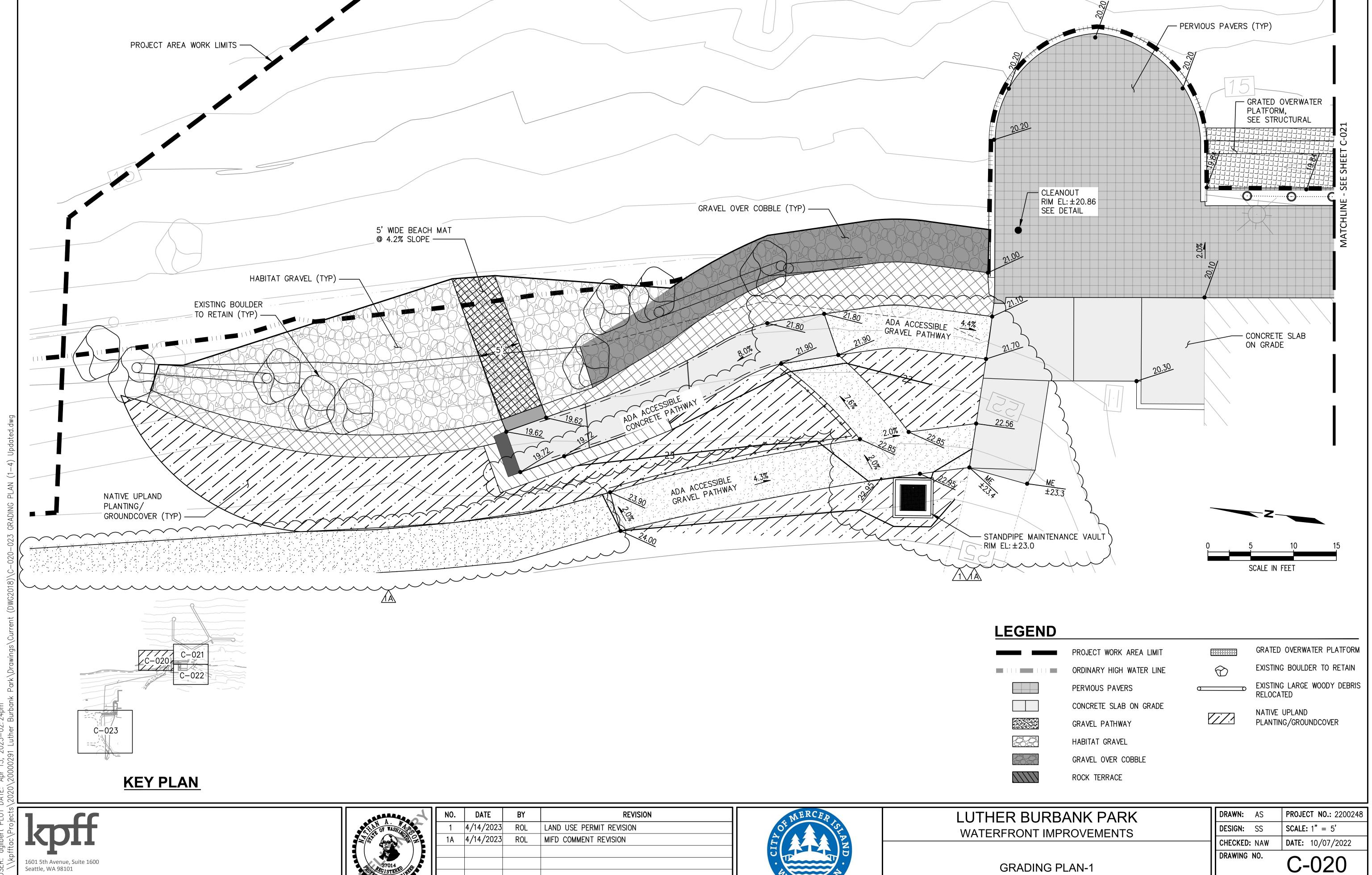
7	NO.	DATE	BY	REVISION
	1	4/14/2023	ROL	LAND USE PERMIT REVISION
	1A	4/14/2023	ROL	MIFD COMMENT REVISION
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LUTHER BURBANK PARK	
WATERFRONT IMPROVEMENTS	

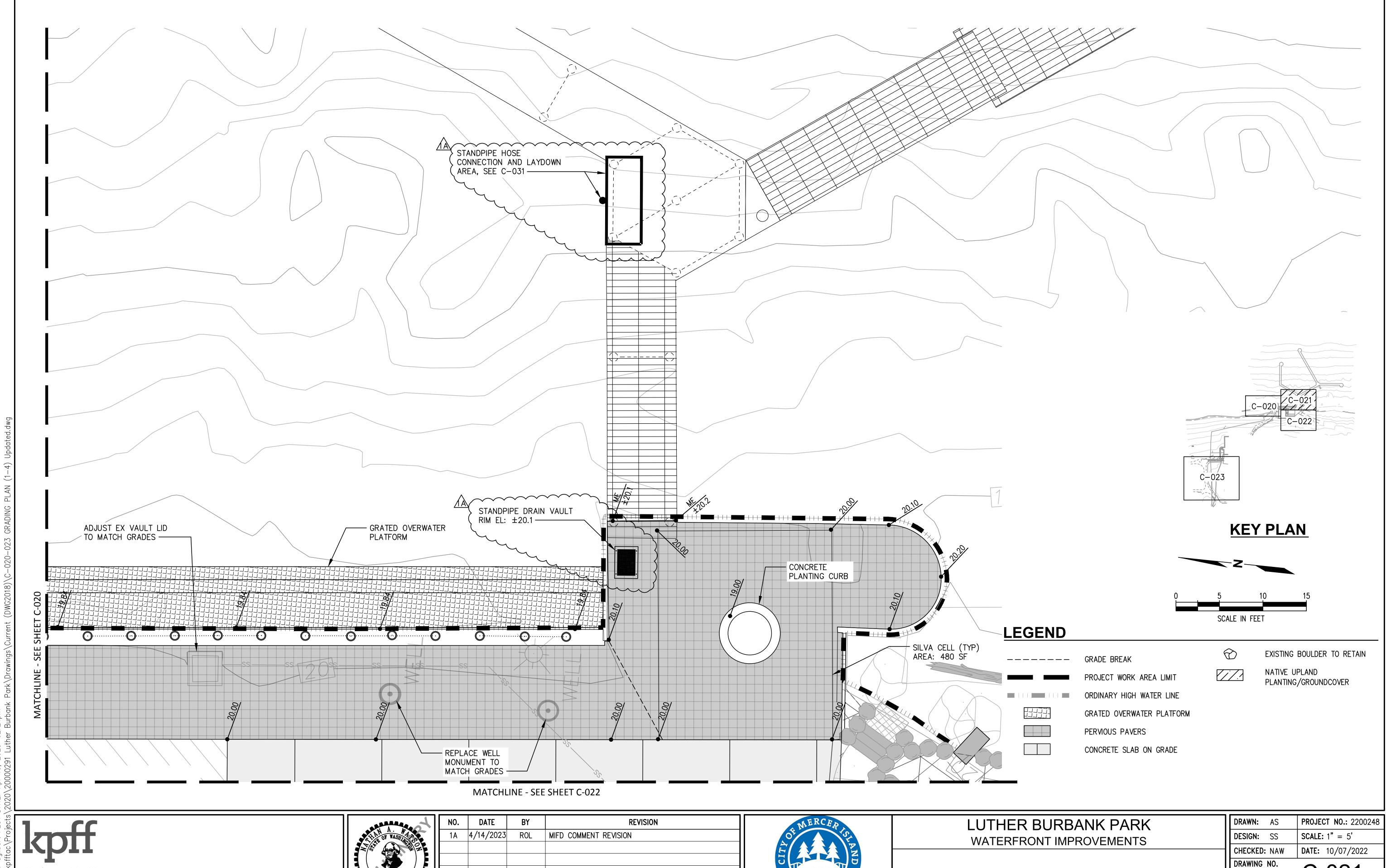
SILVA CELL DETAIL

	DRAWN: AS  DESIGN: SS	PROJECT NO.: 2200248  SCALE: AS SHOWN
	CHECKED: NAW	DATE: 10/07/2022
	DRAWING NO.	C 012
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CAD USER: dqilbert PLOT DATE: Apr 13, 2023-02:24pm

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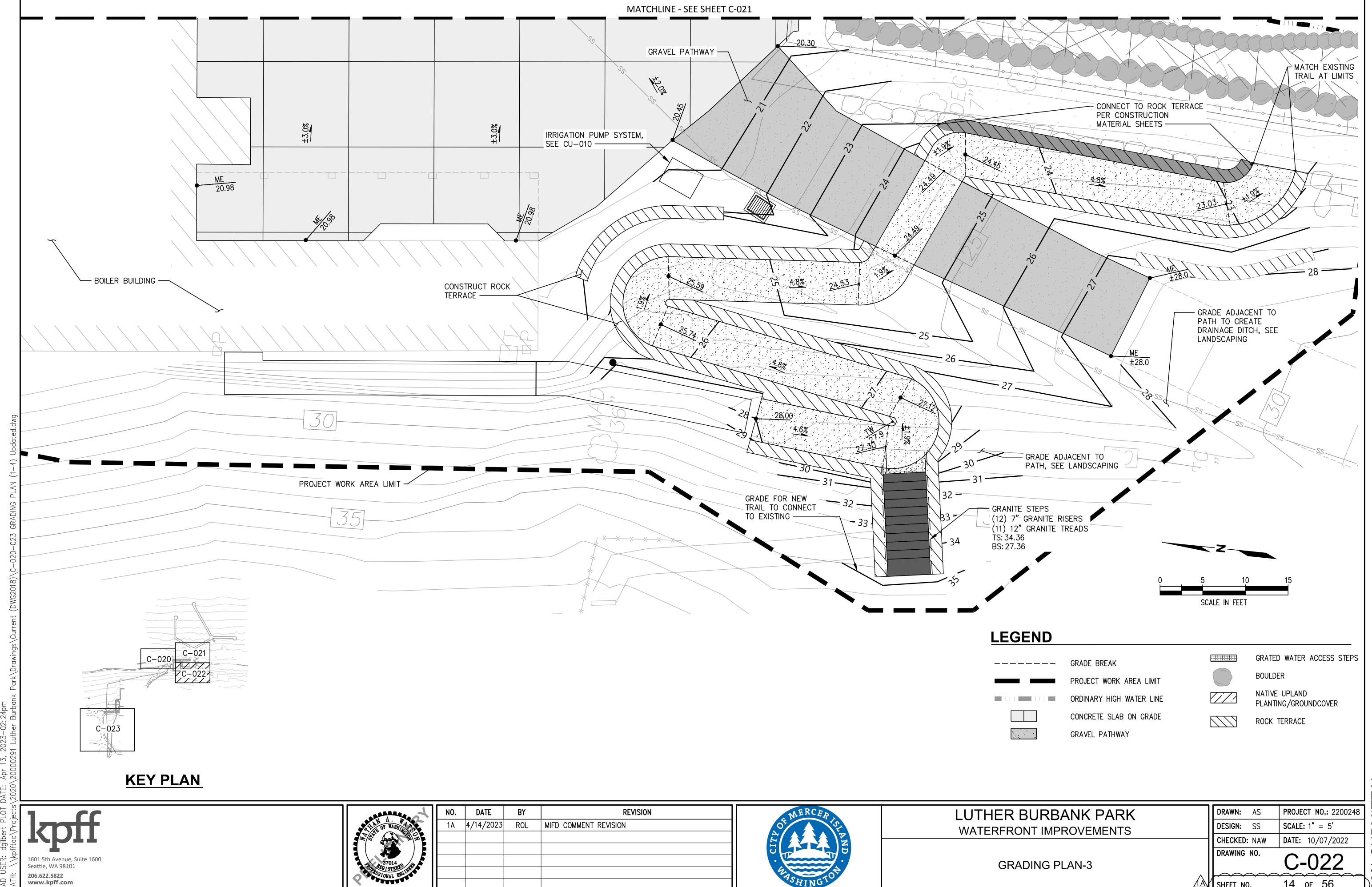


**GRADING PLAN-2** 

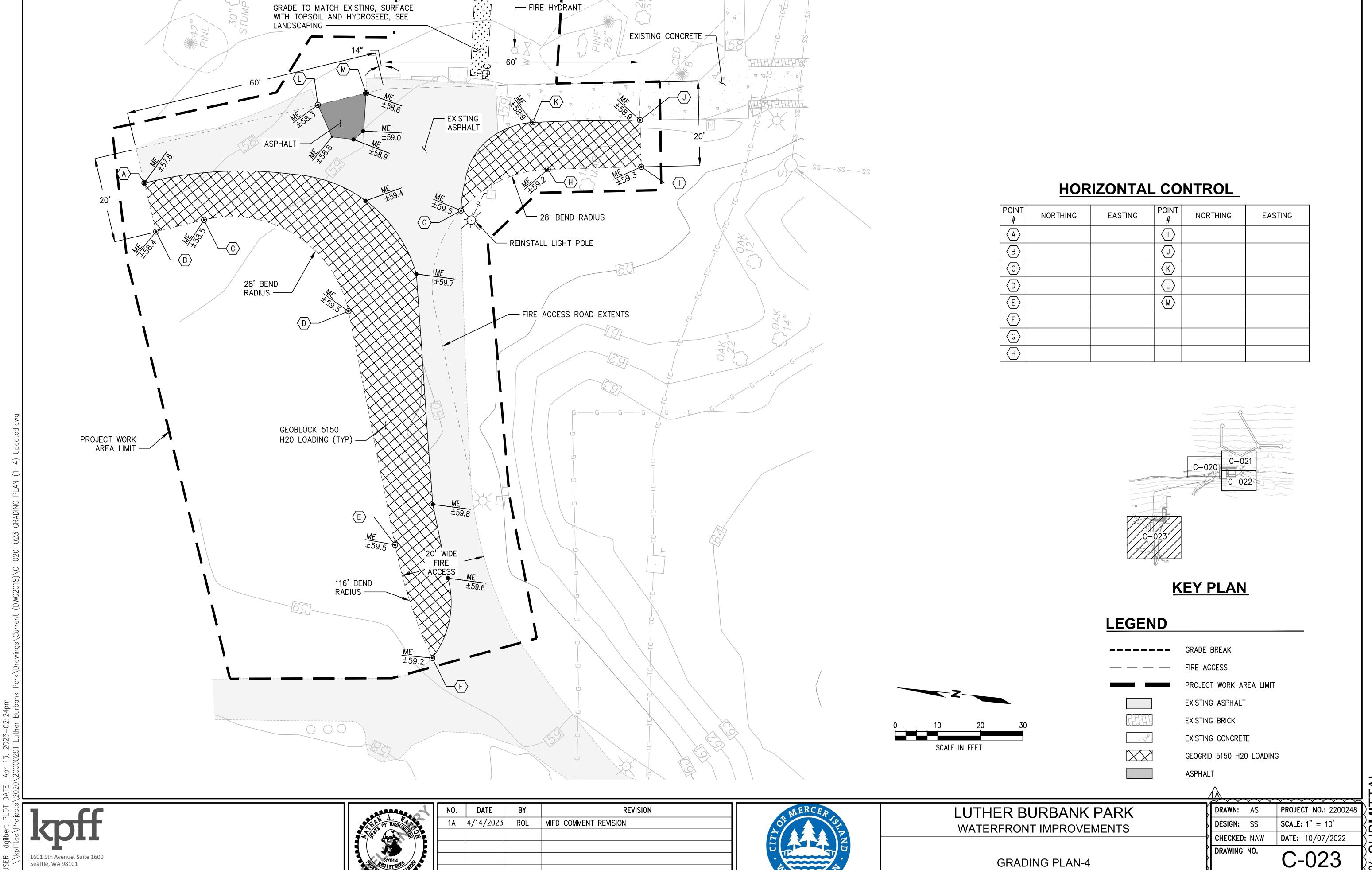
CAD USER: dgilbert PLOT DATE: Apr 13, 2023—02:24pm PATH: \\kpfftac\Projects\2020\20000291 Luther Burbank Park\Drawings\Current (D

Seattle, WA 98101

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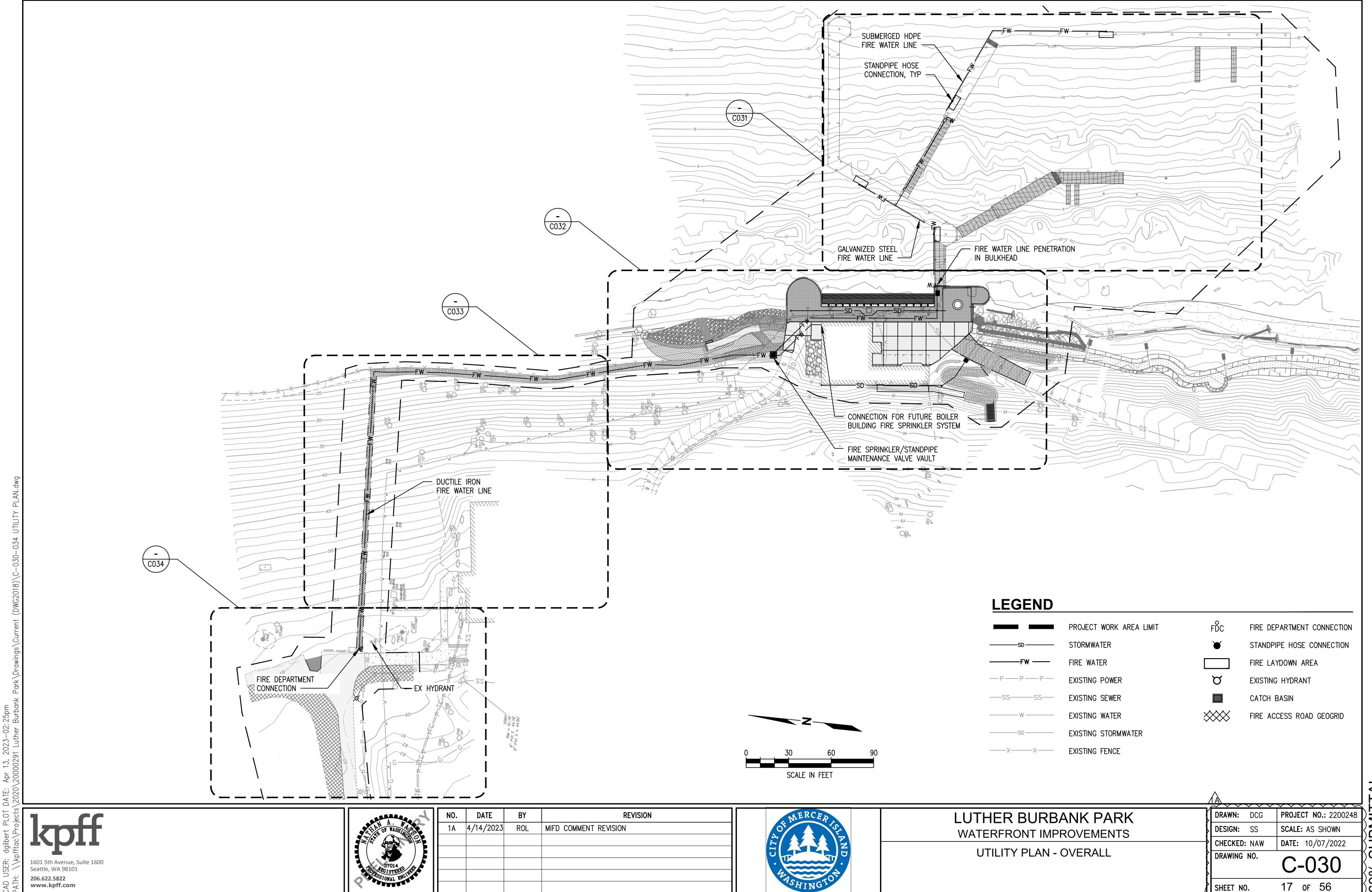
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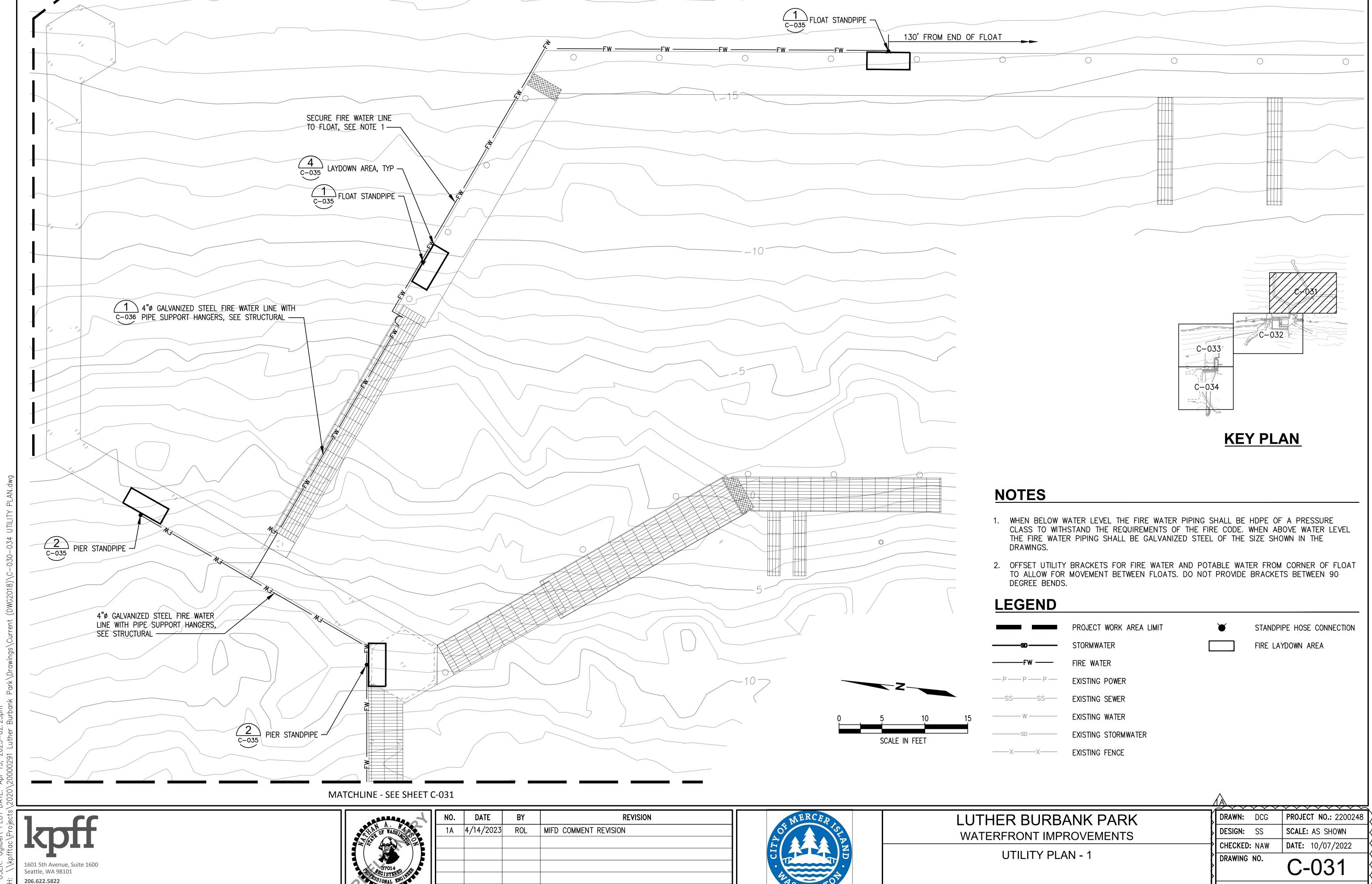


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SHEET NO.

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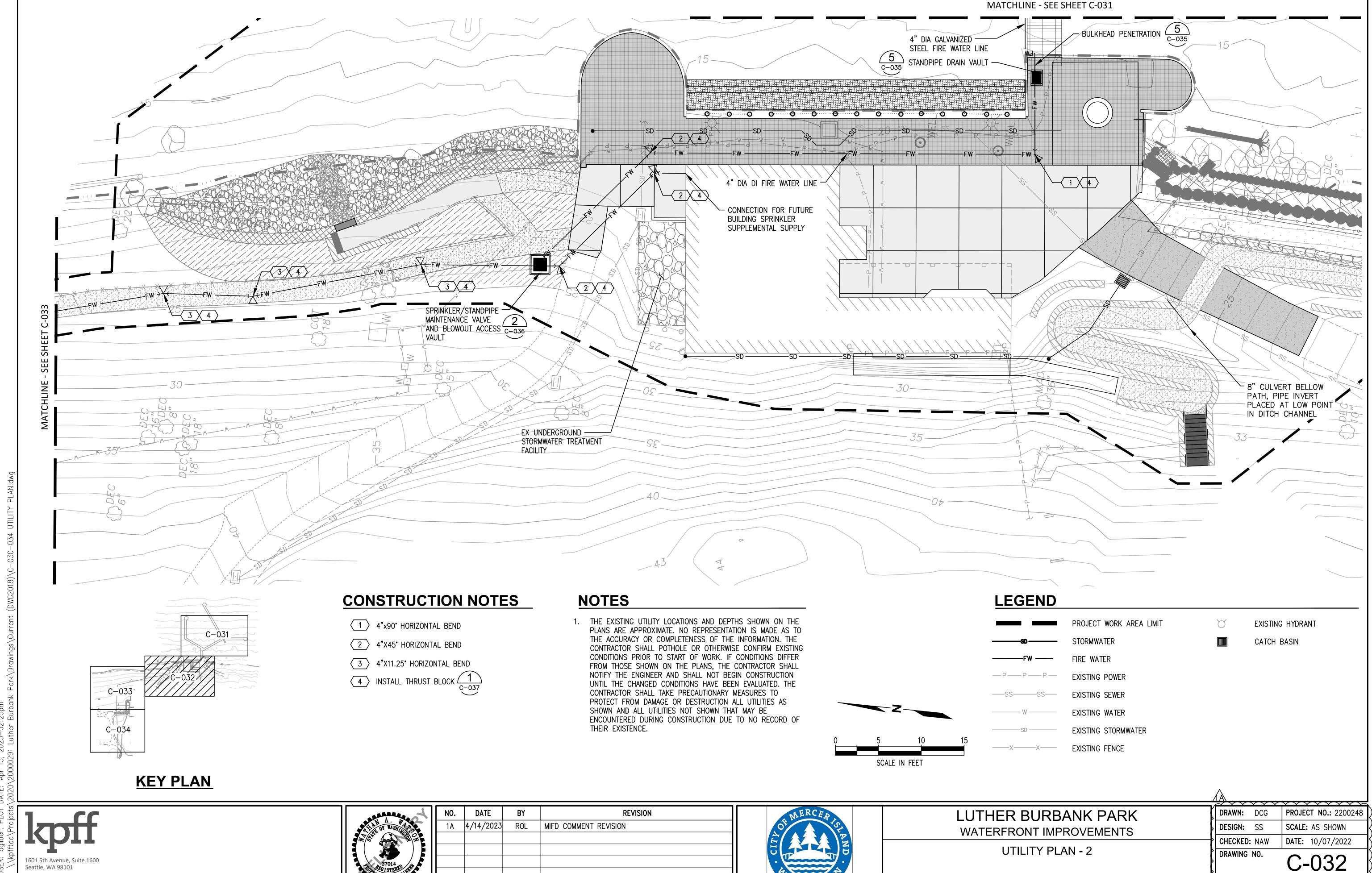


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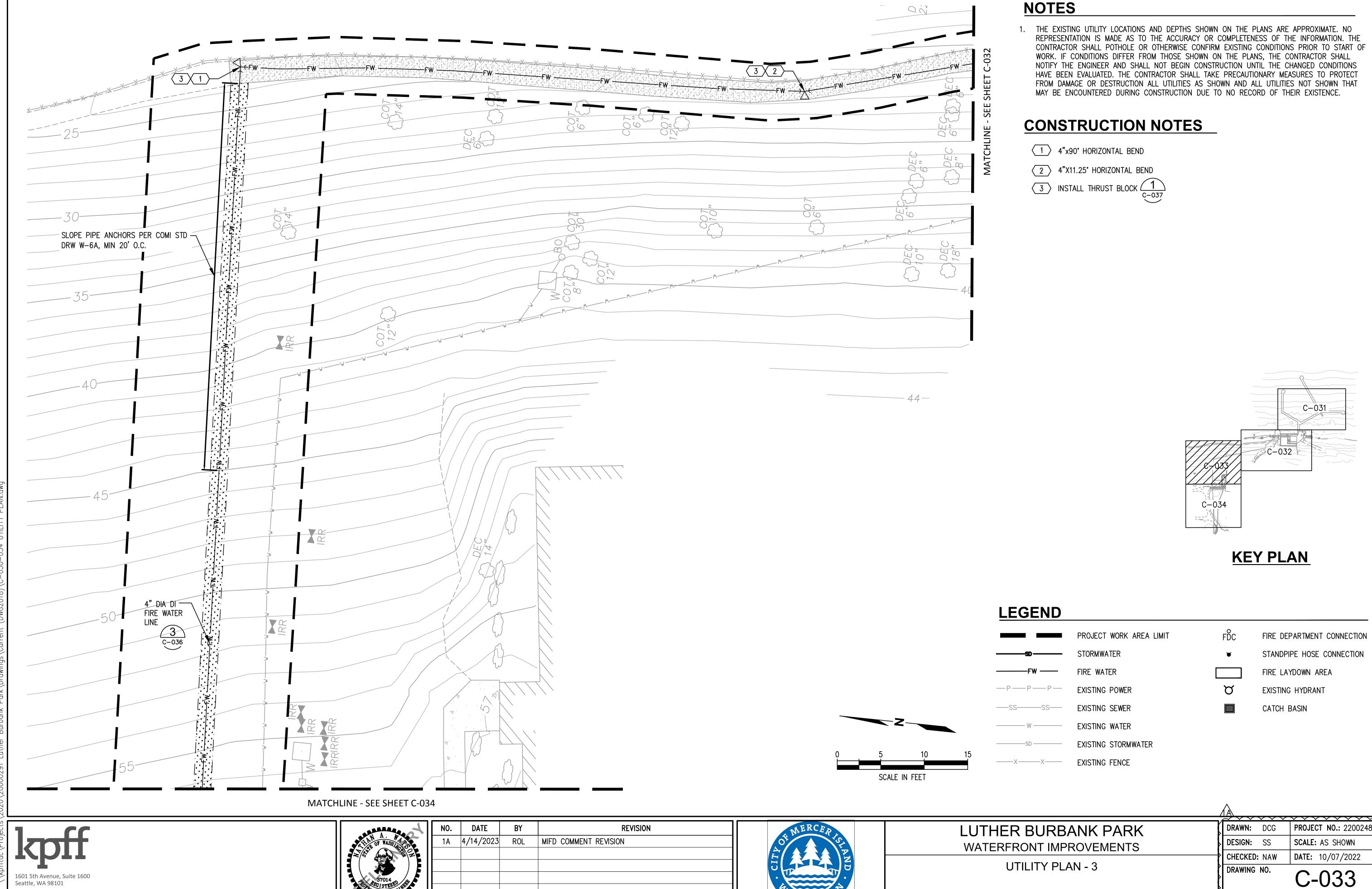
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SHEET NO.

CAD USER: dgilbert PLOT DATE: Apr 13, 2023—02:25pm

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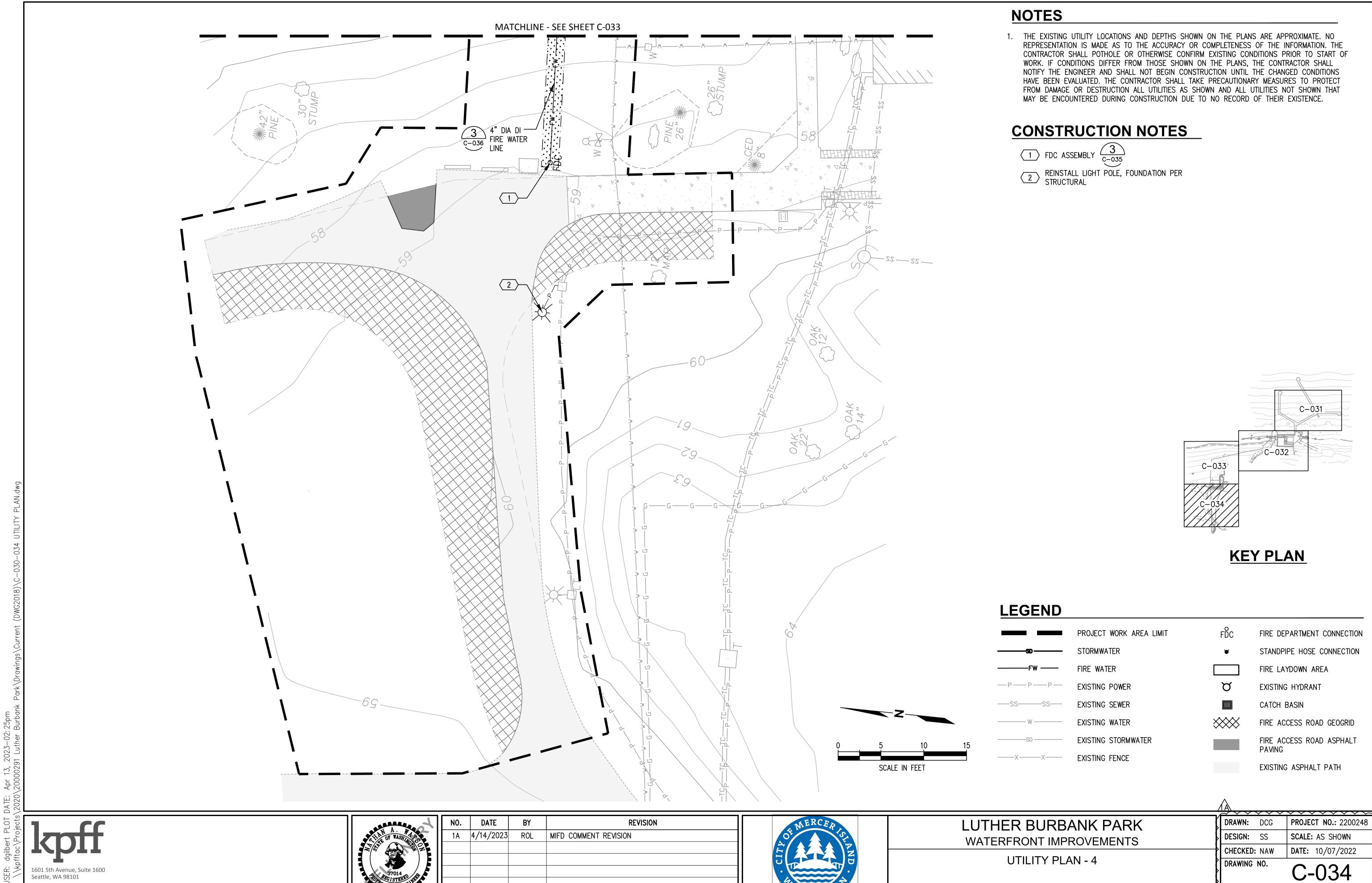


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SHEET NO.



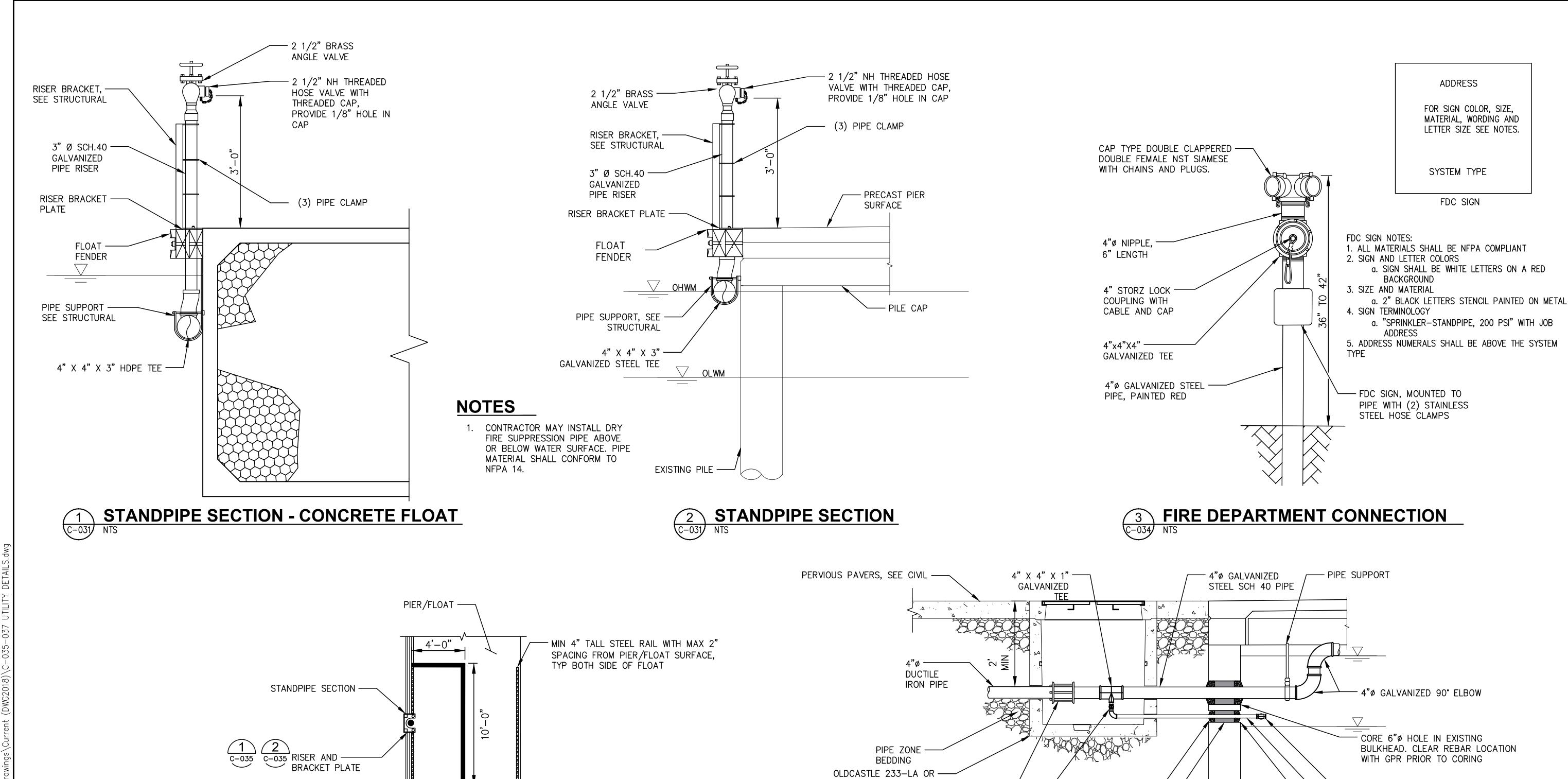
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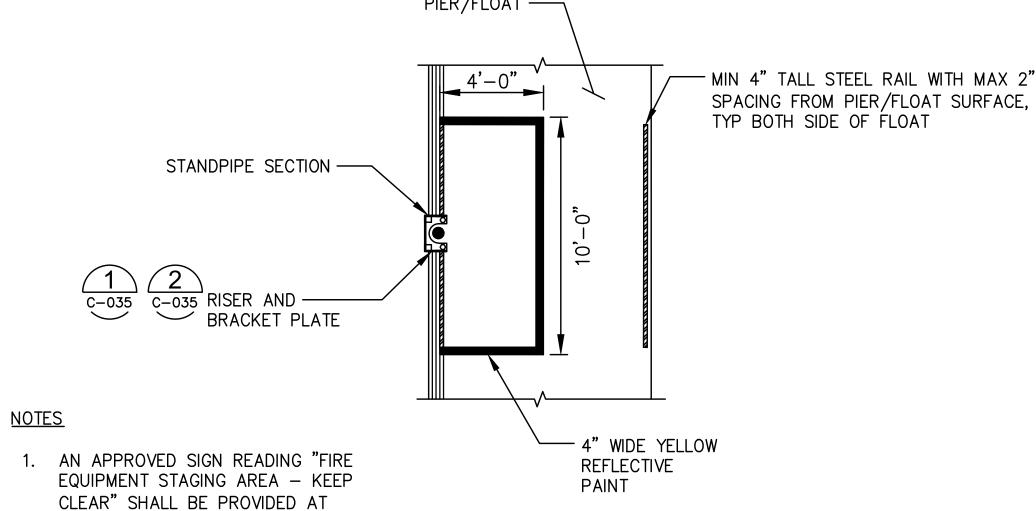
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SHEET NO.





FIRE STAGING AREA

EACH STAGING AREA.



NON-SHRINK

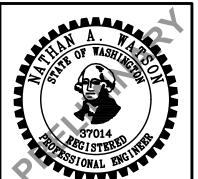
GROUT, TYP

STAINLESS

LINKSEAL

STEEL





1	NO.	DATE	BY	REVISION
	1A	4/14/2023	ROL	MIFD COMMENT REVISION



ROMEC IC501 —

STANDPIPE DRAIN VALVE,

1" GATE VALVE NORMALLY

CLOSED

COUPLING WITH

INSULATING BOOT

EQUAL

**LUTHER BURBANK PARK** WATERFRONT IMPROVEMENTS

**UTILITY DETAILS - 1** 

_/	1A	· · · · · · · · · · · · · · · · · · ·
ľ	DRAWN: DCG	PROJECT NO.: 2200248
<b>\</b>	DESIGN: SS	SCALE: AS SHOWN
ŀ	CHECKED: NAW	<b>DATE:</b> 10/07/2022
<b>^ ^ ^</b>	DRAWING NO.	C-035

SUBMITT

" SWING CHECK

- 1"ø GALVANIZED PIPE

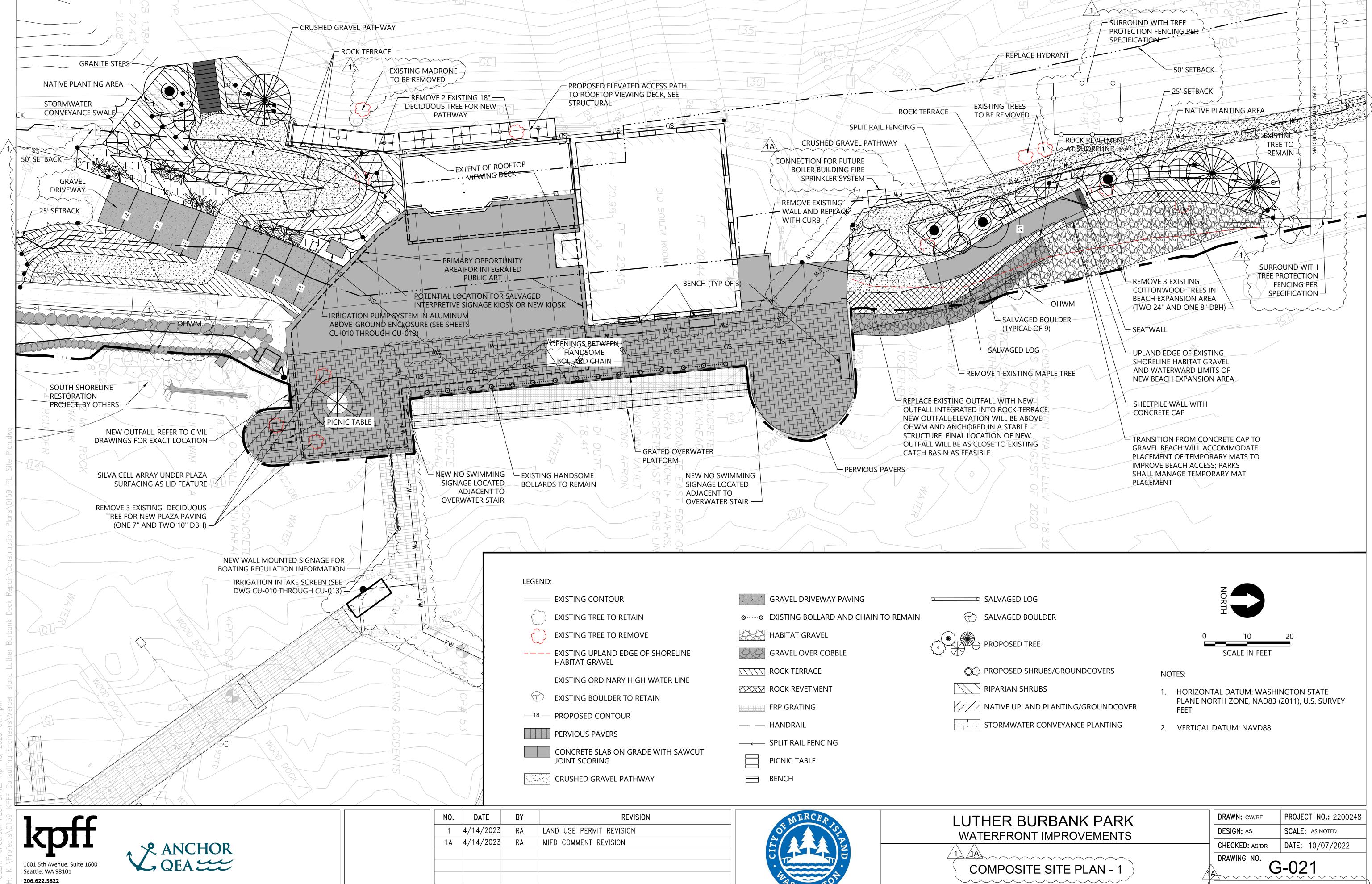
CORING

- CORE 3"Ø HOLE IN EXISTING

LOCATION WITH GPR PRIOR TO

BULKHEAD. CLEAR REBAR

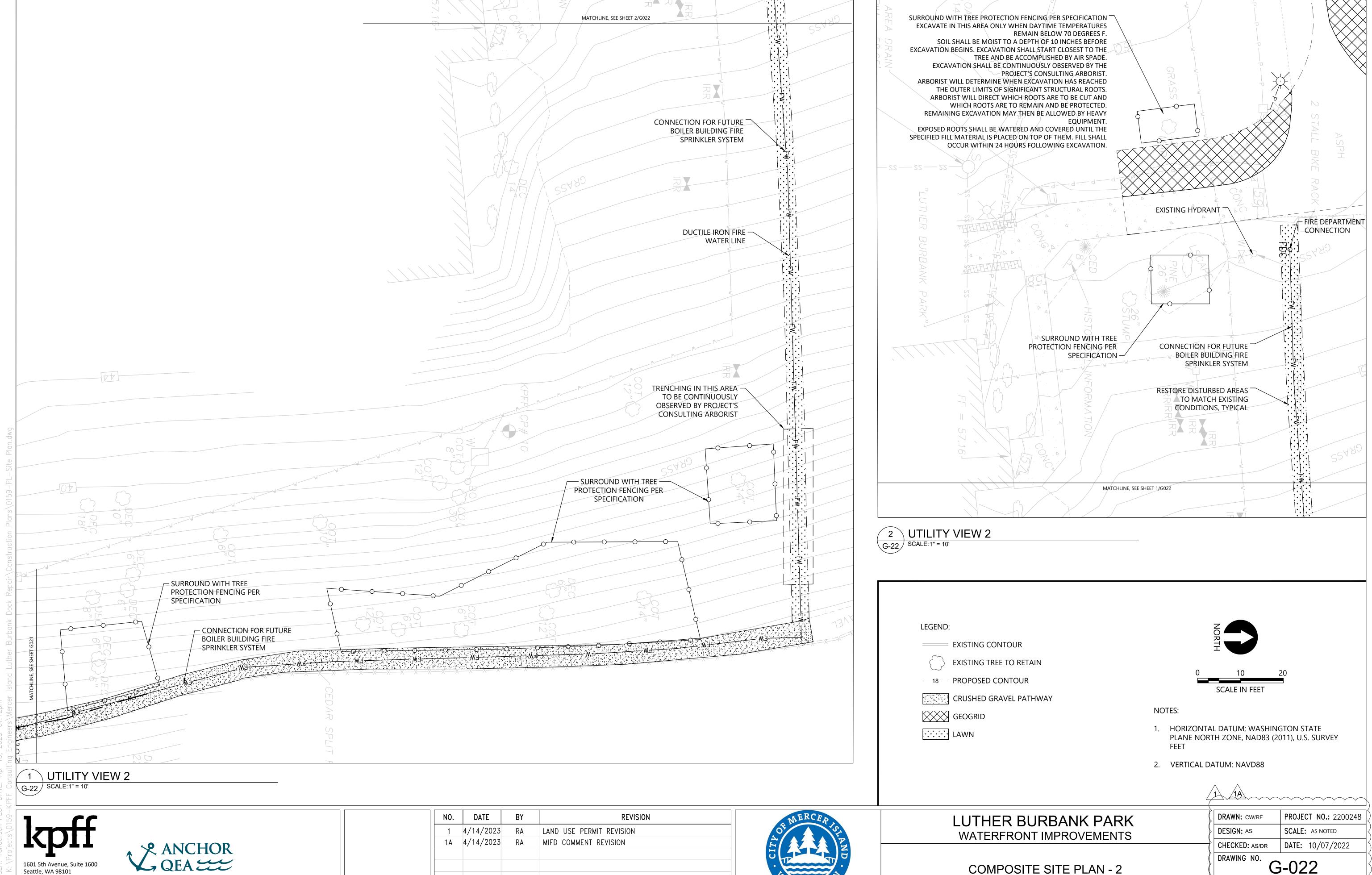
22 of 56 SHEET NO.



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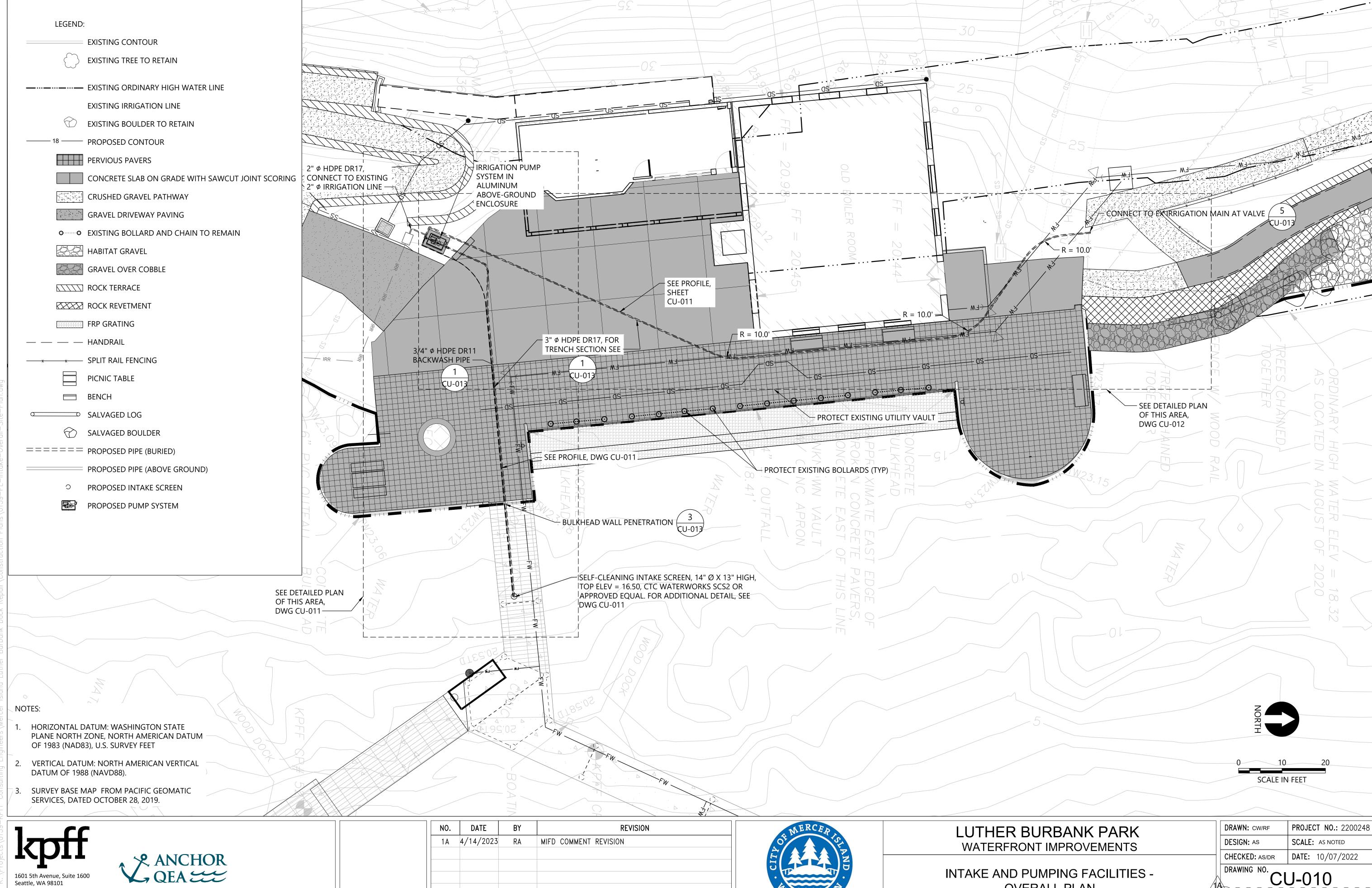
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SHEET NO. 25

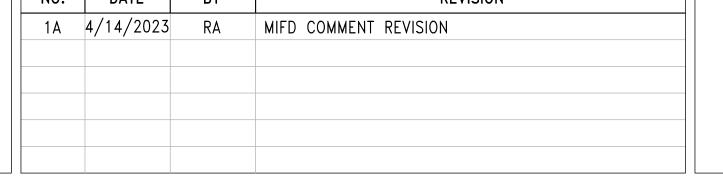


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SHEET NO. 26



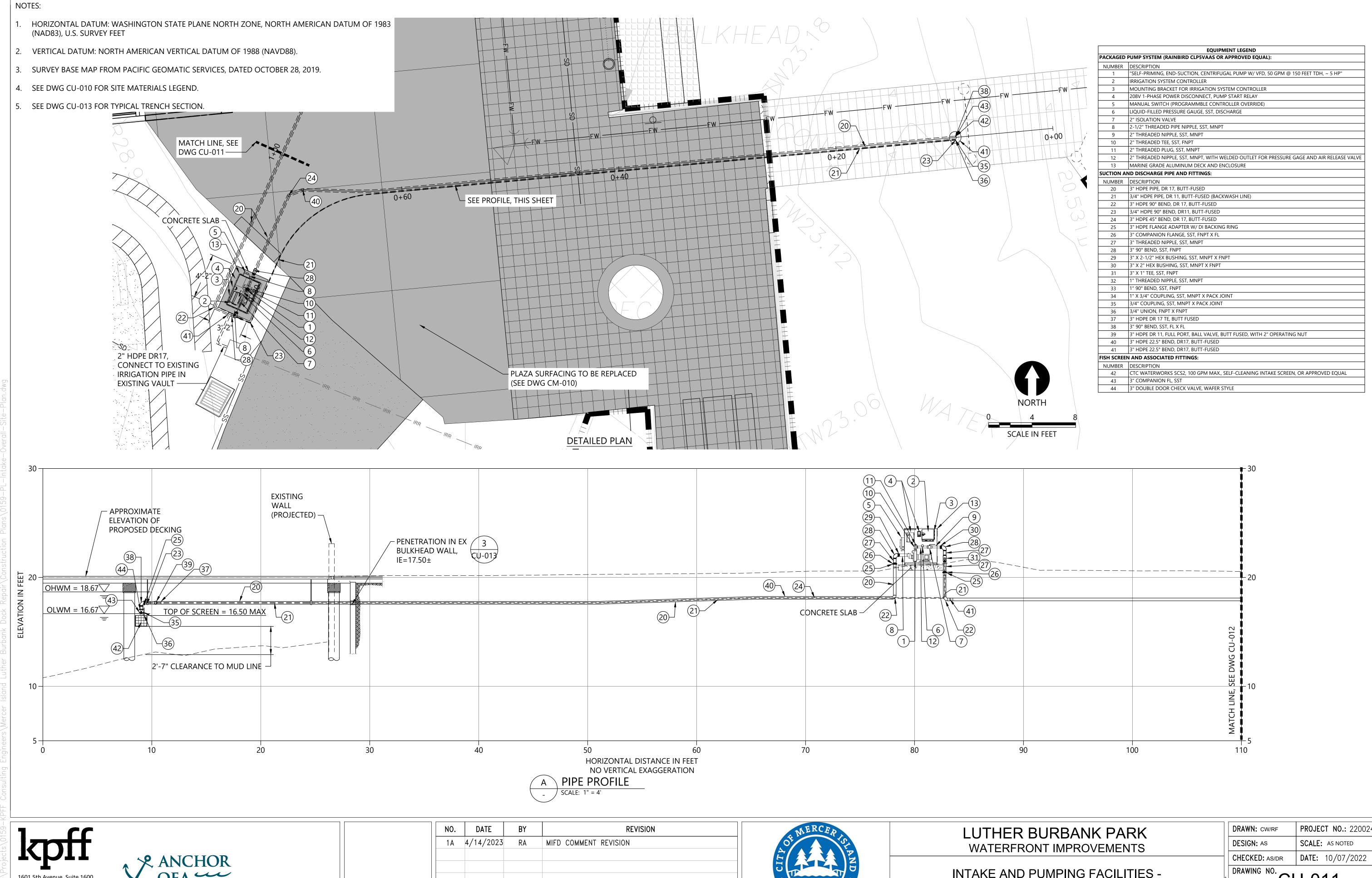






OVERALL PLAN

DRAWN: CW/RF	PROJECT NO.: 2200248				
DESIGN: AS	SCALE: AS NOTED				
CHECKED: AS/DR	<b>DATE:</b> 10/07/2022				
DRAWING NO. CLI_O10					

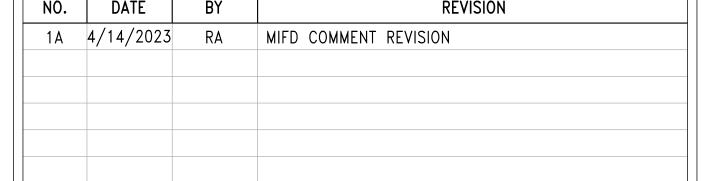




INTAKE AND PUMPING FACILITIES -EQUIPMENT PLAN AND SECTIONS (1 OF 2)

DRAWN: CW/RF	PROJECT NO.: 2200248	ŀ
DESIGN: AS	SCALE: AS NOTED	•
CHECKED: AS/DR	<b>DATE:</b> 10/07/2022	
DRAWING NO. CL	J-011	(
SHEET NO. 28	0F 56	







INTAKE AND PUMPING FACILITIES -EQUIPMENT PLAN AND SECTIONS (2 OF 2)

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DRAWN: CW/RF	PROJECT NO.: 2200248	-		
DESIGN: AS	SCALE: AS NOTED			
CHECKED: AS/DR	<b>DATE:</b> 10/07/2022			
DRAWING NO. CU-012				
	<b>U-U   Z</b>	2		

of **56** 

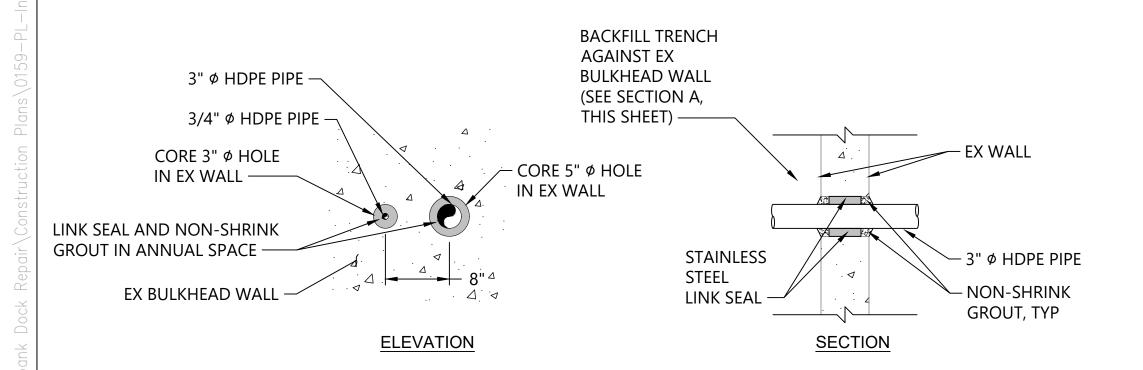
#### TYPICAL TRENCH SECTION NOTES:

- 1. PIPE BEDDING PIPE BEDDING SHALL BE AT LEAST 6 INCHES DEEP AND SHALL MEET THE REQUIREMENTS OF SECTION 9-03.12(3) OF THE STANDARD SPECIFICATIONS. IF EXCAVATED TRENCH BOTTOM IS UNSTABLE OR NOT SUITABLE, THE CONTRACTOR SHALL EXCAVATE TO A DEPTH REQUIRED BY THE ENGINEER AND BACKFILL WITH PIPE BEDDING. PLACE PIPE BEDDING IN MAXIMUM 6-INCH LIFTS AND COMPACT TO 90% OF MAXIMUM DRY DENSITY.
- 2. SELECT BACKFILL SELECT FILL SHALL ALSO MEET THE REQUIREMENTS OF SECTION 9-03.12(3) OF THE STANDARD SPECIFICATIONS. PLACE SELECT BACKFILL IN 6-INCH LIFTS TO A MINIMUM DEPTH OF 6 INCHES ABOVE THE CROWN OF THE PIPE AND COMPACT TO 90% OF MAXIMUM DRY DENSITY.
- 3. FINAL BACKFILL UNDER PATHWAYS AND OTHER HARD SURFACES, THE CONTRACTOR SHALL BACKFILL THE REMAINING PORTION OF THE TRENCH TO THE LINES AND GRADES SHOW WITH 5/8-INCH MINUS CRUSHED ROCK SURFACING COMPACTED TO 95% OF MAXIMUM DRY DENSITY.
- 4. TRACER TAPE AND LOCATOR WIRE TRACER TAPE SHALL MEET THE REQUIREMENTS OF SECTION 9-15.18 OF THE STANDARD SPECIFICATIONS. LOCATOR WIRE SHALL BE 12 GA. COPPER MULTI-STRAND RHW, CERTIFIED FOR DIRECT BURIAL. THE TRACER TAPE AND LOCATOR WIRE SHALL BE INSTALLED ALONG THE ENTIRE PROFILE OF THE PIPE.
- 5. DRAIN ROCK WHERE THE PIPE IS INSTALLED ACROSS OR UNDER DRAIN ROCK PLACED AS PART OF AN INFILTRATION TRENCH ADJACENT TO THE BOILER BUILDING, THE CONTRACTOR SHALL BACKFILL THE TRENCH WITH DRAIN ROCK AND OTHER MATERIALS TO MATCH THE MATERIALS EXCAVATED TO PLACE THE IRRIGATION PIPE.

1 TYPICAL TRENCH SECTION

#### SCALE: NTS

PLACEHOLDER - DETAIL TO BE ADDED AT 90% DESIGN



4 DRAIN VALVE DETAIL
#### SCALE: NTS

PLACEHOLDER - DETAIL TO BE ADDED AT 90% DESIGN

3 BULKHEAD WALL PENETRATION DETAIL
#### SCALE: 1" = 1'

5 EXISTING IRRIGATION MAIN CONNECTION DETAIL

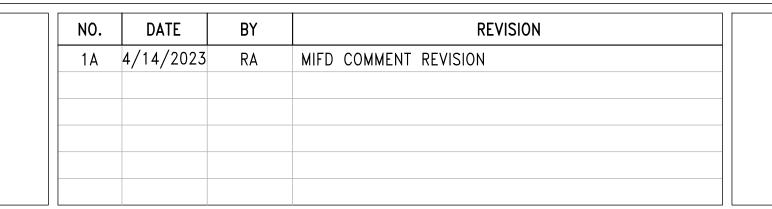
SCALE: NTS

PLACEHOLDER - DETAIL TO BE ADDED AT 90% DESIGN











## LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

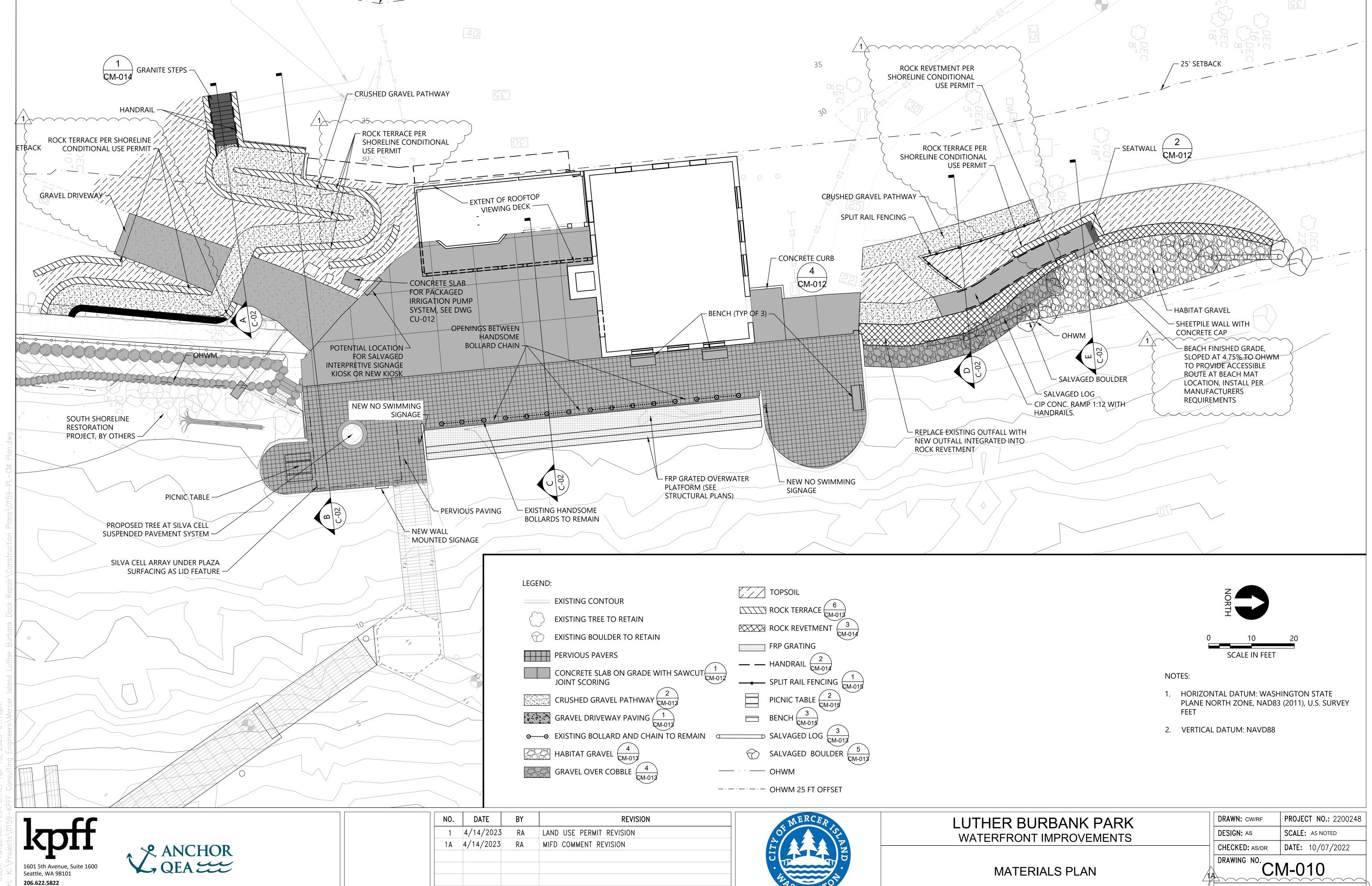
INTAKE AND PUMPING FACILITIES - DETAILS

DRAWN: CW/RF	PROJECT NO.: 2200248			
DESIGN: AS	SCALE: AS NOTED			
CHECKED: AS/DR	<b>DATE:</b> 10/07/2022			
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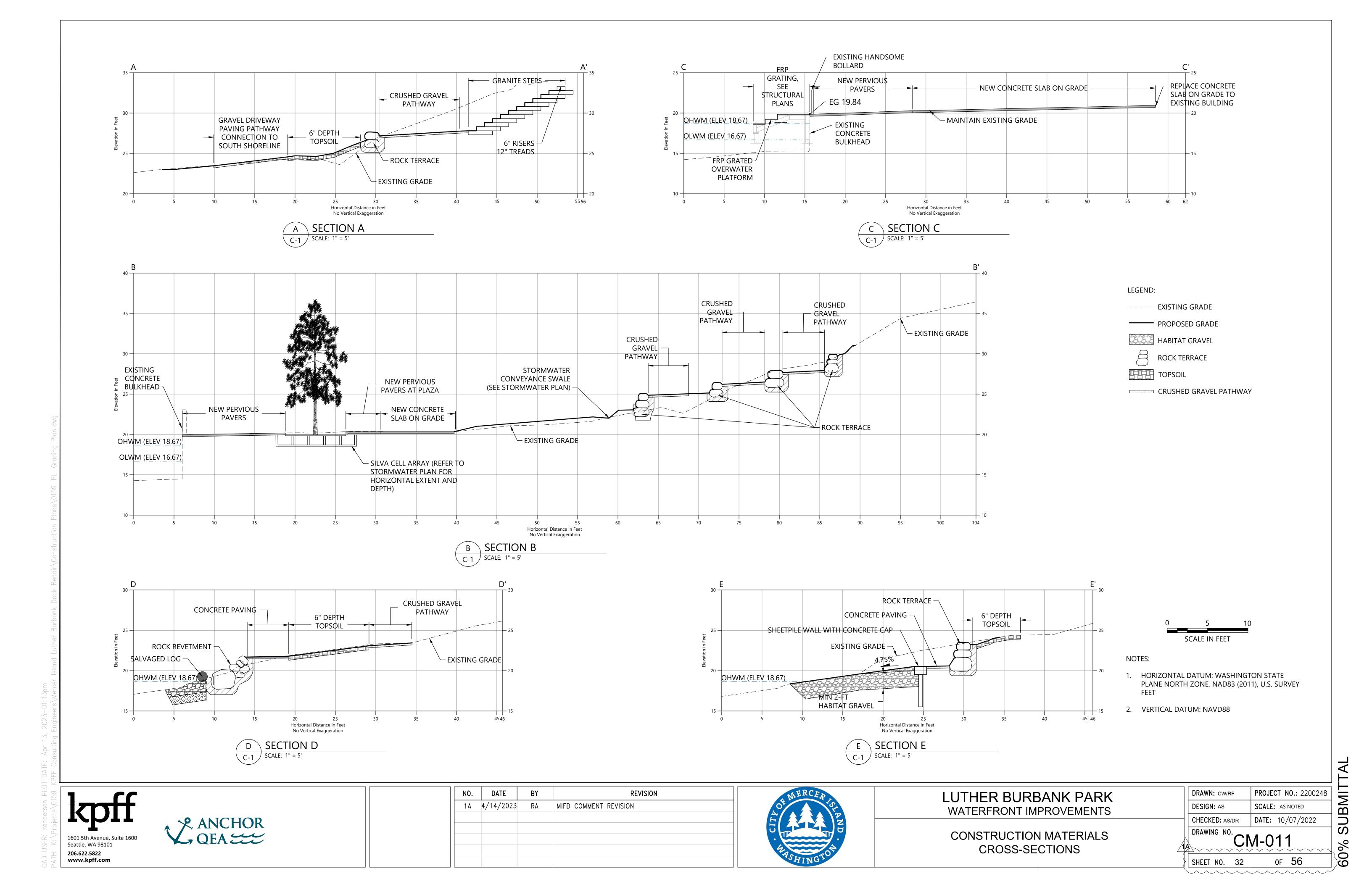
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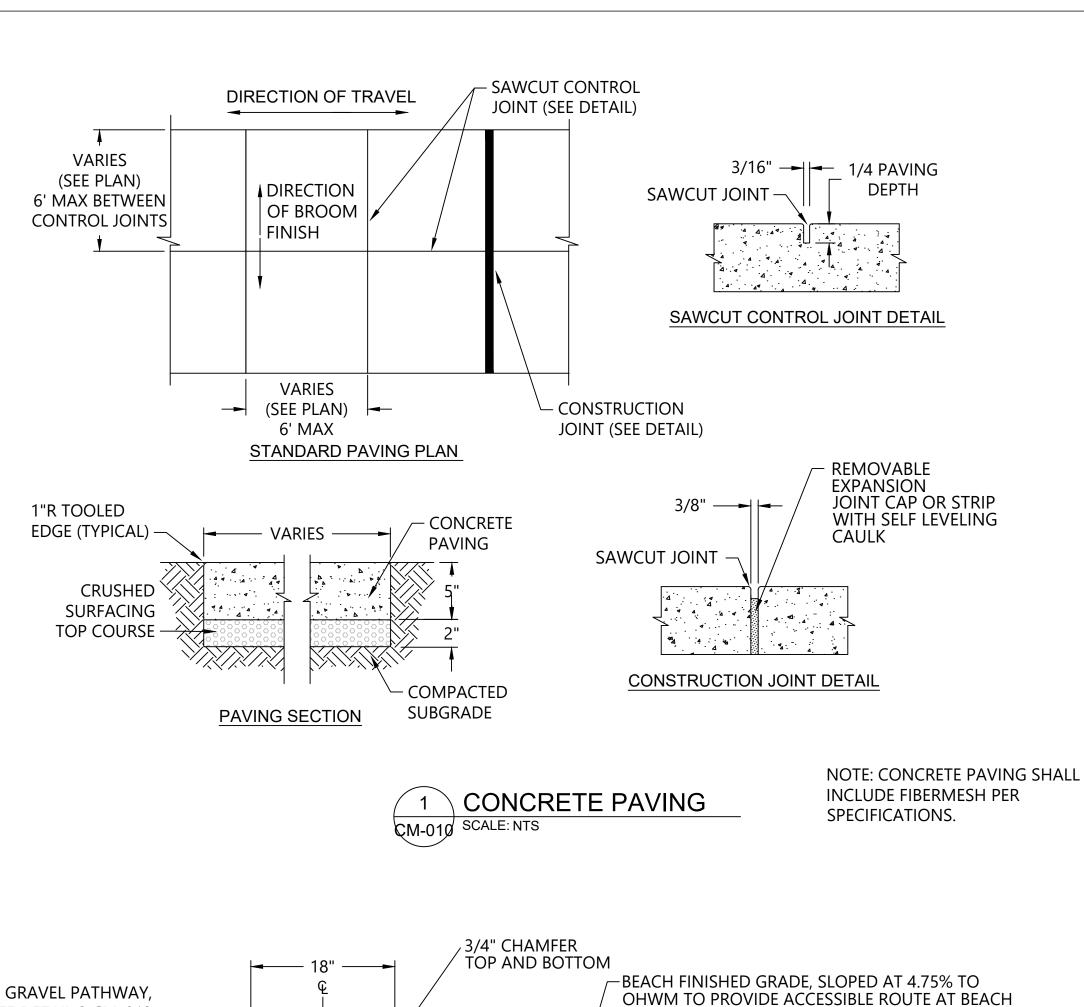


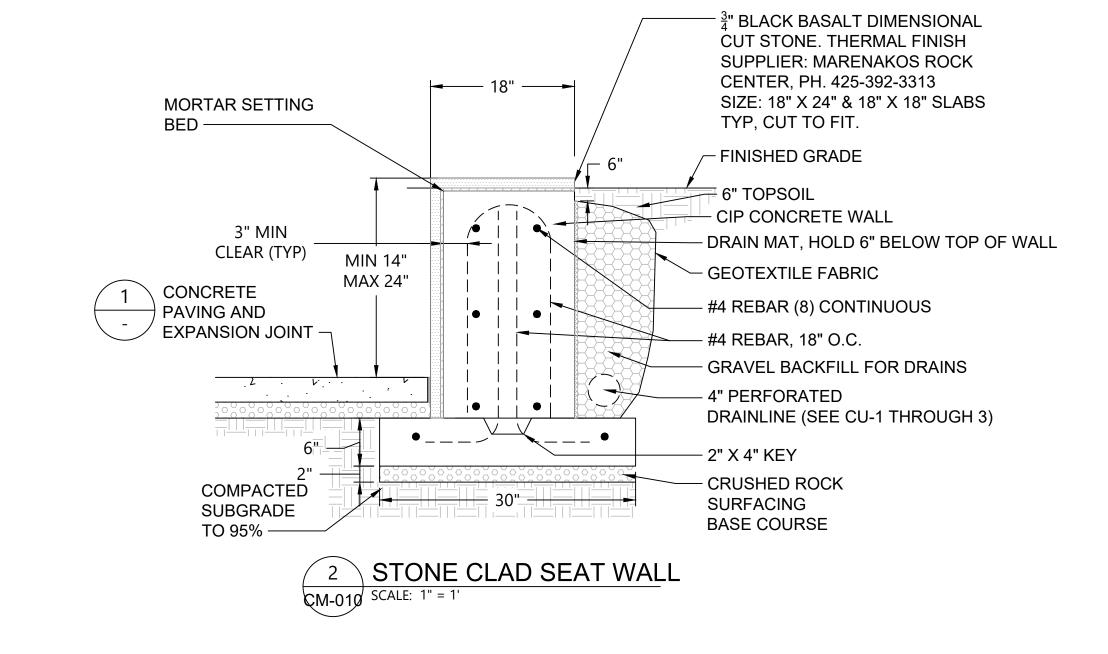
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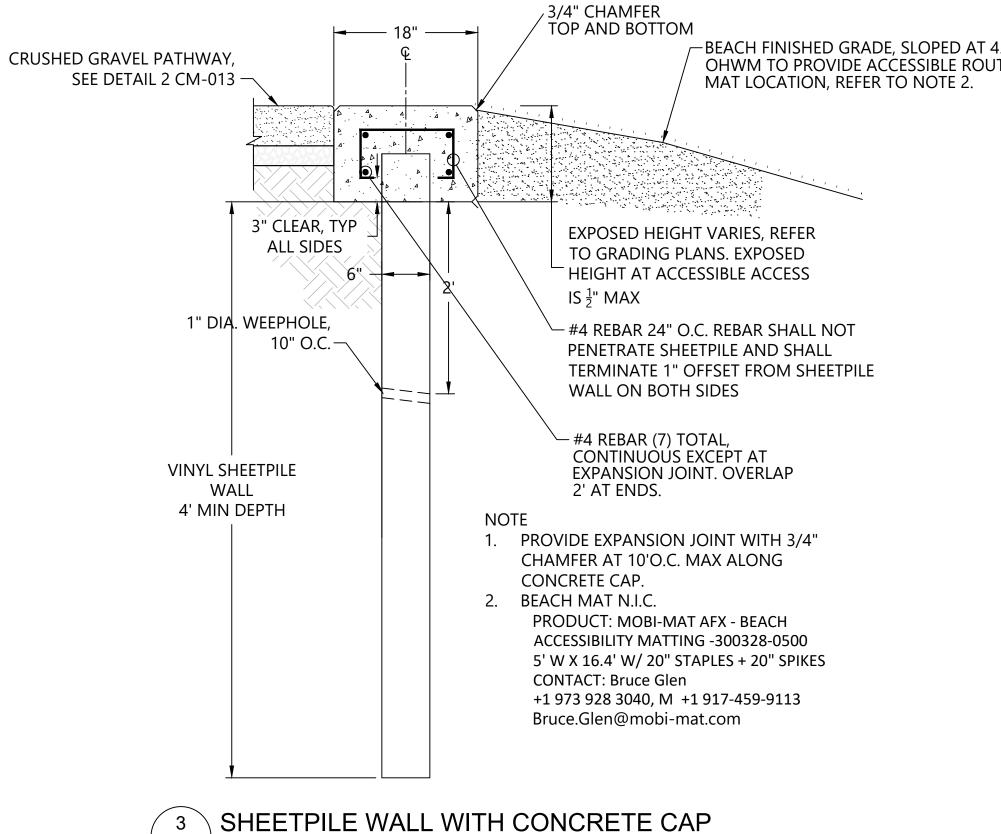
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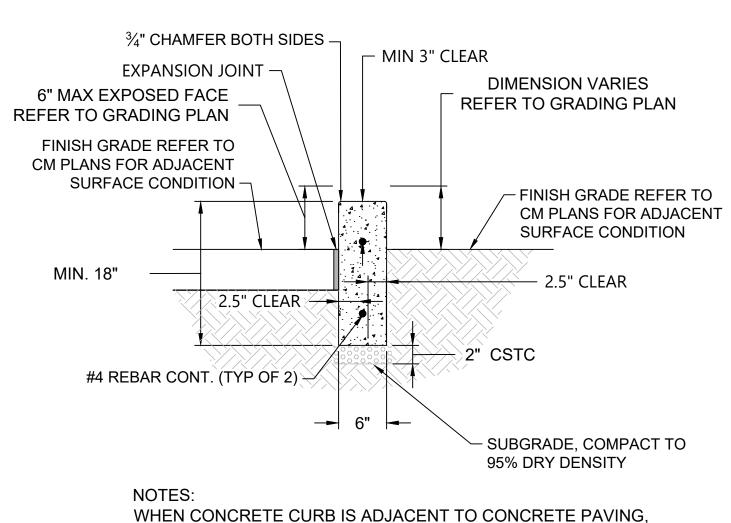
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PLACEHOLDER - DETAIL TO BE ADDED AT 90% DESIGN

INSTALL PRE-MOLDED JOINT FILLER AT INTERFACE, USE SNAP LOCK FILLED WITH SIKA FLEX SELF-LEVELING CAULK.

4 CONCRETE CURB CM-010 SCALE: 1" = 1.0'

OUTFALL CM-010 SCALE: 1" = 1'

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CM-010 SCALE: 1" = 1.0'

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	1A	4/14/2023	RA	MIFD COMMENT REVISION



### LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

CONSTRUCTION MATERIALS DETAILS (SHEET 1 OF 4)

DRAWN: CW/RF	PROJECT NO.: 2200248
DESIGN: AS	SCALE: AS NOTED
CHECKED: AS/DR	<b>DATE:</b> 10/07/2022
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PLACEHOLDER - DETAIL TO BE ADDED AT 90% DESIGN

GRAVEL DRIVEWAY PAVING

CM-010 SCALE: 1" = 1'

FLUSH AT EDGE

4" MIN.

2% MAX. CROSS SLOPE

ADJACENT CONDITIONS AND GRADES VARY

2" MIN. DEPTH 1/4" TO DUST CRUSHED SURFACING COMPACT SUBGRADE TO 95%

2" MIN. DEPTH CRUSHED SURFACING BASE COURSE COMPACTED TO 95% DRY DENSITY

2 CRUSHED GRAVEL PATHWAY

CM-010 SCALE: 1" = 1'

2 CRIMPED - 3/4" PLAIN STEEL **FASTENERS** (UNGALVANIZED) SHACKLE AT EACH SWAGGED END - 3-FT 1/2" DIA. STAINLESS STEEL CABLE WITH SWAGGED END **BURIED IN 2-FT EXISTING** BEACH SURFACE THREADED ROD WITH EYELET PROVIDED WITH ANCHOR EMBEDDED 7-10 FEET PER MANUFACTURER'S INSTRUCTIONS. **CONNECTION DETAIL** 3 SALVAGED LOG CM-010 SCALE: NTS

3-FT CABLE WITH

SWAGGED END

BURIED IN 2-FT (TYP.)

- 3/4" PLAIN STEEL

- SALVAGED LOG

(UNGALVANIZED) CHAIN

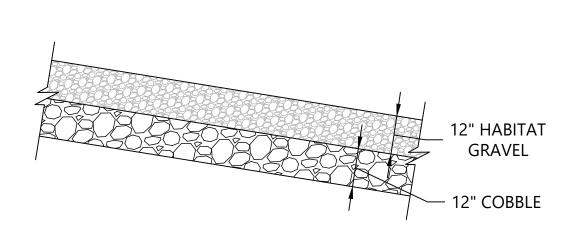
3/4" PLAIN STEEL

THREADED ROD (TYP.)

ANCHOR (TYP.)

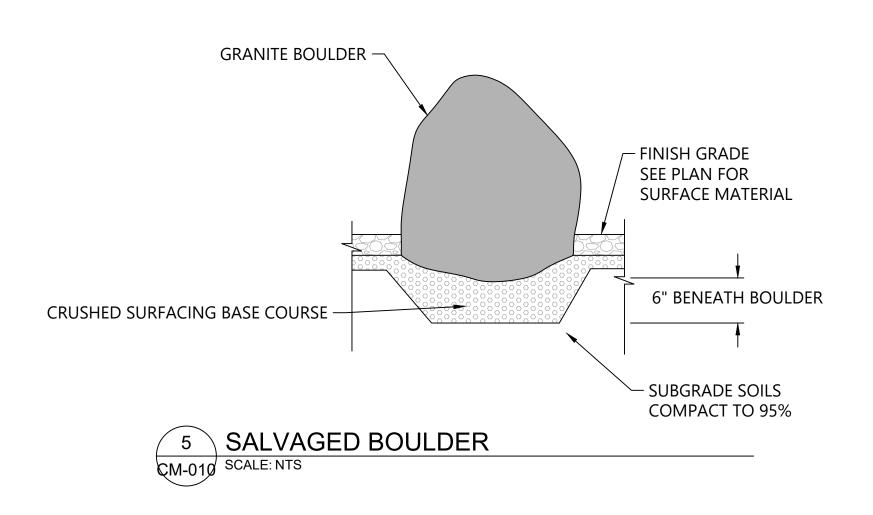
(UNGALVANIZED) CHAIN (TYP.)

SEE CONNECTION DETAIL (TYP.)

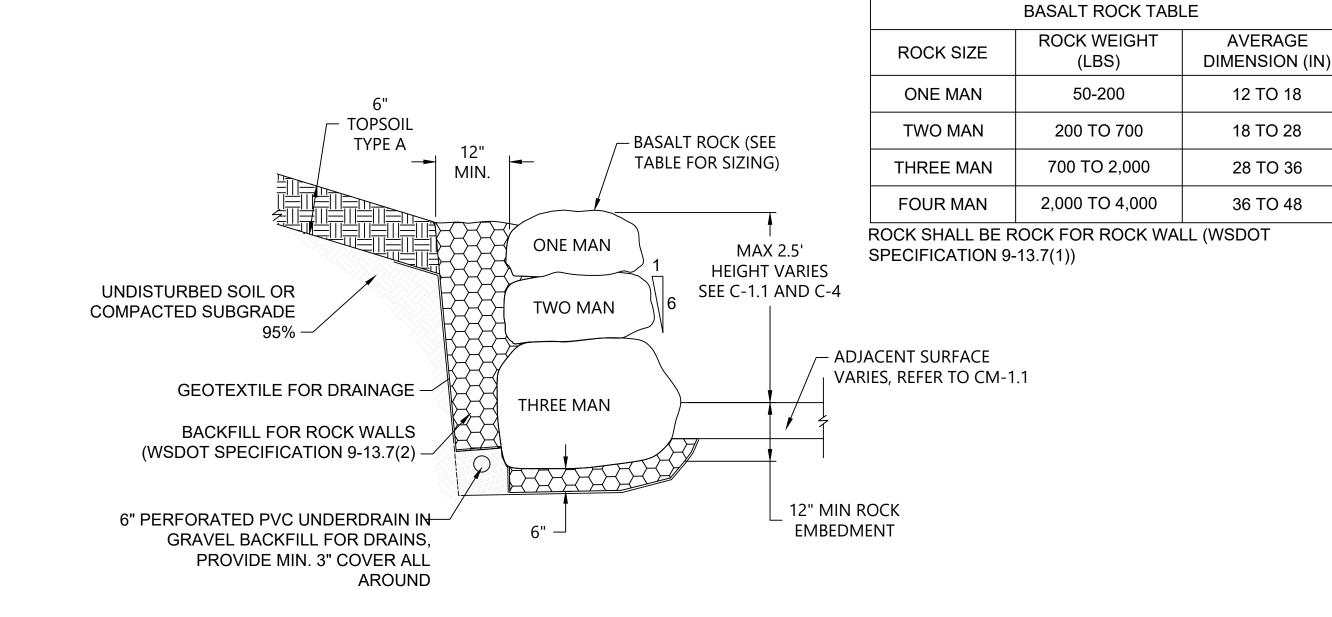


4 HABITAT GRAVEL OVER COBBLE CM-010 SCALE: NOT TO SCALE

REFER TO WSDOT SPECFICAITION SPECIAL PROVISION 9-13.6(1) FOR HABITAT GRAVEL AND 9-13.6(2) BEACH SAND. COBBLE SHALL BE 8" STREAMBED COBBLE PER WSDOT SPECIFICATION 9-03.11(2).



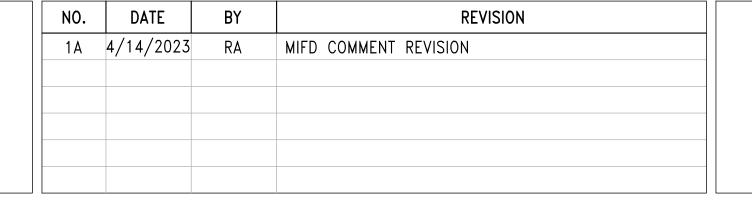
NOTE: INSTALL CRUSHED SURFACING TOP COURSE OR RECYCLED AC BASE FOR BOULDERS 4 C FEET AND LARGER. FOR BOULDERS UNDER 4 CUBIC FEET PLACE ON 95% COMPACTED SUBGRA AND BURY IN 6-12" OF BEACH GRAVEL/RIVER ROCK.





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## LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

BASALT ROCK TERRACE

CM-010 SCALE: NOT TO SCALE

CONSTRUCTION MATERIALS DETAILS (SHEET 2 OF 4)

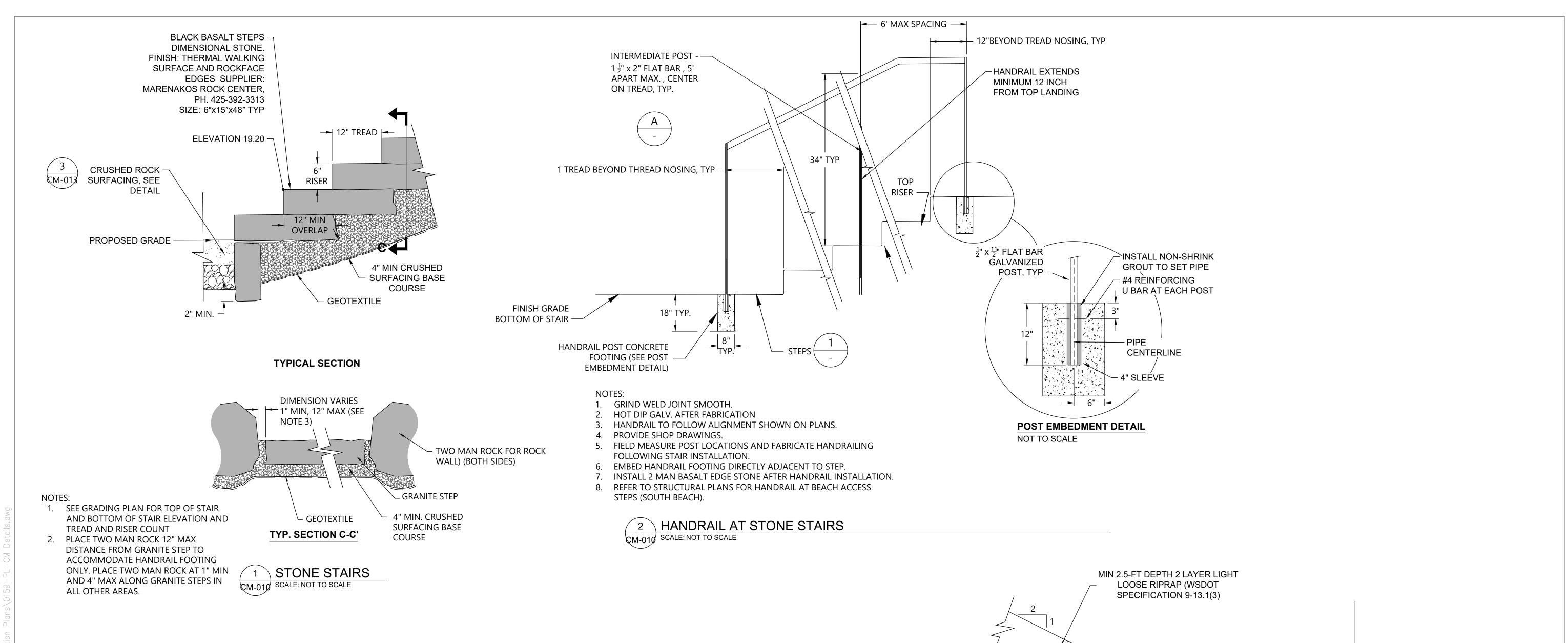
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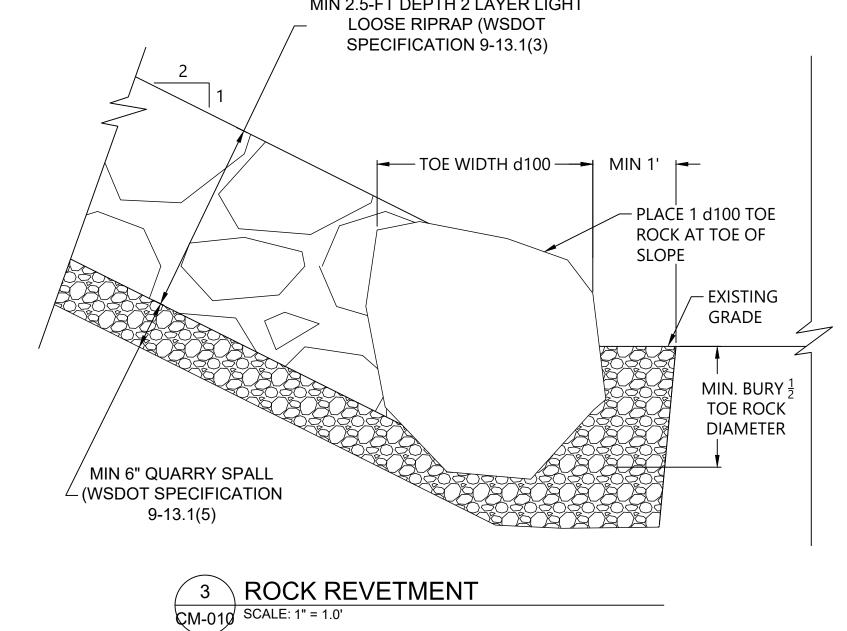
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- SALVAGED LOG

MITTA %09









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4/14/2023	RA	MIFD COMMENT REVISION
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### LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

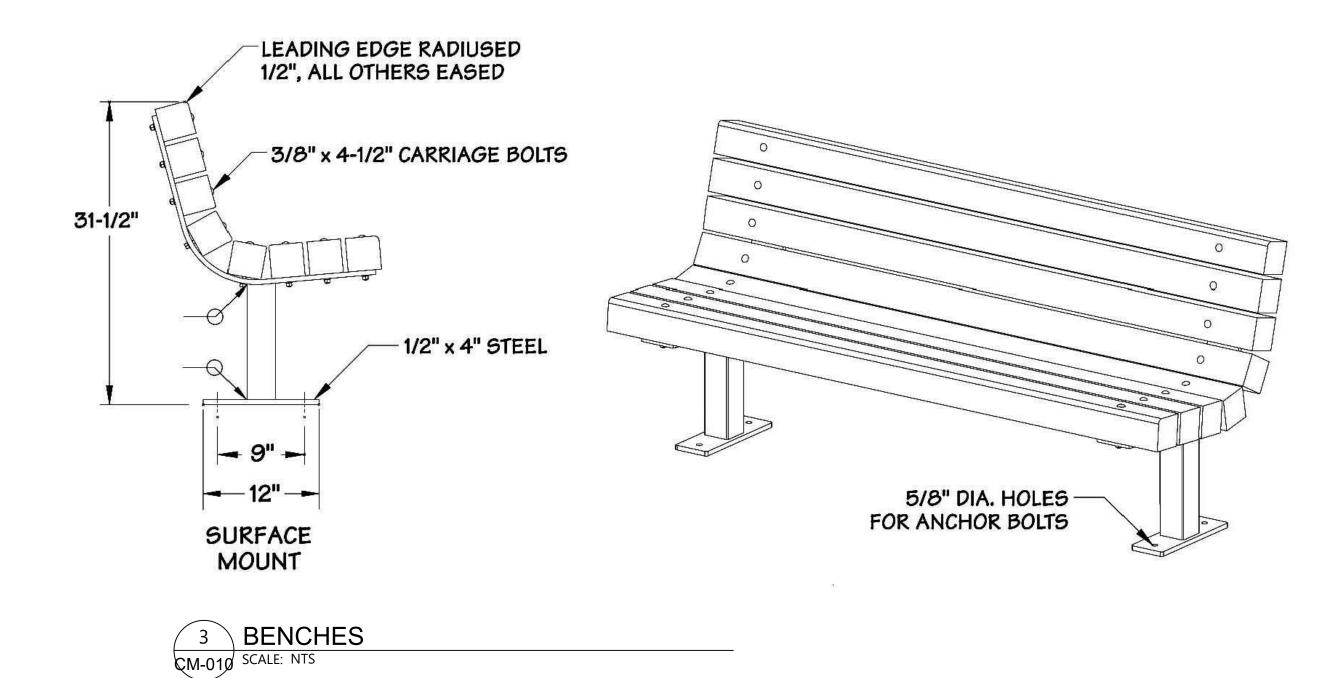
CONSTRUCTION MATERIALS DETAILS (SHEET 3 OF 4)

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>	SHEET NO. 35	of <b>56</b>	90



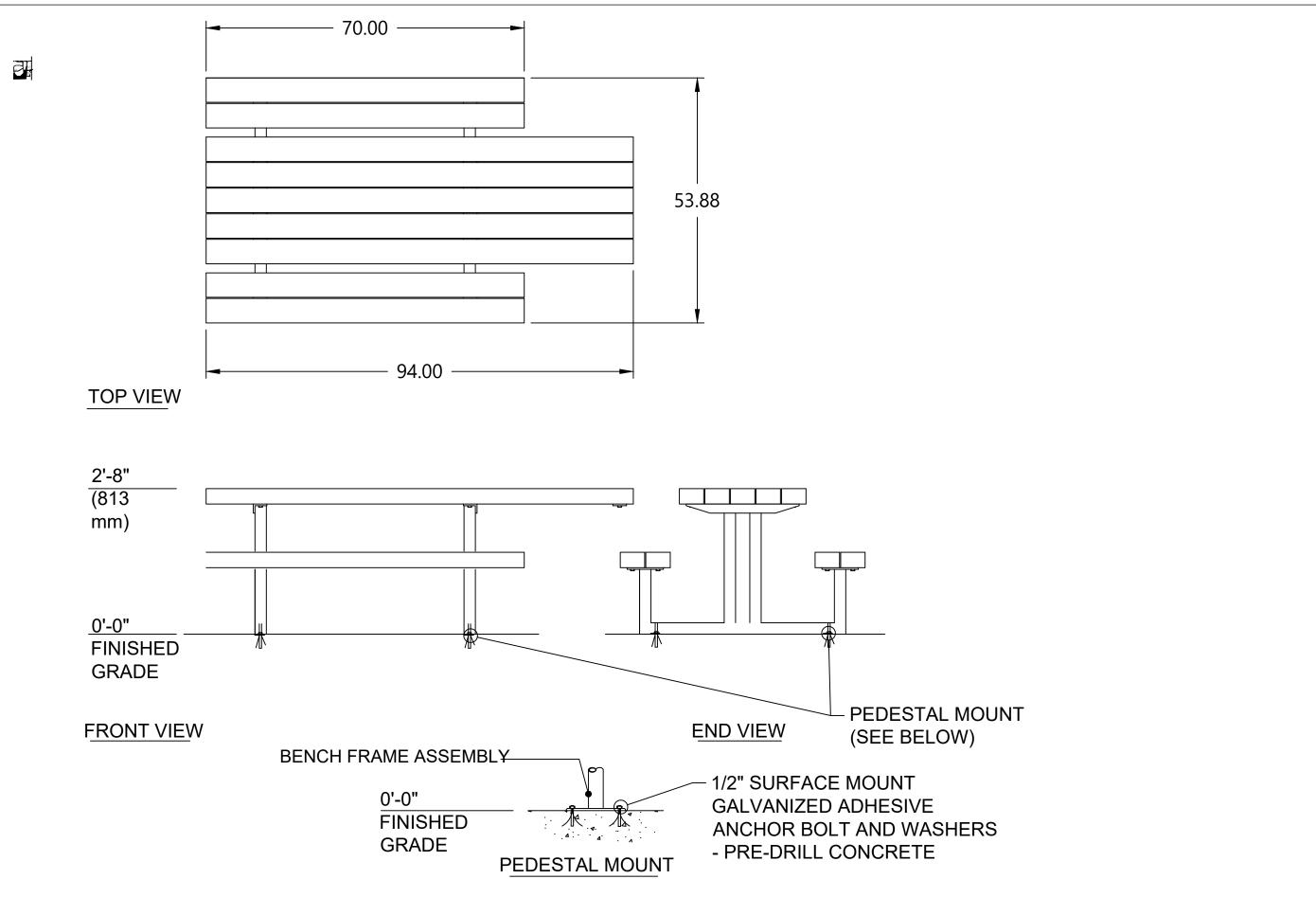
→ 6" MIN.

**FRONT VIEW** 



CONCRETE FOOTING

<del>-</del> 24" <del>-</del>





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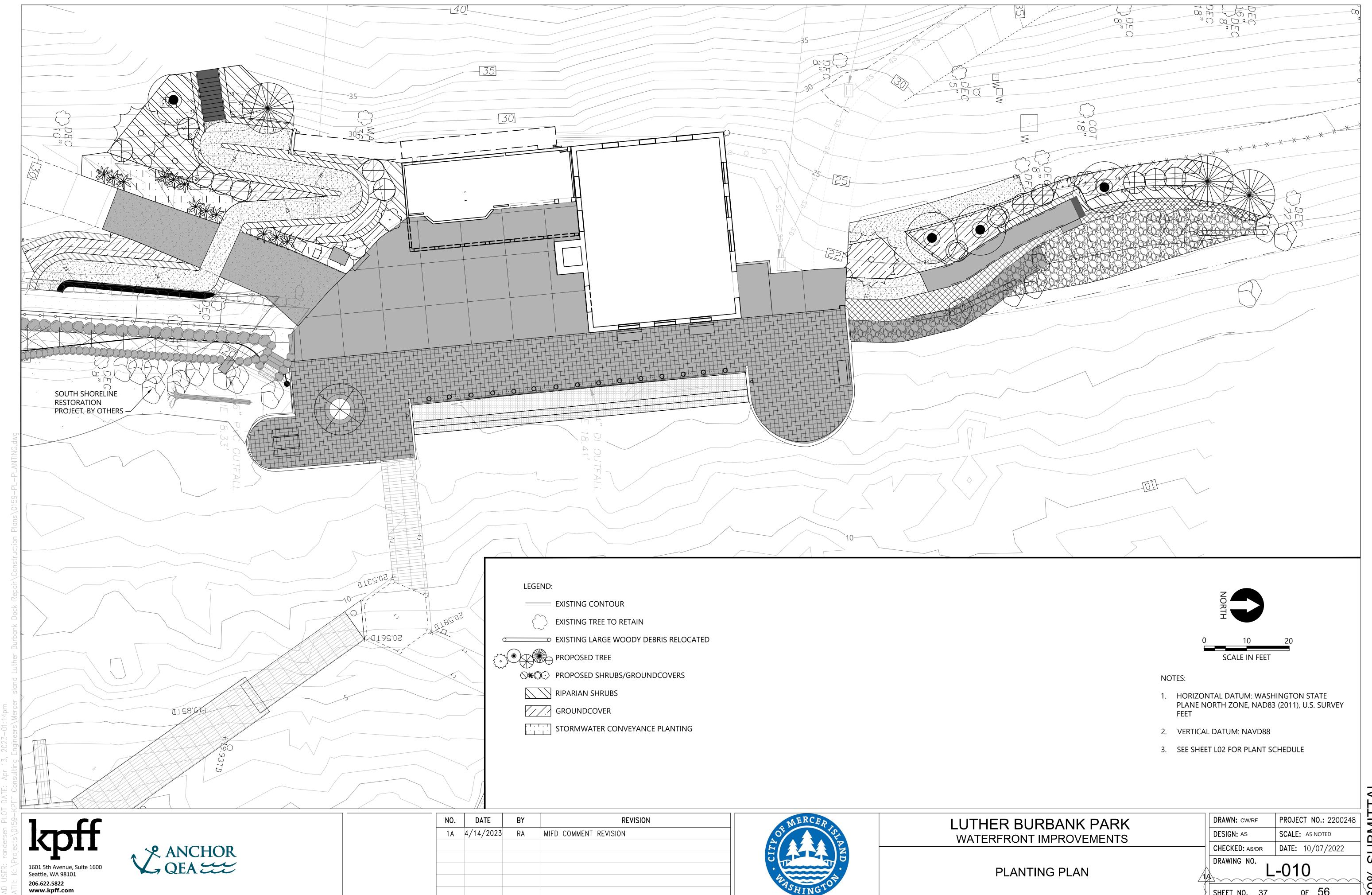


### LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

CONSTRUCTION MATERIALS DETAILS (SHEET 4 OF 4)

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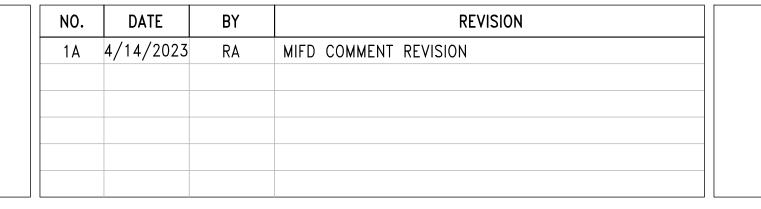
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PLANT QUANTITIES WILL BE PROVIDED AT 90%

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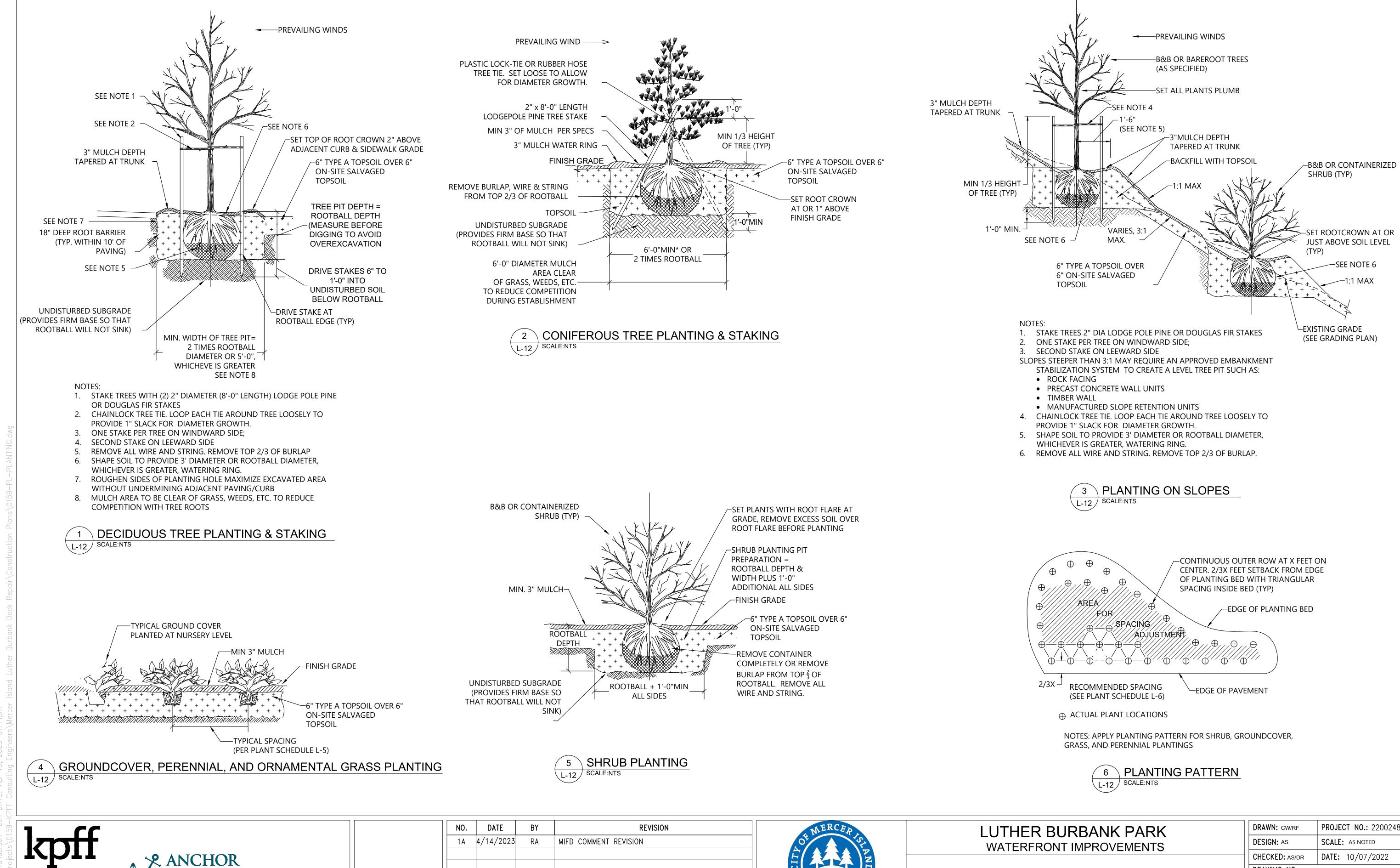


#### LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

PLANT SCHEDULE

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DESIGN: AS	SCALE: AS NOTED	
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PLANTING DETAILS

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^	DRAWING NO.	-012
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DEAD LOADS SHALL INCLUDE THE SELF WEIGHT OF MATERIALS AND COMPONENTS LISTED

REINFORCED CONCRETE = 150 PCF STRUCTURAL STEEL = 490 PCF WOOD/TIMBER = 35 PCF

GANGWAY DL ASSUMED = 105 PLF (HALF SUPPORTED AT EACH END)

DESIGN FREEBOARDS UNDER DEAD LOAD: WAVE ATTENUATOR/MOORING FLOAT = 24" ±2" FINGER FLOATS = 1'-6" ±2" GENERAL PURPOSE FLOAT = 9" ±1" KAYAK FINGER FLOATS = 9" ±1" ADA KAYAK LAUNCH = 9" ±1"

FLOAT CROSS SLOPES UNDER DEAD LOAD ONLY SHALL NOT EXCEED 2% (1:50). SEE MAXIMUM SLOPES BELOW.

FLOATS CONSIST OF FLOAT CONCRETE, STRUCTURAL STEEL, WOOD/TIMBER, FLOTATION PONTOONS, FRP GRATING, FOAM, RUBRAILS, BULLRAILS, CONNECTION HARDWARE, PILE GUIDE BRACKETS. GANGWAYS AND ALL HARDWARE PERMANENTLY ATTACHED TO THE FLOATS INCLUDING STANDPIPES, ETC.

### **LIVE LOADS (ULL & CLL)**

IN ADDITION TO THE DEAD LOADS, THE FOLLOWING LIVE LOADS SHALL BE USED FOR DESIGN. LIVE LOAD REDUCTION IS PER IBC SECTION 1607.11.

FIXED PIER = 50 PSF UNIFORM LIVE LOAD (ULL) GANGWAYS = 100 PSF ULL (FOR DELEGATED STRUCTURAL DESIGN) = 50 PSF ULL (FOR SUPPORT REACTIONS AT EACH END) FLOATS = 25 PSF ULL, 400 LB CONCENTRATED LIVE LOAD (CLL) CLL SHALL ACT OVER A 6"x6" AREA, APPLIED AT ANY POINT ON THE FLOAT DECK NOT CLOSER THAN 12" FROM ANY EDGE.

OTHER = 100 PSF ULL, 300 LB CLL

### MAXIMUM SLOPES

UNDER DL ONLY, DL + ULL, OR DL + CLL: SHALL NOT EXCEED 2% (1:50)

### MAXIMUM LONGITUDINAL SLOPE

UNDER DL ONLY, AND DL +ULL: 1/8 INCH PER FOOT, NOT TO EXCEED 1 INCH IN 10 FEET UNDER DL AND CLL: 1/4 INCH PER FOOT, NOT TO EXCEED 2 INCHES IN 10 FEET

SNOW LOAD THE SNOW LOAD IS DETERMINED USING CHAPTER 7 OF ASCE 7 IN ACCORDANCE WITH IBC SECTION 1608 AND WITH THE FOLLOWING FACTORS:

### MINIMUM DESIGN LOAD 25 PSF PER MERCER ISLAND BUILDING CODE

### SEISMIC LOADS

THE SEISMIC FORCE-RESISTING SYSTEM (SFRS) USED TO RESIST EARTHQUAKE AND WIND LOADS IS COMPRISED OF CANTILEVERED STEEL PILES DESIGNED TO REMAIN ELASTIC FOR CODE LEVEL EARTHQUAKE. EARTHQUAKE DESIGN IS BASED ON THE EQUIVALENT LATERAL FORCE PROCEDURE IN ASCE 7 SECTION 12.8 WITH THE FOLLOWING FACTORS:

```
SITE CLASS C
RISK CATEGORY II
SEISMIC DESIGN CATEGORY D
I_{\rm e} = 1.00
                                                 SECONDS
S_s = 1.388 \, c
                                    R = \overline{1.00}
S_1 = 0.482 \, q
       1.110 g
                                    Ω =
S_{DS} =
       0.483 q
                                          = \overline{1.110}
```

THE SEISMIC FORCE-RESISTING SYSTEM IS COMPRISED OF THE STRUCTURAL CANTILEVERED STRUCTURAL STEEL PILES IDENTIFIED IN PLAN AND/OR ELEVATION.

WIND LOAD IS DETERMINED USING CHAPTERS 26-31 OF ASCE 7 IN ACCORDANCE WITH IBC SECTION 1609 WITH THE FOLLOWING FACTORS:

 $V = C_sW = KIPS$ 

RISK CA	ATEGORY II	$K_{zt}$	=	1.0
EXPOSI	JRE CATEGORY D	K <sub>e</sub>	=	1.0
V =	97.5 MPH	$G_{cpi}$	=	0.0
$V_{asd} =$	75.5 MPH	op.		

MINIMUM WIND LOAD APPLIED TO ALL PROJECTED VERTICAL SURFACES OF DOCKS, FLOATS, AND BERTHED BOATS SHALL NOT BE LESS THAN 15 PSF, APPLIED ABOVE THE WATER SURFACE.

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SOIL LOADS
ALLOWABLE SOIL-BEARING PRESSURE 4000 PSF DL + LL 5333 PSF DL + LL + SEISMIC/WIND PILE CAPACITY

5000 PSF DOWNWARD, END BEARING 350 PSF/LF DOWNWARD, SKIN FRICTION 300 PSF/LF UPWARD, SKIN FRICTION 2000 PSF LATERAL, 0 TO 5FT

4500 PSF LATERAL, > 5FT

# WAVE LOADS

<del>~~~</del> ~~~~	~~~~~	~~~~~
	WAVE ATTENUATOR	GENERAL PURPOSE
	MOORING FLOAT	FLOAT
50-YR WIND WAVES		
EXTREME WAVE HEIGHT	1.8 FT	1.4 FT
PERIOD	1.9 SECONDS	2.7 SECONDS
LENGTH	18.5 FT	37.2 FT
HORIZONTAL WAVE LOAD	600 PLF	200 PLF
BOAT WAKE		
EXTREME WAVE HEIGHT	1.5 FT	0.7 FT
PERIOD	1.9 SECONDS	2.7 SECONDS
LENGTH	18.5 FT	37.2 FT
HORIZONTAL WAVE LOAD	300 PLF	100 PLF

\_\_\_\_\_\_

## **GENERAL NOTES**

SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER PRIOR TO ANY FABRICATION OR CONSTRUCTION FOR ALL STRUCTURAL ITEMS, INCLUDING THE FOLLOWING: CONCRETE OR MASONRY REINFORCEMENT, EMBEDDED STEEL ITEMS, STRUCTURAL STEEL, AND GRATING.

IF THE SHOP DRAWINGS DIFFER FROM OR ADD TO THE DESIGN OF THE STRUCTURAL DRAWINGS, THEY SHALL BEAR THE SEAL AND SIGNATURE OF THE WASHINGTON STATE REGISTERED PROFESSIONAL ENGINEER WHO IS RESPONSIBLE FOR THE DESIGN.

### **DEFERRED SUBMITTALS**

PER IBC SECTION 107.3.4.1, DRAWINGS AND CALCULATIONS FOR THE DESIGN AND FABRICATION OF ITEMS THAT ARE DESIGNED BY OTHERS SHALL BEAR THE SEAL AND SIGNATURE OF THE WASHINGTON STATE REGISTERED PROFESSIONAL ENGINEER WHO IS RESPONSIBLE FOR THE DESIGN AND SHALL BE SUBMITTED TO THE ENGINEER AND THE BUILDING DEPARTMENT FOR REVIEW PRIOR TO FABRICATION. DEFERRED SUBMITTALS INCLUDE BUT ARE NOT LIMITED TO THE FOLLOWING:

### **GANGWAY** WAVE ATTENUATOR/MOORING FLOAT FINGER FLOAT **GENERAL PURPOSE FLOAT** KAYAK FINGER FLOAT

ADA KAYAK LAUNCH

### NONSTRUCTURAL COMPONENTS

ESIGN, DETAILING AND ANCHORAGE OF ALL NONSTRUCTURAL COMPONENTS SHALL BE IN ACCORDANCE WITH IBC SECTION 1613. ASCE 7 CHAPTER 13. AND THE PROJECT SPECIFICATIONS. NONSTRUCTURAL COMPONENTS DESIGNED BY OTHERS SHALL NOT INDUCE TORSIONAL LOADING INTO SUPPORTING STRUCTURAL MEMBERS WITHOUT ADDITIONAL BRACING OF THOSE MEMBERS TO ELIMINATE TORSIONAL FORCES. TORSIONAL BRACING SHALL BE DESIGNED BY THE NONSTRUCTURAL COMPONENT DESIGNER AND APPROVED BY THE ENGINEER.

DESIGN, DETAILING AND CONSTRUCTION OF ALL NONSTRUCTURAL COMPONENTS WHICH ATTACH TO STRUCTURE SHALL ACCOMMODATE CONSTRUCTION TOLERANCES AS ESTABLISHED BY THE STRUCTURAL SPECIFICATIONS

SPECIAL INSPECTION PER IBC CHAPTER 17 SHALL BE PERFORMED BY AN APPROVED TESTING AGENCY AS INDICATED IN THE STATEMENT OF SPECIAL INSPECTIONS AND TESTING. ALL PREPARED SOIL-BEARING SURFACES SHALL BE INSPECTED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF REINFORCING STEEL. SOIL COMPACTION SHALL BE SUPERVISED BY AN APPROVED TESTING AGENCY OR GEOTECHNICAL ENGINEER.

STRUCTURAL OBSERVATION OF THE SFRS WILL BE PERFORMED BY THE STRUCTURAL ENGINEER OF RECORD IN ACCORDANCE WITH IBC SECTION 1704.6. STRUCTURAL OBSERVATION CONSISTS OF VISUAL OBSERVATION OF THE STRUCTURAL SYSTEMS FOR GENERAL CONFORMANCE TO THE CONSTRUCTION DOCUMENTS AND DOES NOT INCLUDE OR WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED BY THE IBC AND AS SHOWN IN THE SPECIAL INSPECTIONS SCHEDULE. CONTRACTOR SHALL PROVIDE A

72 HOURS NOTICE BEFORE CONCEALING THE FOLLOWING STRUCTURAL COMPONENTS FROM VIEW:

REINFORCING STEEL FOR THE FIRST PLACEMENT OF THE FOLLOWING ELEMENTS: SFRS

STRUCTURAL OBSERVATIONS IN ADDITION TO THOSE REQUIRED BY IBC SECTION 1704.6 MAY BE PERFORMED AT THE ENGINEER'S DISCRETION. TIMING OF THESE SHALL BE DISCUSSED AT THE PREINSTALLATION CONFERENCE.

CONTRACTOR SHALL VERIFY ALL LEVELS, DIMENSIONS, AND EXISTING CONDITIONS IN THE FIELD BEFORE PROCEEDING. CONTRACTOR SHALL NOTIFY THE ENGINEER OF ANY DISCREPANCIES OR FIELD CHANGES PRIOR TO INSTALLATION OR FABRICATION. IN CASE OF DISCREPANCIES BETWEEN THE EXISTING CONDITIONS AND THE DRAWINGS, THE CONTRACTOR SHALL OBTAIN DIRECTION FROM THE ENGINEER BEFORE PROCEEDING. DIMENSIONS NOTED AS PLUS OR MINUS (±) INDICATE UNVERIFIED DIMENSIONS AND ARE APPROXIMATE. NOTIFY ENGINEER IMMEDIATELY OF CONFLICTS OR EXCESSIVE VARIATIONS FROM INDICATED DIMENSIONS. NOTED DIMENSIONS TAKE PRECEDENCE OVER SCALED DIMENSIONS--<u>DO NOT SCALE DRAWINGS</u>. DIMENSIONS OF EXISTING CONDITIONS ARE BASED ON THE 1973 PIER PLANS AND 1988 FINGER PIER PLANS AND ARE TO BE FIELD-VERIFIED BY THE CONTRACTOR.

CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS BEFORE COMMENCING ANY DEMOLITION, CONTRACTOR SHALL PROVIDE ADEQUATE SHORING AND BRACING OF ALL STRUCTURAL MEMBERS, EXISTING CONSTRUCTION AND SOIL EXCAVATIONS, AS REQUIRED, AND IN A MANNER SUITABLE TO THE WORK SEQUENCE. TEMPORARY SHORING AND BRACING SHALL NOT BE REMOVED UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE DRAWINGS AND MATERIALS HAVE ACHIEVED

FIELD LOCATE REINFORCING BARS, TENDONS, AND EMBEDS AND PROVIDE A MINIMUM OF 2" CLEARANCE TO ALL CONCRETE CORES AND CUTS. NO REINFORCING BARS, TENDONS, OR EMBEDS IN EXISTING CONSTRUCTION SHALL BE CUT UNLESS DIRECTED TO BY THE ENGINEER OR AS SHOWN ON THE DRAWINGS.

CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES REQUIRED TO PERFORM THE

SEE THE GEOTECHNICAL REPORT BY GEOENGINEERS, INC, DATED MORE COMPLETE INFORMATION. EARTHWORK MATERIAL, BACKFILL AND COMPACTION SHALL BE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT. BACKFILL BEHIND WALLS SHALL NOT BE PLACED BEFORE THE WALLS AND SUPPORTING SLABS ACHIEVE 28 DAY CONCRETE STRENGTH OR THE WALLS ARE TEMPORARILY BRACED. ALL TOPSOIL ORGANICS AND LOOSE SURFACE SOIL SHALL BE REMOVED FROM BENEATH FILL SUPPORTING CONCRETE SLABS OR PAVING.

ALL FRAMING MEMBERS SHALL BE EQUALLY SPACED BETWEEN GRID LINES, COLUMNS, AND DIMENSIONED FRAMING UNLESS NOTED OTHERWISE.

### CONCRETE

CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF IBC CHAPTER 19

STRUCTURAL NOTES

CONCRETE MIXTURES SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:

	CONCRETE MIXTURES							
f'c(PSI)	TEST	<b>EXPOSURE CLASS</b>		ASS	USE			
	AGE(DAYS)	F	S	W	C	03L		
4,000	28	F_	S_	W_	C_	FOUNDATIONS, CONCRETE PILE CAPS		

CONCRETE MIXTURES SHALL CONFORM TO THE MOST STRINGENT REQUIREMENTS FOR EXPOSURE CLASSES SPECIFIED IN THE TABLE ABOVE AND ACI 318 TABLE 19.3.2.1.

WATER-REDUCING ADMIXTURES MAY BE INCORPORATED IN CONCRETE MIX DESIGNS, BUT SHALL CONFORM TO ASTM C 494, AND BE USED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. CaCl2 OR OTHER WATER-SOLUBLE CHLORIDE ADMIXTURES SHALL NOT BE USED.

WATER/CEMENTITIOUS MATERIALS RATIO SHALL BE MEASURED BY WEIGHT AND SHALL BE BASED ON THE TOTAL CEMENTITIOUS MATERIAL. WATER/CEMENTITIOUS MATERIALS RATIO AND WATER CONTENT SHALL BE DETERMINED BY THE SUPPLIER BASED ON STRENGTH REQUIREMENTS AND SHALL NOT EXCEED THE MAXIMUM WATER/CEMENTITIOUS MATERIAL RATIO AND/OR WATER CONTENT IF SHOWN ABOVE OR IN ACI 318 TABLE 19.3.2.1 FOR THE EXPOSURE CLASSES LISTED.

FIELD-MEASURED SLUMP SHALL CONFORM TO THE SUBMITTED CONCRETE MIX DESIGN. TOLERANCE OF SLUMP SHALL CONFORM TO ASTM C 94.

ALL CONCRETE SUBJECT TO EXPOSURE CLASSES F1. F2 OR F3 SHALL BE AIR ENTRAINED. AIR-ENTRAINING AGENTS SHALL CONFORM TO ASTM C 260. THE AMOUNT OF ENTRAINED AIR SHALL BE ACCORDING TO ACI 318 TABLE 19.3.3.1 WITH A FIELD TOLERANCE OF ±1.5 PERCENT BY VOLUME. THE AMOUNT OF ENTRAINED AIR SHALL BE MEASURED IN THE FIELD AT THE DISCHARGE FROM THE TRUCK.

THE CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS FOR APPROVAL 2 WEEKS PRIOR TO PLACING ANY CONCRETE. THE MIX DESIGN SHALL BE IN CONFORMANCE WITH ACI 318, CHAPTER 19. THE SUBMITTAL SHALL INDICATE WHERE EACH CONCRETE MIX IS TO BE USED ON THE PROJECT, AS WELL AS THE MAXIMUM AGGREGATE SIZE OF EACH MIX. MAXIMUM AGGREGATE SIZE SHALL CONFORM TO THE PROJECT SPECIFICATIONS.

IF THE AIR TEMPERATURE WILL EXCEED 75 DEGREES F WITHIN 48 HOURS OF PLACING CONCRETE, A MOIST CURE SHALL BE APPLIED TO THE CONCRETE FOR A PERIOD OF 36 HOURS AFTER FINISHING CONCRETE SURFACES. REFER TO THE PROJECT SPECIFICATIONS FOR CURING REQUIREMENTS.

### REINFORCING STEEL **DEFORMED BARS**

HEADED DEFORMED BARS

ASTM A 615, GRADE 60 ASTM A 970, HEAD TYPE HA

REINFORCING SHALL BE SUPPORTED AS SPECIFIED BY THE PROJECT SPECIFICATIONS AND THE CRSI MANUAL OF STANDARD PRACTICE. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH ACI STANDARD OF PRACTICE AS OUTLINED IN ACI 315, "GUIDE TO PRESENTING REINFORCING STEEL DESIGN DETAILS".

LAP ALL REINFORCING BARS AS NOTED ON THE DRAWINGS. WHERE SPLICE LENGTH IS NOT SHOWN, USE TYPE Lb (Lbt FOR TOP BARS) SPLICE PER DEVELOPMENT AND SPLICE LENGTH SCHEDULE. MECHÀNICAL SPLICES CALLED OUT ON THE PLANS SHALL BE TYPE 1, UNLESS OTHERWISE NOTED. TYPE 1 SPLICES SHALL DEVELOP 125 PERCENT OF THE YIELD CAPACITY OF THE SPLICED BARS IN BOTH TENSION AND COMPRESSION. TYPE 2 SPLICES SHALL DEVELOP THE SPECIFIED TENSILE STRENGTH OF THE SPLICED BARS IN TENSION IN ADDITION TO MEETING TYPE 1 SPLICE REQUIREMENTS. SUBMIT ICC-ES OR IAPMO UES REPORT VALID FOR THE 2018 IBC DEMONSTRATING COMPLIANCE OF COUPLERS WITH THESE REQUIREMENTS.

AT THE CONTRACTOR'S OPTION AND WITH THE ENGINEER'S APPROVAL HEADED DEFORMED BARS MAY BE USED IN LIEU OF REINFORCING BARS SHOWN WITH STANDARD 90 OR 180 DEGREE HOOKS AND MECHANICAL SPLICES MAY BE USED IN LIEU OF LAP SPLICES. USE OF HEADED DEFORMED BARS IS SUBJECT TO CONFORMANCE WITH ACI 318 SECTION 25.4.4. USE OF MECHANICAL SPLICES IS SUBJECT TO CONFORMANCE WITH ACI 318 SECTION 18.2.7 AND REQUIRES SUBMITTAL OF AN ICC-ES OR IAPMO UES REPORT VALID FOR THE 2018 IBC.

REINFORCING STEEL SHALL HAVE PROTECTION AS FOLLOWS, UNLESS NOTED

<u>USE</u>	
BEAM STIRRUPS AND COLUMN TIES	
NONSTRUCTURAL SLAB-ON-GRADE	
STRUCTURAL SLAB-AT-GRADE BOTTOM BAF	₹S
WALL BARS:	

**EXPOSED TO EARTH OR WEATHER** 

FOOTING, PILE CAP, GRADE BEAM BOTTOM BARS TOP BARS

1 1/2" (#5 AND SMALLER) (#6 AND LARGER) (CAST AGAINST ÉARTH) 1 1/2" (#6 AND LARGER WHERE **EXPOSED TO EARTH OR** WEATHER)

PER DETAILS

ASTM A 706, GRADE 60, LOW ALLOY

WELDING OF REINFORCING, WHERE APPROVED BY THE ENGINEER, SHALL BE PERFORMED USING LOW HYDROGEN ELECTRODES AND PREHEATED IN ACCORDANCE WITH AWS D1.4, REINFORCING STEEL WELDING CODE. WELDERS AND WELDING PROCEDURES SHALL BE QUALIFIED IN ACCORDANCE WITH AWS D1.4. MATERIALS SHALL CONFORM TO THE FOLLOWING:

E80XX

SIDE BARS

WELDING ELECTRODES

REINFORCING BARS TO BE WELDED

NONSHRINK GROUT
BASE PLATE GROUT SHALL BE NONSHRINK TYPE WITH MINIMUM fc = 8,000 PSI. ALL OTHER NONSHRINK GROUT SHALL HAVE MINIMUM f'c = 5,000 PSI.

# **ABBREVIATIONS LIST**

<u>+</u>	PLUS OR MINUS, TOLERANCE, CONTRACTOR VERIFY
<b>@</b>	AT
ADA	AMERICANS WITH DISABILITIES ACT
APPROX	APPROXIMATE, APPROXIMATELY
BLDG	BUILDING
BTM	BOTTOM
CL, Q	CENTERLINE
CLR	CLEAR

CONT CONTINUOUS DBL DOUBLE DIA, Ø DIAMETER EACH EΑ **EXISTING GRADE** ELEVATION EL, ELEV EQUAL, EQUALLY

CONCRETE

CONNECTION

CLR

CONC

CONN

EXIST, (E) EXISTING FDC FIRE DEPARTMENT CONNECTION FRP FIBERGLASS REINFORCED PLASTIC GALV GALVANIZED HDG HOT DIPPED GALVANIZED

HOLLOW STRUCTRUAL SECTION INVERT ELEVATION INFO INFORMATION JOINT ANGLE POUND MAX MAXIMUM MB MALLEABLE BOLT

MIN MINIMUM NTS NOT TO SCALE OC ON CENTER **OHWM** ORDINARY HIGH WATER MARK, ELEVATION 18.67 ORDINARY LOW WATER MARK, ELEVATION 16.75

SYMMETRY, SYMMETRICAL

OPP OPPOSITE PCF POUNDS PER CUBIC FOOT PLATE PLF POUNDS PER LINEAR FOOT PT PRESSURE TREATED REINF REINFORCEMENT SIM SIMILAR SPA SPACED, SPACING, SPACES

TYP **TYPICAL UHMW** ULTRA HIGH MOLECULAR WEIGHT UNO UNLESS NOTED OTHERWISE UV **ULTRA VIOLET** 

VIF VERIFY IN FIELD **WORK POINT** 

**SYMM** 

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	NO.	DATE	BY	REVISION
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**LUTHER BURBANK PARK** WATERFRONT IMPROVEMENTS

DESIGN: WBC SCALE: AS SHOWN CHECKED: AKB DATE: 10/7/2022 DRAWING NO.

DRAWN: RRT

| PROJECT NO.: 2200248

STRUCTURAL NOTES

# STRUCTURAL NOTES

### **ANCHORS**

**POST-INSTALLED ANCHORS** 

PROVIDE POST-INSTALLED ANCHORS PER THE FOLLOWING SCHEDULE UNLESS NOTED OTHERWISE:

ANCHORS IN CONCRETE							
ANCHOR TYPE	APPROVED ANCHOR(S)	<b>EVALUATION REPORT</b>					
ADHESIVE	-	ICC-ES ESR					
MECHANICAL	-	ICC-ES ESR					

ADHESIVE REINFORCING DOWEL MATERIALS

ASTM A 615, GRADE 60 ADHESIVE REINFORCING DOWELS (ARD) THREADED ARD ASTM F 1554, GRADE 36 (CARBON STEEL) ASTM A193 B8M CLASS 1 (STAINLESS)

ANCHOR EMBEDMENT DEPTHS LISTED SHALL BE CONSIDERED EFFECTIVE EMBEDMENT DEPTHS AS DEFINED IN THE ICC-ES OR IAPMO UES EVALUATION REPORTS. PROVIDE ANCHOR LENGTH AND HOLE PER EVALUATION REPORT TO ACCOMMODATE THE EFFECTIVE EMBEDMENT SPECIFIED IN THESE DRAWINGS.

MECHANICAL AND ADHESIVE ANCHORS SHALL BE ZINC PLATED CARBON STEEL UNLESS NOTED OTHERWISE. MECHANICAL AND ADHESIVE ANCHORS EXPOSED TO WEATHER SHALL BE STAINLESS STEEL.

DO NOT DAMAGE EXISTING REINFORCEMENT. IF LOCATION OF REINFORCEMENT IS UNKNOWN, SCAN FOR EXISTING REINFORCING STEEL PRIOR TO DRILLING.

USE OF ALTERNATE PRODUCTS, OR OF POST-INSTALLED ANCHORS AT LOCATIONS NOT SHOWN IN THESE DRAWINGS, IS SUBJECT TO THE APPROVAL OF THE ENGINEER. SUBMIT PROPOSED ANCHORS TO THE ENGINEER WITH AN ICC-ES OR IAPMO UES REPORT VALID FOR THE 2018 IBC AND DOCUMENTATION SHOWING THAT THE ALTERNATE PRODUCTS PROVIDE EQUIVALENT CAPACITY FOR ALL CONDITIONS IN THIS PROJECT, SUBMITTED ICC-ES AND IAPMO UES REPORTS SHALL DEMONSTRATE THAT THE ANCHORS ARE SUITABLE FOR USE IN CRACKED CONCRETE. WHERE ANCHORS RESIST SEISMIC LOADS OR SUSTAINED TENSION, SUBMITTED ICC-ES AND IAPMO UES REPORTS SHALL DEMONSTRATE THAT THE ANCHORS ARE SUITABLE FOR THE RESISTANCE OF SEISMIC LOADS OR SUSTAINED TENSION (AS APPLICABLE). DOCUMENTATION OF CAPACITY FOR ALTERNATE PRODUCTS MUST BE INCLUDED AS A DEFERRED SUBMITTAL.

ADHESIVES SHALL NOT BE INSTALLED PRIOR TO THE CONCRETE REACHING AN AGE OF 21 DAYS AS REQUIRED BY ACI 318.

ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCLINED TO SUPPORT SUSTAINED TENSION LOADS SHALL BE INSTALLED BY PERSONNEL CERTIFIED BY THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT PROGRAM.

WELDED HEADED STUDS, WELDED THREADED STUDS, AND DEFORMED BAR ANCHORS ALL STUDS AND DEFORMED BAR ANCHORS (DBA) SHALL BE AUTOMATICALLY END WELDED IN SHOP OR FIELD WITH EQUIPMENT RECOMMENDED BY MANUFACTURER WITH LENGTH AFTER WELD AS SHOWN ON THE STRUCTURAL DRAWINGS.

WELDED HEADED STUDS WELDED THREADED STUDS

DEFORMED BAR ANCHORS

AWS D1.1 TYPE B

<u>SIZE</u> 3/4"Ø UNLESS NOTED OTHERWISE AWS D1.1 TYPE A PER DETAILS **ASTM A 1064** 1/2"Ø UNLESS NOTED OTHERWISE

AISC 360 "SPECIFICATION FOR STRUCTURAL STEEL **BUILDINGS**"

AWS PREQUALIFIED JOINT DETAILS

HIGH STRENGTH BOLTS RCSC "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS"

STRUCTURAL STEEL

AWS D1.1, TYPICAL WELDING

WELDER CERTIFICATION AMERICAN WELDING SOCIETY (AWS) WASHINGTON ASSOCIATION OF BUILDING

**STEEL MATERIALS** WIDE FLANGE SHAPES (W AND WT)

PLATES (PL), BARS ANGLES (L), CHANNELS (C AND MC) ASTM A 500, GRADE C

STRUCTURAL TUBES (HSS) STEEL PIPE STEEL PIPE PILES STRUCTURAL BOLTS ANCHOR RODS

THREADED RODS

WELDING ELECTRODES

ASTM A 992 ASTM A 36 TYPICAL. ASTM A 572 GRADE 50 WHERE NOTED ASTM A 36

OFFICIALS (WABO)

ASTM A 53, GRADE B ASTM A 252, GRADE 3 (MOD) FY = 50 KSI ASTM F 3125, GRADE A 325 ASTM F 1554, GRADE 36 **UNLESS NOTED OTHERWISE** ASTM A 36, UNLESS NOTED OTHERWISE 70 KSI, LOW HYDROGEN, TYPICAL

STRUCTURAL STEEL DESIGN, FABRICATION AND ERECTION SHALL CONFORM TO THE REQUIREMENTS OF IBC CHAPTER 22. ALL MEMBERS ARE TO BE ERECTED WITH NATURAL MILL CAMBER OR INDUCED CAMBER UP, UNLESS OTHERWISE NOTED ON THE PLANS. SUBSTITUTION OF MEMBER SIZES OR STEEL GRADE WILL NOT BE ALLOWED WITHOUT

60 KSI, MINIMUM, STEEL DECK AND COLD-FORMED

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDS AND JOINT PREPARATIONS THAT INCLUDE, BUT ARE NOT LIMITED TO, ERECTION ANGLES, LIFT HOLES AND OTHER AIDS, WELDING PROCEDURES, REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPES, SURFACE ROUGHNESS VALUES, AND UNEQUAL PARTS.

FRAMING

PRIOR APPROVAL BY THE ENGINEER. A MINIMUM OF TWO BOLTS IS REQUIRED FOR ALL

BEAM CONNECTIONS. ALTERNATIVE CONNECTIONS TO THOSE SHOWN ON THESE

DRAWINGS WILL REQUIRE PRIOR APPROVAL BY THE ENGINEER

STRUCTURAL STEEL AND CONNECTIONS, INCLUDING PLATES AND OTHER STEEL ITEMS EMBEDDED IN CONCRETE, WHICH ARE EXPOSED TO WEATHER AND NOT TO BE PAINTED ACCORDING TO THE ENGINEER, SHALL BE HOT-DIPPED GALVANIZED AFTER FABRICATION IN COMPLIANCE WITH ASTM A 123. ALL FIELD WELDS ON GALVANIZED MATERIAL SHALL BE COATED WITH BRUSH APPLIED ZINC-RICH PAINT COMPLYING WITH THE SPECIFICATIONS.

ALL COATINGS ARE TO FOLLOW THE SPECIFICATIONS AND PRODUCT MANUFACTURER'S INSTRUCTIONS.

ALL WELDING SHALL BE IN CONFORMANCE WITH AISC AND AWS STANDARDS, AND SHALL BE PERFORMED BY AWS-WABO-CERTIFIED WELDERS. ONLY WELDS THAT ARE PREQUALIFIED, AS DEFINED BY AWS, OR QUALIFIED BY TESTING SHALL BE USED. SHOP DRAWINGS SHALL SHOW ALL WELDING WITH AWS A2.4 SYMBOLS. WELDS SHOWN ON THE DRAWINGS ARE MINIMUM SIZES. INCREASE WELD SIZE TO AWS MINIMUM SIZES BASED ON THICKNESS. MINIMUM WELD SIZE SHALL BE 3/16-INCH, UNLESS NOTED OTHERWISE. THE WELDS SHOWN ARE FOR THE FINAL CONNECTIONS. FIELD WELD SYMBOLS ARE SHOWN WHERE FIELD WELDS ARE REQUIRED BY THE STRUCTURAL DESIGN. WHERE FIELD WELD IS NOT INDICATED. THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING IF A WELD SHOULD BE SHOP OR FIELD-WELDED IN ORDER TO FACILITATE THE STRUCTURAL STEEL

## PILE DRIVING

PILES SHALL BE DRIVEN TO: THE REQUIRED TIP ELEVATION DEFINED IN THE PLANS, NO GREATER THAN 12" ABOVE FINAL CUTOFF ELEVATION, WITHIN 2 INCHES OF PLAN LOCATION: WITHIN 1% FROM VERTICAL.

PILE DRIVING BY VIBRATORY, IMPACT OR OTHER INSTALLATION MEANS MAY BE REQUIRED. TO REACH REQUIRED TIP ELEVATION. OBSTRUCTIONS MAY BE ENCOUNTERED DURING PILE DRIVING.

PILE DRIVING AND PILE ACCEPTANCE CRITERIA SHALL BE ESTABLISHED IN COORDINATION WITH THE GEOTECHNICAL ENGINEER BASED ON EQUIPMENT SELECTED BY THE CONTRACTOR.

### WOOD

WOOD CONSTRUCTION SHALL CONFORM TO ALL REQUIREMENTS OF IBC CHAPTER 23.

SAWN LUMBER SHALL CONFORM TO THE LATEST EDITION OF "GRADING AND DRESSING RULES" BY WCLIB OR "WESTERN LUMBER GRADING RULES" BY WWPA. LUMBER SHALL BE SEASONED DRY WITH A MAXIMUM MOISTURE CONTENT OF 19% AND BE THE SPECIES AND GRADE SPECIFIED BELOW.

E (DOI) (OINIOLE LIGE

<u>USE</u>	<u>GRADE</u>	F <sub>b</sub> (PSI) (SINGLE USE)
PLANKING & PLATES 2" TO 4" THICK, 2" AND WIDER	HEM-FIR NO. 2 DOUGLAS FIR-LARCH NO. 2	850 900
JOISTS & RAFTERS 2" TO 4" THICK, 2" AND WIDER	HEM-FIR NO. 2 DOUGLAS FIR-LARCH NO. 2	850 900
BEAMS & STRINGERS 5"x5" AND LARGER	DOUGLAS FIR-LARCH NO. 1	1,350
POSTS 5"x5" AND LARGER 4"X4"	DOUGLAS FIR-LARCH NO. 1 DOUGLAS FIR-LARCH NO. 1	1,200 1,000

ROUND TIMBER PILES AND POLES

MBER PILES SHALL CONFORM TO ASTM D 25 AND TIMBER POLES SHALL CONFORM TO ASTM D 3200. PILES AND POLES SHALL BE PACIFIC COAST DOUGLAS FIR AND SHALL HAVE A MINIMUM CIRCUMFERENCE OF INCHES WITH LENGTHS AS INDICATED. PILES AND POLES SHALL HAVE A MINIMUM ALLOWABLE COMPRESSIVE STRESS OF 1,300 PSI PARALLEL TO GRAIN.

PRESERVATIVE TREATMENT

SAWN LUMBER FOR THE OVERWATER STRUCTURE SHALL BE PRESERVATIVE TREATED PER IBC 2304.12.3. PRESERVATIVE AND FINAL RETENTION SHALL BE IN ACCORDANCE WITH AWPA U1 FOR FRESH WATER EXPOSURE (USE CATEGORY 4A, COMMODITY SPECIFICATION A). PENTACHLOROPHENOL, CREOSOTE, CHROMATED COPPER ARSENATE (CCA) OR COMPARABLY TOXIC COMPOUNDS ARE NOT PERMITTED. PRESERVATIVE TREATMENTS SHALL MEET ALL CURRENT BEST MANAGEMENT PRACTICES AS DESCRIBED BELOW.

TIMBER PILES SHALL BE PRESERVATIVE TREATED PER IBC 2304.12.3.1 AND 1810.3.2.4.1. PRESERVATIVE AND MINIMUM FINAL RETENTION SHALL BE IN ACCORDANCE WITH AWPA U1 FOR FRESH WATER EXPOSURE (USE CATEGORY 4C, COMMODITY SPECIFICATION E). PENTACHLOROPHENOL, CREOSOTE, CCA OR COMPARABLY TOXIC COMPOUNDS ARE NOT PERMITTED. AMMONIACAL COPPER ZINC ARSENATE (ACZA) IS PERMITTED AND SHALL MEET ALL CURRENT BEST MANAGEMENT PRACTICES AS DESCRIBED BELOW.

PRESERVED WOOD PRODUCTS SHALL BE PRODUCED IN ACCORDANCE WITH THE MOST CURRENT VERSION OF THE "PRODUCTION GUIDE - BEST MANAGEMENT PRACTICES FOR THE USE OF PRESERVED WOOD IN AQUATIC AND SENSITIVE ENVIRONMENTS" (BMP'S) ISSUED BY WESTERN WOOD PRESERVERS INSTITUTE (WWPI) AND AVAILABLE AT WWW.PRESERVEDWOOD.COM.

ALL PRESERVED WOOD SHALL BE CERTIFIED BY AN INDEPENDENT THIRD-PARTY INSPECTION AGENCY TO HAVE BEEN PRODUCED IN COMPLIANCE WITH THE BMP'S. COMPLIANCE WILL BE DOCUMENTED BY THE PRESENCE OF THE BMP MARK LEGIBLY STAMPED, BRANDED, MARKED, END TAGGED, OR AN EQUIVALENT DESIGNATION ON EACH PIECE OF MATERIAL OR LOT ARRIVING ON SITE; OR IN LIEU OF PLACING THE BMP MARK ON EACH PIECE OF MATERIAL OR LOT, A CERTIFICATE OF COMPLIANCE ISSUED AND SIGNED BY A QUALIFIED INSPECTION AGENCY CERTIFYING THE MATERIAL AND/OR ITS PRODUCTION WAS INSPECTED IN COMPLIANCE WITH PROCEDURES PUBLISHED IN THE MOST CURRENT VERSION OF THE BMP'S. THE BMP MARK SHALL BE SHOWN ON THE CERTIFICATE OF COMPLIANCE.

COMPLIANCE FOR PRODUCTS NOT BEARING THE BMP MARK WILL BE DOCUMENTED BY A CERTIFICATE OF COMPLIANCE ISSUED AND SIGNED BY AN INSPECTION AGENCY CERTIFYING THAT THE MATERIAL AND/OR ITS PRODUCTION WAS INSPECTED IN COMPLIANCE WITH PROCEDURES PUBLISHED IN THE MOST CURRENT VERSION OF THE BMP'S. AN INDEPENDENT WOOD INSPECTION AGENCY OF THE PRODUCER'S CHOICE AND ACCEPTABLE TO THE PRODUCER CAN BE USED TO PROVIDE THE INSPECTION SERVICE

FOLLOW CURRENT WWPI BMP'S FOR ALL WORK RELATED TO PRESERVATIVE TREATED WOOD INCLUDING BUT NOT LIMITED TO: PURCHASING, TRANSPORTATION, HANDLING, INSPECTION, STORAGE BOTH ON- AND OFF-SITE, FIELD TREATMENT OF CUTS, ABRASIONS, BORINGS, AND/OR OTHER INJURIES. CONTRACTOR SHALL SUBMIT BMP'S TO THE OWNER AND ENGINEER FOR REVIEW AND COMMENT PRIOR TO STARTING THE WORK.

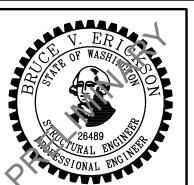
### **GANGWAYS**

GANGWAYS SHALL BE MADE OF ALUMINUM. ALL STRUCTURAL ALUMINUM, INCLUDING TUBES, PLATES, ANGLES AND PIPE SHALL BE ALLOY 6061-T6. ALL BOLTS FOR ALUMINUM CONSTRUCTION SHALL BE STAINLESS STEEL APPROPRIATE FOR USE WITH ALUMINUM IN MARINE ENVIRONMENTS. ISOLATORS SHALL BE USED WHEN CONNECTING DISSIMILAR MATERIALS, SUCH AS STEEL AND ALUMINUM.

### **FLOATS**

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1A	4/14/2023	WBC	MIFD COMMENT REVISION



**LUTHER BURBANK PARK** WATERFRONT IMPROVEMENTS

STRUCTURAL NOTES

	DRAWN: RRT	PROJECT NO.: 2200248
	DESIGN: WBC	SCALE: AS SHOWN
	CHECKED: AKB	DATE: 10/7/2022
	DRAWING NO.	S-002
	·····	<u>J-002</u>
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Xrefs: VI RPR\_BDR22v34

USER: RICKT PLOT DATE: Apr 12, 2023—11:05a1 : V:\2000291 (Luther Burbank Pier Repairs)\02

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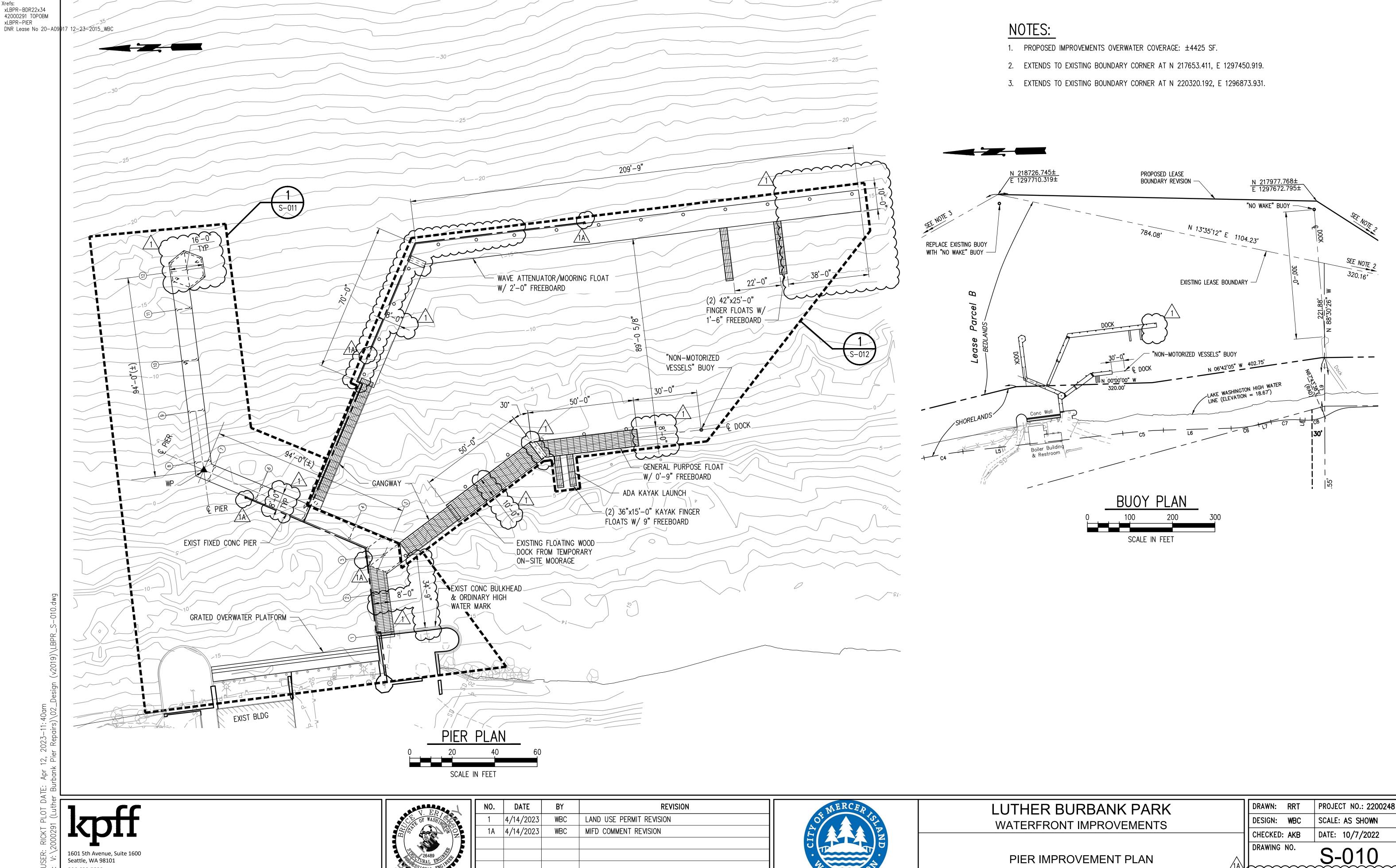


# LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

STATEMENT OF SPECIAL INSPECTIONS

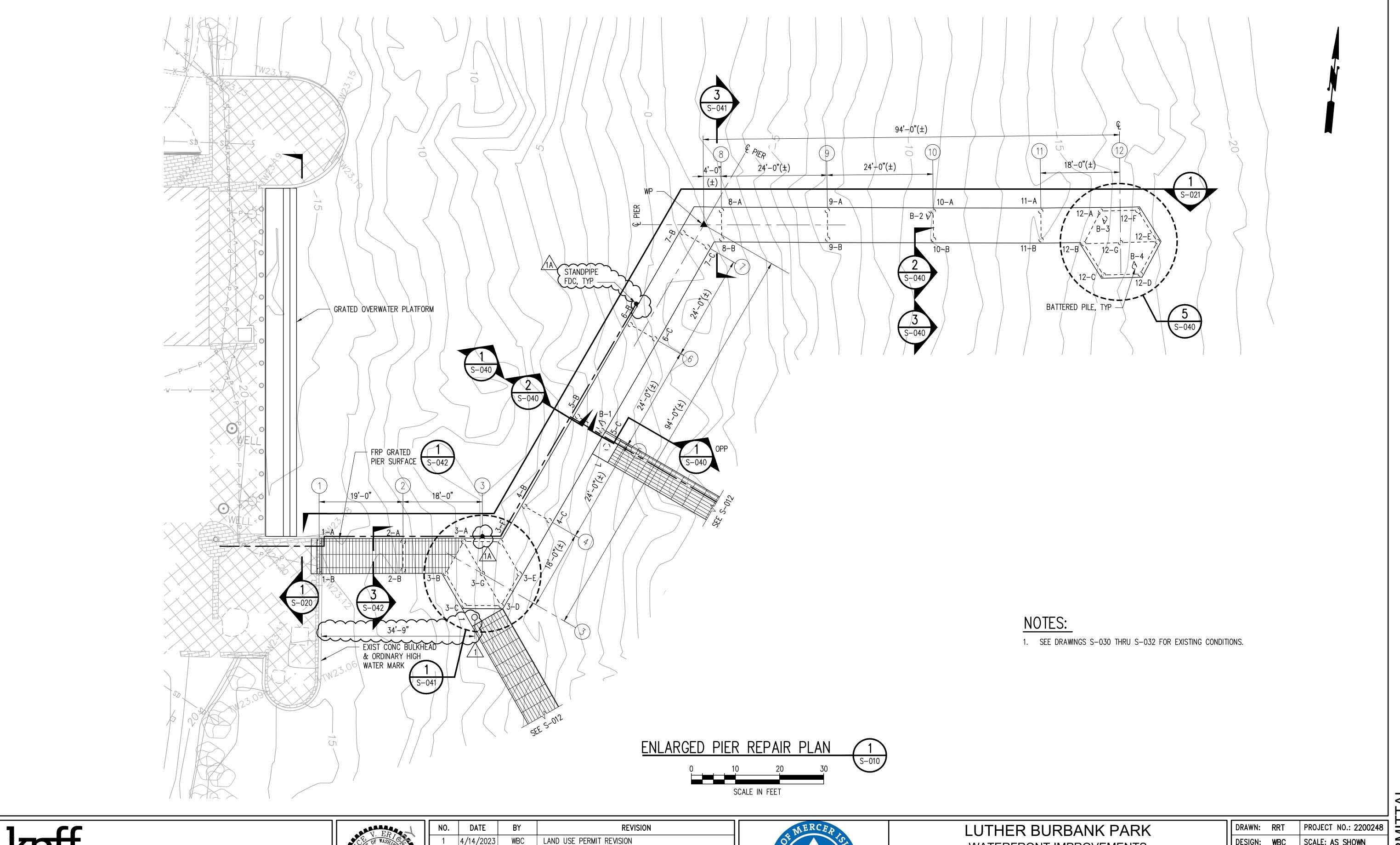
DRAWING NO.	S-003
CHECKED: AKB	DATE: 10/7/2022
DESIGN: WBC	SCALE: AS SHOWN
DRAWN: RRT	PROJECT NO.: 2200248

SHEET NO. 42 OF 56



xLBPR-PIER

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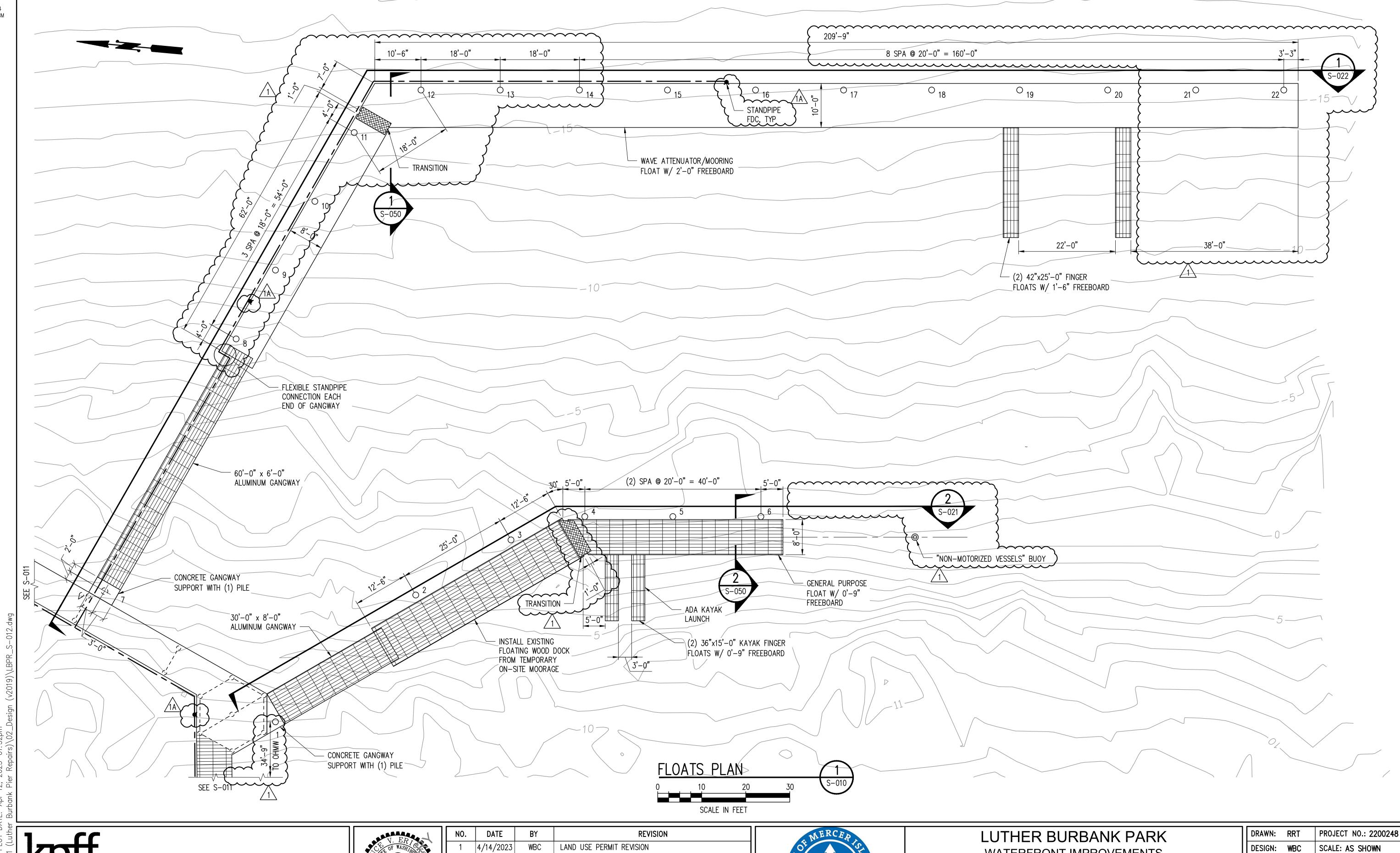
WATERFRONT IMPROVEMENTS

ENLARGED PIER REPAIR PLAN

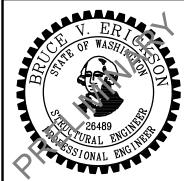
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CHECKED: AKB	DATE: 10/7/2022
DESIGN: WBC	SCALE: AS SHOWN
DRAWN: RRT	PROJECT NO.: 2200248

44 of 56

xLBPR-BDR22x34 42000291 TOPOBM xLBPR-PIER



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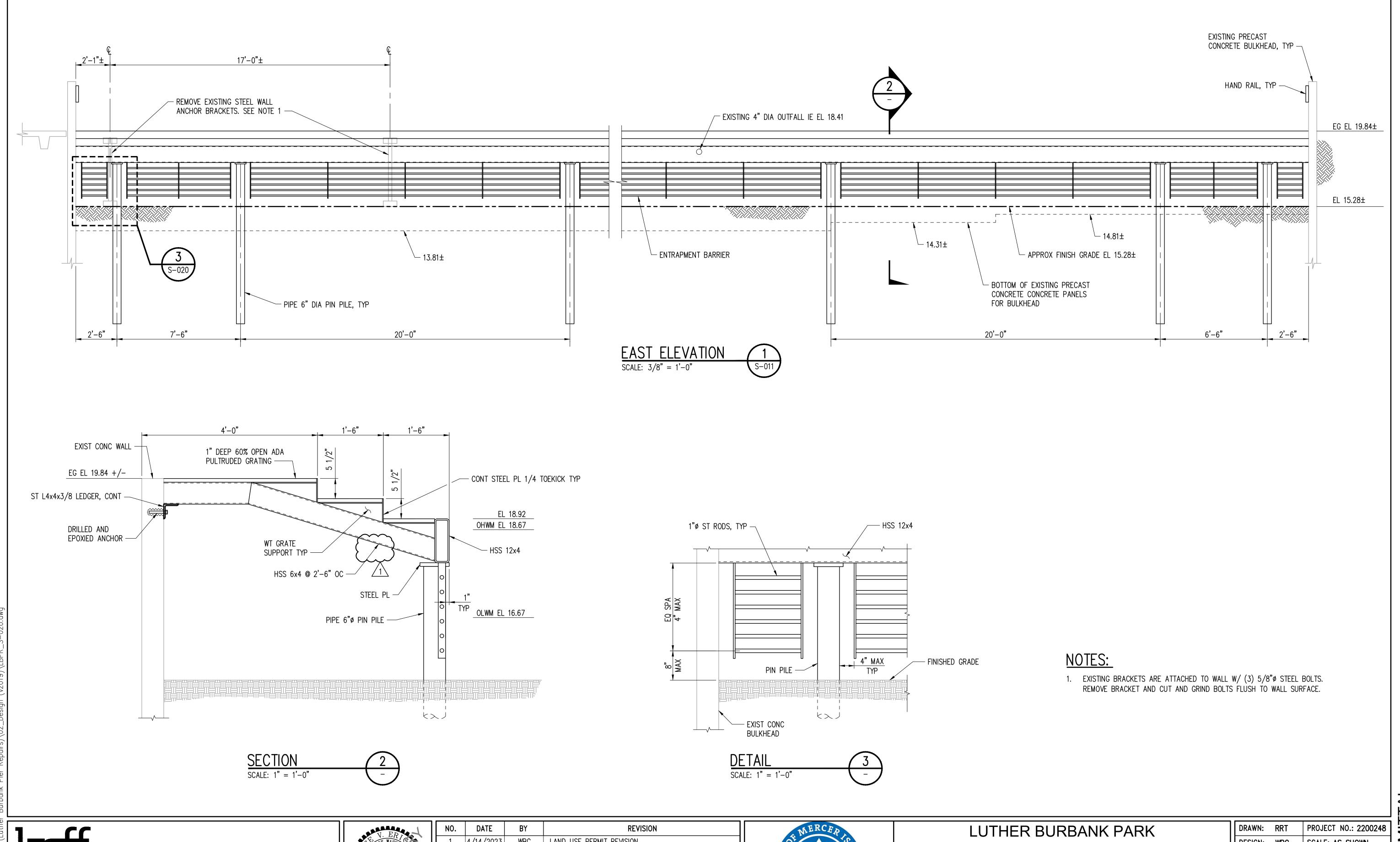


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WATERFRONT IMPROVEMENTS

DATE: 10/7/2022 DRAWING NO. FLOATS PLAN

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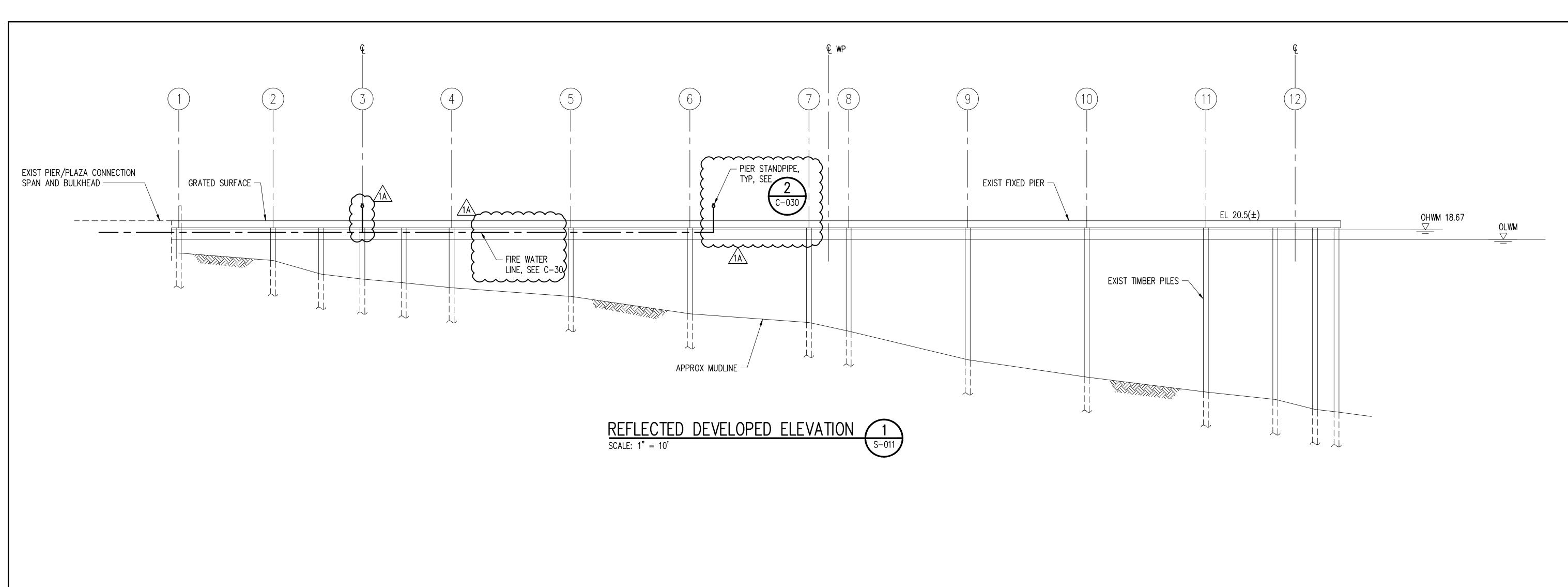
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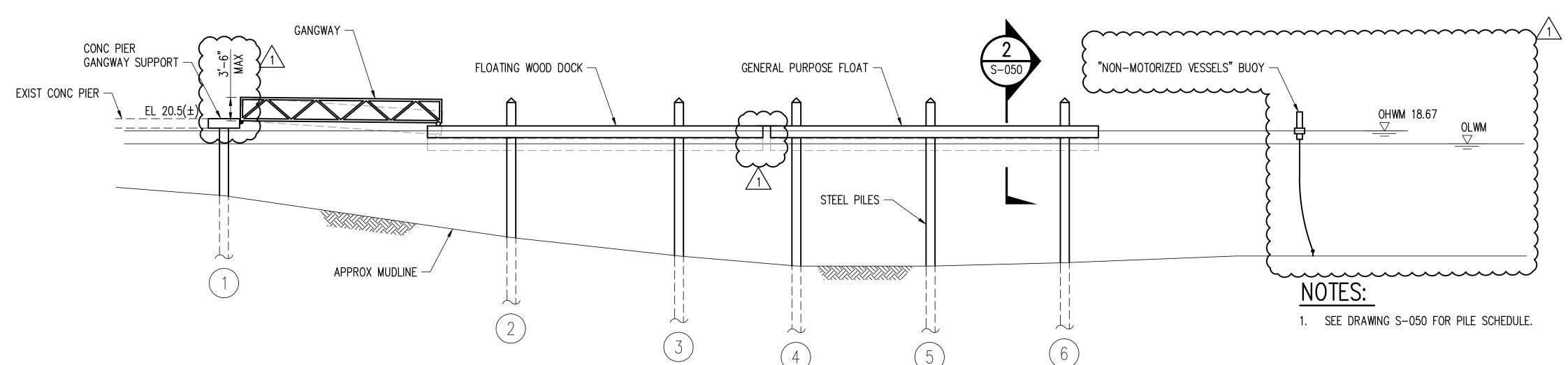


WATERFRONT IMPROVEMENTS

GRATED OVERWATER PLATFORM DETAILS

PROJECT NO.: 2200248
SCALE: AS SHOWN
DATE: 10/7/2022
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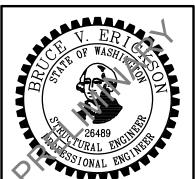


REFLECTED DEVELOPED ELEVATION 2
SCALE: 1" = 10'

S-012

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LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

PIER AND FLOATS ELEVATION

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LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

PIER AND FLOATS ELEVATION

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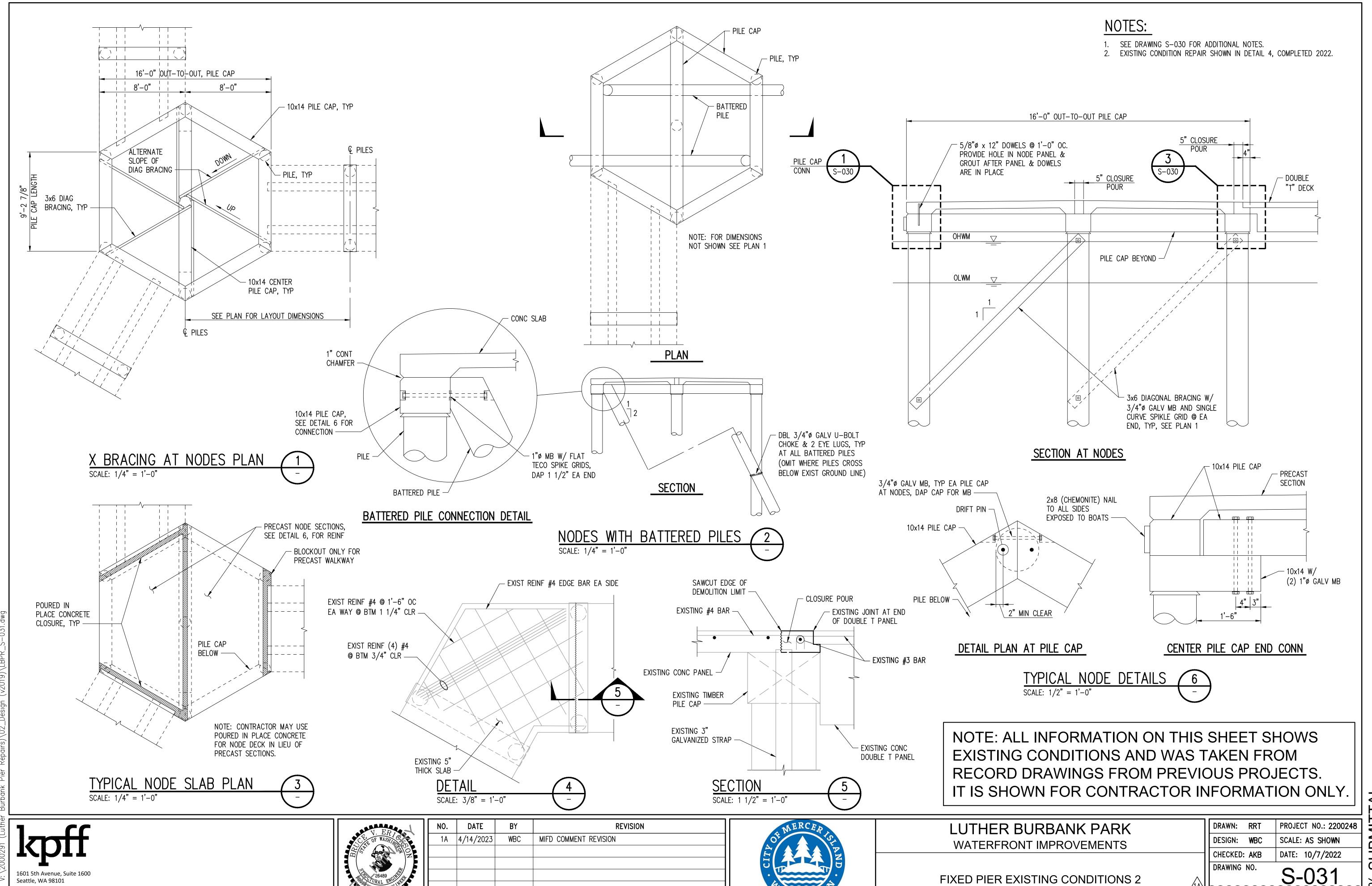


LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

FIXED PIER EXISTING CONDITIONS 1

DRAWN: RRT	PROJECT NO.: 2200248	
DESIGN: WBC	SCALE: AS SHOWN	
CHECKED: AKB	DATE: 10/7/2022	
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SHEET NO. 49 OF 56



FIXED PIER EXISTING CONDITIONS 2

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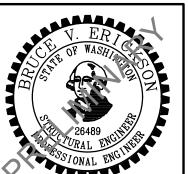
# NOTES:

- 1. SEE DRAWING S-030 FOR ADDITIONAL NOTES.
- 2. EXISTING CONDITION TEMPORARY REPAIR SHOWN IN SECTION DETAIL 1 COMPLETED 2022.

NOTE: ALL INFORMATION ON THIS SHEET SHOWS
EXISTING CONDITIONS AND WAS TAKEN FROM
RECORD DRAWINGS FROM PREVIOUS PROJECTS.
IT IS SHOWN FOR CONTRACTOR INFORMATION ONLY.



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SCALE: 1/2" = 1'-0"

PIER/PLAZA CONNECTION

NO.	DATE	BY	REVISION
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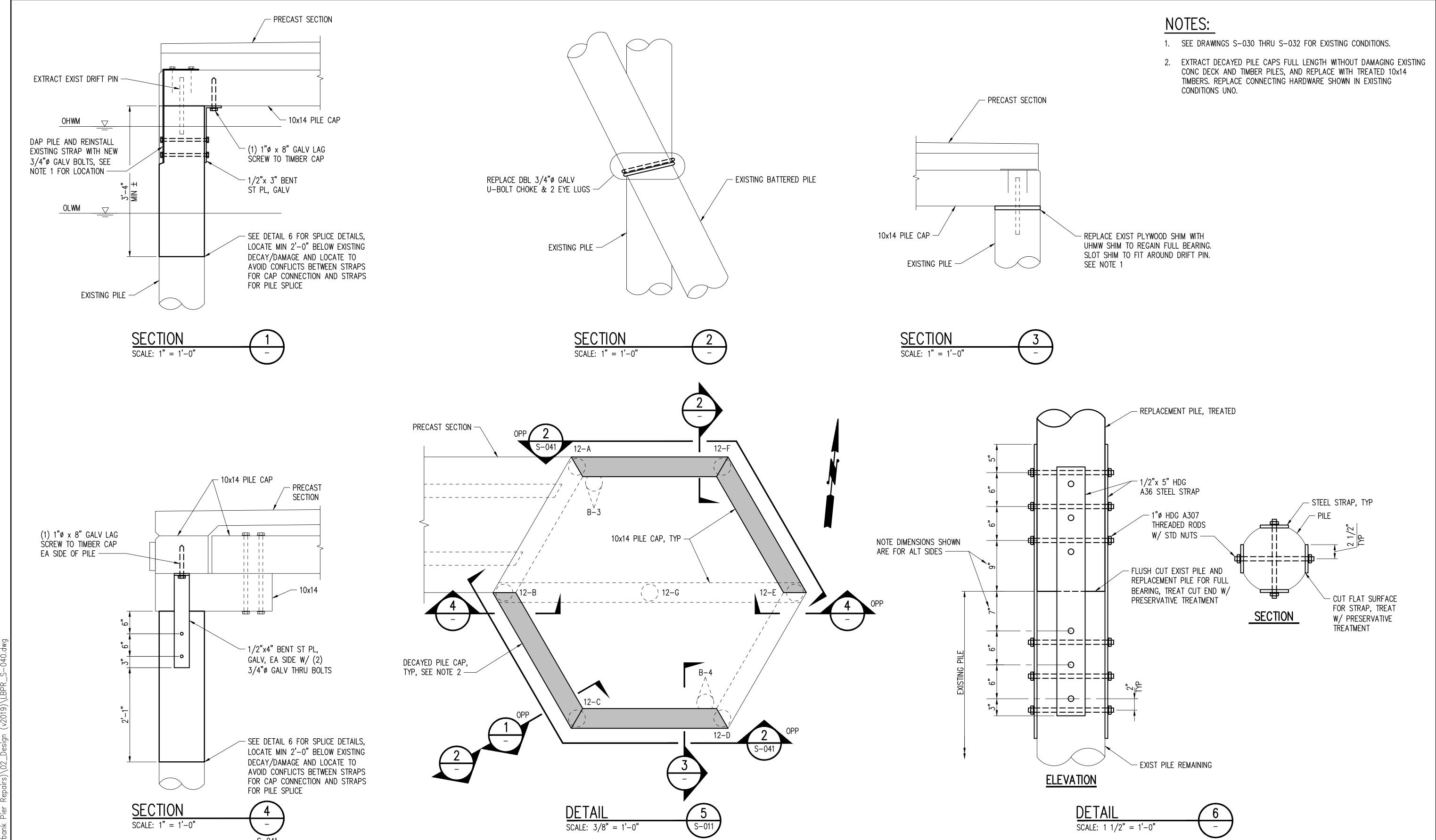


LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

FIXED PIER EXISTING CONDITIONS 3

DRAWING NO.	S-032
CHECKED: AKB	DATE: 10/7/2022
DESIGN: WBC	SCALE: AS SHOWN
DRAWN: RRT	PROJECT NO.: 2200248

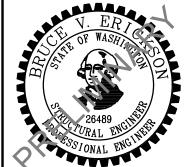
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	1A	4/14/2023	WBC	MIFD COMMENT REVISION
<b>5</b>				



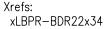
LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

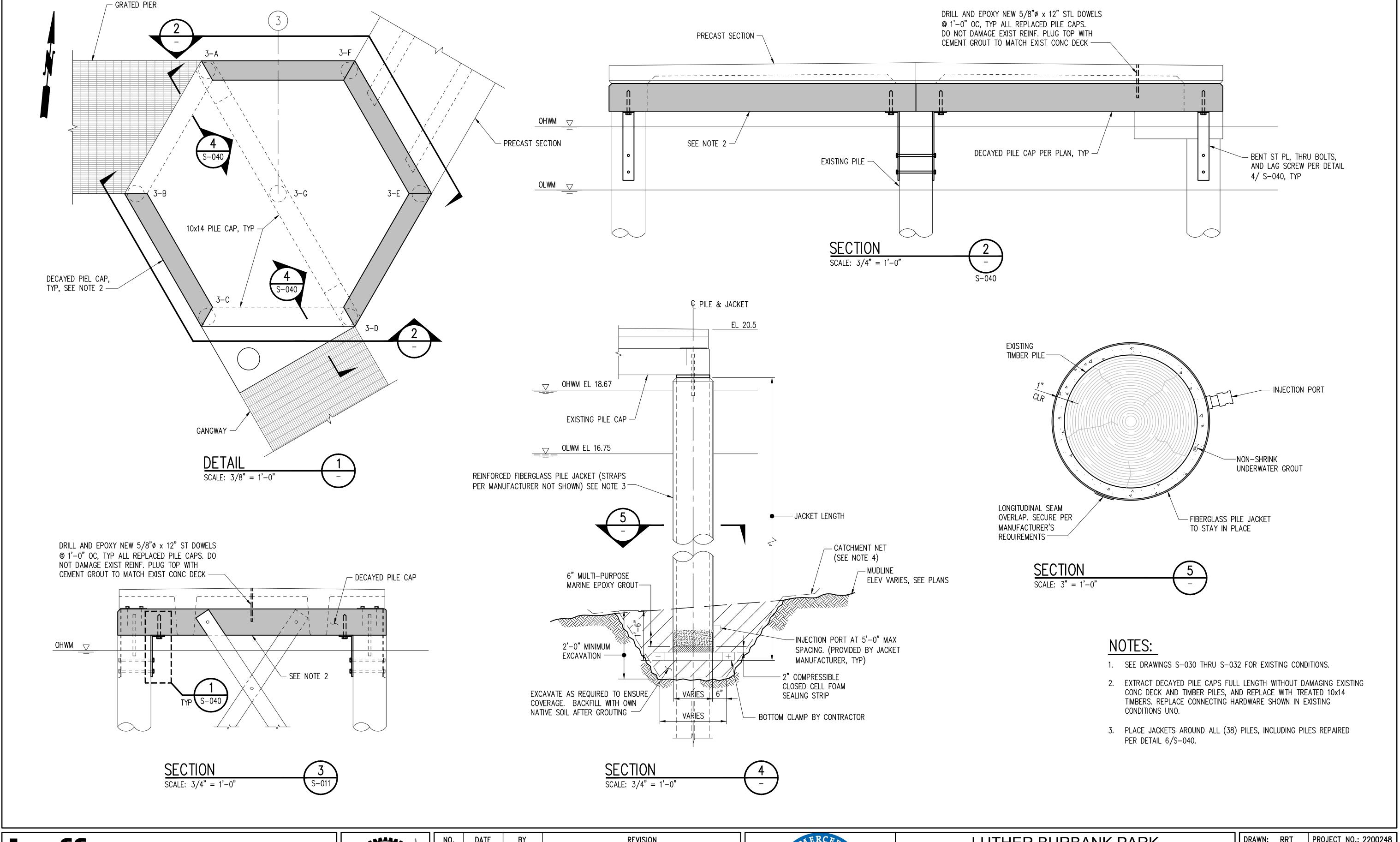
FIXED PIER REPAIR DETAILS

DRAWING NO.	S-040
CHECKED: AKB	DATE: 10/7/2022
DESIGN: WBC	SCALE: AS SHOWN
DRAWN: RRT	PROJECT NO.: 2200248

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CAD USER: RICKT PLOT DATE: Apr 12, 2023-03;11pm





1601 5th Avenue, Suite 1600 Seattle, WA 98101 206.622.5822 www.kpff.com



NO.	DATE	BY	REVISION	
1A	4/14/2023	WBC	MIFD COMMENT REVISION	



LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

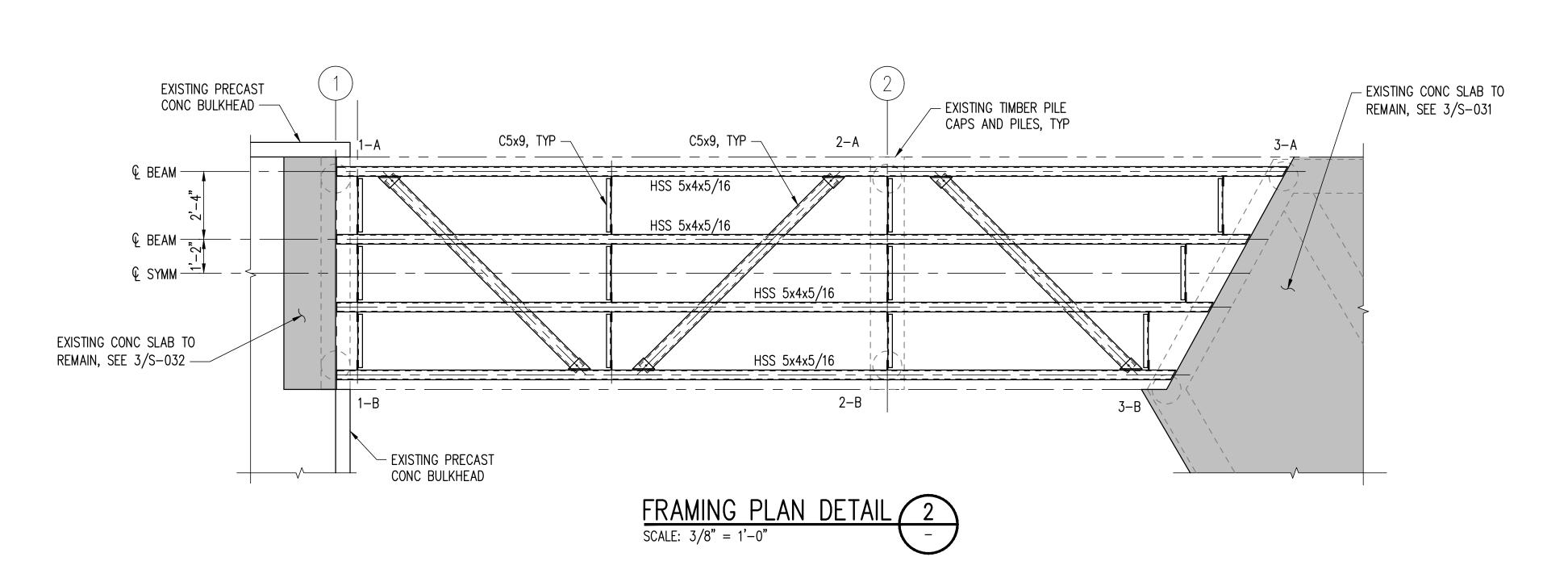
FIXED PIER REPAIR DETAILS

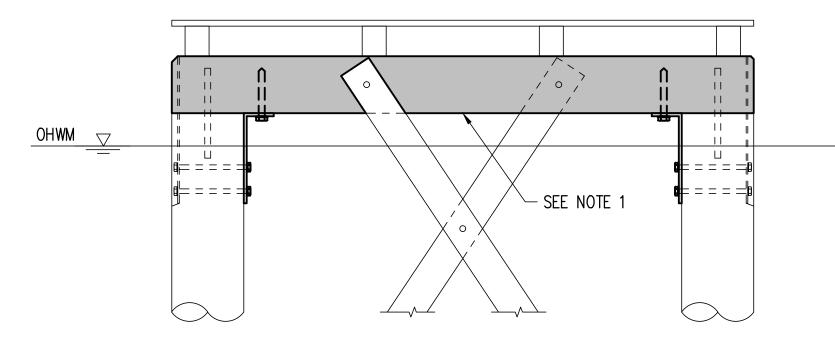
DRAWN: RRT DESIGN: WBC	PROJECT NO.: 2200248  SCALE: AS SHOWN
CHECKED: AKB	DATE: 10/7/2022
DRAWING NO.	S-041

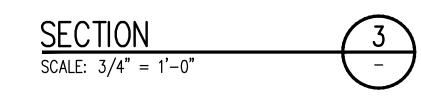
SHEET NO.

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60% SUBMITTAL





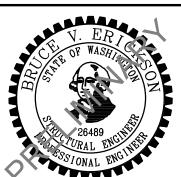


# NOTES:

1. EXTRACT DECAYED PILE CAP AT BENT 2 FULL LENGTH WITHOUT DAMAGING EXISTING TIMBER PILES. REPLACE WITH TREATED 10x14 TIMBER. REPLACE CONNECTING HARDWARE SHOWN IN EXISTING CONDITIONS UNO.

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	NO.	DATE	BY	REVISION	Ш
	1A	4/14/2023	WBC	MIFD COMMENT REVISION	$\mathbb{I}$
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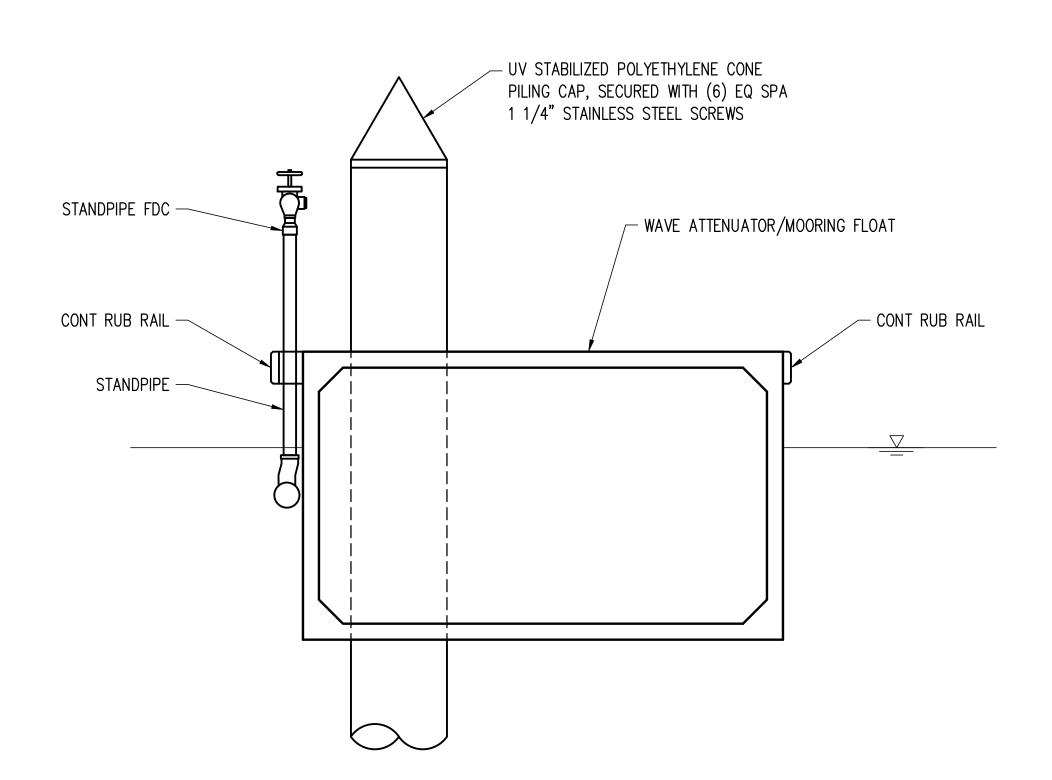


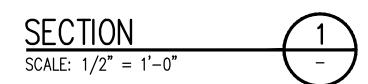
LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

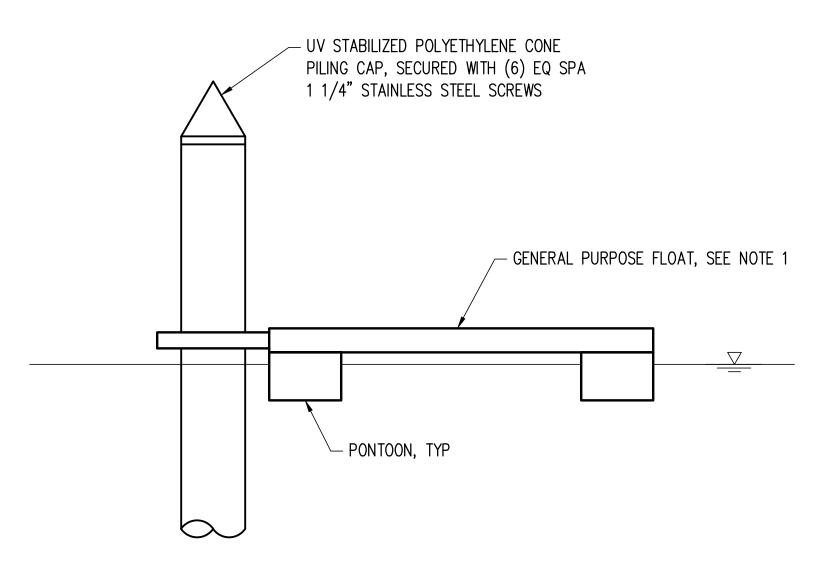
FIXED PIER GRATING DETAILS

DRAWN: RRT	PROJECT NO.: 2200248	<u> </u>
DESIGN: WBC	SCALE: AS SHOWN	2
CHECKED: AKB	DATE: 10/7/2022	=
DRAWING NO.	S-042	2
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1. 40% MINIMUM LIGHT TRANSMISSION IS REQUIRED. SEE STRUCTURAL NOTES AND SPECIFICATIONS FOR MORE INFORMATION



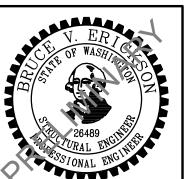




SECTION	$\overline{2}$
SCALE: $1/2" = 1'-0"$	<u> </u>

Pile ID	Nominal Dia (in)	Wall t (in)	Cutoff Elev (ft)	Approx Mudline Elev (ft)	Embed (ft)	Tip Elev (ft)
1	16	0.625	20.00	9.00	20.00	-11.00
2	16	0.625	22.00	2.75	20.00	-17.25
3	16	0.625	22.00	-0.25	20.00	-20.25
4	16	0.625	22.00	-1.75	20.00	-21.75
5	16	0.625	22.00	-1.50	20.00	-21.50
6	16	0.625	22.00	-1.00	20.00	-21.00
7	16	0.625	20.00	5.50	20.00	-14.50
8	24	0.625	25.00	-7.50	28.00	-35.50
9	24	0.625	25.00	-10.75	28.00	-38.75
10	24	0.625	25.00	-13.00	28.00	-41.00
11	24	0.625	25.00	-16.00	28.00	-44.00
12	24	0.625	25.00	-16.50	28.00	-44.50
13	24	0.625	25.00	-16.25	28.00	-44.25
14	24	0.625	25.00	-16.25	28.00	-44.25
15	24	0.625	25.00	-16.25	28.00	-44.25
16	24	0.625	25.00	-16.25	28.00	-44.25
17	24	0.625	25.00	-16.00	28.00	-44.00
18	24	0.625	25.00	-15.75	28.00	-43.75
19	24	0.625	25.00	-15.50	28.00	-43.50
20	24	0.625	25.00	-15.50	28.00	-43.50
21	24	0.625	25.00	-15.50	28.00	-43.50
22	24	0.625	25.00	-14.75	28.00	_42.75





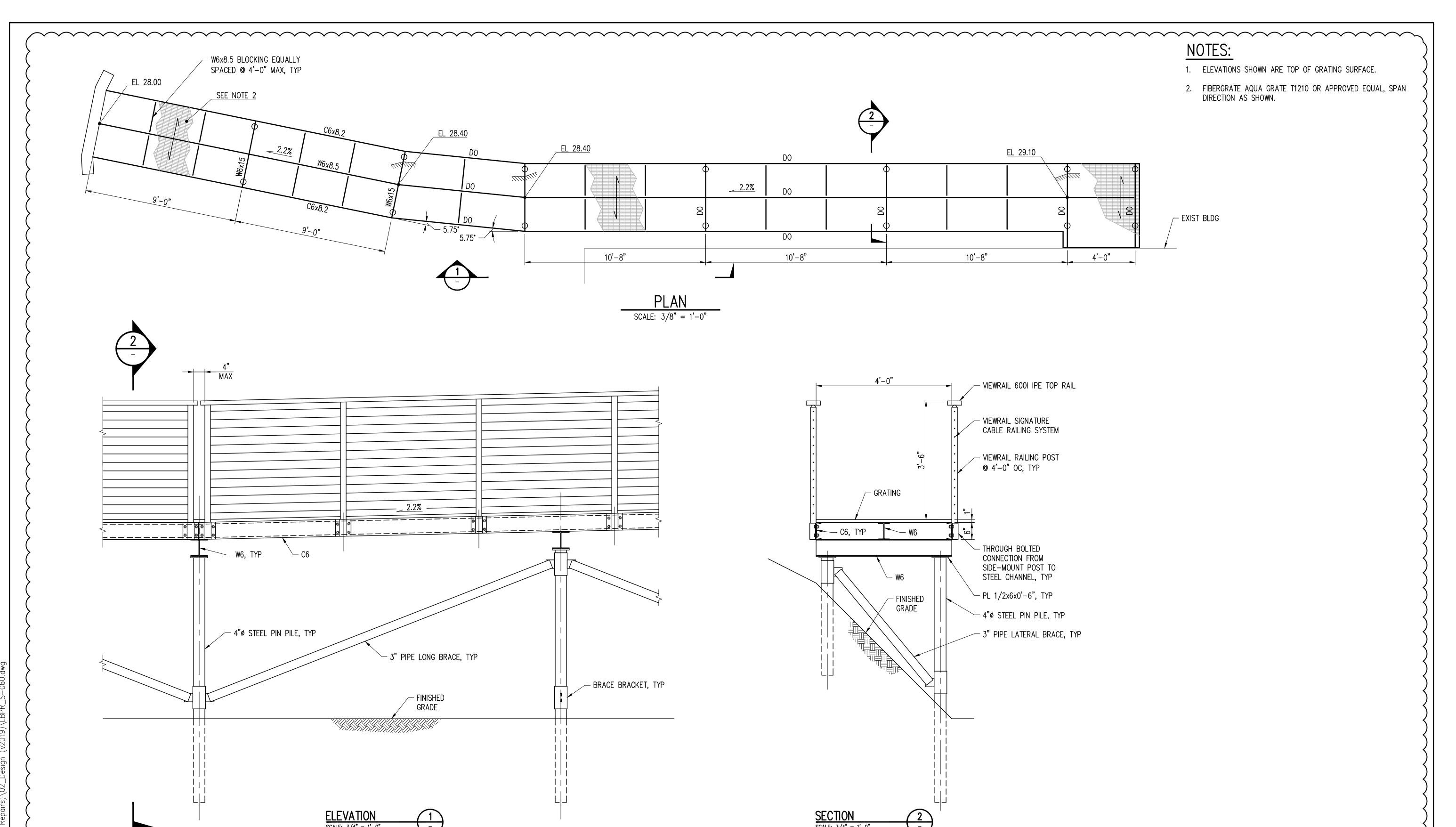
	NO.	DATE	BY	REVISION
	1	4/14/2023	WBC	LAND USE PERMIT REVISION
	1A	4/14/2023	WBC	MIFD COMMENT REVISION
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# LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

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DRAWN: RRT	PROJECT NO.: 2200248	•
DESIGN: WBC	SCALE: AS SHOWN	
CHECKED: AKB	DATE: 10/7/2022	
DRAWING NO.	S-050	(
·····	<b>3-030</b>	, ?





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NO.	DATE	BY	REVISION	
1	4/14/2023	WBC	LAND USE PERMIT REVISION	
1A	4/14/2023	WBC	MIFD COMMENT REVISION	
	1	1 4/14/2023	1 4/14/2023 WBC	1 4/14/2023 WBC LAND USE PERMIT REVISION



# LUTHER BURBANK PARK WATERFRONT IMPROVEMENTS

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	DRAWN: RRT	PROJECT NO.: 2200248
	DESIGN: WBC	SCALE: AS SHOWN
	CHECKED: AKB	DATE: 10/7/2022
	DRAWING NO.	S 060
		S-060

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CAD USER: RICKT PLOT DATE: Apr 12, 2023-03:17

VIEWING DECK ELEVATED ACCESS PATH